

## From Humboldt to Sioux Falls

	-Mail: Contact Info:	Phil Gundvaldson, P.E.
	Contact Info:	Phil Gundvaldeon P F
Lomments / Lillestions		271-5527 (office) PhilG@InfrastructureDG.com
Hoy 38 and 19 totorse Not suce und todo Just	ection on	observettion.

Project website: http://https://www.sd38corridorstudy.com/







Please place this comment/question form in the designated box on the way out of the meeting, mail to the address on the opposite side of this card, or email comments and questions to <a href="mailto:PhilG@InfrastructureDG.com">PhilG@InfrastructureDG.com</a>, by July 14th, 2023.

## From Humboldt to Sioux Falls

Name:	Rod Kramm	Telephone:	605270-9296
Address:_	204 S CA,130N	E-Mail:	
		Contact Info:	Phil Gundvaldson, P.E. 271-5527 (office)
Shoo	s/Questions: uld Be a sign That CVOS		PhilG@InfrastructureDG.com Stop where /Live 19
meet	s 38 by Friendlys in Hun	poolt	,
	/		

1122

Project website: http://https://www.sd38corridorstudy.com/







Please place this comment/question form in the designated box on the way out of the meeting, mail to the address on the opposite side of this card, or email comments and questions to <a href="mailto:Philo@infrastructureDG.com\_by">Philo@infrastructureDG.com\_by</a> July 14<sup>th</sup>, 2023.

## From Humboldt to Sioux Falls

Name: BRENT HOFFMAN	Telephone:	605.215.7014
Address: 2608 N. Coron Aye #213	E-Mail: Bre	ent. Hoffman
Sienx Falls, SD 57107  Comments / Questions:	Contact Info:	Phil Gundvaldson, P.E. 271-5527 (office) Phil G@InfrastructureDG.com
Many residents feel the north on	a of the I-go	D/Hwy 38 Interchange
is inherently unsafe, largely due to		, , , , , ,
off-ramp but there are also concerns		
east because of the line of sight.	If the road	could be widered and
lowered through there it would in	mprove visibili	ty. There are similar
concerns about the How 30 connec	tion with Mic	Kelson and some residents
have suggested the roadway could	be lowered	and for the curve smoothed.
out. Thanks for the opportunity	to comment.	Sen. Hoffman (D-9)

Project website: http://https://www.sd38corridorstudy.com/







Please place this comment/question form in the designated box on the way out of the meeting, mail to the address on the opposite side of this card, or email comments and questions to PhilG@InfrastructureDG.com\_by July 14th, 2023.

## From Humboldt to Sioux Falls

Name: Paul Dyke Address: 46572 25777 St.		205-528-3941  Daule locker 1157. Com  Phil Gundvaldson, P.E. 271-5527 (office)  Phil Gainfrastructure DG com			
Hastford, SD 57033 Comments / Questions:	Contact Info:				
Right turn lane needed (1	heading	west) at 468TH S			



Project website: http://https://www.sd38corridorstudy.com/





Please place this comment/question form in the designated box on the way out of the meeting, mail to the address on the opposite side of this card, or email comments and questions to <a href="https://example.com/Philo@InfrastructureDG.com/">Philo@InfrastructureDG.com/</a> by July 14th, 2023.

From Humboldt to Sioux Falls

Name: Peggy Hoggestraat	Telephone: 605-214-0623
Name: Peggy Hoogestraat Address: 27575 462nd Ave	E-Mail: garden gal peggy @gmail.com
Chancellor, SD 57015	Contact Info: Phil Gundvaldson, P.E.
	271-5527 (office)
Comments / Questions:	PhilG@InfrastructureDG.com
I attended the June 8 meeting	n Hartford and already left comment
Later I was told by my son, 1	Matt. who travels the corridor more
than I that a great concern	for the neighborhood is the
traffic entering + leaving the r	ace track. Safety for all is so
important.	
Thank you for involving the c	ommunities.
Pear	
3	J. J

Project website: http://https://www.sd38corridorstudy.com/







Please place this comment/question form in the designated box on the way out of the meeting, mail to the address on the opposite side of this card, or email comments and questions to <a href="mailto:PhilG@InfrastructureDG.com">PhilG@InfrastructureDG.com</a> by July 14<sup>th</sup>, 2023.

## SD Department of Transportation Public Meeting March 13, 2024

## NH 0042(80)371, Minnehaha County, PCN 06YP SD38 – From Humboldt to W of Marion Road in Sioux Falls Corridor Study

Individual stakeholder meetings followed a Public Meeting were held at West Central High School in Hartford, SD on March 13, 2024. A video was made available on the website to provide information and generate questions on design, environmental, right-of-way, and access management which are typical for most highway projects.

Comments were made to the Design Team by email, comment cards, website contact form, online survey, and online interactive map. Comments will be reviewed by the Design Team for consideration in the design process.

#### **Overview of Interactive Map Comments Received**

Comments were submitted through the comment map on the Corridor Study's website. Their main concerns are speed, amount of traffic and safety.

- Intersection of Highway 38 and 261<sup>st</sup> St
  - Submitted through interactive map (2/29/24)
    - "Enough people live in this neighborhood to make a turn lane a good option. It is difficult to wait to turn in or out with traffic moving at 65+."

#### **Overview of Comment Cards Received**

Comments were from residents who attended the Public Meeting held on March 13, 2024 at West Central High School in Humboldt or watched the presentation on the project website. Their concerns are summarized as follows:

- a median stopping snow and having to make U-turns
- the need for turn lanes at the racetrack
- reducing the speed limit primarily from the high school east to the interchange
- intersection sight distance issues at the interchange
- keeping their existing driveway access and configuration

- concerns with a wider highway affecting private property, fencing, existing wells, farm operations, etc.
- concerns with property acquisition and/or reduction of property values, changes to access locations, medians restricting access, and affecting existing billboard's location or visibility

The handwritten comment cards have been re-written below.

- Jeanne Foster, 200 E. 5<sup>th</sup> Street, Crooks, SD 57020
  - "No Median 3 lanes or 4 lanes ok along 38 between 468 Ave and Ellis Road. Need to be able to move farm equipment going both east and west on 38. There are a number of properties that would need to go east and west in this section. Median will stop snow on parts of this part of the road"
- Linda Hatle, 46735 SD Highway 38, Sioux Falls, SD 57107
  - "I'm not against a 4-lane road, just against not getting a turn lane to get into my driveway. Continuing to make a U-turn to me is going to be much more dangerous. In the winter when the plows have not cleared the roads good, I could possibly get stuck in the snow making a U-turn and get frost bitten trying to shovel any vehicle out of the snow. Also the snow will not be able to blow across due to a median and if there is a drift there will be no way to get around it. When pulling a long trailer into my yard, a U-turn will not be practical. I do not wish to drive further west to go home. I want and need a turn lane to my driveway! The state needs to consider property owners should have a right to have a lane to go home without driving further."
- Allan and Angelia Martens, 46061 SD Highway 38, Hartford, SD 57033
  - "Need a no passing zone between 460<sup>th</sup> & 461 Street. Also need a left and right turn lane for racetrack between 460 and 461<sup>st</sup> Street. Both for safety concerns."
- Mike and Jana Miles, 45570 258<sup>th</sup> St., Humboldt, SD 57035
  - We are landowners along Highway 38 on 45816 Hwy 38 which borders 258<sup>th</sup> Street near Humboldt, SD. I attended the afternoon meeting on March 13<sup>th</sup> in Sioux Falls and from what your plan shows you are wanting to take out our driveway access to Highway 38. We have managed this farm for 44 years and never had an issue with our access to Highway 38. We have been very grateful for this access because of the snow accumulation that this stretch of land creates in the winter. In the winter 258<sup>th</sup> street is not always plowed due to the extremely deep snow so this will create an issue if they were to merge our driveway to 258th street. We have a cattle operation that we need to access at least twice a day to monitor and feed cattle. Adding 500 feet to our driveway would be detrimental to our operation. I understand adding a curve on the gravel road on 258th would be a benefit for those traveling 258th street but extending our driveway to join it would not be a benefit to us. Our septic system runs right up to the edge of our property by the driveway and feedlot which would also create problems if disturbed. The best solution would be to leave the driveway access from 45816 Hwy 38 as is. Thank you, Mike and Jana Miles."

#### **Overview of Comments from the Public Meeting**

Some comments were written on the strip maps that were displayed at the Public Meeting held on March 13, 2024, at West Central High School in Humboldt and other comments were made verbally to the staff at the public meeting. Their main concerns are speed, amount of traffic and safety.

- 1. No medians
- 2. Signal at Ellis Road dangerous
- 3. Can we reduce speed?
  - a. DOT will be doing a speed study soon on the portion if SD Highway 38 from the high school to the interchange
- 4. Accidents near Dorothy Ave
- 5. Property owner potentially ok with median if frontage road connects his and his neighbor's driveways to a median cut location near 476<sup>th</sup>
- 6. Buffalo Ridge property owner concerned with median
  - a. Most of his business comes from the west so needs a median cut for entrance into his property ¾ access shown in some options but not all
  - b. Does not want to lose any part of his land for interchange reconfiguration
- 7. City of Hartford
  - a. Sidewalk initiative planned for the next year
    - i. Discussion about timing of project through Hartford
      - 1. TA grant possibility to construct prior to project
        - a. Would help possibility of receiving a grant if city had a sidewalk plan in place
          - i. Working with SECOG to develop
    - ii. Would like to see a pedestrian connection from Humboldt to Hartford –
       especially for school
      - 1. Make it part of a plan and it will be easier to add to the project
  - b. Teresa Sidel will send pictures of new signage

- c. City of Hartford purchased land for new WWTP and does not want to give land up for interchange options
- d. Concerns with access to City property on the north side of I-90 near the interchange
  - i. Existing access on west side of property need to get across the creek to access the rest of the property
- e. Mayor of Hartford does not favor roundabouts
- f. Within Hartford, potentially shift the roadway north at the curve to avoid purchasing ROW for trail. Verify with final survey and ROW location.

#### 8. Highway 19

- a. No proposed changes beyond the stop signs that were added last year. Still monitoring and adding additional signage as necessary.
- 9. Too many interchange options presented
- 10. Need a "No Passing Zone" between 460<sup>th</sup> and 461<sup>st</sup> Streets. Also need a left and right turn lane for the race track due to safety concerns.

#### Overview of Individual landowner/Stakeholder Meetings

Two individual meetings were held with individual landowners and stakeholders following the public meeting. The first meeting was held via Zoom on March 25, 2024 with Wyatt Haines who lives at 25973 466<sup>th</sup> Avenue, just north of the Exit 390 interchange. Wyatt was also representing his neighboring property owners, the Melin family, and Haase family. Ben White and Phil Gundvaldson participated in the call and presented an overview of the project. Wyatt was primarily interested in interchange and how it may affect properties, the adjacent roadway network, and modify access. Wyatt was appreciative of the presentation and would like himself and other area landowners to be kept informed as the project progresses.

The second meeting was held in person at Hartford City Hall on April 3, 2024, with the City of Hartford and the Hartford Area Development Foundation (HADF). Ben White and Phil Gundvaldson participated in the presentation and started by playing the recorded presentation from the website. The various options for the mainline and interchange were presented and

discussed. The questions received were like those mentioned by others at the public meeting. The group was appreciative of the presentation and would like to be kept informed as the project progresses, particularly phases through Hartford and the interchange.



#### STAKEHOLDER MEETING SIGN-IN

## Wednesday, March 13, 2024

## Project: HP 5596(25)P, Minnehaha County

Name	Address	Phone #(s)	Own Property on Project (Yes or No)
Phil Gundvaldson	116 W. 69Th St. SFSD 57108	605-271-5527	No
2. Jana Miles	45570 2584n ST Jun boldt SD	605-366-9437	Yes
3. CHUCK RUNGE	26731 465th HARTFORD	(05-366-2754	YES
1. Tom allany Stoffeedin	45938 SD Hwy 38 Numboldt, SD 57035	605-359-8834	Yes
5. Steve Gramm	700 E. Broadway Ave	605-773-3281	res
6. Katoma Fire Khand	IM E. Brad way Lyenne	645- 773-6641	Kes
7. Monica Foster	24109 S Robin Drive	701-330-6286	yes
8. Knistic Ellis	201 S Main St	409 363 378	NO
9. Teresa Sidel	125 Nmaire - Hartford, SD	605-523687	yes
10. Amy M. Fair	7	605-528-3338	- 11
11 Steve Cram	Minnehale Corny Highway Dept		
12.			
13.			
14.			
15.			



#### STAKEHOLDER MEETING SIGN-IN

## Wednesday, March 13, 2024

## Project: HP 5596(25)P, Minnehaha County

Name	Address	Phone #(s)	Own Property on Project (Yes or No)
1. Jestie Murphy	221 W capibl Suik 103; Pierre, SD 57501	605.280.5430	No
2. BEN WHITE		605-221-2651	NO
Lynnae Redenius	45935 SD-Hy 38 Humboldt SD57	035605-310-1505	yes
4. Res Staffens	315 N Ford St. Hutoldf		
Larry Deur	700 E. Bradway Ave Fierre	605 713-3157	No
6. Tom Kloxin	46729 DUIST SS	665 528-7462	YES
7. Linda Hatle	46735 SD Highway 38	605-360-6375	yes
8. Kas Hum	24555 466th Ave G1 try 51)	605-359-3058	Yes
9. Krista May	HIGH LATE ST. STEZOU SESD STILD	1005-271-5527	No
10. Arden Jones	508 PATROCKAR HART-ROND 50	605-310-4663	No
11.			
12.			
13.			
14.			
15.			



#### **PUBLIC MEETING SIGN-IN**

## Wednesday, March 13, 2024

## Project: HP 5596(25)P, Minnehaha County

Name	Address	Phone #(s)	Own Property on Project (Yes or No)
1. Steve Gramm	700 E. Broadway Ave Pierre	605-713-3281	No
Matther Bru	1	605-221-2655	No
3. Koling For Khars	l.	605-773-6641	16
Leslie Murphy	221 W Capital; Suile 103; Pierre, SD 57501	605.280.5930	No
5. David Tuch	P.O. Box 205 Hartford S.D	605-261-0580	no
6. En thought	102 2nd St. Hartfal 50	605-528-3217	School
Andy Wieczorek	46711 261275+ Sioux Falls	605-201-7682	Yes
8. Fanne Faster	200 E 5th Street	605-359-1267	yes
Tereso Sidel	1250 man Au Hantford (City)		401
Linda Lambeth	1302 E. North St. Humboldt, SD	605-363-3545	yes
11. Down Howey	46068 SD38 Hartford	605-691-3503 1005-838-7460	Yes
12. Allan Martons	46061 SD 38 Hortford	605-380-5206	Yes
13. Buth Kelix Berson	26/32 5. Robin Dr. Sionx Fala SD	701-212-3639	Ves
Franka Kan Habart	25911 461 SE Ave Hartford	605-802-3556	Yes
15. Curt Marson	S316 W 60th St N Siwx Falls SIS 87107	605-941-4481	NO



#### **PUBLIC MEETING SIGN-IN**

## Wednesday, March 13, 2024

Project: HP 5596(25)P, Minnehaha County

Name		Addr	Phone #(s)	Own Property on Project (Yes or No)	
1. David No/2	26153	466 Ave	605-310-3574		
BRAID SONISTAD	46638	Hwy 38	BUFFARD RIDGE	6053669794	YES
Ron Van Heerde	46896	they 38	SIOUX Falls	405 351 5590	yes
5.				1	
6.					
7.					
8.					
9.					
10.					
11.					
12.					
13.					
14.					
15.					



#### PUBLIC MEETING SIGN-IN

## Wednesday, March 13, 2024

Project: HP 5596(25)P, Minnehaha County

Name	Address	Phone #(s)	Own Property on Project (Yes or No)
1. CARYCLELAND	5316 W 60TH ST N	367-5680	No
2. Krista May	The w. 69th Street Suite 200	605-271-5527	No
3. Harry Johnston	5316 W 60th St N SF SD	605-360-6053	NO
4. Steve Crowe	46689 S.D. Hwy38 SFSD		VPS
Thomas Soisak	46370 263 of St Hartford SD	605 940 9349	VS
6. Sean Henri	500 N Veston Ave Sier Falls SD	605-691-8176	No
7. Traws Druss		605-940-1165	No
8. Londa Pagintan	46727 261 St Sieux Falls SD	605 261-7422	Yes
9. Muchas Those	2594 46 7 DANEHOTE J SD	605-366-9333	Xes
10. Vanet Foster	2904 W 334 St# 135 SF SD	605-610-5553	1
11. Michael Rodenburgh	156 - City of Hookford Engineers	0.0	NO
12. Physics momarty	46535 Jeannine Dr Hartford	605-518-3433	Hes
13.	Transition of the state of the		7
14.			
15.			



Banner Associates, Inc.
409 22nd Avenue South
Brookings, SD 57006
Tel 605.692.6342
Toll Free 855.323.6342
www.bannerassociates.com

## AGENCY, STAKEHOLDER, AND PUBLIC MEETING SUMMARY

DATE	March 13, 2024
LOCATION	SDDOT Area Office; 5316 W 60 <sup>th</sup> Street N; Sioux Falls, SD 57107 (1-4 PM) West Central High School; 705 E 2 <sup>nd</sup> Street; Hartford, SD 57033 (5:30-7:30 PM)
PROJECT	SD 38 Corridor Study

#### Stakeholder/Agency/Public Comments

**SAT Member Comment** 

#### **Common Questions/Concerns:**

- 1. Timing of Project
- 2. Impairment of access to residences/businesses with mainline improvements
- 3. Safety Improvements speed limits, hill re-grading
- 4. Roundabouts
- 5. Property Encroachments

#### SDDOT Area Office; 5316 W 60th Street N; Sioux Falls, SD 57107 (1-4 PM)

Questions were asked concerning the timing of the project.

- SDDOT has a placeholder in 2031 for projects derived from this study

#### Individual residing in the Songbird Development between Sioux Falls and Hartford

- Concerned with encroachment of land if the roadway widens to a 4-lane
- Safety there are adjacent houses to the road already; these will become closer to the roadway if widened to a 4-lane
- Speed limit should be 55mph for the entire route
- Why the changes to access points and the side roads?
  - Squaring up intersections for safety and truck traffic. Entry points preferred perpendicular to the roadway.

#### Will there be a traffic signal on Tea/Ellis Road?

- Nothing is warranted for the near future, but this may need re-evaluation as traffic volumes increase
- o The City of Sioux Falls' long-term plan for Tea/Ellis Road is a 4-lane all the way through

#### Center turn lane vs. concrete barrier median:

- Center turn lane would be preferred
  - Building a median would set the stage for future development and access points by limiting connections to SD 38. This would increase safety and reduce accidents.

#### Changes since the first 2023 agency/stakeholder/landowner meeting:

- Traffic analysis is complete



- Alternatives are complete; next step will be to select a preferred alternative Will there be any cut and fill to hillsides to improve vision?

- Likely yes, especially if we know there are problematic locations.

#### Property owner at 258<sup>th</sup> Street intersection, east of Humboldt

- Questioning the road realignment of 258<sup>th</sup> Street
  - Moving landowner to one point of access and changing alignment of 258<sup>th</sup> to allow for a better vantagepoint when entering SD 38.
- Currently the property is used for an Air B&B (she does not reside there, but does have cattle there and farmground)
- Will 258<sup>th</sup> stay gravel? Yes.
- Cattle lot has been moved back from the ROW already; fence is guardrail
- Hill (pink arrow) to the west of the house blocks snow; would not mind it being graded down

 A 7:1 backslope could be considered to the hill. Take the hill out to the ROW, provide more snow storage. May need a temporary easement to cut the backslope.



- Eagle nest to the west of her property.
  - Banner confirmed. Approximate lat/long 43.646921, -97.053676. Two adults, potential eaglets in nest.

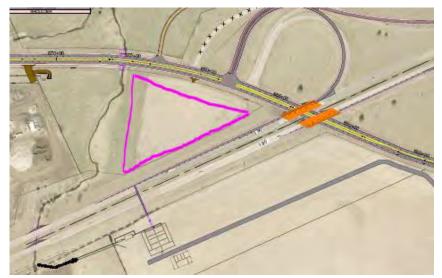
#### Property owners at 45938 SD 38:

- Shed to the east of the house is also theirs, used by Nortec Seeds
- No change to access; no concerns.

#### City of Hartford -

- Liked Interchange Options 1 and 9 the best.
- Bike Trail in Hartford on south side of road becomes very close to roadway along one stretch
  - o North side of SD 38 has more room, but also more access points to deal with
- Owns triangle piece south of 38, west of I90 (pink polygon below)





- Do not prefer alternatives that cut the triangle parcel off
- They would like to sell this parcel, but access becomes problematic with the interchange improvements – a roundabout design would work, but they don't like roundabouts.
  - For access to this parcel off the interchange, it would need FHWA approval and has to be a public roadway; cannot be private. Anything that would tie to the ramp cannot be a private road.
- Likely would use approach access to parcel coming from the west across the creek.
- O Have plans to expand the WWTP to the east, so avoid doing any improvements/alternatives that would affect that area.
- Bike path timing?
  - If DOT would do a path in conjunction with a SD 38 project, the City would have to wait for that project to come along. If they decide to do it before a SD 38 project, they could apply for TAP funding and use those funds to construct the path. If the road project would impact an existing bike path, SDDOT would replace the path.
- In recent years, have been annexing more land to the east.
- Pedestrian usage at the High School crossings does this area need a light?
  - o would warrant a light at 2<sup>nd</sup> Street.
  - o Also would be a good spot for a roundabout since the road is skewed.
- City overall would like to see:
  - Avoidance of City-owned land
  - Reduced speeds east of town
  - o Incorporation of bike trail on south side, sidewalk on the north side
  - Median



#### West Central High School; 705 E 2<sup>nd</sup> Street; Hartford, SD 57033 (5:30-7:30 PM)

Resident along the route:

- With a raised median (18-20 feet wide), a person could end up going to the next mile marker to turn around
- Concern with lack of breaks to turn around and driving farther to do so in rural cross sections.
  - o Raised medians will reduce turning vehicle accidents.

#### Resident near Hartford:

- What about roundabouts for kids going to school? That seems dangerous.
  - o Younger generations actually navigate roundabouts better than older generations.
- Concern about impacts to existing trees and shelterbelts.

#### Resident on Middle Drive:

- Concern of how access will be provided to residences during construction.

#### Resident adjacent to SD 38:

- (Husband) Sometimes there is no passing right now due to traffic and sight restrictions and there is no median
- (Wife) Likes the 4-lanes, dislikes the median.
- Concerns with maneuvering horse trailer or hay loads from their house (south of SD 38) across the road to their pasture north of SD 38.
- Safety concerns with riding horses across 4-lanes and a median.
- With the reconfiguration of the access road, they like the idea of the whole development not driving in front of their house.

Make sure culvert drainages are adequate.

Make a 4-way stop on Western / SD 38 – funnel traffic to the interstate.

Property owner near Pheasant Run Avenue – realignment on south side of the road

- Would like more turn-around locations
- Heavy farm equipment have to go to turn around at Ellis Road already heavy traffic there
- If they turn to the west, there's a hill; line of sight isn't good
- Would prefer center turn lane as opposed to median.

#### Brad Songstad – likes Option 1 and 3 for Mainline for his business

- Option 4, no driveway to his parcel south of SD 38
- Utility easement present
- Current striping in front of his business on the north side of SD 38 confuses customers
- If there was a raised median, no one would get into his business. 99% of his business comes from WB I- 90, no place to turn in. He needs a median break.
- Aerial backgrounds are old; there is development in the lots east of his parcel south of 38

Landowner on Tea Ellis Road – northwest corner of intersection – consolidate to one access point.

- Concern with median

## Social Pinpoint Comments - March 26, 2024

Created on	Туре	Threads	Comment	Up Votes	Email	Phone	Postcode	Firstname	Lastname	Device Type	Region	City
5/31/2023 18:15	Safety	Safety-01	Event traffic is a challenge for congestion	0	tthoreen@hrgreen.com	6513989333	55104	Timothy	Thoreen	Desktop	Illinois	Chicago
6/1/2023 11:06	Safety	Safety-02	-	1	tthoreen@hrgreen.com	6513989333	55104	Timothy	Thoreen	Desktop	Illinois	Chicago
6/1/2023 11:08	Other	Other-1	Other comment option	0	tthoreen@hrgreen.com	6513989333	55104	Timothy	Thoreen	Desktop	Illinois	Chicago
6/5/2023 11:26	Safety	Safety-03	This four-lane section of hwy 38 is poorly lit up to the Marion Rd. intersection. This, combined with high speeds and a straight stretch of road all the way to the interstate makes the road ideal for racers and speeders. Many times I've witnessed cars blowing the red light at night, sometimes without headlights on or smoked-out headlights, making them difficult to see when making a legal crossing of hwy 38.	0	anyhoo@gmail.com					Mobile		
6/5/2023 11:36	Traffic	Traffic-1	Seeing cars coming east-bound (from Hartford) when exiting off the off-ramp into Hwy 38 is difficult.	2	anyhoo@gmail.com					Mobile		
6/5/2023 11:37	Traffic	Traffic-2	Traffic backs up here sometimes when cars are trying to enter I-90	0	anyhoo@gmail.com					Mobile		
6/5/2023 11:40	Safety	Safety-04	Stoplights are badly needed here. It's a large intersection and with development now north of Hwy 38. Vehicles tend to drive over 50mph on this road, so crossing hwy 38 is dangerous.	3	anyhoo@gmail.com					Mobile		
6/5/2023 11:44	Safety	Safety-05	Very difficult to see north-bound drivers on Marion Rd when trying to turn into Marion Rd from N 54th Street. There's a hill just south of Marion/N 54th that creates a very short sight- line. Perhaps a stop light is needed here?	3	anyhoo@gmail.com					Mobile		
6/7/2023 12:27	Safety	Safety-06	Need to slow the speed down through town to 35 all the way west to 19	0	siemonsmaelectric@yahoo.com					Mobile	South Dakota	Winner
6/7/2023 14:06	Safety	Safety-02-child	This intersection is very dangerous, especially for students atempting to turn west going to school at the same time commuters are driving east at 65+ mph. It will only get more difficult when the	2	mark.heath@sanfordhealth.org					Desktop	South Dakota	Sioux Falls
6/15/2023 10:28	Traffic	Traffic-3	Traffic does not slow down coming into Hartford, making it difficult for multi-modal traffic to enter, exit, or cross Highway 38 at Western Avenue.	2	mr.atlasboy@gmail.com					Desktop		
6/15/2023 10:29	Traffic	Traffic-4	The speed limit of 65 MPH feels too fast for this stretch of Highway 38 near Hartford Heights, especially with the number and spacing of access points and the bike trail along the highway.	0	mr.atlasboy@gmail.com					Desktop		
6/15/2023 10:34	Safety	Safety-07	The 90-degree-angle correction greatly improved safety at this intersection, but traffic on Highway 38 still creates safety concerns for cross traffic. Consider a traffic signal, roundabout, or other traffic calming technique at this intersection.		mr.atlasboy@gmail.com					Desktop		

6/15/2023 10:38	Safety	Safety-08	As development around this intersection continues to blossom, consider a traffic signal, roundabout, or other traffic calming technique to 2 improve safety.	mr.atlasboy@gmail.com					Desktop		
6/15/2023 10:40	Safety	Safety-09	Due to traffic coming from multiple directions at this intersection, consider an all-way stop, roundabout, or other traffic calming technique to improve safety.	mr.atlasboy@gmail.com					Desktop		
6/26/2023 6:30	Safety	Safety-10	Flatten road so it is easier to see traffic heading  East when turning off mesa.	alysia.boysen@gmail.com					Mobile	Nebraska	Omaha
6/26/2023 6:31	Safety	Safety-11	Add execration lanes at Ellis road so that traffic can safely merge onto hwy 38	alysia.boysen@gmail.com					Mobile	Nebraska	Omaha
3/13/2024 14:26	Traffic	Traffic-5	Enough people live in this neighborhood to make a turn lane a good option. It is difficult to wait to $1$ turn in or out with traffic moving at 65+	kristen.foster88@gmail.com	7013309738	57107	Kristen	Hall	Mobile	South Dakota	Sioux Falls
3/24/2024 9:17	Traffic	Traffic-3-child	add a roundabout 0	bud7997@gmail.com					Desktop	South Dakota	Hartford
3/24/2024 9:18	Safety	Safety-07-child	looks like a great place for a roundabout 0	bud7997@gmail.com					Desktop	South Dakota	Hartford
3/24/2024 9:25	Safety	Safety-08-child	this intersection is a tricky one from both directions on 38. The cars turning into and from Mickleson, and add the fact that its a curve and a hill. Great spot for a roundabout	bud7997@gmail.com					Desktop	South Dakota	Hartford
3/24/2024 9:33	Traffic	Traffic-2-child	hard to see W bound 38 traffic when getting off W bound 90. I like Option 6 of the interchange plan. ORoundabouts will at least slow the flow but keep things moving.	bud7997@gmail.com					Desktop	South Dakota	Hartford
3/24/2024 9:41	Safety	Safety-11-child	N bound Ellis Road traffic turning East on 38 could use a turning lane onto an acceleration lane. I'm a W bound turner myself, not sure if there could be a left turning acceleration lane	bud7997@gmail.com					Desktop	South Dakota	Hartford
3/26/2024 11:12	Traffic	Traffic-2-child	Strongly recommend a right turn lane, coming off the intestate exit, with no stop sign (maybe a yield sign) which extends past the service road to allow merging vehicles to get up to speed and to allow easier commute to Hartford and also reducing traffic that backs up at the exit.	wwhaines@icloud.com					Mobile	Arizona	Phoenix

From: White, Ben <bwhite@hrgreen.com>
Sent: Friday, March 22, 2024 8:59 AM

**To:** Philip Gundvaldson

**Subject:** FW: SD38 Corridor Study Comment

Follow Up Flag: Follow up Flag Status: Flagged

FYI

#### Ben White, PE, LS

Senior Project Manager | Regional Director - Transportation HR Green® | Building Communities. Improving Lives.





431 N. Phillips Avenue | Suite 400 | Sioux Falls, SD 57104-5933 **Direct** 605.221.2651 | **Cell** 605.400.4947 HRGREEN.COM

The contents of this transmission and any attachments are confidential and intended for the use of the individual or entity to which it is addressed. If the reader of this message is not the intended recipient, you are hereby notified that any dissemination, distribution or copying of this communication is prohibited.

From: SD38 Corridor Study Webflow Forms <no-reply-forms@webflow.com>

Sent: Thursday, March 21, 2024 7:44 PM

To: Whitver, Heidi <a href="https://www.tthoreen@hrgreen.com">https://www.tthoreen@hrgreen.com</a>; White, Ben

<bwhite@hrgreen.com>; steve.gramm@state.sd.us

**Subject:** SD38 Corridor Study Comment

This email came from outside the HR Green organization. Please use caution when clicking on hyperlinks and opening attachments

You just got a form submission!

#### **Form**

Contact Form

#### Site

SD38 Corridor Study

#### **Submitted content**

Name: Mike and Jana Miles Email: <a href="mailto:jmiles@siouxvalley.net">jmiles@siouxvalley.net</a>

Comments: We are land owners along Highway 38 on 45816 Hwy 38 which borders 258th street near Humboldt SD. I attended the afternoon meeting on March 13th in Sioux Falls and from what your plan

shows you are wanting to take out our driveway access to Highway 38. We have managed this farm for 44 years and never had an issue with our access to Highway 38. We have been very grateful for this access because of the snow accumulation that this stretch of land creates in the winter. In the winter 258th street is not always plowed due to the extremely deep snow so this will create an issue if they were to merge our driveway to 258th street. We have a cattle operation that we need to access at least twice a day to monitor and feed cattle. Adding 500 feet to our driveway would be detrimental to our operation. I understand adding a curve on the gravel road on 258th would be a benefit for those traveling 258th street but extending our driveway to join it would not be a benefit to us. Our septic system runs right up to the edge or our property by the driveway and feedlot which would also create problems if disturbed. The best solution would be to leave the driveway access from 45816 Hwy 38 as is. Thank you, Mike and Jana Miles

#### Number of submissions received

7/500 this month March 1st – March 31st

#### Need more submissions?

Please contact your website administrator.

If you believe this is a spam submission, please forward to <a href="mailto:form-spam-reports@support.webflow.com">form-spam-reports@support.webflow.com</a>

Unsubscribe from notifications for this site.

From: SD38 Corridor Study Webflow Forms <no-reply-forms@webflow.com>

Sent: Wednesday, March 13, 2024 8:48 PM

To: Whitver, Heidi; Thoreen, Timothy; White, Ben; steve.gramm@state.sd.us

**Subject:** SD38 Corridor Study Comment

This email came from outside the HR Green organization. Please use caution when clicking on hyperlinks and opening attachments

You just got a form submission!

#### **Form**

Contact Form

#### Site

SD38 Corridor Study

#### **Submitted content**

Name: Kristen Hall

Email: kristen.hall88@outlook.com

Comments: Hello, I live at the house right at the junction of 261st and Highway 38. I was concerned that the meeting tonight only discussed closing our access to the highway. Visibility to enter 38 is much worse at 467th Ave and many people in the neighborhood use the 261st access for this reason. I am also concerned about how the larger road will affect our well that we share with two other houses. The access to the neighborhood is also beneficial due to large vehicles such as buses or trucks being able to use 261st to safely turn around if needing to go back in the other direction. As someone very much affected by this decision, I feel our best outcome would be a four lane road past the neighborhood with an optional turn lane to 261st St. If there seems to be no way around closing 261st, then it would be beneficial to at least have 467th paved down to 12th St.

#### Number of submissions received

4/500 this month March 1st – March 31st

#### Need more submissions?

Please contact your website administrator.

If you believe this is a spam submission, please forward to form-spam-reports@support.webflow.com

Unsubscribe from notifications for this site.

**From:** noreply@socialpinpoint.com

Sent: Wednesday, March 13, 2024 2:27 PM

**To:** Thoreen, Timothy; White, Ben

**Subject:** New Comment[Traffic] created on project: SD 38 Corridor Study [A58153]

This email came from outside the HR Green organization. Please use caution when clicking on hyperlinks and opening attachments





The following Comment from <a href="mailto:kristen.foster88@gmail.com">kristen.foster88@gmail.com</a> was posted regarding the project: SD 38 Corridor Study

Enough people live in this neighborhood to make a turn lane a good option. It is difficult to wait to turn in or out with traffic moving at 65+

**Review the Comment now** 

## Powered by Social Pinpoint



From: SD38 Corridor Study Webflow Forms <no-reply-forms@webflow.com>

Sent: Wednesday, March 13, 2024 2:06 PM

To: Whitver, Heidi; Thoreen, Timothy; White, Ben; steve.gramm@state.sd.us

**Subject:** SD38 Corridor Study Comment

This email came from outside the HR Green organization. Please use caution when clicking on hyperlinks and opening attachments

You just got a form submission!

#### **Form**

Contact Form

#### Site

SD38 Corridor Study

#### **Submitted content**

Name: Peggy Hoogestraat

Email: gardengalpeggy@gmail.com

Comments: Today, March 13, 2024, I listened to the prerecorded presentation for the project. Please note that on the traffic volume projections map, the I-90 speedway entrance and the 459th Ave are marked incorrectly. I discovered that when reviewing where my own property is along Hwy 38. From what I understand, there will be no additional changes from Humboldt to Hartford's Western Avenue as a result of this study. Please let me know if that is correct. I will not be able to attend the open house tonight. Thank you

#### Number of submissions received

3/500 this month March 1st – March 31st

#### Need more submissions?

Please contact your website administrator.

If you believe this is a spam submission, please forward to form-spam-reports@support.webflow.com

Unsubscribe from notifications for this site.

From: SD38 Corridor Study Webflow Forms <no-reply-forms@webflow.com>

Sent: Wednesday, March 6, 2024 5:53 PM

**To:** Whitver, Heidi; Thoreen, Timothy; White, Ben; steve.gramm@state.sd.us

**Subject:** SD38 Corridor Study Comment

This email came from outside the HR Green organization. Please use caution when clicking on hyperlinks and opening attachments

You just got a form submission!

#### **Form**

Contact Form

#### Site

SD38 Corridor Study

#### **Submitted content**

Name: Kristi Nimick

Email: usnimicks@yahoo.com

Comments: your flyer you sent out says the meeting is on Wednesday March 13, this web page says the

14th? Which is correct?

#### Number of submissions received

1/500 this month

March 1st - March 31st

#### Need more submissions?

Please contact your website administrator.

If you believe this is a spam submission, please forward to form-spam-reports@support.webflow.com

Unsubscribe from notifications for this site.

From Humboldt to Sioux Falls

Name: Jeanne Faster,	Telephone: 605 - 359 - 1267
Address: 200 E 5th Street	E-Mail: 2001, eanne@gmail.com
Rrooks SD 57020	Contact Info: Phil Gundvaldson, P.E.
Comments / Outstand	605-271-5527 (office)
Comments / Questions:	PhilG@InfrastructureDG.com
No median - Slones of 4	lares ok along 31 between
468 Que and FIL'S Road.	0
Need to be able to move	tarm egiripment going hoth
east and west on 38	by this
The are a number of properties	that would need to go east I west in swoton
Median with stop stow show	u a parts of this part of the
Road	

Project website: https://www.sd38corridorstudy.com/







Please place this comment/question form in the designated box on the way out of the meeting, mail to the address on the opposite side of this card, or email comments and questions to <a href="mailto:PhilG@InfrastructureDG.com">PhilG@InfrastructureDG.com</a> by April 19th, 2024.

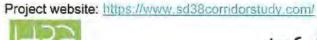
## From Humboldt to Sioux Falls

Name: Lindor Hatle	Telephone: 605/360-6375
Address: 46735 SD Highway 38	E-Mail: Hatlelinda @gmail.com
Sioux Falls, South Dakota 57107	Contact Info: Phil Gundvaldson, P.E.
Comments / Questions:	605-271-5527 (office) PhilG@InfrastructureDG.com
_ Im not against a 4 lane road, just ag	cinst not gotting a turn lane to get into
my driveway. Continuing on to make a &	turn to me is going to be much more,
dangerous In the winter when the plo	we have not cleared the roads good.
I could possibly get stack in the mow	making a U-turm and get frost
bitton trying to shovel my wehicle out of	
able to flow-across due to a mediamian	
way toget around it. When pulling a long tra	ilar into my yard, all-turn will not be
practical. I do not wish to drive further west	so home. I want and need a turn lane to my
Project website: https://www.sd38corridorstudy.com/ dzirver	Swald have a right to have a lane to
	we without driving further. SD
HRGreen infrastru	ucture DOT

Please place this comment/question form in the designated box on the way out of the meeting, mail to the address on the opposite side of this card, or email comments and questions to <a href="Philo@InfrastructureDG.com">Philo@InfrastructureDG.com</a> by April 19th, 2024.

From Humboldt to Sioux Falls

Allan and Angelia Martens 46061 SD HWY 38	Telephone:E-Mail:				
Comments / Questions:	Contact Info: Phil Gundvaldson, P.E. 605-271-5527 (office) PhilG@InfrastructureDG.com				
	between 460 m + 461 street. turn lave for race track t. Both for safety concerns				



HRGreen





Please place this comment/question form in the designated box on the way out of the meeting, mail to the address on the opposite side of this card, or email comments and questions to PhilG@InfrastructureDG.com by April 19th, 2024.

# South Dakota Highway 38 Corridor Study

From Humboldt to Sioux Falls

	110111	1 Idil in	1	= 7/1-GUS10	1 266
la.	That Have Mills	6	Telephone:	105-364 19	init
Name: In	unic Janoch		E-Mail: \mi		
Address: L	1557 N QSK -31035		Contact Info:	Phil Gundvaldson, P.E. 605-271-5527 (office) PhilG@InfrastructureDG.com	
th	I Questions:  e are land owners along Highway 38 of the afternoon meeting on March 13 <sup>th</sup> in the liveway access to Highway 38. We have been very grateful reates in the winter. In the winter 258 reate an issue if they were to merge out operation. I understand adding a contract but extending our driveway to just the driveway to go to the contract of the driveway to go to the contract of the driveway to go to the contract of the driveway to go to the driveway t	ye managed this farm ful for this access becate street is not alway our driveway to 258th and feed cattle. Add curve on the gravel re oin it would not be a and feedlot which way access from 4581	n for 44 years and cause of the snow ys plowed due to street. We have ing 500 feet to ou oad on 258th wou benefit to us. Ou ould also create per 6 Hwy 38 as is. The	street near Humboldt SD. I attended shows you are wanting to take out a never had an issue with our access to accumulation that this stretch of life the extremely deep snow so this was cattle operation that we need to be a detrimental to life the abenefit for those traveling 2 or septic system runs right up to the problems if disturbed. The best mank you, Mike and Jana Miles	and ————————————————————————————————————
143	3	infrastr	ucture		וטנ
HRG	1080 - this comment/auestion form	in the designated t	oox on the way o	out of the meeting, mail to the ad structureDG com_by April 19 <sup>th</sup> , 20	dress on the )24.

#### **Buffalo Ridge Corporation**

46614 Hwy 38-Buffalo Ridge, SD 57107 (605)528-3931 (605)366-9794 cell

17 April 24

Steve Gramm

SDDOT

Greetings,

It was a pleasure meeting everyone at the SD38 Corridor Study public open house at West Central High School. It is always nice meeting fellow SDSU Civil Engineering graduates.

We own properties adjacent to the SE corner of the I-90/SD38 interchange. The main property being north and south of I-90, bound to the south by SD38, and to the west by the east-bound ramp right-of-way (depicted as A). A smaller six-acre parcel adjacent to the SE corner of the SD38/466<sup>th</sup> Ave intersection rights-of-way (depicted as B).

We paid a premium for these properties due to their location adjacent to their intersection rights-of way. The six-acre lot was purchased at auction over twenty years ago. It sold for more than ten times per acre than the adjacent property south and east, resulting from its intersection location.

We support planned options that mitigate reduction of our property values. Several planned options show relocations of right-of-way for the interstate, highway, and 466<sup>th</sup> Ave. To mitigate reduction in property value, our property lines common to existing rights-of-way must remain common to any relocated rights-of-way. Such scenarios are depicted as hatched areas south of the interstate on the attached aerial photographs.

We support options that least affect our property, including but not limited to land, structures, billboards/signs, utilities, fences etc. We would expect to be compensated/reimbursed for any loss or relocation of any such property. Transfer of state-owned land would be considered as depicted by hatched areas north of the interstate.

Options least affecting our properties include options 5, 8 and 9, with 5 being most desirable. Option 3.2 could be considered if slightly modified. The lot adjacent east of parcel B is owned by ANCO. The proposed alignment of 466<sup>th</sup> Ave runs through the newly

constructed ANCO building. Cemcast is adjacent to parcel B to the south and southeast. Cemcast would probably be agreeable to swap their NW triangular corner for an equal SE corner of parcel B, squaring off both lots. 466<sup>th</sup> Ave could then be aligned dividing parcel B roughly into equal parcels, depicted on option 3.2 and attached Figure A. Again, we would expect compensation to relocate any billboards, structures, utilities, fences etc. We would consider a land transfer of state-owned land for compensation of land lost (hatched area across interstate).

Options 1, 2, 3, 4, 6 & 7 have a much more drastic effect on our properties. We oppose moving 466th Ave unless our existing common property line is moved along with relocated 466th Ave right-of-way. These options also affect significantly more of our land, billboards, structures, utilities and fence etc. Again, state-owned land across interstate could be considered for compensation.

We do not support any medians, raised or painted, prohibiting customers entering or leaving our properties. Several years ago, a painted median prohibiting eastbound traffic from accessing our business was created along SD38. Almost all of our customers exit off I-90 arriving at our business eastbound on SD38. This has resulted in a very negative impact not only on our business but a major inconvenience for our customers.

Most drive past our driveway slowly, prohibited by the painted median. Most find a place to turn around anywhere from 0.1 to 3.0 miles down the highway. Some drive all the way to Marion Road back to I-90W. Many of our customers are big RVs, trailers of all kinds, including campers and semi-trucks, which are difficult to U-turn. We were dumbfounded that not only did nobody ask for our input, but we were also never informed of it beforehand.

Of the three mainline options, we prefer option one, painted, not raised medians. We support a center turn lane allowing both right and left turns similar to the center turn lane just across I-90 along Hartford Heights, a stone's throw away. They have similar driveways and intersections with a center two-way turn lane, approved with a higher traffic count.

In addition, for years the directional guide sign just before the stop sign on the east bound off ramp indicated gas 1.0 mile to the left, when it is 0.1-0.2 miles. Recently the 1.0 mile has been changed to 0.4 mile. This also causes gas customers to drive past only to inconveniently search for a place to turn around without running out of gas.

And last, we do not support the removal of the residential driveway to the east. Both residences each have their own separate driveway. The centerline of the west driveway is thirty feet west of the common property line. If the east property driveway is removed, the

west driveway will have to be moved thirty feet east to straddle the property line (Fig. B). A tree or two would also have to be removed.

In summary, our first preference would be to leave existing conditions unchanged with the exception of the painted median prohibiting access to our business driveway and correcting the directional guide sign at the off ramp.

Our next preference would be options 5,8 & 9. We expect our property lines common with existing rights-of-way will move along with the new rights-of-way depicted as hatched area west of parcel A. Driveway shown across SD38 from 466th intersection. Hatched area west of B and state-owned land across I-90 could be considered for transfer as compensation for property loss. Option 3.2 may be ok with a revised alignment due to the existing building.

Options 1,3, 4 & 6 are preferred least unless parcel B property line commonly shared with the existing right-of-way will move along with new right-of-way. Not doing so would greatly reduce the value of parcel B. The smaller hatched area further west and state-owned land across I-90 could be considered for transfer to compensate for property loss. Parcel B may also have to have driveway access to SD38.

A SD38 painted center turning lane allowing both right and left turns similar to the other side of the I-90 bridge along Hartford Heights is preferred. It is permitted with a higher traffic count with similar driveways and street intersections.

Last, we prefer the existing driveway not to be removed, requiring relocation of the other driveway to straddle the property line and tree removal.

Please feel free to contact me to discuss further.

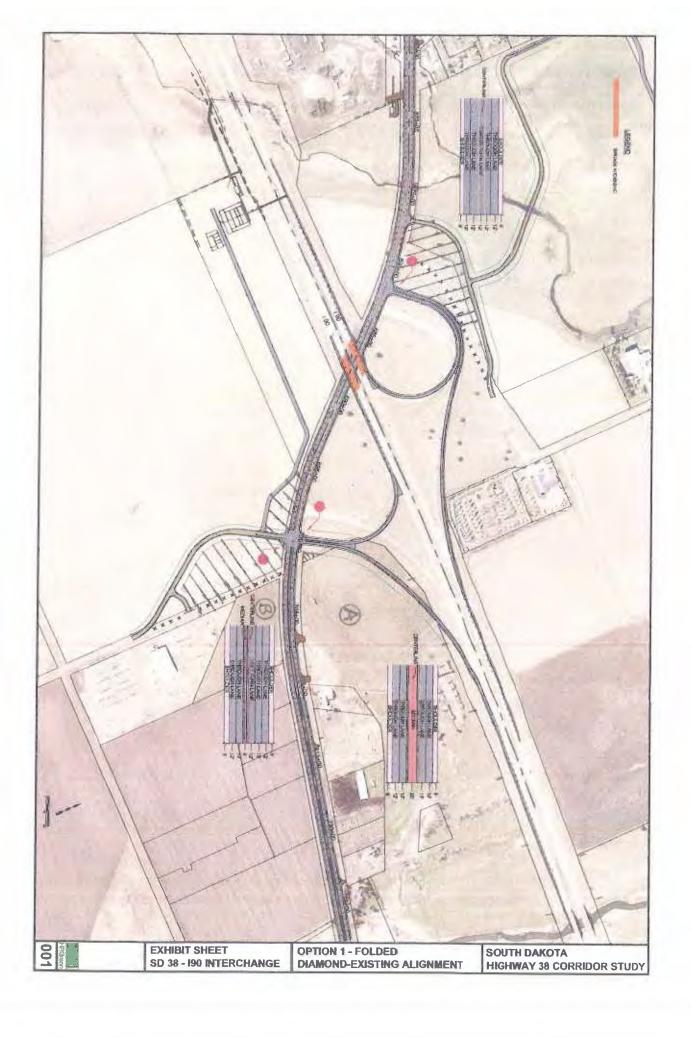
Best Regards,

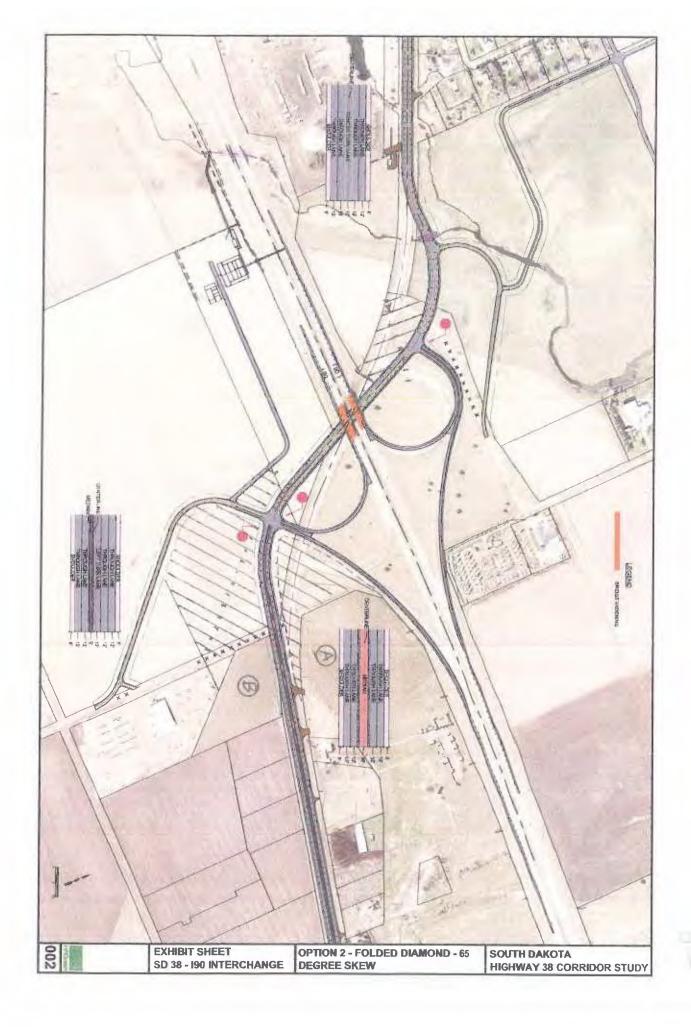
Brad Songstad, PE

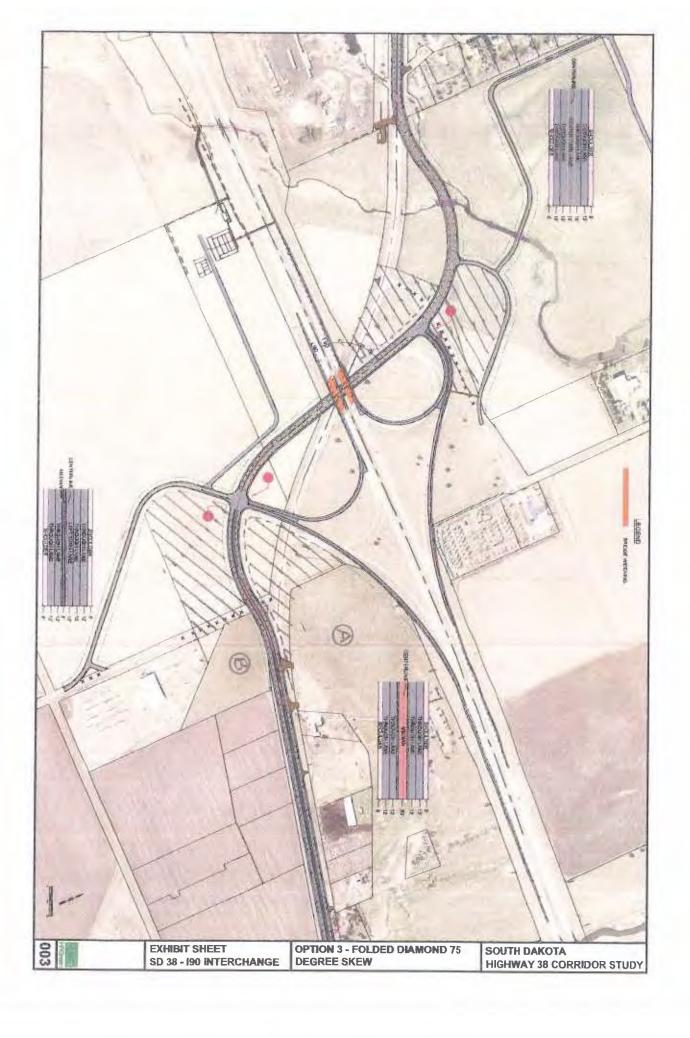
Bro Songstel

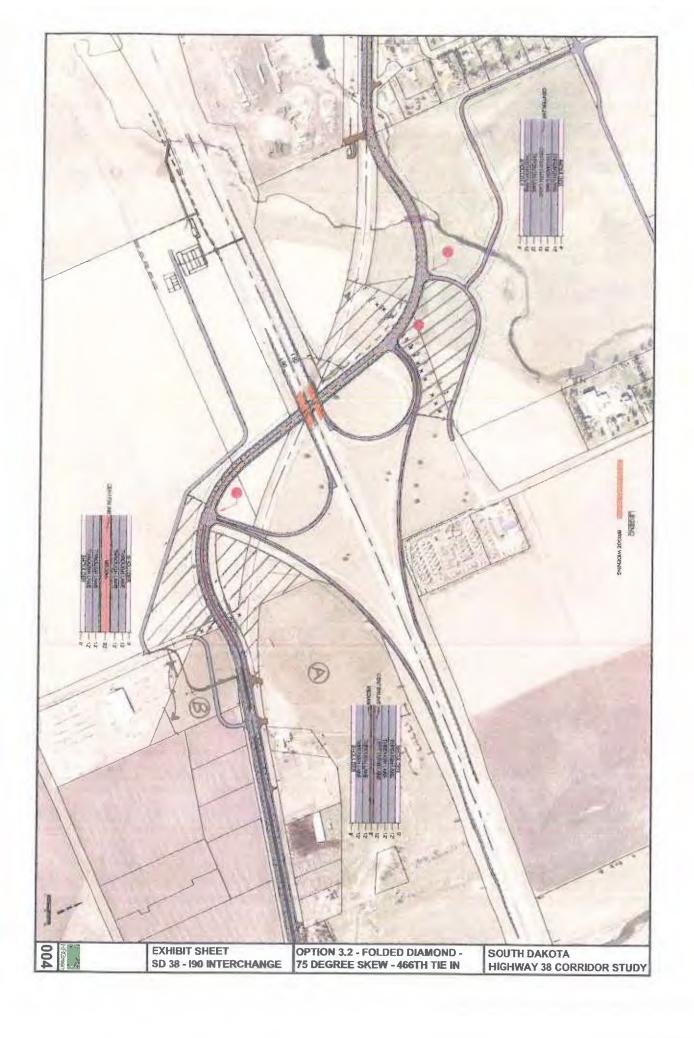
Cc: Michael Paulson-Christopherson, Anderson, Paulson & Fideler

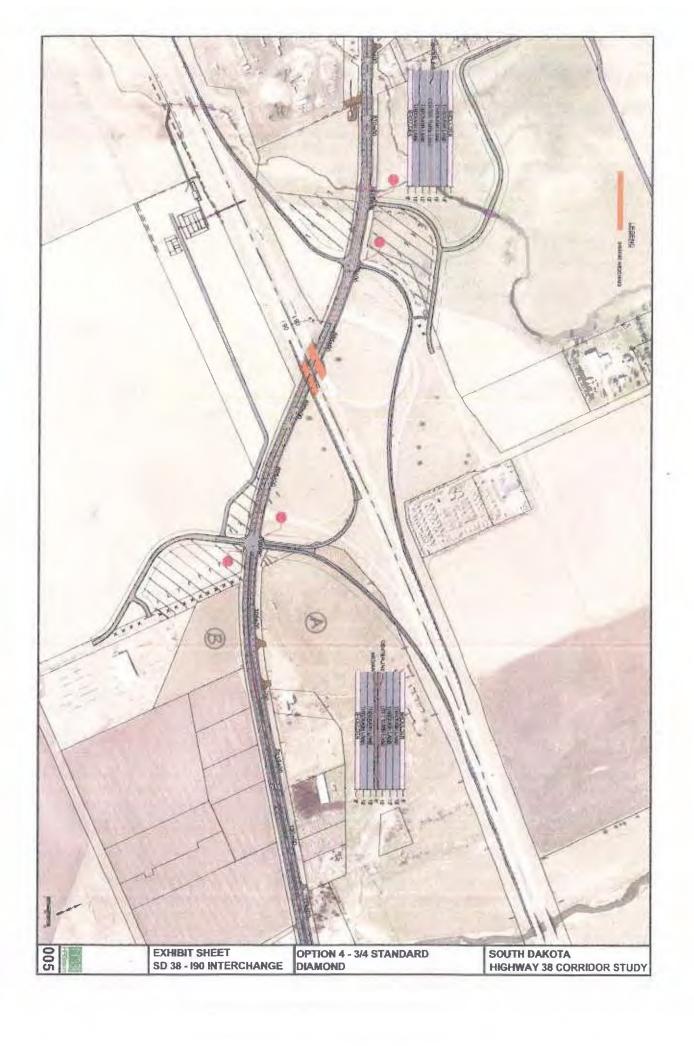
Clint Sargent-Meierhenry Sargent

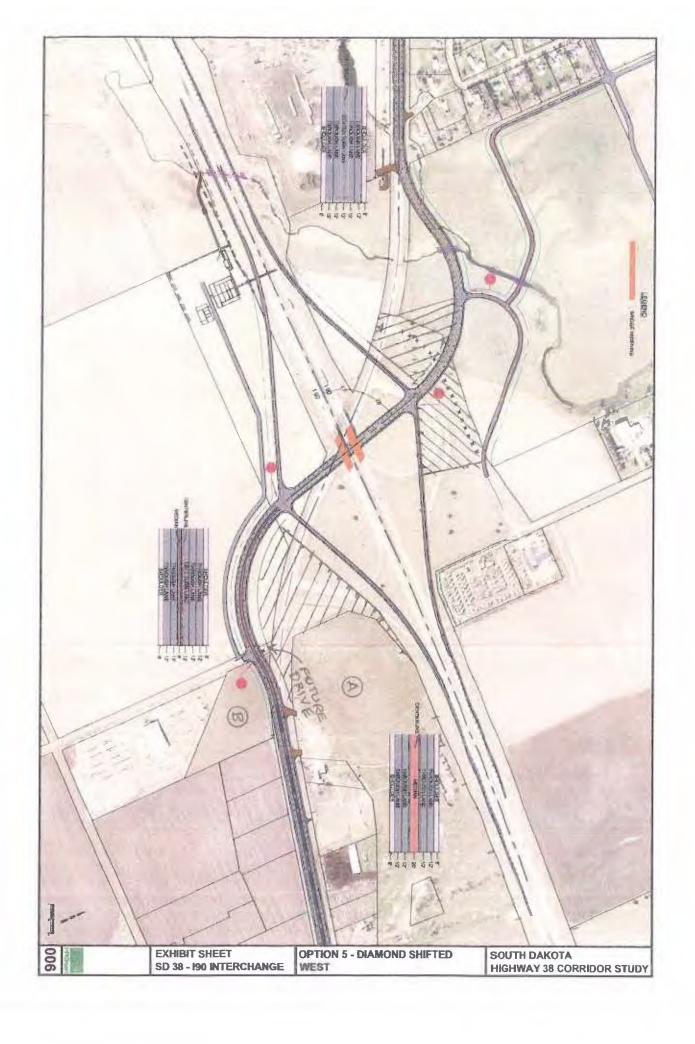


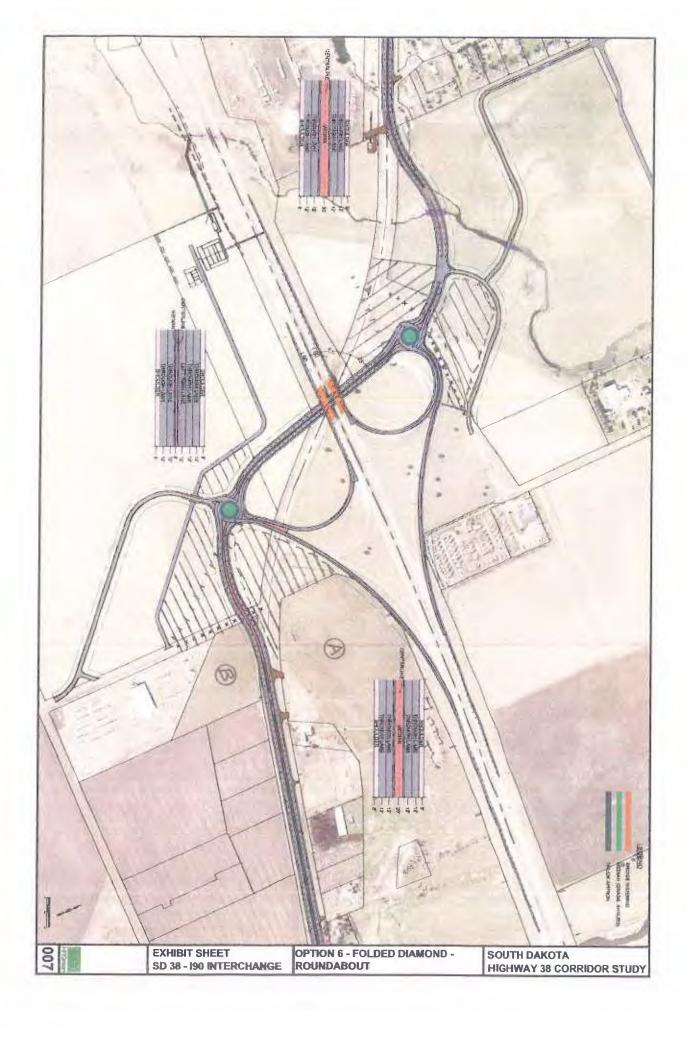












# **Philip Gundvaldson**

From:noreply@socialpinpoint.comSent:Sunday, March 3, 2024 1:58 PMTo:Thoreen, Timothy; White, Ben

**Subject:** New response for survey: SD 38 Corridor Study on project: SD 38 Corridor Study

This email came from outside the HR Green organization. Please use caution when clicking on hyperlinks and opening attachments





The following response on survey: SD 38 Corridor Study from an unknown stakeholder was submitted regarding the project: SD 38 Corridor Study

5/4 Complete

1. Tell us about how you relate to the corridor (check all that apply):

I live near Highway 38

I rely on Highway 38 for my daily commute to Sioux Falls

2.How often do you travel on Highway 38?

Daily

3. What would you like improved on Highway 38? Please rank based on high priority and low priority.

Safety on Highway 38:

1 (high)

Traffic Flow:

4

Pedestrian crossin:

5 (low)

Bike options:

5 (low)

Access to or from Interstate 90:

1 (high)

Access to adjacent land use:

2

Safety at intersections:

1 (high)

4.If I could fix one thing about the Highway 38 corridor, it would be:

Morning commute is busy with buses and trucks. Entry to the highway from intersections is an important need, especially when visibility is limited.

**Review the Survey response now** 

Powered by **Social Pinpoint** 

<u>Unsubscribe From This List</u> | <u>Manage Email Preferences</u>

# **Philip Gundvaldson**

**From:** noreply@socialpinpoint.com

**Sent:** Wednesday, March 13, 2024 2:22 PM **To:** Thoreen, Timothy; White, Ben

**Subject:** New response for survey: SD 38 Corridor Study on project: SD 38 Corridor Study

This email came from outside the HR Green organization. Please use caution when clicking on hyperlinks and opening attachments





The following response on survey: SD 38 Corridor Study from an unknown stakeholder was submitted regarding the project: SD 38 Corridor Study

5/4 Complete

1. Tell us about how you relate to the corridor (check all that apply):

I rely on Highway 38 for my daily commute to Sioux Falls
I live near Highway 38

- 2.How often do you travel on Highway 38? Daily
- 3. What would you like improved on Highway 38? Please rank based on high priority and low priority.

Safety on Highway 38:

2

Traffic Flow:

1 (high)

Pedestrian crossin:

5 (low)

Bike options:

5 (low)

Access to or from Interstate 90:

2

Access to adjacent land use:

5 (low)

Safety at intersections:

3

4.If I could fix one thing about the Highway 38 corridor, it would be:

Better turn lane options or more lanes between Marion and Ellis Rd

**Review the Survey response now** 

Powered by **Social Pinpoint** 

<u>Unsubscribe From This List</u> | <u>Manage Email Preferences</u>

# APPENDIX D: FUTURE BUILD CONCEPT TRAFFIC OPERATIONS AND SAFETY ANALYSIS

**SD38 Corridor Study** 





To: Steve Gramm, SDDOT

From: Brian Willham, PE, PTOE / Ben White, PE

Subject: SD Highway 38 – Future Build Concept Traffic Operations and Safety Analysis

Date: July 19, 2024

#### Introduction

The purpose of this technical memorandum is to document the future build concept traffic assessment in support of the study being completed along SD 38. This technical report will provide a future year conditions assessment of the highway and each of the study intersections. **Table 1** depicts the eighteen study intersections reviewed as part of the existing conditions assessment and traffic data review.

TABLE 1: SD 38 STUDY INTERSECTIONS

Main Line	Cross Street(s)
SD Highway 38	SD Highway 19 / 457th Avenue
SD Highway 38	459 <sup>th</sup> Avenue
SD Highway 38	I-90 Speedway Entrance
SD Highway 38	Western Avenue / 463rd Avenue
SD Highway 38	Main Avenue
SD Highway 38	Vandemark Avenue
SD Highway 38	2 <sup>nd</sup> Street
SD Highway 38	West Central High School Entrance
SD Highway 38	Railroad Street / 464th Avenue
SD Highway 38	Mickelson Road / 260th Street
SD Highway 38	466th Avenue (North)
SD Highway 38	WB I-90 Exit 390
SD Highway 38	EB I-90 Exit 390
SD Highway 38	466th Avenue (South)
SD Highway 38	County Highway 141 / 468th Avenue
SD Highway 38	County Highway 139 / 469th Avenue
SD Highway 38	La Mesa Drive / 470 <sup>th</sup> Avenue
SD Highway 38	Marion Road

## Traffic Forecasting

The existing traffic volume data for the SD 38 corridor was developed from 12-hour count data collected on November 2, 2022, for 17 intersections. To develop future traffic conditions, the Sioux Falls Metropolitan Planning Organization (SFMPO) Travel Demand Model (TDM) and SDDOT GIS data was used to establish the 2050 ADT. Available development site plans were sourced and any planned development trips that had not been included in the TDM were incorporated into the future year forecasted volumes. The growth calculated from the ADT values were used to develop 2050 design year morning (AM) and afternoon (PM) peak hour volumes at study intersections. The estimated interim year 2029 morning (AM) and afternoon (PM) peak hour volumes were developed by process of interpolation using straight-line growth assumptions based on the existing year and future year 2050 traffic volumes. Any adjustments that were necessary to relocate traffic due to intersection modifications within concepts were completed manually. The peak hour volumes were previously used to evaluate the existing condition and



future no-build traffic operations for intersections and highway segments within the study area and the same forecasts will be utilized to evaluate the future build concept conditions.

## Traffic Operations Methodology

#### Intersections

Intersection level of service (LOS) is primarily a function of peak hour turning movement volumes, intersection lane configuration, and traffic control. For intersection analysis, the Highway Capacity Manual (HCM) defines LOS in terms of the average control delay at the intersection in seconds per vehicle. The results of a HCM analysis are typically presented in the form of a letter grade (A-F) that provides a qualitative estimate of the operational efficiency or effectiveness of the corridor. Much like an academic report card, LOS A represents the best range of operating conditions (i.e., motorists experiencing little delay or congestion) and LOS F represents the worst (i.e., extreme delay or severe congestion).

**Table 2** defines the control delay range corresponding to each LOS for unsignalized and signalized intersection locations. At intersections, LOS E is considered to be at capacity and typically represents a scenario in which significant queuing is present or traffic signal cycle failure is evident. For unsignalized intersections, the intersection LOS is given by the worst approach LOS. For instance, an intersection with LOS D on one approach and LOS B on the rest would result in LOS D for the intersection.

TABLE 2: LEVEL OF SERVICE FOR CONTROL DELAY (INTERSECTIONS)

	Unsignalized	Traffic Signal
Level Of Service	Control Delay (sec/veh)	Control Delay (sec/veh)
А	≤ 10	≤ 10
В	> 10 and ≤ 15	> 10 and ≤ 20
С	> 15 and ≤ 25	> 20 and ≤ 35
D	> 25 and ≤ 35	> 35 and ≤ 55
Е	> 35 and ≤ 50	> 55 and ≤ 80
F	> 50	> 80

Source: Highway Capacity Manual, 7th Edition.

Following SDDOT guidance, LOS C is the desired minimum traffic operational goal for intersections in rural environments while LOS D is an acceptable operational goal for intersections in dense urban environments. The intersections within the study area have a desired traffic operational goal of LOS C.

#### **Highways**

Two-lane highway LOS is defined by follower density which relates directly to the passing opportunities available to motorists. In two-lane highway analysis, the highway is segmented according to whether passing zones are present or whether passing is prohibited or otherwise unavailable due to geometric limitations. Multilane highway LOS is defined by density which relates to the ability of a motorist to maneuver freely within the traffic stream. For multilane highway analysis, the highway is segmented anywhere that the uniformity of the traffic or roadway conditions change.

Error! Reference source not found. defines the follower density range corresponding to each LOS for two-lane highway segments. On two-lane highways, LOS E is considered to be at capacity. For two-lane highway segments, a LOS B would represent a scenario where some platooning is present with the potential passing demand and passing opportunities balanced while a LOS D would represent a scenario where significant platooning is present and passing demand far exceeds passing opportunities.



TABLE 3: LEVEL OF SERVICE FOR FOLLOWER DENSITY (TWO-LANE HIGHWAYS)

	Speed ≥ 50 mph	Speed < 50 mph
Level Of Service	Follower Density (followers/mi/ln)	Follower Density (followers/mi/ln)
А	≤ 2.0	≤ 2.5
В	> 2.0 - 4.0	> 2.5 – 5.0
С	> 4.0 - 8.0	> 5.0 - 10.0
D	> 8.0 – 12.0	> 10.0 – 15.0
E	> 12.0	> 15.0
F	Demand exce	eeds capacity

Source: Highway Capacity Manual, 7th Edition.

**Table 4** defines the follower density range corresponding to each LOS for multilane highway segments. On multilane highways, LOS E is considered to be at capacity. For multilane highway segments, a LOS B represents a reasonably free-flowing condition with minimal maneuvering restrictions while a LOS D would represent a scenario where speeds begin to decline and freedom to maneuver is limited.

TABLE 4: LEVEL OF SERVICE FOR FOLLOWER DENSITY (MULTILANE HIGHWAYS)

Level Of Service	Free-Flow Speed (mph)	Density (passenger cars/mi/ln)
А		≤ 11.0
В		> 11.0 - 18.0
С		> 18.0 – 26.0
D		> 26.0 - 35.0
	60	> 35.0 - 40.0
E	55	> 35.0 - 41.0
	50	> 35.0 - 43.0
	45	> 35.0 – 45.0
	60	> 43.0
F	55	> 45.0
Г	50	> 43.0
	45	> 45.0

Source: Highway Capacity Manual, 7th Edition.

Following SDDOT guidance, LOS C is the desired traffic operational goal for highways in rural environments and LOS D is considered the minimal acceptable operations for highways in urban environments. The SD 38 highway segments within the study area are categorized as rural with federal functional classification of collector between Humboldt to Hartford and categorized as urban with federal functional classification of minor arterial between Hartford to Sioux Falls. The highway segments within the study area have a desired traffic operational goal of LOS C with minimum allowable LOS D between Hartford to Sioux Falls.

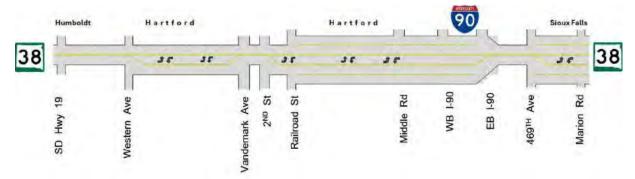
## Future Build Corridor Concepts

Opening Year 2029 and Design Year 2050 traffic volume forecasts were used to evaluate the traffic operations of intersections and the highway corridor under the build concepts. Operational analysis was completed for the AM and PM peak hour periods of each scenario. Build concept plans are available under separate cover.

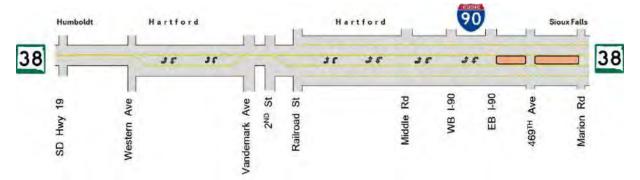
The following 3 Build condition scenarios were evaluated:



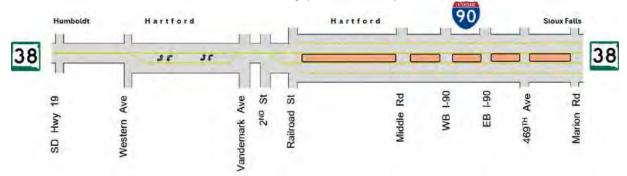
 Alternative 1 – two-lane highway from Humboldt to Hartford (as existing), three-lane roadway from Western Avenue to Railroad Street, five-lane roadway (center TWLTL) between Railroad Street to the I-90 Exit 390 interchange, two-lane highway from the I-90 interchange to 469<sup>th</sup> Ave/County Highway 139, and five-lane roadway (center TWLTL) from 460<sup>th</sup> Ave/County Highway 139 to Sioux Falls.



Alternative 2 – two-lane highway from Humboldt to Hartford (as existing), three-lane roadway from Western
Avenue to Railroad Street, five-lane roadway (center TWLTL) between Railroad Street to the I-90 Exit 390
interchange, four-lane highway (raised median) from the I-90 interchange to Sioux Falls.



• Alternative 3 - two-lane highway from Humboldt to Hartford (as existing), three-lane roadway from Western Avenue to Railroad Street, four-lane roadway (raised median) between Railroad Street to Sioux Falls.



## **Future Traffic Operations**

Traffic operations analysis for the study area intersections included capacity evaluation using the Highway Capacity Manual (HCM) 7<sup>th</sup> Edition two-lane highway and multilane highway methodologies through use of the Highway Capacity Software (HCS) 2022. Output reports from the HCS2022 software are available in the Appendix.

Traffic operations analysis for the study area SD Highway 38 corridor included capacity evaluation using the Highway Capacity Manual (HCM) 7<sup>th</sup> Edition two-lane highway and multilane highway methodologies through use of the Highway Capacity Software (HCS) 2022. The highway traffic operations analysis used conceptual highway geometry, future year traffic volumes, and design speeds.



The future year traffic operations analyses does not include a comparison of concepts for the interchange ramp terminal intersections due to that portion of the corridor being removed from this study for inclusion in a future study to document the potential changes to interstate access.

## Opening Year 2029

The results of the Opening Year 2029 intersection capacity analyses can be seen in **Table 5**. The results of the two-lane highway and multilane highway corridor capacity analyses can be seen in **Table 6** and **Table 7**.

Under the Opening Year 2029 conditions, the traffic operations analyses showed acceptable operations at all intersections within the study area, under all alternative scenarios, with intersections achieving LOS C or greater during both the AM and PM peak hours.

Under the Opening Year 2029 conditions, the traffic operations analyses showed acceptable operations at all of the highway segments within the study area, under all alternative scenarios, with all segments achieving LOS B or greater during both the AM and PM peak hours.

In general, the Opening Year 2029 condition traffic operations demonstrated acceptable performance measures at all intersections and highway segments within the study area. The desired LOS was realized for all intersections and highway segments during the AM and PM peak hours for all concepts.



TABLE 5: HCM TRAFFIC INTERSECTION OPERATIONS – OPENING 2029

		Inters	ection		NO-E	UILD			ALTERN	ATIVE 1			LTERN	ATIVE 2			ALTERN	ATIVE 3	
ID#	SD Hwy 38	Con		AM PEA	K HOUR	PM PEA	K HOUR	AM PEA	( HOUR	PM PEAR	( HOUR	AM PEAK	HOUR	PM PEAR	( HOUR	AM PEA	( HOUR	PM PEAK	HOUR
	Cross Street(s)	No Build	Build	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
1	SD Highway 19 / 457 <sup>th</sup> Avenue	TWSC	TWSC	10.4	В	10.5	В	10.6	В	10.8	В	10.6	В	10.8	В	10.6	В	10.8	В
2	459 <sup>th</sup> Avenue	TWSC	TWSC	10.4	В	11.8	В	7.6	Α	11.8	В	7.6	Α	11.8	В	7.6	А	11.8	В
3	I-90 Speedway Entrance	TWSC	TWSC	0.0	Α	0.0	А	0.0	А	0.0	А	0.0	А	0.0	А	0.0	А	0.0	А
4	Western Avenue / 463 <sup>rd</sup> Avenue	TWSC	TWSC	13.5	В	16.5	С	13.0	В	15.2	С	13.0	В	15.2	С	13.0	В	15.2	С
5	Main Avenue	TWSC	TWSC	12.0	В	15.2	С	11.3	В	12.9	В	11.3	В	12.9	В	11.3	В	12.9	В
6	Vandemark Avenue	TWSC	TWSC	12.6	В	12.7	В	12.7	В	12.7	В	12.7	В	12.7	В	12.7	В	12.7	В
7	2 <sup>nd</sup> Street	TWSC	Signal	16.6	С	18.5	С	6.4	А	6.4	А	6.4	А	6.4	Α	6.4	А	6.4	А
8	West Central High School Entrance	TWSC	TWSC	12.1	В	12.0	В	10.5	В	10.2	В	10.5	В	10.2	В	10.5	В	10.2	В
9	Railroad Street / 464 <sup>th</sup> Avenue	TWSC	TWSC	18.2	С	19.8	С	17.5	С	18.4	С	17.5	С	18.4	С	17.5	В	18.4	В
10	Mickelson Road / 260 <sup>th</sup> Street	TWSC	Signal	24.8	С	54.5	F	11.9	В	11.2	В	11.9	В	11.2	В	11.9	В	11.2	В
11	466th Avenue (North)	TWSC	TWSC	19.5	С	20.3	С	11.6	В	13.9	В	11.6	В	13.9	В	16.3	С	18.8	С
12	WB I-90 Exit 390	TWSC	NA	11.5	В	17.7	С	NA	-	NA	-	NA	-	NA	-	NA	-	NA	-
13	EB I-90 Exit 390	TWSC	NA	12.3	В	15.4	С	NA	-	NA	-	NA	-	NA	-	NA	-	NA	-
14	466th Avenue (South)	TWSC	NA	11.9	В	12.3	В	NA	-	NA	-	NA	-	NA	-	NA	-	NA	-
15	County Highway 141 / 468 <sup>th</sup> Avenue	TWSC	TWSC	13.5	В	14.5	В	13.5	В	14.5	В	14.1	В	13.6	В	14.1	В	13.6	В
16	County Highway 139 / 469 <sup>th</sup> Avenue	TWSC	TWSC	14.2	В	18.5	С	11.8	В	14.9	В	11.4	В	15.4	В	11.4	В	15.4	В
17	La Mesa Drive / 470 <sup>th</sup> Avenue	TWSC	TWSC	17.0	С	21.7	С	15.1	С	17.8	С	15.1	В	17.8	В	15.1	В	17.8	В
18	Marion Road	Signal	Signal	16.2	В	20.6	С	16.2	В	20.6	С	16.2	В	20.6	С	16.2	В	20.6	С

Notes: Bold/Highlighted Color indicates a poor LOS



TABLE 6: HCM TRAFFIC HIGHWAY OPERATIONS - OPENING 2029, EASTBOUND SD 38

	Segme	nt Toma		NO	BUILD			ALTERN	ATIVE 1			ALTERN	ATIVE 2			ALTER	NATIVE 3	
ID#	Segme	пстуре	AM PEAK	HOUR	РМ РЕАК НО	DUR	AM PEAK	HOUR	PM PEAK	HOUR	AM PEAK	HOUR	PM PEAK	HOUR	AM PEA	( HOUR	PM PEA	K HOUR
	No Build	Build	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS
EB 1	Passing Zone	Passing Zone	0.6	Α	0.2	Α	0.6	Α	0.2	Α	0.6	Α	0.2	Α	0.6	Α	0.2	Α
EB 2	Passing Constrained	Passing Constrained	0.7	Α	0.3	Α	0.7	А	0.3	А	0.7	А	0.3	А	0.7	А	0.3	А
EB 3	Passing Zone	Passing Zone	0.5	Α	0.2	Α	0.5	А	0.2	Α	0.5	Α	0.2	Α	0.5	Α	0.2	Α
EB 4	Passing Constrained	Passing Constrained	0.7	Α	0.3	Α	0.7	А	0.3	А	0.7	А	0.3	А	0.7	Α	0.3	А
EB 5	Passing Zone	Passing Zone	0.5	Α	0.2	Α	0.5	Α	0.2	Α	0.5	Α	0.2	Α	0.5	Α	0.2	Α
EB 6	Passing Constrained	Passing Constrained	0.7	Α	0.3	Α	0.7	А	0.3	А	0.7	А	0.3	А	0.7	А	0.3	А
EB 7	Passing Zone	Passing Zone	0.6	Α	0.2	Α	0.6	Α	0.2	А	0.6	Α	0.2	Α	0.6	Α	0.2	Α
EB 8	Passing Zone	Passing Zone	0.5	Α	0.2	Α	0.5	А	0.2	А	0.5	А	0.2	А	0.5	Α	0.2	А
EB 9	Passing Constrained	Passing Constrained	0.7	Α	0.3	Α	0.7	А	0.3	Α	0.7	Α	0.3	Α	0.7	Α	0.3	Α
EB 10	Passing Zone	Passing Zone	0.5	Α	0.2	Α	0.5	А	0.2	А	0.5	А	0.2	А	0.5	Α	0.2	А
EB 11	Passing Zone	Passing Zone	0.6	Α	0.3	Α	0.6	Α	0.3	А	0.6	Α	0.3	Α	0.6	Α	0.3	Α
EB 12	Passing Constrained	Passing Constrained	0.7	Α	0.4	Α	0.7	А	0.4	А	0.7	А	0.4	Α	0.7	Α	0.4	А
EB 13	Passing Zone	Passing Zone	0.6	Α	0.3	Α	0.6	А	0.3	А	0.6	А	0.3	А	0.6	Α	0.3	А
EB 14	Passing Constrained	Passing Constrained	1.3	Α	0.7	Α	1.3	А	0.7	А	1.3	А	0.7	Α	1.3	Α	0.7	А
EB 15	Passing Zone		3.7	В	1.8	Α												
EB 16	Passing Constrained	Multilane	4.1	С	1.9	Α	4.7	Α	3.5	Α	4.7	Α	3.5	Α	4.7	Α	3.5	Α
EB 17	Passing Zone		3.7	В	1.8	Α												
EB 18	Passing Zone	Multilana	3.6	В	1.3	Α	4.2	Α.	2.0	Α	4.2	Δ.	2.0	Α.	4.2	Δ.	2.0	Δ.
EB 19	Passing Constrained	Multilane	3.6	В	1.3	Α	4.3	Α	2.9	Α	4.3	Α	2.9	Α	4.3	Α	2.9	Α
EB 20	Passing Constrained	Multilane	3.8	В	1.4	Α	4.6	Α	2.9	Α	4.6	Α	2.9	Α	4.6	Α	2.9	Α
EB 21	Passing Constrained	Multilane	4.0	С	1.7	Α	4.4	Α	2.8	Α	4.4	Α	2.8	Α	4.4	Α	2.8	Α
EB 22	Passing Constrained	NA	1.2	Α	1.0	Α	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
EB 23	Passing Constrained		1.3	Α	1.0	Α	1.3	А	1.0	А								
EB 24	Passing Zone		1.1	Α	0.9	Α	1.1	А	0.9	А								
EB 25	Passing Constrained	Multilane <sup>2,3</sup>	1.2	Α	1.0	Α	1.2	Α	1.0	А	2.3	^	2.0		2.3	^	2.0	^
EB 26	Passing Zone	iviuitiiai le-,°	1.1	Α	0.8	Α	1.1	Α	0.8	А	2.3	Α	2.0	Α	2.3	Α	2.0	Α
EB 27	Passing Constrained		1.3	Α	1.0	Α	1.3	А	1.0	А								
EB 28	Passing Zone		1.1	Α	0.9	Α	1.1	А	0.9	А								
EB 29	Passing Zone		1.5	Α	1.2	Α	1.5	Α	1.2	А								
EB 30	Passing Constrained	Multilana <sup>23</sup>	1.6	Α	1.3	Α	1.6	А	1.3	А	2.4		2.0		0.4		0.0	
EB 31	Passing Zone	- Multilane <sup>2,3</sup>	1.4	Α	1.2	Α	1.4	А	1.2	А	2.4	Α	2.2	Α	2.4	Α	2.2	Α
EB 32	Passing Constrained	1	1.6	Α	1.3	Α	1.6	А	1.3	А								
EB 33	Passing Constrained		4.2	С	1.3	Α												
EB 34	Passing Zone	Multilane	3.9	В	1.3	Α	3.6	Α	2.2	Α	3.6	Α	2.2	Α	3.6	Α	2.2	Α
EB 35	Passing Constrained		4.0	С	1.2	Α												

Notes: Bold indicates Multilane Highway
Highlighted Color indicates a poor LOS
NA indicates Segment Removed for Build Conditions
Multilane<sup>2,3</sup> indicates segment type for Alternative 2 and 3



TABLE 7: HCM TRAFFIC HIGHWAY OPERATIONS – OPENING 2029, WESTBOUND SD 38

	Soame	ent Type		NO B	UILD		1	ALTERNAT	IVE 1		А	LTERN	IATIVE 2		A	LTERN	ATIVE 3	
ID#	Segille	ant Type	AM PEAK	HOUR	PM PEAP	HOUR	AM PEA	( HOUR	PM PEAK	HOUR	AM PEAK	HOUR	PM PEAK	HOUR	AM PEAR	HOUR	PM PEAK	HOUR
	No Build	Build	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS
WB 1	Passing Constrained		0.7	Α	4.4	С												
WB 2	Passing Zone	Multilane	0.7	Α	4.2	С	1.6	А	3.8	Α	1.6	Α	3.8	Α	1.6	Α	3.8	Α
WB 3	Passing Constrained		0.8	Α	2.1	В												
WB 4	Passing Zone		0.8	Α	2.1	В	0.8	А	2.1	В								
WB 5	Passing Constrained	Multilane <sup>2,3</sup>	0.7	Α	2.0	Α	0.7	А	2.0	А	1.9	Α	2.8	Α	1.9	Α	2.8	Α
WB 6	Passing Zone	Williane *	0.8	Α	2.1	В	0.8	А	2.1	В	1.9	_ A	2.0	_ A	1.5	A	2.0	^
WB 7	Passing Constrained		0.6	Α	1.5	Α	0.6	А	1.5	Α								
WB 8	Passing Constrained		0.7	Α	1.7	Α	0.7	А	1.7	Α								
WB 9	Passing Zone		0.6	Α	1.4	Α	0.6	А	1.4	Α								
WB 10	Passing Constrained	Multilane <sup>2,3</sup>	0.7	Α	1.6	Α	0.7	А	1.6	Α	1.7	Α	2.7	Α	1.6	Α	2.5	Α
WB 11	Passing Zone	Wulliane-,	0.6	Α	1.5	Α	0.6	А	1.5	Α	1.7	A	2.7	A	1.0	A	2.5	A
WB 12	Passing Constrained		0.7	Α	1.7	Α	0.7	А	1.7	Α								
WB 13	Passing Constrained		0.7	Α	1.7	Α	0.7	А	1.7	Α								
WB 14	Passing Constrained	NA	0.9	Α	2.1	В	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
WB 15	Passing Constrained	Multilane	0.8	Α	1.7	Α	1.8	Α	3.0	Α	1.8	Α	3.0	Α	1.8	Α	3.0	Α
WB 16	Passing Constrained	Multilane	1.5	Α	5.1	С	3.1	Α	5.5	Α	3.1	Α	5.5	Α	3.1	Α	5.5	Α
WB 17	Passing Constrained		1.3	Α	4.9	С												
WB 18	Passing Constrained	Multilane	1.4	Α	5.1	С	2.9	Α	5.3	Α	2.9	A	5.3	A	2.9	A	5.3	A
WB 19	Passing Zone	Mulliane	1.3	Α	4.8	С	2.9	A	5.3	A	2.5	A	5.5	A	2.5	A	5.5	A
WB 20	Passing Constrained		1.4	Α	5.1	С												
WB 21	Passing Constrained	Multilane	1.9	Α	5.1	С	2.4	۸	5.6	^	3.4	Α	5.6		3.4	Α.	5.6	A
WB 22	Passing Zone	Mulliane	1.7	Α	4.1	С	3.4	Α	5.6	Α	3.4	A	5.6	Α	3.4	Α	5.6	A
WB 23	Passing Zone	Passing Zone	0.3	Α	0.7	Α	0.3	А	0.7	Α	0.3	Α	0.7	Α	0.3	Α	0.7	Α
WB 24	Passing Constrained	Passing Constrained	0.3	Α	0.7	Α	0.3	А	0.7	Α	0.3	А	0.7	А	0.3	А	0.7	Α
WB 25	Passing Constrained	Passing Constrained	0.3	Α	0.6	Α	0.3	А	0.6	Α	0.3	Α	0.6	Α	0.3	Α	0.6	Α
WB 26	Passing Zone	Passing Zone	0.3	Α	0.8	Α	0.3	А	0.8	А	0.3	А	0.8	Α	0.3	Α	0.8	Α
WB 27	Passing Zone	Passing Zone	0.3	Α	0.6	Α	0.3	А	0.6	Α	0.3	Α	0.6	Α	0.3	Α	0.6	Α
WB 28	Passing Zone	Passing Zone	0.3	Α	0.8	Α	0.3	А	0.8	Α	0.3	А	0.8	А	0.3	Α	0.8	Α
WB 29	Passing Zone	Passing Zone	0.3	Α	0.8	Α	0.3	А	0.8	Α	0.3	Α	0.8	Α	0.3	Α	0.8	Α
WB 30	Passing Constrained	Passing Constrained	0.3	Α	0.6	Α	0.3	А	0.6	А	0.3	А	0.6	А	0.3	А	0.6	Α
WB 31	Passing Zone	Passing Zone	0.3	Α	0.8	Α	0.3	А	0.8	Α	0.3	А	0.8	А	0.3	А	0.8	Α
WB 32	Passing Constrained	Passing Constrained	0.3	Α	0.7	Α	0.3	А	0.7	А	0.3	А	0.7	А	0.3	А	0.7	Α
WB 33	Passing Constrained	Passing Constrained	0.3	Α	0.8	Α	0.3	Α	0.8	А	0.3	А	0.8	А	0.3	Α	0.8	Α
WB 34	Passing Zone	Passing Zone	0.3	Α	0.7	Α	0.3	А	0.7	А	0.3	А	0.7	А	0.3	Α	0.7	А
WB 35	Passing Constrained	Passing Constrained	0.5	Α	1.0	Α	0.5	А	1.0	А	0.5	А	1.0	А	0.5	Α	1.0	Α

8

Notes: Bold indicates Multilane Highway
Highlighted Color indicates a poor LOS
NA indicates Segment Removed for Build Conditions
Multilane<sup>2,3</sup> indicates segment type for Alternative 2 and 3



### Design Year 2050

The results of the Design Year 2050 intersection capacity analyses can be seen in **Table 8**. The results of the two-lane highway and multilane highway corridor capacity analyses can be seen in **Table 9** and **Notes**: Bold indicates Multilane Highway

Highlighted Color indicates a poor LOS NA indicates Segment Removed for Build Conditions Multilane2,3 indicates segment type for Alternative 2 and 3

#### Table 10.

Under the Design Year 2050 conditions, the traffic operations analyses showed acceptable operations at all intersections within the study area, with intersections achieving LOS C or greater during both the AM and PM peak hours. To achieve LOS C goals, it was necessary to convert several intersections from two-way stop control to signalized operations. The following intersections were analyzed under traffic signal control:

- SD 38 & Western Avenue/463rd Avenue
- SD 38 & 2<sup>nd</sup> Street
- SD 38 & Railroad Street/464th Avenue
- SD 38 & Mickelson Road/260th Street
- SD 38 & County Highway 139/469th Avenue
- SD 38 & La Mesa Drive/470th Avenue

Under the Design Year 2050 conditions, the traffic operations analyses showed acceptable operations at all of the highway segments within the study area, under all alternative scenarios, with all segments achieving LOS C or greater during both the AM and PM peak hours.

In general, the Design Year 2050 condition traffic operations demonstrated acceptable performance measures at all intersections and highway segments within the study area. The desired LOS was realized for all intersections and highway segments during the AM and PM peak hours for all concepts.



TABLE 8: HCM TRAFFIC INTERSECTION OPERATIONS – DESIGN 2050

					NO E	UILD			ALTERN	ATIVE 1		Į.	ALTERN	ATIVE 2			ALTERN	IATIVE 3	
ID#	SD Hwy 38 Cross Street(s)	Intersection	on Control	AM PEA	K HOUR	PM PEA	K HOUR	AM PEA	( HOUR	PM PEAI	( HOUR	AM PEAR	( HOUR	PM PEA	( HOUR	AM PEA	K HOUR	PM PEAK	K HOUR
	31333 341334(3)	No Build	Build	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
1	SD Highway 19 / 457 <sup>th</sup> Avenue	TWSC	TWSC	12.2	В	12.3	В	12.2	В	12.6	В	12.2	В	12.6	В	12.2	В	12.6	В
2	459 <sup>th</sup> Avenue	TWSC	TWSC	11.6	В	13.5	В	11.7	В	13.5	В	11.7	В	13.5	В	11.7	В	13.5	В
3	I-90 Speedway Entrance	TWSC	TWSC	0.0	Α	0.0	Α	0.0	А	0.0	А	0.0	А	0.0	А	0.0	А	0.0	А
4	Western Avenue / 463 <sup>rd</sup> Avenue	TWSC	Signal	21.5	С	63.1	F	11.4	В	12.1	В	11.4	В	12.1	В	11.4	В	12.1	В
5	Main Avenue	TWSC	TWSC	14.4	В	25.5	D	13.2	В	17.6	С	13.2	В	17.6	С	13.2	В	17.6	С
6	Vandemark Avenue	TWSC	TWSC	15.4	С	16.8	С	15.5	С	16.9	С	15.5	С	16.9	С	15.5	С	16.9	С
7	2 <sup>nd</sup> Street	TWSC	Signal	31.1	С	38.3	E	8.8	Α	7.2	Α	8.8	А	7.2	Α	8.8	А	7.2	Α
8	West Central High School Entrance	TWSC	TWSC	15.4	С	14.8	В	11.9	В	11.3	В	11.9	В	11.3	В	11.9	В	11.3	В
9	Railroad Street / 464 <sup>th</sup> Avenue	TWSC	Signal	43.6	E	43.8	E	11.1	В	8.4	А	11.1	В	8.4	А	11.1	В	8.4	А
10	Mickelson Road / 260 <sup>th</sup> Street	TWSC	Signal	19.2	В	21.3	С	16.7	В	18.2	В	16.7	В	18.2	В	16.7	В	18.2	В
11	466th Avenue (North)	TWSC	TWSC	31.6	D	31.4	D	12.9	В	17.3	С	12.9	В	17.3	С	12.9	В	17.3	С
12	WB I-90 Exit 390	-	-	14.9	В	66.1	F	-	-	-	-	-	-	-	-	-	-	-	-
13	EB I-90 Exit 390	-	-	18.4	С	30.0	D	-	-	-	-	-	-	-	-	-	-	-	-
14	466 <sup>th</sup> Avenue (South)	-	-	13.9	В	15.7	С	-	-	-	-	-	-	-	-	-	-	-	-
15	County Highway 141 / 468 <sup>th</sup> Avenue	TWSC	TWSC	16.7	С	21.3	С	16.9	С	21.3	С	17.9	С	18.8	С	17.9	С	18.8	С
16	County Highway 139 / 469 <sup>th</sup> Avenue	TWSC	Signal	43.1	E	266.3	F	19.5	В	13.7	В	17.6	В	10.8	В	17.6	В	10.8	В
17	La Mesa Drive / 470 <sup>th</sup> Avenue	TWSC	Signal	39.2	E	81.5	F	10.6	В	16.8	В	10.6	В	16.8	В	10.6	В	16.8	В
18	Marion Road	TWSC	Signal	19.1	В	32.1	С	19.1	В	32.1	С	19.1	В	32.1	С	19.1	В	32.1	С

Notes: Bold/Highlighted Color indicates a poor LOS



TABLE 9: HCM TRAFFIC HIGHWAY OPERATIONS – DESIGN 2050, EASTBOUND SD 38

	Segme	ent Type		NO B	UILD			ALTERNA <sup>*</sup>	TIVE 1		А	LTERN	IATIVE 2			LTERN	ATIVE 3	
ID#			AM PEAK	HOUR	PM PEA	K HOUR	AM PEA	K HOUR	PM PEAK	HOUR	AM PEAK	HOUR	PM PEAK	HOUR	AM PEAK	HOUR	PM PEAK	HOUR
	No Build	Build	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS
EB 1	Passing Zone	Passing Zone	1.2	Α	0.6	Α	1.2	Α	0.6	Α	1.2	Α	0.6	Α	1.2	Α	0.6	Α
EB 2	Passing Constrained	Passing Constrained	1.3	Α	0.6	Α	1.3	А	0.6	Α	1.3	Α	0.6	Α	1.3	Α	0.6	Α
EB 3	Passing Zone	Passing Zone	1.1	Α	0.5	Α	1.1	А	0.5	Α	1.1	Α	0.5	Α	1.1	Α	0.5	Α
EB 4	Passing Constrained	Passing Constrained	1.3	Α	0.6	Α	1.3	А	0.6	Α	1.3	А	0.6	А	1.3	А	0.6	А
EB 5	Passing Zone	Passing Zone	1.1	Α	0.5	Α	1.1	А	0.5	Α	1.1	Α	0.5	Α	1.1	А	0.5	Α
EB 6	Passing Constrained	Passing Constrained	1.3	Α	0.6	Α	1.3	А	0.6	А	1.3	А	0.6	А	1.3	А	0.6	А
EB 7	Passing Zone	Passing Zone	1.2	Α	0.6	Α	1.2	Α	0.6	Α	1.2	Α	0.6	Α	1.2	Α	0.6	Α
EB 8	Passing Zone	Passing Zone	1.1	Α	0.6	Α	1.1	А	0.6	Α	1.1	А	0.6	А	1.1	А	0.6	А
EB 9	Passing Constrained	Passing Constrained	1.3	Α	0.7	Α	1.3	Α	0.7	Α	1.3	Α	0.7	Α	1.3	Α	0.7	Α
EB 10	Passing Zone	Passing Zone	1.1	Α	0.5	Α	1.1	А	0.5	А	1.1	А	0.5	Α	1.1	Α	0.5	А
EB 11	Passing Zone	Passing Zone	1.1	Α	0.5	Α	1.1	А	0.5	Α	1.1	Α	0.5	Α	1.1	Α	0.5	Α
EB 12	Passing Constrained	Passing Constrained	1.3	Α	0.7	Α	1.3	А	0.7	Α	1.3	А	0.7	Α	1.3	А	0.7	Α
EB 13	Passing Zone	Passing Zone	1.1	Α	0.5	Α	1.1	А	0.5	Α	1.1	Α	0.5	Α	1.1	А	0.5	Α
EB 14	Passing Constrained	Passing Constrained	2.1	В	1.1	Α	2.1	В	1.1	Α	2.1	В	1.1	А	2.1	В	1.1	Α
EB 15	Passing Zone		6.7	С	3.4	В												
EB 16	Passing Constrained	Multilane Highway	7.1	С	3.5	В	6.7	Α	4.8	Α	6.7	Α	4.8	Α	6.7	Α	4.8	Α
EB 17	Passing Zone	1	6.7	С	3.4	В	1											
EB 18	Passing Zone	Multilege Highway	8.1	D	3.3	В		Α	4.0				4.0		0.0		4.0	
EB 19	Passing Constrained	Multilane Highway	7.9	С	3.1	В	6.0	Α	4.0	Α	6.0	Α	4.0	Α	6.0	Α	4.0	Α
EB 20	Passing Constrained	Multilane Highway	8.3	D	3.4	В	6.4	Α	4.0	Α	6.4	Α	4.0	Α	6.4	Α	4.0	Α
EB 21	Passing Constrained	Multilane Highway	9.2	D	4.2	С	6.2	Α	4.0	Α	6.2	Α	4.0	Α	6.2	Α	4.0	Α
EB 22	Passing Constrained	NA	3.2	В	2.9	В	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
EB 23	Passing Constrained		2.4	В	1.9	Α	3.0	В	2.3	В								
EB 24	Passing Zone		2.2	В	1.8	Α	2.8	В	2.2	В								
EB 25	Passing Constrained	Multilana Hisburg 23	2.3	В	1.9	Α	2.9	В	2.3	В	2.0		0.0		2.0		2.0	
EB 26	Passing Zone	Multilane Highway <sup>2,3</sup>	2.1	В	1.7	Α	2.7	В	2.1	В	3.2	Α	2.8	Α	3.2	Α	2.8	Α
EB 27	Passing Constrained		2.4	В	1.9	Α	3.0	В	2.3	В								
EB 28	Passing Zone		2.2	В	1.8	Α	2.8	В	2.2	В								
EB 29	Passing Zone		3.3	В	2.8	В	3.4	В	3.0	В								
EB 30	Passing Constrained	Manufattana 118 15 23	3.5	В	2.9	В	3.6	В	3.1	В		_	0.0		0.5			
EB 31	Passing Zone	Multilane Highway <sup>2,3</sup>	3.2	В	2.8	В	3.3	В	2.9	В	3.5	Α	3.2	Α	3.5	A	3.2	A
EB 32	Passing Constrained		3.5	В	2.9	В	3.6	В	3.0	В								
EB 33	Passing Constrained		8.2	D	2.9	В												
EB 34	Passing Zone	Multilane Highway	8.0	С	2.8	В	6.2	Α	3.1	Α	6.2	Α	3.1	Α	6.2	Α	3.1	Α
EB 35	Passing Constrained	J	8.0	С	2.7	В												

Notes: Bold indicates Multilane Highway
Highlighted Color indicates a poor LOS
NA indicates Segment Removed for Build Conditions
Multilane<sup>2,3</sup> indicates segment type for Alternative 2 and 3



TABLE 10: HCM TRAFFIC HIGHWAY OPERATIONS - DESIGN 2050, WESTBOUND SD 38

	Comm	ant Time		NO E	BUILD		<b>A</b>	LTERNA	TIVE 1		А	LTERN	ATIVE 2		А	LTERN	ATIVE 3	
ID#	Segm	ent Type	AM PEA	HOUR	PM PEAI	K HOUR	AM PEAK	HOUR	PM PEAK	HOUR	AM PEAK	HOUR	PM PEAK	HOUR	AM PEAK	HOUR	PM PEAK	K HOUR
	No Build	Build	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS	Density	LOS
WB 1	Passing Constrained		1.6	Α	8.7	D												
WB 2	Passing Zone	Multilane	1.5	Α	8.5	D	2.4	Α	5.6	Α	2.4	Α	5.6	Α	2.4	Α	5.6	Α
WB 3	Passing Constrained		1.8	Α	4.4	С												
WB 4	Passing Zone		1.8	Α	4.4	С	2.0	Α	4.7	С								
WB 5	Passing Constrained	Multilane <sup>2,3</sup>	1.7	Α	4.2	С	1.8	Α	4.5	С	2.7	_	4.1	Α	2.7	Α	4.1	_
WB 6	Passing Zone	wulliane-,	1.8	Α	4.4	С	2.0	Α	4.7	С	2.1	Α	4.1	A	2.1	A	4.1	Α
WB 7	Passing Constrained		1.3	Α	2.9	В	1.8	Α	4.4	С								
WB 8	Passing Constrained		1.4	Α	3.2	В	1.7	Α	3.7	В								
WB 9	Passing Zone		1.3	Α	2.8	В	1.5	Α	3.4	В								
WB 10	Passing Constrained	Multilane <sup>2,3</sup>	1.4	Α	3.1	В	1.7	Α	3.6	В	0.4		2.7		0.4		2.7	
WB 11	Passing Zone	- wuttiane <sup>2,9</sup>	1.3	Α	2.9	В	1.6	А	3.5	В	2.4	Α	3.7	Α	2.4	Α	3.7	A
WB 12	Passing Constrained		1.4	Α	3.2	В	1.7	А	3.7	В								
WB 13	Passing Constrained		1.4	Α	3.2	В	1.7	А	3.7	В								
WB 14	Passing Constrained	NA	2.4	В	5.4	С	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	N/
WB 15	Passing Constrained	Multilane	1.9	А	4.3	С	2.5	Α	4.2	Α	2.5	Α	4.2	Α	2.5	Α	4.2	Α
WB 16	Passing Constrained	Multilane	3.3	В	10.9	D	4.3	Α	7.6	Α	4.3	Α	7.6	Α	4.3	Α	7.6	Α
WB 17	Passing Constrained		3.0	В	10.5	D												
WB 18	Passing Constrained		3.2	В	10.8	D												
WB 19	Passing Zone	Multilane	3.1	В	10.7	D	3.9	Α	7.6	Α	3.9	Α	7.6	Α	3.9	Α	7.6	A
WB 20	Passing Constrained		3.2	В	10.8	D												
WB 21	Passing Constrained		3.3	В	8.7	D												
WB 22	Passing Zone	Multilane	3.1	В	7.4	С	4.6	Α	7.9	Α	4.6	Α	7.9	Α	4.6	Α	7.9	A
WB 23	Passing Zone	Passing Zone	0.6	Α	1.4	Α	0.6	Α	1.4	Α	0.6	Α	1.4	Α	0.6	Α	1.4	А
WB 24	Passing Constrained	Passing Constrained	0.6	Α	1.4	Α	0.6	А	1.4	Α	0.6	Α	1.4	А	0.6	А	1.4	А
WB 25	Passing Constrained	Passing Constrained	0.6	Α	1.4	Α	0.6	А	1.4	Α	0.6	Α	1.4	Α	0.6	Α	1.4	А
WB 26	Passing Zone	Passing Zone	0.7	Α	1.7	Α	0.7	А	1.7	Α	0.7	Α	1.7	А	0.7	А	1.7	А
WB 27	Passing Zone	Passing Zone	0.6	Α	1.4	Α	0.6	Α	1.4	Α	0.6	Α	1.4	Α	0.6	Α	1.4	А
WB 28	Passing Zone	Passing Zone	0.7	Α	1.7	Α	0.7	А	1.7	А	0.7	А	1.7	Α	0.7	А	1.7	А
WB 29	Passing Zone	Passing Zone	0.7	Α	1.7	Α	0.7	Α	1.7	Α	0.7	Α	1.7	Α	0.7	Α	1.7	А
WB 30	Passing Constrained	Passing Constrained	0.6	А	1.4	Α	0.6	А	1.4	А	0.6	Α	1.4	А	0.6	Α	1.4	А
WB 31	Passing Zone	Passing Zone	0.7	Α	1.7	Α	0.7	А	1.7	Α	0.7	Α	1.7	Α	0.7	Α	1.7	А
WB 32	Passing Constrained	Passing Constrained	0.6	А	1.5	Α	0.6	А	1.5	А	0.6	А	1.5	А	0.6	А	1.5	A
WB 33	Passing Constrained	Passing Constrained	0.7	Α	1.7	Α	0.7	А	1.7	Α	0.7	А	1.7	Α	0.7	Α	1.7	1
WB 34	Passing Zone	Passing Zone	0.6	А	1.5	Α	0.6	А	1.5	А	0.6	А	1.5	Α	0.6	А	1.5	F
WB 35	Passing Constrained	Passing Constrained	0.9	A	2.1	В	0.9	A	2.1	В	0.9	A	2.1	В	0.9	A	2.1	E

Notes: Bold indicates Multilane Highway
Highlighted Color indicates a poor LOS
NA indicates Segment Removed for Build Conditions
Multilane<sup>2,3</sup> indicates segment type for Alternative 2 and 3

12 12



# Predictive Safety Analysis

Safety analysis of locations within the SD Highway 38 study corridor area of influence was completed for the Build scenarios. Predictive crash analysis was completed using the Interactive Highway Safety Design Model (IHSDM) Crash Prediction analysis tool to evaluate the safety effects and predict the expected change in crashes between design year scenarios. IHSDM reports are available in the Appendix.

The crash analysis determined the predicted crash frequency within the SD Highway 38 area of influence resulting from the Build roadway conditions. Predicted crash frequency is a measure of safety performance based on segments or intersections of a common facility type. Predictive crash frequency accounts for changes in traffic volume, and roadway characteristics, and is appropriate for comparing the variations in crash frequency that may result from added travel lanes or other geometric modifications.

A summary of the predicted crashes for the SD Highway 38 segments between the intersections with SD Highway 19 and Marion Road are provided in **Table 11**. The predicted crash analysis showed a significant reduction in crashes for build scenarios compared to the no-build scenario (two-lane highway). The addition of lanes, wider shoulder widths, median, and decrease in density are some of the factors causing the reduction in crashes for the build scenarios. The predicted frequency of crashes between build scenarios is consistent between the SD Highway 19 and Railroad Street segments, where there were no major geometric changes, with noticeable differences between the Railroad Street to Marion Road segments where the Build scenarios represented changes to the number of lanes, shoulder width and/or median type.

Furthermore, Alternative 3 would be safer compared to Alternatives 1 and 2. With higher volumes of opposing traffic, raised medians limit left turn movements to certain concentrated points, thereby reducing conflicting movements between vehicles. The predicted crash analysis demonstrated the potential for crash reductions in Alternative 3 with a raised median from Railroad Street to Marion Road with a reduction of 194.7 total segment crashes compared to Alternative 1 with TWLTL and a two-lane cross section between Railroad Street and Marion Road. The predictive crash analysis also showed that Alternative 2 with a TWLTL and raised median cross section between Railroad Street and Marion Road would result in a reduction of 160.34 total segment crashes compared to Alternative 1.

A summary of the predicted crashes for the SD Highway 38 intersections are provided in **Table 12**. At study intersections, the predicted crash frequency was consistent from SD Highway 19 to Railroad Street, where no differences between the corridor or intersection geometrics existed. There were noticeable changes in the predicted crash frequency at the County Highway 141/468<sup>th</sup> Avenue intersection and the County Highway 139/469<sup>th</sup> Avenue where the influence of the corridor can be seen to also have an effect on the safety at these intersections resulting in a reduction of 89.9 total crashes with the five-lane cross section.

TABLE 11: SD 38 SEGMENT CRASH FREQUENCY

		Segment	No Build	l Predicted	Crashes (20	025-2050)	Alterna	tive 1 Predi 20	cted Crashe 50)	es (2025-	Alterna	tive 2 Predi 20	cted Crashe 50)	es (2025-	Alterna	tive 3 Predi 20	cted Crash (50)	es (2025-
	Location	Length (Miles)	Total Crashes	Total Crashes/ Year	FI Crashes/ Year	PDO Crashes/ Year												
Segment 1:	SD Highway 19 to 459 <sup>th</sup> Avenue	2.05	47.76	1.83	0.58	1.24	42.37	1.62	0.52	1.10	42.37	1.62	0.52	1.10	42.37	1.62	0.52	1.10
Segment 2:	459th Avenue to Western Avenue	4.08	94.87	3.64	1.17	2.47	84.92	3.26	1.04	2.21	84.92	3.26	1.04	2.21	84.92	3.26	1.04	2.21
Segment 3:	Western Avenue to Main Avenue	0.24	18.36	0.70	0.22	0.47	18.09	0.69	0.22	0.47	18.09	0.69	0.22	0.47	18.09	0.69	0.22	0.47
Segment 4:	Main Avenue to Vandemark Avenue	0.31	24.91	0.95	0.30	0.65	24.71	0.95	0.30	0.65	24.71	0.95	0.30	0.65	24.71	0.95	0.30	0.65
Segment 5:	Vandemark Avenue to 2 <sup>nd</sup> Street	0.47	39.24	1.50	0.48	1.02	32.72	1.25	0.41	0.85	32.72	1.25	0.41	0.85	32.72	1.25	0.41	0.85
Segment 7:	2 <sup>nd</sup> Street to West Central High School	0.06	5.85	0.22	0.07	0.15	5.62	0.22	0.07	0.15	5.62	0.22	0.07	0.15	5.62	0.22	0.07	0.15
Segment 8:	West Central High School Entrance to Railroad Street	0.20	18.27	0.70	0.22	0.47	15.41	0.59	0.19	0.41	15.41	0.59	0.19	0.41	15.41	0.59	0.19	0.41
Segment 9:	Railroad Street to Mickelson Road	0.45	65.00	2.50	0.80	1.69	41.66	1.60	0.94	0.66	32.96	1.27	0.74	0.52	24.80	0.96	0.49	0.46
Segment 10:	Mickelson Road to 466 <sup>th</sup> Avenue (North)	1.40	165.99	6.38	2.04	4.33	175.21	6.73	3.89	2.84	130.47	5.00	2.90	2.11	106.78	4.10	2.08	2.01
Segment 11:	466 <sup>th</sup> Avenue (North) to WB I-90 Ramps	0.07	7.14	0.27	0.08	0.18	13.54	0.52	0.16	0.35	6.68	0.25	0.15	0.10	3.97	0.15	0.07	0.07
Segment 12:	WB I-90 Ramps to EB I-90 Ramps	0.28	21.29	0.81	0.26	0.55	22.97	0.88	0.28	0.60	17.65	0.67	0.40	0.27	18.13	0.69	0.37	0.33
Segment 13:	EB I-90 Ramps to 466 <sup>th</sup> Avenue (South)	0.07	6.62	0.25	0.08	0.17	-	-	-	-	-	-	-	-	-	-	-	-
Segment 14:	466 <sup>th</sup> Avenue (South) to County Highway 141	2.02	132.89	5.11	1.64	3.47	117.62	4.52	1.45	3.07	84.92	3.26	1.75	1.51	84.92	3.26	1.75	1.51
Segment 15:	County Highway 141 to County Highway 139	1.00	71.03	2.73	0.87	1.85	60.22	2.31	0.74	1.57	46.21	1.77	0.95	0.85	46.21	1.77	0.93	0.84
Segment 16:	County Highway 139 to La Mesa Drive	1.00	79.29	3.04	0.97	2.07	87.56	3.36	1.99	1.37	54.71	2.10	1.11	1.00	54.58	2.09	1.10	0.99
Segment 17:	La Mesa Drive to Marion Road	0.97	58.75	2.25	0.71	1.53	60.34	2.32	1.18	1.13	45.18	1.74	0.75	0.98	45.00	1.73	0.74	0.98
Total	All SD 38 Segments	14.67	857.26	32.88	10.49	22.31	802.96	30.82	13.38	17.43	642.62	24.64	11.50	13.18	608.23	23.33	10.28	13.03

Source: Interactive Highway Safety Design Model (IHSDM) 2021 Release, v17.0.0, HR Green, 2023.



TABLE 12: SD 38 INTERSECTION CRASH FREQUENCY

		No Build	Predicted (	Crashes (20	)25-2050)	Alternat		cted Crashe 50)	es (2025-	Alternat	ive 2 Predic 20	cted Crashe 50)	es (2025-	Alternat	tive 3 Predic 20	cted Crashe 50)	es (2025-
	Location	Total Crashes	Total Crashes/ Year	FI Crashes/ Year	PDO Crashes/ Year												
Intersection 1:	SD Highway 19 / 457th Avenue	21.11	0.81	0.33	0.47	41.06	1.57	0.68	0.89	41.06	1.57	0.68	0.89	41.06	1.57	0.68	0.89
Intersection 2:	459th Avenue	27.93	1.07	0.46	0.61	12.69	0.48	0.21	0.27	12.69	0.48	0.21	0.27	12.69	0.48	0.21	0.27
Intersection 3:	I-90 Speedway Entrance	NA	NA	NA	NA												
Intersection 4:	Western Avenue / 463rd Avenue	169.48	6.51	2.80	3.70	94.91	3.65	1.57	2.07	94.91	3.65	1.57	2.07	94.91	3.65	1.57	2.07
Intersection 5:	Main Avenue	132.77	5.10	2.20	2.90	132.77	5.10	2.20	2.90	132.77	5.10	2.20	2.90	132.77	5.10	2.20	2.90
Intersection 6:	Vandemark Avenue	74.90	2.88	1.24	1.63	74.90	2.88	1.24	1.63	74.90	2.88	1.24	1.63	74.90	2.88	1.24	1.63
Intersection 7:	2nd Street	166.63	6.40	2.76	3.64	67.18	2.58	1.11	1.47	67.18	2.58	1.11	1.47	67.18	2.58	1.11	1.47
Intersection 8:	West Central High School Entrance	73.62	2.83	1.17	1.65	73.62	2.83	1.17	1.65	73.62	2.83	1.17	1.65	73.62	2.83	1.17	1.65
Intersection 9:	Railroad Street / 464th Avenue	137.23	5.27	2.27	3.00	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	NA
Intersection 10:	Mickelson Road/260th Street	168.08	6.46	2.19	4.26	54.28	2.08	0.98	1.10	54.28	2.08	0.98	1.10	54.28	2.08	0.98	1.10
Intersection 11:	466th Avenue North	33.89	1.30	0.54	0.76	37.74	1.45	0.64	0.80	36.85	1.41	0.62	0.79	36.85	1.41	0.62	0.79
Intersection 12:	WB I-90 Exit 390	15.08	0.58	0.19	0.38	15.32	0.58	0.20	0.38	15.23	0.58	0.19	0.38	11.67	0.44	0.14	0.30
Intersection 13:	EB I-90 Exit 390	58.66	2.25	0.93	1.31	75.51	2.90	1.25	1.65	59.52	2.28	1.21	1.07	59.52	2.28	1.21	1.07
Intersection 14:	466th Avenue South	75.53	2.90	1.20	1.69	-	-	-	-	-	-	-	-	-	-	-	-
Intersection 15:	County Highway 141 / 468th Avenue	87.10	3.35	1.44	1.90	87.10	3.35	1.44	1.90	45.50	1.75	0.83	0.91	45.50	1.75	0.83	0.91
Intersection 16:	County Highway 139 / 469th Avenue	57.44	2.20	0.91	1.29	97.65	3.76	1.62	2.13	50.17	1.93	1.02	0.91	50.17	1.93	1.02	0.91
Intersection 17:	La Mesa Drive / 470th Avenue	61.03	2.34	1.01	1.33	46.79	1.80	1.01	0.78	46.79	1.80	1.01	0.78	46.79	1.80	1.01	0.78
Intersection 18:	Marion Road	55.22	2.12	0.69	1.42	49.96	1.92	0.63	1.29	49.96	1.92	0.63	1.29	49.96	1.92	0.63	1.29
Total	All SD 38 Intersections	1415.70	54.37	22.33	31.94	961.48	36.92	15.95	20.91	855.43	32.83	14.67	18.10	851.87	32.69	14.62	18.02

Source: Interactive Highway Safety Design Model (IHSDM) 2021 Release, v17.0.0, HR Green, 2023.

15

## Summary

The purpose of this technical memorandum is to document the future build concept traffic assessment at the eighteen study intersections and associated highway corridor segments along the SD Highway 38 corridor, from the SD Highway 19 intersection in Humboldt, South Dakota to the Marion Road intersection in Sioux Falls, South Dakota.

Using the Future year 2050 traffic forecasts, the traffic operations at study intersections and along the highway were evaluated for the three build corridor concepts.

- Alternative 1 two-lane highway from Humboldt to Hartford (as existing), three-lane roadway from Western Avenue to Railroad Street, five-lane roadway (center TWLTL) between Railroad Street to the I-90 Exit 390 interchange, two-lane highway from the I-90 interchange to 469<sup>th</sup> Ave/County Highway 139, and five-lane roadway (center TWLTL) from 460<sup>th</sup> Ave/County Highway 139 to Sioux Falls.
- Alternative 2 two-lane highway from Humboldt to Hartford (as existing), three-lane roadway from Western Avenue to Railroad Street, five-lane roadway (center TWLTL) between Railroad Street to the I-90 Exit 390 interchange, four-lane highway (raised median) from the I-90 interchange to Sioux Falls.
- Alternative 3 two-lane highway from Humboldt to Hartford (as existing), three-lane roadway from Western Avenue to Railroad Street, four-lane roadway (raised median) between Railroad Street and Sioux Falls.

Under the Opening Year 2029 conditions, the traffic operations analyses showed acceptable operations at all intersections within the study area, under all alternative scenarios, with intersections achieving LOS C or greater during both the AM and PM peak hours. The traffic operations analyses showed acceptable operations at all of the highway segments within the study area, under all alternative scenarios, with all segments achieving LOS B or greater during both the AM and PM peak hours. The desired LOS was realized for all intersections and highway segments during the AM and PM peak hours for all concepts

Under the Design Year 2050 conditions, the traffic operations analyses showed acceptable operations at all intersections within the study area, with intersections achieving LOS C or greater during both the AM and PM peak hours. To achieve LOS C goals, it was necessary to convert several intersections from two-way stop control to signalized operations. The traffic operations analyses showed acceptable operations at all of the highway segments within the study area, under all alternative scenarios, with all segments achieving LOS C or greater during both the AM and PM peak hours. The desired LOS was realized for all intersections and highway segments during the AM and PM peak hours for all concepts.

The predictive safety analysis of the SD Highway 38 study corridor revealed the potential for crash reductions in segments that contained a raised median with a reduction of 194.7 total crashes in segments with a raised median compared to without a raised median. The predictive safety analysis of the study intersections showed that there were noticeable changes in the predicted crash frequency at the intersections where the two-lane highway was maintained compared to the concepts with a five-lane cross section with a reduction of 89.9 total crashes with the five-lane cross section.

#### Recommendations

Based on the evaluations and conclusions documented for this corridor study, it is recommended to modify the existing SD 38 corridor as seen in Alternative 3. This alternative provides sufficient capacity to handle future traffic demand while increasing the overall safety of the corridor. Intersections along the study corridor should be monitored for traffic demand changes and plan for future capacity improvements or installation of traffic signal controls, if warranted.

Appendix A – HCS Output

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2023
Agency	HR Green	Analysis Year	2050 Build
Jurisdiction	SD 38 Build Option 1	Time Analyzed	AM
Project Description	464th_MickelsonRd_2050_ AM	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	55.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	55.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	638	Heavy Vehicle Adjustment Factor (fHV)	0.980
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	370
Total Trucks, %	2.00	Capacity (c), pc/h/ln	2100
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2100
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.18
Direction 1 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	55.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	6.7
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	362	Effective Speed Factor (St)	4.62
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.66
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	55.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	8.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	53.0	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	380	Heavy Vehicle Adjustment Factor (fHV)	0.885
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	244
Total Trucks, %	13.00	Capacity (c), pc/h/ln	2060
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2060
Tractor-Trailers (TT), %	- Volume-to-Capacity Ratio (v/c)		0.12
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	53.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.6
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	2.0		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	216	Effective Speed Factor (St)	4.62
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	6.14
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

Copyright © 2024 University of Florida. All Rights Reserved.

HCSTM Highways Version 2023 464th\_MickelsonRd\_2050\_AM.xuf Generated: 05/02/2024 08:56:09

	HCS Multilane	Highway Report		
Project Information				
Analyst	NM	Date	2/27/2023	
Agency	HR Green	Analysis Year	2050	
Jurisdiction	SD38 Build Option 1	Time Analyzed	PM	
Project Description	464th_MickelsonRd_PM	Units	U.S. Customary	
Direction 1 Geometric Data				
Direction 1	EB			
Number of Lanes (N), In	2	Terrain Type	Level	
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-	
Base Free-Flow Speed (BFFS), mi/h	55.0	Grade Length, mi	-	
Lane Width, ft	12	Access Point Density, pts/mi	0.0	
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6	
Free-Flow Speed (FFS), mi/h	55.0	Total Lateral Clearance (TLC), ft	12	
Direction 1 Adjustment Fact	ors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000	
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000	
Driver Population CAF	1.000			
Direction 1 Demand and Ca	pacity			
Volume (V) veh/h	441	Heavy Vehicle Adjustment Factor (fHV)	0.943	
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	266	
Total Trucks, %	6.00	Capacity (c), pc/h/ln	2100	
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2100	
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.13	
Direction 1 Speed and Dens	ity			
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	55.0	
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.8	
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А	
Access Point Density Adjustment (fA)	0.0			
Direction 1 Bicycle LOS		,		
Flow Rate in Outside Lane (vol.), veh/h	251	Effective Speed Factor (St)	4.62	
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.56	
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	D	

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	55.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	8.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	53.0	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	730	Heavy Vehicle Adjustment Factor (fHV)	0.990
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	419
Total Trucks, %	1.00	Capacity (c), pc/h/ln	2060
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2060
Tractor-Trailers (TT), %	-	- Volume-to-Capacity Ratio (v/c)	
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	53.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	7.9
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	2.0		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	415	Effective Speed Factor (St)	4.62
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.50
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	В

Copyright © 2024 University of Florida. All Rights Reserved.

HCSTM Highways Version 2023 464th\_MickelsonRd\_2050\_PM.xuf Generated: 05/02/2024 09:10:58

		HCS Two-La	ne l	Highway Re	port	
Pro	ject Information					
Anal	yst	MJV		Date		2/27/2024
Age	ncy	HRG		Analysis Year		2050
Juris	diction	SDDOT		Time Analyzed		AM
Proje	ect Description	SD 38_466th_469th_EB Build Option 1		Units		U.S. Customary
		Se	egn	nent 1		
Vel	nicle Inputs					
Segr	ment Type	Passing Constrained		Length, ft		1331
Mea	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity					
Dire	ctional Demand Flow Rate, veh/h	414		Opposing Deman	d Flow Rate, veh/h	-
Peak	Hour Factor	0.88		Total Trucks, %		5.26
Segr	ment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.24
Int	ermediate Results					
Segment Vertical Class		1		Free-Flow Speed, mi/h		70.0
Speed Slope Coefficient (m)		4.57394		Speed Power Coefficient (p)		0.41674
PF Slope Coefficient (m)		-1.29259		PF Power Coefficient (p)		0.75846
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		3.0
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0
Sul	osegment Data					
#	Segment Type	Length, ft	Rad	lius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	1331	-		-	67.2
Vel	nicle Results					
Aver	rage Speed, mi/h	67.2		Percent Followers	, %	48.4
Segr	ment Travel Time, minutes	0.23		Follower Density (FD), followers/mi/ln		3.0
Vehi	cle LOS	В				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h 414			Bicycle Effective Width, ft		24	
Bicycle LOS Score 3.80			Bicycle Effective Speed Factor		5.07	
Bicy	cle LOS	D				
		Se	egn	nent 2		
Vel	nicle Inputs					
Sear	ment Type	Passing Zone		Length, ft		1877
		1				1

Measured FFS	70.0				
Demand and Capacity	·				·
Directional Demand Flow Rate, veh/h	414		Opposing Deman	d Flow Rate, veh/h	295
Peak Hour Factor	0.88		Total Trucks, %		5.26
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.24
Intermediate Results					
Segment Vertical Class	1	Free-Flow Speed,		mi/h	70.0
Speed Slope Coefficient (m)	4.36033	4.36033		fficient (p)	0.51615
PF Slope Coefficient (m)	-1.23039		PF Power Coefficie	ent (p)	0.81159
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	2.8
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					·
# Segment Type	Length, ft	Rad	ius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	1877	-		-	67.6
Vehicle Results					
Average Speed, mi/h	67.6		Percent Followers,	%	45.2
Segment Travel Time, minutes	0.32		Follower Density (FD), followers/mi/ln		2.8
Vehicle LOS	В				
Bicycle Results					
Percent Occupied Parking 0			Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	414		Bicycle Effective W	/idth, ft	24
Bicycle LOS Score	3.80	Bicycle Effective Speed Fact		peed Factor	5.07
Bicycle LOS	D				
	S	egm	nent 3		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		1872
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	414		Opposing Demand Flow Rate, veh/h		-
Peak Hour Factor	0.88		Total Trucks, %		5.26
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.24
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed, mi/h		70.0
Speed Slope Coefficient (m)	4.58354		Speed Power Coefficient (p)		0.41674
PF Slope Coefficient (m)	-1.26676		PF Power Coefficie	·	0.76864
In Passing Lane Effective Length? No			Total Segment De	<u> </u>	2.9
%Improvement to Percent Followers 0.0					

Sul	osegment Data						
#	Segment Type	Length, ft		Radius, ft		Superelevation, %	Average Speed, mi/h
1	Tangent	1872		-		-	67.2
Vel	nicle Results						
Aver	rage Speed, mi/h	67.2		Percent Follov	vers,	%	47.4
Segr	ment Travel Time, minutes	0.32	0.32		sity (	FD), followers/mi/ln	2.9
Vehi	cle LOS	В	В				
Bic	ycle Results						
Perc	ent Occupied Parking	0		Pavement Cor	nditio	on Rating	4
Flow	Rate Outside Lane, veh/h	414		Bicycle Effecti	ve W	/idth, ft	24
Bicy	cle LOS Score	3.80		Bicycle Effecti	ve S <sub>l</sub>	peed Factor	5.07
Bicy	cle LOS	D					
			Se	gment 4			
Veł	nicle Inputs						
Segr	ment Type	Passing Zone		Length, ft			3603
Mea	sured FFS	Measured		Free-Flow Spe	eed, i	mi/h	70.0
Dei	mand and Capacity			<u>'</u>			•
Directional Demand Flow Rate, veh/h 414			Opposing Demand Flow Rate, veh/h			295	
Peak Hour Factor		0.88	0.88		Total Trucks, %		5.26
Segment Capacity, veh/h		1700		Demand/Capa	acity	(D/C)	0.24
Int	ermediate Results						
Segr	ment Vertical Class	1	1		Free-Flow Speed, mi/h		70.0
Spee	ed Slope Coefficient (m)	4.38398	4.38398		Speed Power Coefficient (p)		0.51615
PF S	lope Coefficient (m)	-1.18638	-1.18638		PF Power Coefficient (p)		0.82825
In Pa	assing Lane Effective Length?	No	No		Total Segment Density, veh/mi/ln		2.7
%lm	provement to Percent Followers	0.0	0.0		%Improvement to Speed		0.0
Sul	osegment Data						
#	Segment Type	Length, ft		Radius, ft		Superelevation, %	Average Speed, mi/h
1	Tangent	3603		-	-		67.6
Veł	nicle Results						
Average Speed, mi/h 67.6			Percent Followers, %		43.5		
Segment Travel Time, minutes		0.61	0.61		Follower Density (FD), followers/mi/ln		2.7
Vehi	cle LOS	В					
Bic	ycle Results						
Perc	ent Occupied Parking	0		Pavement Cor	nditio	on Rating	4
Flow	Rate Outside Lane, veh/h	414		Bicycle Effective Width, ft		24	
Bicycle LOS Score		3.80	3.80		Bicycle Effective Speed Factor		5.07

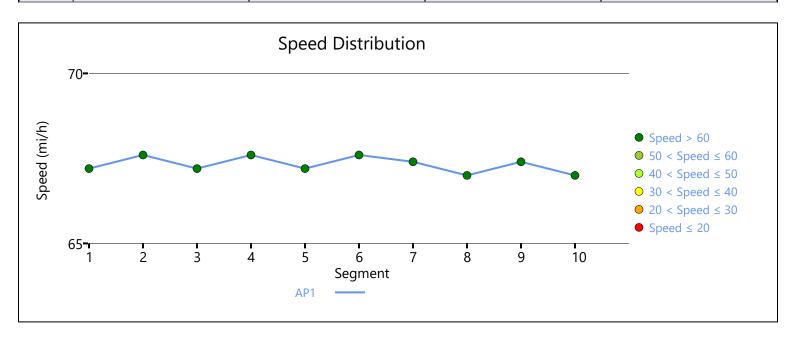
Bicycle LOS	D				
	S	Segn	nent 5		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		1053
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity	·				
Directional Demand Flow Rate, veh/h	414		Opposing Deman	d Flow Rate, veh/h	-
Peak Hour Factor	0.88		Total Trucks, %		5.26
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.24
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29321		PF Power Coefficie	ent (p)	0.75821
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	3.0
%Improvement to Percent Followers	0.0	0.0		Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rad	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	1053	1-	-		67.2
Vehicle Results					
Average Speed, mi/h	67.2		Percent Followers,	, %	48.4
Segment Travel Time, minutes	0.18		Follower Density (FD), followers/mi/ln		3.0
Vehicle LOS	В				
Bicycle Results					
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	414		Bicycle Effective Width, ft		24
Bicycle LOS Score	3.80		Bicycle Effective Speed Factor		5.07
Bicycle LOS	D	D			
	S	Segn	nent 6		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		1120
Measured FFS Measured		Free-Flow Speed, mi/h		70.0	
Demand and Capacity					
Directional Demand Flow Rate, veh/h	414		Opposing Deman	d Flow Rate, veh/h	244
Peak Hour Factor	0.88		Total Trucks, %		5.26
Segment Capacity, veh/h	1700		Demand/Capacity (D/C) 0.24		
Intermediate Results					

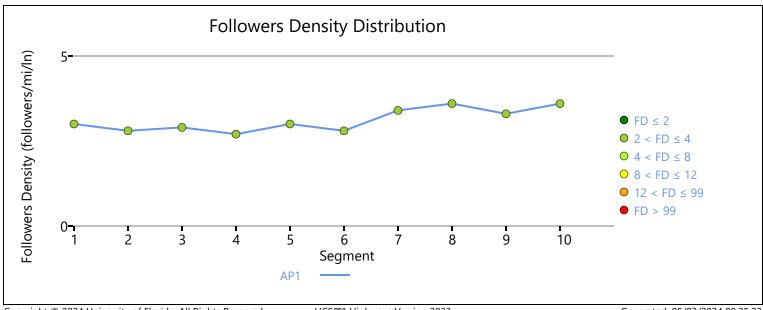
Seam	nent Vertical Class	1		Free-Flow Speed,	mi/h	70.0
	d Slope Coefficient (m)	4.33428		Speed Power Coefficient (p)		0.52768
•	ope Coefficient (m)	-1.24745		PF Power Coefficient (p)		0.80382
	ssing Lane Effective Length?	No		Total Segment De		2.8
	provement to Percent Followers	0.0		%Improvement to		0.0
Sub	segment Data				•	
#	Segment Type	Length, ft	Length, ft Radiu		Superelevation, %	Average Speed, mi/h
1	Tangent	1120	-	<u> </u>	-	67.6
Veh	icle Results					
Avera	age Speed, mi/h	67.6		Percent Followers	%	45.9
	nent Travel Time, minutes	0.19			FD), followers/mi/ln	2.8
	cle LOS	В		Tollower Density (	1 D), 10110WC13/1111/111	2.0
	/cle Results					
	ent Occupied Parking	0		Pavement Conditi	on Pating	4
	Rate Outside Lane, veh/h	414				24
	le LOS Score	3.80		Bicycle Effective Width, ft  Bicycle Effective Speed Factor		5.07
	le LOS	D		bicycle Effective Speed Factor		3.07
Dicyc	10.00		Comm	7		
			Segn	nent 7		
Veh	icle Inputs					
Segm	nent Type	Passing Zone		Length, ft		1272
Meas	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Den	nand and Capacity					
Direc	tional Demand Flow Rate, veh/h	466		Opposing Deman	d Flow Rate, veh/h	318
Peak	Hour Factor	0.88		Total Trucks, %		5.09
Segm	nent Capacity, veh/h	1700		Demand/Capacity (D/C)		0.27
Inte	ermediate Results					
Segm	nent Vertical Class	1		Free-Flow Speed, mi/h		70.0
Speed	d Slope Coefficient (m)	4.35715		Speed Power Coefficient (p)		0.51152
PF Slo	ope Coefficient (m)	-1.25973		PF Power Coefficient (p)		0.79928
In Pas	ssing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	3.4
%lmp	provement to Percent Followers	0.0		%Improvement to	Speed	0.0
Sub	segment Data					
#	Segment Type			dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	1272	-		-	67.4
Veh	icle Results					
Avera	age Speed, mi/h	67.4		Percent Followers	, %	49.5
5 1		Follower Density (FD), followers/mi/ln				

Vehi	icle LOS	В				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Condition Rating		4
Flow	v Rate Outside Lane, veh/h	466		Bicycle Effective W	/idth, ft	24
Bicy	cle LOS Score	3.80		Bicycle Effective Sp	peed Factor	5.07
Bicy	cle LOS	D				
			Segn	nent 8		
Vel	hicle Inputs					
Segi	ment Type	Passing Constrained		Length, ft		625
Mea	sured FFS	Measured		Free-Flow Speed, 1	mi/h	70.0
De	mand and Capacity					·
Dire	ctional Demand Flow Rate, veh/h	466		Opposing Demand	d Flow Rate, veh/h	-
Peak	k Hour Factor	0.88		Total Trucks, %		5.09
Segi	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.27
Int	ermediate Results					
Segi	ment Vertical Class	1		Free-Flow Speed, 1	mi/h	70.0
Spe	ed Slope Coefficient (m)	4.57372		Speed Power Coef	ficient (p)	0.41674
PF S	lope Coefficient (m)	oefficient (m) -1.29323		PF Power Coefficie	ent (p)	0.75819
In Pa	assing Lane Effective Length?	No		Total Segment Der	nsity, veh/mi/ln	3.6
%lm	provement to Percent Followers	0.0		%Improvement to Speed		0.0
Sul	bsegment Data	·				
#	Segment Type	Length, ft	Rac	lius, ft Superelevation, %		Average Speed, mi/h
1	Tangent	625	-	-		67.0
Vel	hicle Results					
Avei	rage Speed, mi/h	67.0		Percent Followers,	%	51.6
Segi	ment Travel Time, minutes	0.11		Follower Density (	FD), followers/mi/ln	3.6
Vehi	icle LOS	В		9, 7, 2, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,		
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Condition	on Rating	4
	v Rate Outside Lane, veh/h	466		Bicycle Effective W		24
Bicy	cle LOS Score	3.80		Bicycle Effective Sp	peed Factor	5.07
Bicy	cle LOS	D				
			Segn	nent 9		
Vel	hicle Inputs					
	ment Type	Passing Zone		Length, ft		1995

Demand and Capacity					
Directional Demand Flow Rate, veh/h	466		Opposing Demand Flow Rate, veh/h		318
Peak Hour Factor	0.88		Total Trucks, %		5.09
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.27
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.36896	:	Speed Power Coe	fficient (p)	0.51152
PF Slope Coefficient (m)	-1.22932		PF Power Coefficie	ent (p)	0.81204
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	3.3
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Length, ft Radiu		Superelevation, %	Average Speed, mi/h
1 Tangent	1995	-		-	67.4
Vehicle Results					
Average Speed, mi/h	67.4		Percent Followers,	, %	48.4
Segment Travel Time, minutes	0.34		Follower Density (FD), followers/mi/ln		3.3
Vehicle LOS	В				
Bicycle Results					
Percent Occupied Parking 0			Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	466		Bicycle Effective W	Vidth, ft	24
Bicycle LOS Score	3.80		Bicycle Effective Speed Factor		5.07
Bicycle LOS	D				
	Se	egme	ent 10		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		1399
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	466	1	Opposing Demand Flow Rate, veh/h		-
Peak Hour Factor	0.88		Total Trucks, %		5.09
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.27
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57524	:	Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.28884		PF Power Coefficie	ent (p)	0.75993
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	3.6
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					

#	Segment Type	Length, ft	Radius, ft		Superelevation, %	Average Speed, mi/h			
1	Tangent	1399	-		-	67.0			
Veh	Vehicle Results								
Avera	age Speed, mi/h	67.0	Perc	ent Followers,	%	51.4			
Segn	nent Travel Time, minutes	0.24	Follo	ower Density (	FD), followers/mi/ln	3.6			
Vehic	cle LOS	В							
Bicycle Results									
Perce	ent Occupied Parking	0	) Pavemen		on Rating	4			
Flow	Rate Outside Lane, veh/h	466	Bicy	cle Effective W	/idth, ft	24			
Bicyc	le LOS Score	3.80	Bicy	cle Effective S <sub>I</sub>	peed Factor	5.07			
Bicyc	le LOS	D							
Faci	ility Results								
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS			
1	290	0.16			3.0	В			





Copyright © 2024 University of Florida. All Rights Reserved.

HCSTM Highways Version 2023 I90EBramp466th\_469th\_AM\_2050\_EB.xuf

Generated: 05/02/2024 09:25:23

		HCS Two-La	ne	Highway Re	port	
Pro	ject Information					
Ana	lyst	MJV		Date		2/27/2024
Age	ncy	HRG		Analysis Year		2050
Juris	diction	SDDOT		Time Analyzed		PM
Proj	ect Description	466th_469th_EB Build Option 1		Units		U.S. Customary
		Se	egn	nent 1		
Vel	nicle Inputs					
Segi	ment Type	Passing Constrained		Length, ft		1331
Mea	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity					·
Dire	ctional Demand Flow Rate, veh/h	355		Opposing Deman	d Flow Rate, veh/h	-
Peak	Hour Factor	0.88		Total Trucks, %		5.26
Segi	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.21
Int	ermediate Results					
Segment Vertical Class		1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)		4.57394		Speed Power Coe	fficient (p)	0.41674
PF S	lope Coefficient (m)	-1.29259		PF Power Coefficie	ent (p)	0.75846
In Pa	assing Lane Effective Length?	No	No Tot		nsity, veh/mi/ln	2.3
%lm	provement to Percent Followers	0.0	.0 %Improvement to Spe		Speed	0.0
Sul	bsegment Data					
#	Segment Type	Length, ft	Rad	lius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	1331	-		-	67.4
Vel	nicle Results					
Avei	rage Speed, mi/h	67.4		Percent Followers	, %	44.5
Segi	ment Travel Time, minutes	0.22		Follower Density (FD), followers/mi/ln		2.3
Vehi	cle LOS	В				İ
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow	r Rate Outside Lane, veh/h	355		Bicycle Effective V	Vidth, ft	24
Bicy	cle LOS Score	3.72		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	D				
		Sc	egn	nent 2		
Vel	nicle Inputs					
	ment Type	Passing Zone		Length, ft		1877
9'	: 7 F =	1 9		- 9 - 4 - 5		<u> </u>

Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	355		Opposing Dem	and Flow Rate, veh/h	477
Peak Hour Factor	0.88		Total Trucks, %		5.26
Segment Capacity, veh/h	1700		Demand/Capac	ity (D/C)	0.21
Intermediate Results					
Segment Vertical Class	1		Free-Flow Spee	d, mi/h	70.0
Speed Slope Coefficient (m)	4.40861		Speed Power C	oefficient (p)	0.48517
PF Slope Coefficient (m)	-1.25153		PF Power Coeff	icient (p)	0.80198
In Passing Lane Effective Length?	No		Total Segment	Density, veh/mi/ln	2.2
%Improvement to Percent Followers	0.0		%Improvement	to Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rad	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	1877	-		-	67.7
Vehicle Results					
Average Speed, mi/h	67.7		Percent Followe	ers, %	42.0
Segment Travel Time, minutes	0.31	0.31		ry (FD), followers/mi/ln	2.2
Vehicle LOS	В				
Bicycle Results					
Percent Occupied Parking	0		Pavement Cond	dition Rating	4
Flow Rate Outside Lane, veh/h	355		Bicycle Effective	e Width, ft	24
Bicycle LOS Score	3.72		Bicycle Effective Speed Factor		5.07
Bicycle LOS	D				
		Segr	nent 3		
Vehicle Inputs					
Segment Type	Passing Constrain	ned	Length, ft		1872
Measured FFS	Measured		Free-Flow Spee	d, mi/h	70.0
Demand and Capacity			•		
Directional Demand Flow Rate, veh/h	355		Opposing Demand Flow Rate, veh/h		-
Peak Hour Factor	0.88		Total Trucks, %		5.26
Segment Capacity, veh/h	1700		Demand/Capac	city (D/C)	0.21
Intermediate Results					
Segment Vertical Class	1		Free-Flow Spee	rd, mi/h	70.0
Speed Slope Coefficient (m)	4.58354		Speed Power C	oefficient (p)	0.41674
PF Slope Coefficient (m)	-1.26676		PF Power Coeff	icient (p)	0.76864
In Passing Lane Effective Length?	No		Total Segment	Density, veh/mi/ln	2.3
%Improvement to Percent Followers	0.0		%Improvement	to Speed	0.0

Sul	osegment Data						
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h	
1	Tangent	1872	-		-	67.4	
Vel	nicle Results	•				•	
Aver	rage Speed, mi/h	67.4		Percent Followers	, %	43.5	
Segr	ment Travel Time, minutes	0.32		Follower Density	(FD), followers/mi/ln	2.3	
Vehi	cle LOS	В					
Bic	ycle Results						
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4	
Flow	Rate Outside Lane, veh/h	355		Bicycle Effective V	Vidth, ft	24	
Bicy	cle LOS Score	3.72		Bicycle Effective S	peed Factor	5.07	
Bicy	cle LOS	D					
			Segi	ment 4			
Vel	nicle Inputs						
Segr	ment Type	Passing Zone	Passing Zone		Length, ft		
Mea	sured FFS	Measured		Free-Flow Speed,	Free-Flow Speed, mi/h		
De	mand and Capacity	·					
Dire	ctional Demand Flow Rate, veh/h	355		Opposing Deman	d Flow Rate, veh/h	477	
Peak	Hour Factor	0.88		Total Trucks, %		5.26	
Segr	nent Capacity, veh/h	1700		Demand/Capacity	/ (D/C)	0.21	
Int	ermediate Results						
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0	
Spe	ed Slope Coefficient (m)	4.43226	4.43226		Speed Power Coefficient (p)		
PF S	lope Coefficient (m)	-1.20666	-1.20666		PF Power Coefficient (p)		
In Pa	assing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		2.1	
%lm	provement to Percent Followers	0.0		%Improvement to	%Improvement to Speed		
Sul	osegment Data						
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h	
1	Tangent	3603	-		-	67.7	
Vel	nicle Results						
Aver	age Speed, mi/h	67.7		Percent Followers	Percent Followers, %		
Segr	ment Travel Time, minutes	0.60		Follower Density	(FD), followers/mi/ln	2.1	
Vehi	cle LOS	В					
Bic	ycle Results						
Perc	ent Occupied Parking	0		Pavement Conditi	ion Rating	4	
Flow	Rate Outside Lane, veh/h	355		Bicycle Effective V	Bicycle Effective Width, ft		
Bicy	cle LOS Score	3.72		Bicycle Effective S	peed Factor	5.07	

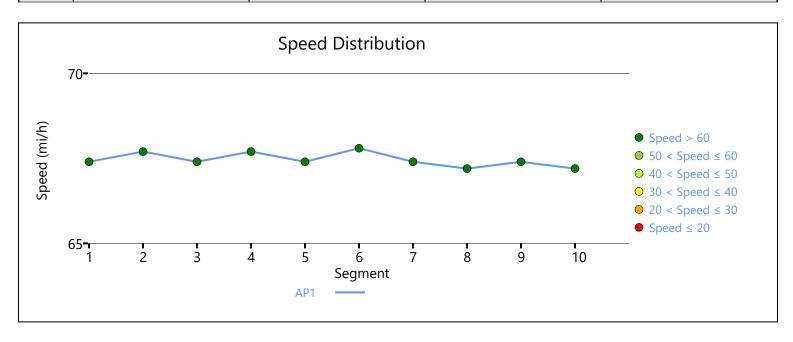
Bicycle LOS	D				
	S	egn	nent 5		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		1053
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	355		Opposing Deman	d Flow Rate, veh/h	-
Peak Hour Factor	0.88		Total Trucks, %		5.26
Segment Capacity, veh/h	1700		Demand/Capacity	' (D/C)	0.21
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29321		PF Power Coefficie	ent (p)	0.75821
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	2.3
%Improvement to Percent Followers	0.0		%Improvement to Speed		0.0
Subsegment Data					
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	1053	1-		-	67.4
Vehicle Results					
Average Speed, mi/h	67.4		Percent Followers	, %	44.5
Segment Travel Time, minutes	0.18		Follower Density (FD), followers/mi/ln		2.3
Vehicle LOS	В				
Bicycle Results					
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	355		Bicycle Effective Width, ft		24
Bicycle LOS Score	3.72		Bicycle Effective Speed Factor		5.07
Bicycle LOS	D				
	S	egn	nent 6		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		1120
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity	<u>'</u>				•
Directional Demand Flow Rate, veh/h	355		Opposing Deman	d Flow Rate, veh/h	403
Peak Hour Factor	0.88		Total Trucks, %		5.26
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.21
Intermediate Results	•				

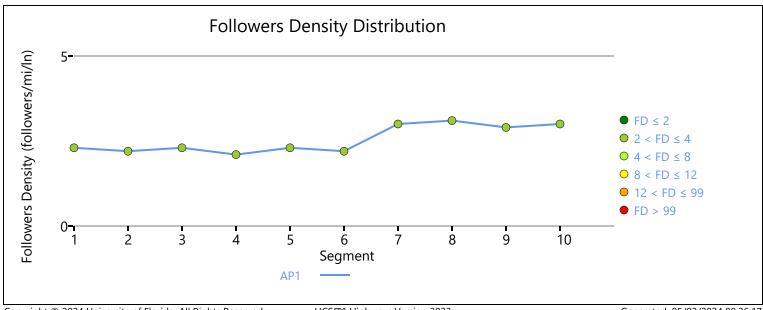
Seam	nent Vertical Class	1		Free-Flow Speed,	mi/h	70.0
	d Slope Coefficient (m)	4.38045		Speed Power Coefficient (p)		0.49627
	ope Coefficient (m)	-1.27058		PF Power Coefficient (p)		0.79479
	ssing Lane Effective Length?	No	No		nsity, veh/mi/ln	2.2
	provement to Percent Followers	0.0		%Improvement to		0.0
Sub	segment Data				•	
#	Segment Type	Length, ft	Length, ft Radiu		Superelevation, %	Average Speed, mi/h
1	Tangent	1120	-	<u> </u>	-	67.8
Veh	icle Results					
Avera	age Speed, mi/h	67.8		Percent Followers	%	42.7
	nent Travel Time, minutes	0.19			FD), followers/mi/ln	2.2
	cle LOS	В		Tollower Bensity	1 D), 10110WC13/1111/111	E.E.
	/cle Results	"				
				Day and and Canaditi	an Dation	
	ent Occupied Parking	355		Pavement Conditi		24
	Rate Outside Lane, veh/h le LOS Score	3.72		Bicycle Effective Width, ft  Bicycle Effective Speed Factor		5.07
	le LOS	D D		bicycle Effective Speed Factor		5.07
ысус	le LO3		_	. =		
			Segn	nent 7		
Veh	icle Inputs					
Segn	nent Type	Passing Zone		Length, ft		1272
Meas	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Den	nand and Capacity					
Direc	tional Demand Flow Rate, veh/h	420		Opposing Deman	d Flow Rate, veh/h	557
Peak	Hour Factor	0.88		Total Trucks, %		5.09
Segn	nent Capacity, veh/h	1700		Demand/Capacity (D/C)		0.25
Inte	ermediate Results					
Segn	nent Vertical Class	1		Free-Flow Speed, mi/h		70.0
Spee	d Slope Coefficient (m)	4.41686		Speed Power Coefficient (p)		0.47488
PF Slo	ope Coefficient (m)	-1.28420		PF Power Coefficient (p)		0.78783
In Pa	ssing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	3.0
%lmp	provement to Percent Followers	0.0		%Improvement to	Speed	0.0
Sub	segment Data					
#	Segment Type	Length, ft Rac		lius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	1272	-		-	67.4
Veh	icle Results					
Avera	age Speed, mi/h	67.4		Percent Followers	, %	47.7
Segment Travel Time, minutes 0.21		Follower Density (FD), followers/mi/ln				

Vehi	icle LOS	В				
Bic	cycle Results					
Perc	cent Occupied Parking	0		Pavement Condition Rating		4
Flov	v Rate Outside Lane, veh/h	420		Bicycle Effective W	/idth, ft	24
Bicy	rcle LOS Score	3.75		Bicycle Effective Sp	peed Factor	5.07
Bicy	rcle LOS	D				
			Segn	nent 8		
Vel	hicle Inputs					
Seg	ment Type	Passing Constrained	d	Length, ft		625
Mea	asured FFS	Measured		Free-Flow Speed, 1	mi/h	70.0
De	mand and Capacity					
Dire	ectional Demand Flow Rate, veh/h	420		Opposing Demand	d Flow Rate, veh/h	-
Peal	k Hour Factor	0.88		Total Trucks, %		5.09
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.25
Int	ermediate Results					
Seg	ment Vertical Class	1		Free-Flow Speed, 1	mi/h	70.0
Spe	ed Slope Coefficient (m)	4.57372		Speed Power Coef	ficient (p)	0.41674
PF S	Slope Coefficient (m)	-1.29323		PF Power Coefficie	ent (p)	0.75819
In Pa	assing Lane Effective Length?	No		Total Segment Der	nsity, veh/mi/ln	3.1
%lm	nprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Sul	bsegment Data					
#	Segment Type	Length, ft	Rad	dius, ft Superelevation, %		Average Speed, mi/h
1	Tangent	625	-	-		67.2
Vel	hicle Results					<u>'</u>
Ave	rage Speed, mi/h	67.2		Percent Followers,	%	48.9
Seg	ment Travel Time, minutes	0.11		Follower Density (	FD), followers/mi/ln	3.1
Vehi	icle LOS	В				
Bic	cycle Results	<u>'</u>				
Perc	cent Occupied Parking	0		Pavement Condition	on Rating	4
	v Rate Outside Lane, veh/h	420		Bicycle Effective W		24
Bicy	rcle LOS Score	3.75		Bicycle Effective Speed Factor		5.07
Bicy	rcle LOS	D				
			Segn	nent 9		
	hiala lumuta					
Vel	hicle Inputs					
	ment Type	Passing Zone		Length, ft		1995

Demand and Capacity					
Directional Demand Flow Rate, veh/h	420		Opposing Demand Flow Rate, veh/h		557
Peak Hour Factor	0.88		Total Trucks, %		5.09
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.25
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.42866		Speed Power Coe	fficient (p)	0.47488
PF Slope Coefficient (m)	-1.25311		PF Power Coefficie	ent (p)	0.80020
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	2.9
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Radi	lius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	1995	-		-	67.4
Vehicle Results					
Average Speed, mi/h	67.4		Percent Followers	, %	46.6
Segment Travel Time, minutes	0.34		Follower Density (FD), followers/mi/ln		2.9
Vehicle LOS	В				
Bicycle Results					
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	420		Bicycle Effective V	Vidth, ft	24
Bicycle LOS Score	3.75		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	D				
	Se	egm	ent 10		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		1399
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	420		Opposing Demand Flow Rate, veh/h		-
Peak Hour Factor	0.88		Total Trucks, %		5.09
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.25
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57524		Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.28884		PF Power Coefficie	ent (p)	0.75993
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	3.0
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					

#	Segment Type	Length, ft	Radius, ft		Superelevation, %	Average Speed, mi/h			
1	Tangent	1399	-		-	67.2			
Veh	Vehicle Results								
Avera	age Speed, mi/h	67.2	Perc	ent Followers,	%	48.7			
Segn	nent Travel Time, minutes	0.24	Follo	ower Density (	FD), followers/mi/ln	3.0			
Vehic	cle LOS	В							
Bicycle Results									
Perce	ent Occupied Parking	0	Pavement Condition Rating		4				
Flow	Rate Outside Lane, veh/h	420	Bicy	cle Effective W	/idth, ft	24			
Bicyc	le LOS Score	3.75	Bicy	cle Effective S <sub>l</sub>	peed Factor	5.07			
Bicyc	le LOS	D							
Faci	ility Results								
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS			
1	253	0.13			2.5	В			





Copyright © 2024 University of Florida. All Rights Reserved.

HCSTM Highways Version 2023 I90EBRamp466th\_469th\_PM\_2050\_EB.xuf

Generated: 05/02/2024 09:26:17

		HCS Two-La	ne l	Highway Re	port	
Pro	oject Information					
Ana	lyst	MJV		Date		11/2/2023
Age	ncy	HRG		Analysis Year		2050
Juris	sdiction	SDDOT		Time Analyzed		AM
Proj	ect Description	466th_469th_AM_WB_I d_Option1	Buil	Units		U.S. Customary
		Se	egn	nent 1		
Ve	hicle Inputs					
Seg	ment Type	Passing Constrained		Length, ft		718
Mea	esured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity					
Dire	ctional Demand Flow Rate, veh/h	318		Opposing Deman	d Flow Rate, veh/h	-
Peal	k Hour Factor	0.88		Total Trucks, %		17.04
Seg	ment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.19
Int	ermediate Results					
Segment Vertical Class		1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)		4.57372	4.57372		fficient (p)	0.41674
PF S	Slope Coefficient (m)	-1.29182	-1.29182		ent (p)	0.75993
In P	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	2.0
%ln	provement to Percent Followers	0.0		%Improvement to	Speed	0.0
Su	bsegment Data					
#	Segment Type	Length, ft	Rad	lius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	718	-		-	67.6
Ve	hicle Results					
Ave	rage Speed, mi/h	67.6		Percent Followers	, %	41.8
Seg	ment Travel Time, minutes	0.12		Follower Density (FD), followers/mi/ln		2.0
Veh	icle LOS	А				
Bic	ycle Results					
Perd	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flov	v Rate Outside Lane, veh/h	318		Bicycle Effective V	Vidth, ft	24
Bicy	cle LOS Score	9.01		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	F				
		Se	egn	nent 2		
Ve	hicle Inputs					
	ment Type	Passing Zone		Length, ft		1738
						1

Measured FFS Measured		Free-Flow Speed,	mi/h	70.0	
Demand and Capacity					
Directional Demand Flow Rate, veh/h 318		Opposing Demand Flow Rate, veh/h		466	
Peak Hour Factor	0.88		Total Trucks, %		17.04
Segment Capacity, veh/h	1700		Demand/Capacity	y (D/C)	0.19
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.40359		Speed Power Coe	efficient (p)	0.48677
PF Slope Coefficient (m)	-1.25494		PF Power Coeffici	ent (p)	0.80196
In Passing Lane Effective Length?	No		Total Segment De	ensity, veh/mi/ln	1.8
%Improvement to Percent Followers	0.0		%Improvement to	o Speed	0.0
Subsegment Data	·				
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	1738	-		-	67.9
Vehicle Results					
Average Speed, mi/h	67.9	67.9		s, %	39.4
Segment Travel Time, minutes	0.29		Follower Density (FD), followers/mi/ln		1.8
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condit	ion Rating	4
Flow Rate Outside Lane, veh/h	318	318		Vidth, ft	24
Bicycle LOS Score	9.01		Bicycle Effective Speed Factor		5.07
Bicycle LOS	F				
		Segn	nent 3		
Vehicle Inputs					
Segment Type	Passing Constrain	ned	Length, ft		579
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					•
Directional Demand Flow Rate, veh/h	318		Opposing Demar	nd Flow Rate, veh/h	-
Peak Hour Factor	0.88		Total Trucks, %		17.04
Segment Capacity, veh/h	1700		Demand/Capacity	y (D/C)	0.19
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29182		PF Power Coeffici	ent (p)	0.75993
In Passing Lane Effective Length?	No		Total Segment De	ensity, veh/mi/ln	2.0
In Passing Lane Effective Length? No			%Improvement to Speed		+

Sul	osegment Data					
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	579	-		-	67.6
Vel	nicle Results	•				
Aver	age Speed, mi/h	67.6		Percent Followers	, %	41.8
Segr	ment Travel Time, minutes	0.10		Follower Density (	(FD), followers/mi/ln	2.0
Vehi	cle LOS	Α				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	ion Rating	4
Flow	Rate Outside Lane, veh/h	318		Bicycle Effective V	Vidth, ft	24
Bicy	cle LOS Score	9.01		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	F				
			Segi	ment 4		
Vel	nicle Inputs					
Segr	ment Type	Passing Zone		Length, ft	Length, ft	
Mea	sured FFS	Measured		Free-Flow Speed,	Free-Flow Speed, mi/h	
De	mand and Capacity	·				
Dire	ctional Demand Flow Rate, veh/h	318		Opposing Deman	d Flow Rate, veh/h	466
Peak	Hour Factor	0.88		Total Trucks, %		18.44
Segr	ment Capacity, veh/h	1700	1700		/ (D/C)	0.19
Int	ermediate Results					
Segr	nent Vertical Class	1		Free-Flow Speed,	70.0	
Spe	ed Slope Coefficient (m)	4.41190	4.41190		Speed Power Coefficient (p)	
PF S	lope Coefficient (m)	-1.23534		PF Power Coefficient (p)		0.80987
In Pa	assing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		1.8
%lm	provement to Percent Followers	0.0		%Improvement to Speed		0.0
Sul	osegment Data					
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	2262	-		-	
Vel	nicle Results					
Aver	age Speed, mi/h	67.9		Percent Followers	, %	38.7
Segr	ment Travel Time, minutes	0.38		Follower Density (	(FD), followers/mi/ln	1.8
Vehi	cle LOS	A				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	ion Rating	4
Flow	Rate Outside Lane, veh/h	318		Bicycle Effective V	Vidth, ft	24
Bicy	cle LOS Score	9.85		Bicycle Effective S	Bicycle Effective Speed Factor	

Bicycle LOS	F				
	Se	egm	nent 5		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		980
Measured FFS Measured			Free-Flow Speed,	mi/h	70.0
Demand and Capacity					·
Directional Demand Flow Rate, veh/h	295		Opposing Deman	d Flow Rate, veh/h	-
Peak Hour Factor	0.88		Total Trucks, %		18.44
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29166		PF Power Coefficie	ent (p)	0.76014
In Passing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		1.7
%Improvement to Percent Followers	0.0		%Improvement to Speed		0.0
Subsegment Data					
# Segment Type	Length, ft	Radi	ius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	980	-		-	67.7
Vehicle Results					
Average Speed, mi/h	67.7		Percent Followers	, %	40.0
Segment Travel Time, minutes	0.16		Follower Density (FD), followers/mi/ln		1.7
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	295		Bicycle Effective Width, ft		24
Bicycle LOS Score	9.81		Bicycle Effective Speed Factor		5.07
Bicycle LOS	F				
	Se	egm	nent 6		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		3667
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	295		Opposing Deman	d Flow Rate, veh/h	414
Peak Hour Factor	0.88		Total Trucks, %		18.44
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17
Intermediate Results	•				•

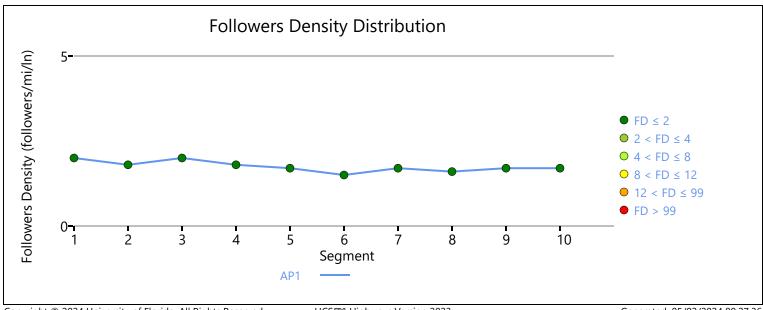
Sean	nent Vertical Class	1		Free-Flow Speed,	mi/h	70.0
	d Slope Coefficient (m)	4.41738		Speed Power Coe		0.49463
	ope Coefficient (m)	-1.19837		PF Power Coefficient (p)		0.82363
In Pa	ssing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.5
%lmp	provement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data				1 -		
#	Segment Type	Length, ft	Length, ft Radiu		Superelevation, %	Average Speed, mi/h
1	Tangent	3667	-		-	68.0
Veh	icle Results					<u>'</u>
Avera	age Speed, mi/h	68.0		Percent Followers	. %	35.5
	nent Travel Time, minutes	0.61			(FD), followers/mi/ln	1.5
	cle LOS	A			(-),	1
Bicy	/cle Results					
	ent Occupied Parking	0		Pavement Conditi	on Rating	4
	Rate Outside Lane, veh/h	295		Bicycle Effective V		24
	le LOS Score			Bicycle Effective Speed Factor		5.07
Bicyc	le LOS	F				
			Segn	nent 7		
Veh	icle Inputs					
Segn	nent Type	Passing Constrain	ned	Length, ft		1846
Meas	sured FFS	Measured	Measured		mi/h	70.0
Der	nand and Capacity			•		
Direc	tional Demand Flow Rate, veh/h	295		Opposing Deman	d Flow Rate, veh/h	-
Peak	Hour Factor	0.88				18.44
Segn	nent Capacity, veh/h	1700	1700		/ (D/C)	0.17
Inte	ermediate Results					
Segn	nent Vertical Class	1		Free-Flow Speed, mi/h		70.0
Spee	d Slope Coefficient (m)	4.58311		Speed Power Coefficient (p)		0.41674
PF SI	ope Coefficient (m)	-1.26629		PF Power Coefficient (p)		0.77017
In Pa	ssing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		1.7
%lmp	provement to Percent Followers	0.0		%Improvement to	Speed	0.0
Sub	segment Data					
#	Segment Type	Length, ft	Rad	lius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	1846	-		-	67.7
Veh	icle Results					
Avera	age Speed, mi/h	67.7		Percent Followers	, %	39.1
Segment Travel Time, minutes		0.31		Follower Density (FD), followers/mi/ln		

Vehi	icle LOS	А				
Bic	cycle Results					
Perc	ent Occupied Parking	0		Pavement Condition	on Rating	4
Flow	v Rate Outside Lane, veh/h	295		Bicycle Effective W	/idth, ft	24
Bicy	cle LOS Score	9.81	9.81 E		peed Factor	5.07
Bicy	cle LOS	F				
		S	egm	nent 8		
Vel	hicle Inputs					
Segi	ment Type	Passing Zone L		Length, ft		2174
Mea	asured FFS	Measured		Free-Flow Speed, 1	mi/h	70.0
De	mand and Capacity					
Dire	ectional Demand Flow Rate, veh/h	295		Opposing Demand	d Flow Rate, veh/h	414
Peak	k Hour Factor	0.88		Total Trucks, %		18.44
Segi	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17
Int	ermediate Results					
Segi	ment Vertical Class	1		Free-Flow Speed, mi/h		70.0
Spe	ed Slope Coefficient (m)	4.39765		Speed Power Coef	ficient (p)	0.49463
PF S	PF Slope Coefficient (m) -1.23320			PF Power Coefficie	ent (p)	0.81133
In Pa	assing Lane Effective Length?	No		Total Segment Der	nsity, veh/mi/ln	1.6
%lm	nprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Sul	bsegment Data					
#	Segment Type	Length, ft	Rad	lius, ft Superelevation, %		Average Speed, mi/h
1	Tangent	2174	-	-		68.0
Vel	hicle Results	•				
Aver	rage Speed, mi/h	68.0		Percent Followers,	%	36.8
Segi	ment Travel Time, minutes	0.36		Follower Density (I	FD), followers/mi/ln	1.6
Vehi	icle LOS	A		7, 7		
Bic	cycle Results					
Perc	cent Occupied Parking	0		Pavement Condition	on Rating	4
	v Rate Outside Lane, veh/h	295		Bicycle Effective W		24
Bicy	rcle LOS Score	9.81		Bicycle Effective Speed Factor		5.07
Bicycle LOS F						
Bicy	rcle LOS					
Bicy	cle LOS		egm	nent 9		
	hicle Inputs		egm	ent 9		
Vel			egm	Length, ft		1277

1 Tangent 1277	70.0 0.41674 0.76014
Peak Hour Factor  0.88  Total Trucks, %  Segment Capacity, veh/h  1700  Demand/Capacity (D/C)  Intermediate Results  Segment Vertical Class  1 Free-Flow Speed, mi/h  Speed Slope Coefficient (m)  4.57372  Speed Power Coefficient (p)  In Passing Lane Effective Length?  No  Total Segment Density, veh  %Improvement to Percent Followers  0.0  Subsegment Data  # Segment Type  Length, ft  Radius, ft  Superel  1 Tangent  1277  -	70.0 0.41674 0.76014
Segment Capacity, veh/h  Intermediate Results  Segment Vertical Class  1 Free-Flow Speed, mi/h Speed Slope Coefficient (m)  4.57372 Speed Power Coefficient (p)  In Passing Lane Effective Length?  No Total Segment Density, veh  Mimprovement to Percent Followers  5 Subsegment Data  # Segment Type  Length, ft  Radius, ft  Superel  1 Tangent  1277  -	70.0 p) 0.41674 0.76014
Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h  Speed Slope Coefficient (m) 4.57372 Speed Power Coefficient (p)  PF Slope Coefficient (m) -1.29166 PF Power Coefficient (p)  In Passing Lane Effective Length? No Total Segment Density, very  %Improvement to Percent Followers 0.0 %Improvement to Speed  Subsegment Data  # Segment Type Length, ft Radius, ft Superel  1 Tangent 1277 -	70.0 p) 0.41674 0.76014
Segment Vertical Class 1 Free-Flow Speed, mi/h  Speed Slope Coefficient (m) 4.57372 Speed Power Coefficient (p)  PF Slope Coefficient (m) -1.29166 PF Power Coefficient (p)  In Passing Lane Effective Length? No Total Segment Density, very  %Improvement to Percent Followers 0.0 %Improvement to Speed  Subsegment Data  # Segment Type Length, ft Radius, ft Superel  1 Tangent 1277 -	0.41674 0.76014
Speed Slope Coefficient (m)  4.57372  Speed Power Coefficient (p)  PF Slope Coefficient (m)  In Passing Lane Effective Length?  No  Total Segment Density, veryold with the percent Followers  Subsegment Data  # Segment Type  Length, ft  Radius, ft  Superel  Tangent  Speed Power Coefficient (p)  Road Subsegment Density, veryold with the percent Followers  Subsegment Data  Framework  Superel  Tangent  Speed Power Coefficient (p)  Radius, ft  Superel  -	0.41674 0.76014
PF Slope Coefficient (m)  In Passing Lane Effective Length?  No  Total Segment Density, veh  Mimprovement to Percent Followers  0.0  Subsegment Data  # Segment Type Length, ft Radius, ft Superel  1 Tangent  PF Power Coefficient (p)  Total Segment Density, veh  Mimprovement to Speed  Subsegment Data	0.76014
In Passing Lane Effective Length?  No Total Segment Density, veh %Improvement to Percent Followers  0.0  %Improvement to Speed  Subsegment Data  # Segment Type Length, ft Radius, ft Superel 1 Tangent  1277  -	
%Improvement to Percent Followers  O.0 %Improvement to Speed  Subsegment Data  # Segment Type Length, ft Radius, ft Superel  1 Tangent 1277	h/mi/ln 17
Subsegment Data   # Segment Type Length, ft Radius, ft Superel   1 Tangent 1277	1.7
# Segment Type Length, ft Radius, ft Superel 1 Tangent 1277	0.0
1 Tangent 1277	
	levation, % Average Speed, mi/h
	67.7
Vehicle Results	
Average Speed, mi/h 67.7 Percent Followers, %	40.0
Segment Travel Time, minutes 0.21 Follower Density (FD), follower De	owers/mi/ln 1.7
Vehicle LOS A	
Bicycle Results	
Percent Occupied Parking 0 Pavement Condition Rating	g 4
Flow Rate Outside Lane, veh/h 295 Bicycle Effective Width, ft	24
Bicycle LOS Score 9.81 Bicycle Effective Speed Fac	5.07
Bicycle LOS F	
Segment 10	
Vehicle Inputs	
Segment Type Passing Constrained Length, ft	898
Measured FFS Measured Free-Flow Speed, mi/h	70.0
Demand and Capacity	
Directional Demand Flow Rate, veh/h 295 Opposing Demand Flow Rate	late, veh/h
Peak Hour Factor 0.88 Total Trucks, %	18.44
Segment Capacity, veh/h 1700 Demand/Capacity (D/C)	0.17
Intermediate Results	
Segment Vertical Class 1 Free-Flow Speed, mi/h	70.0
Speed Slope Coefficient (m)  4.57372  Speed Power Coefficient (p)	o) 0.41674
PF Slope Coefficient (m) -1.29166 PF Power Coefficient (p)	0.76014
In Passing Lane Effective Length? No Total Segment Density, veh	h/mi/ln 1.7
%Improvement to Percent Followers 0.0 %Improvement to Speed	
Subsegment Data	0.0

#	Segment Type	Length, ft	Radius, ft		Superelevation, %	Average Speed, mi/h			
1	Tangent	779	-		-	67.7			
Veh	Vehicle Results								
Avera	age Speed, mi/h	67.7	Perc	ent Followers,	%	40.0			
Segn	nent Travel Time, minutes	0.15	Follo	ower Density (	FD), followers/mi/ln	1.7			
Vehic	cle LOS	A							
Bicy	Bicycle Results								
Perce	ent Occupied Parking	0	Pave	ement Conditi	on Rating	4			
Flow	Rate Outside Lane, veh/h	295	Bicy	cle Effective W	/idth, ft	24			
Bicyc	le LOS Score	9.81	Bicycle Effective Speed Factor		5.07				
Bicyc	le LOS	F							
Faci	ility Results								
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS			
1	204	0.09			1.7	А			





Copyright © 2024 University of Florida. All Rights Reserved.

HCS™ Highways Version 2023 I90EBRamp466th\_469th\_AM\_2050\_WB.xuf

Generated: 05/02/2024 09:27:36

		HCS Two-Lai	ne l	Highway Re	port	
Pro	ject Information					
Anal	yst	MJV		Date		2/27/2024
Age	ncy	HRG		Analysis Year		2050
Juris	diction	SDDOT		Time Analyzed		PM
Proje	ect Description	466th_469th_PM_WB_E d_Option1	Buil	Units		U.S. Customary
		Se	egn	nent 1		
Vel	nicle Inputs					
Segr	ment Type	Passing Constrained		Length, ft		718
Mea	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity					
Dire	ctional Demand Flow Rate, veh/h	557		Opposing Deman	d Flow Rate, veh/h	-
Peak	Hour Factor	0.88		Total Trucks, %		17.04
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.33
Int	ermediate Results					
Segment Vertical Class		1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)		4.57372		Speed Power Coe	fficient (p)	0.41674
PF S	lope Coefficient (m)	-1.29182		PF Power Coefficie	ent (p)	0.75993
In Pa	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	4.7
%lm	provement to Percent Followers	0.0		%Improvement to	Speed	0.0
Sul	osegment Data					
#	Segment Type	Length, ft	Rad	ius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	718	-		-	66.7
Vel	nicle Results				•	
Aver	rage Speed, mi/h	66.7		Percent Followers,	, %	56.3
Segr	ment Travel Time, minutes	0.12		Follower Density (FD), followers/mi/ln		4.7
Vehi	cle LOS	С				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow	Rate Outside Lane, veh/h	557		Bicycle Effective W	Vidth, ft	24
Bicy	cle LOS Score	9.30		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	F				
		Se	egn	nent 2		
Vel	nicle Inputs					
Segr	ment Type	Passing Zone		Length, ft		1738
_		1				The second secon

Measured FFS Measured		Free-Flow Speed,	mi/h	70.0	
Demand and Capacity					
Directional Demand Flow Rate, veh/h 557		Opposing Demand Flow Rate, veh/h		420	
Peak Hour Factor	0.88		Total Trucks, %		17.04
Segment Capacity, veh/h	1700		Demand/Capacit	y (D/C)	0.33
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.39239		Speed Power Coe	efficient (p)	0.49356
PF Slope Coefficient (m)	-1.25055		PF Power Coeffici	ient (p)	0.80414
In Passing Lane Effective Length?	No		Total Segment De	ensity, veh/mi/ln	4.5
%Improvement to Percent Followers	0.0		%Improvement to	o Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	1738	-		-	67.0
Vehicle Results					
Average Speed, mi/h	67.0		Percent Followers, %		54.2
Segment Travel Time, minutes	0.29		Follower Density	(FD), followers/mi/ln	4.5
Vehicle LOS	С	С			
Bicycle Results					·
Percent Occupied Parking	0		Pavement Condit	ion Rating	4
Flow Rate Outside Lane, veh/h	557		Bicycle Effective \	Width, ft	24
Bicycle LOS Score	9.30	9.30		Speed Factor	5.07
Bicycle LOS	F				
		Segn	nent 3		
Vehicle Inputs					
Segment Type	Passing Constrain	ned	Length, ft		579
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity			1		
Directional Demand Flow Rate, veh/h	557		Opposing Demar	nd Flow Rate, veh/h	-
Peak Hour Factor	0.88		Total Trucks, %	· · ·	17.04
Segment Capacity, veh/h	1700		Demand/Capacit	y (D/C)	0.33
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe		0.41674
PF Slope Coefficient (m)	-1.29182		PF Power Coeffici	·	0.75993
In Passing Lane Effective Length?	No		Total Segment De		4.7
In Passing Lane Effective Length?		%Improvement to Speed			

Sul	osegment Data					
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	579	-		-	66.7
Vel	nicle Results	•				
Aver	rage Speed, mi/h	66.7		Percent Followers	, %	56.3
Segr	ment Travel Time, minutes	0.10		Follower Density (	(FD), followers/mi/ln	4.7
Vehi	cle LOS	С				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow	Rate Outside Lane, veh/h	557		Bicycle Effective V	Vidth, ft	24
Bicy	cle LOS Score	9.30		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	F				
			Segi	ment 4		
Vel	nicle Inputs					
Segr	ment Type	Passing Zone	Passing Zone		Length, ft	
Mea	sured FFS	Measured		Free-Flow Speed,	Free-Flow Speed, mi/h	
De	mand and Capacity					
Dire	ctional Demand Flow Rate, veh/h	557		Opposing Deman	d Flow Rate, veh/h	420
Peak	Hour Factor	0.88		Total Trucks, %		18.44
Segr	ment Capacity, veh/h	1700	1700		γ (D/C)	0.33
Int	ermediate Results					
Segr	ment Vertical Class	1		Free-Flow Speed,	70.0	
Spe	ed Slope Coefficient (m)	4.40070	4.40070		Speed Power Coefficient (p)	
PF S	lope Coefficient (m)	-1.23103		PF Power Coefficient (p)		0.81211
In Pa	assing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		4.4
%lm	provement to Percent Followers	0.0		%Improvement to Speed		0.0
Sul	osegment Data					
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	2262	-		-	
Vel	nicle Results					
Aver	age Speed, mi/h	67.0		Percent Followers	, %	53.5
Segr	ment Travel Time, minutes	0.38		Follower Density (	(FD), followers/mi/ln	4.4
Vehi	cle LOS	С				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow	Rate Outside Lane, veh/h	557		Bicycle Effective V	Vidth, ft	24
Bicy	cle LOS Score	10.13		Bicycle Effective S	peed Factor	5.07

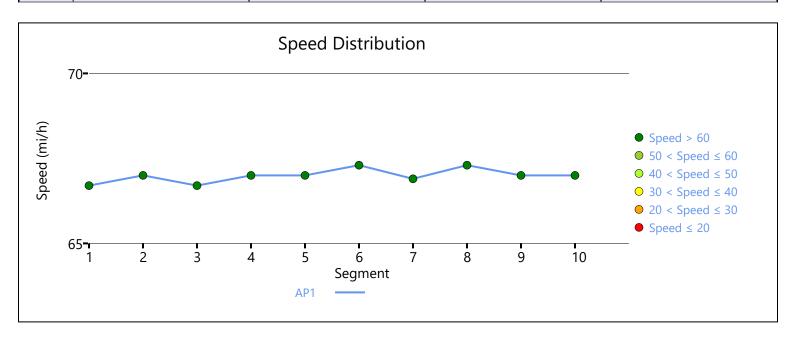
Bicycl	e LOS	F				
		S	egn	nent 5		
Vehi	icle Inputs					
Segm	ent Type	Passing Constrained		Length, ft		980
Meası	ured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Den	nand and Capacity					
Direct	ional Demand Flow Rate, veh/h	477		Opposing Deman	d Flow Rate, veh/h	-
Peak I	Hour Factor	0.88		Total Trucks, %		18.44
Segm	ent Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.28
Inte	rmediate Results					
Segm	ent Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed	l Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF Slo	pe Coefficient (m)	-1.29166		PF Power Coefficie	ent (p)	0.76014
In Pas	sing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		3.7
%lmp	rovement to Percent Followers	0.0		%Improvement to Speed		0.0
Sub	segment Data					
#	Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	980	-		-	67.0
Vehi	icle Results					
Avera	ge Speed, mi/h	67.0		Percent Followers	, %	52.1
Segm	ent Travel Time, minutes	0.17		Follower Density (FD), followers/mi/ln		3.7
Vehicl	e LOS	В				
Bicy	cle Results					
Percei	nt Occupied Parking	0		Pavement Conditi	on Rating	4
Flow F	Rate Outside Lane, veh/h	477		Bicycle Effective Width, ft		24
Bicycl	e LOS Score	10.05		Bicycle Effective S	peed Factor	5.07
Bicycl	e LOS	F				
		S	egn	nent 6		
Vehi	icle Inputs					
Segm	ent Type	Passing Zone		Length, ft		3667
Meası	ured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Den	nand and Capacity					
Direct	ional Demand Flow Rate, veh/h	477		Opposing Deman	d Flow Rate, veh/h	355
Peak I	Hour Factor	0.88		Total Trucks, %		18.44
Segm	ent Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.28
Inte	rmediate Results					

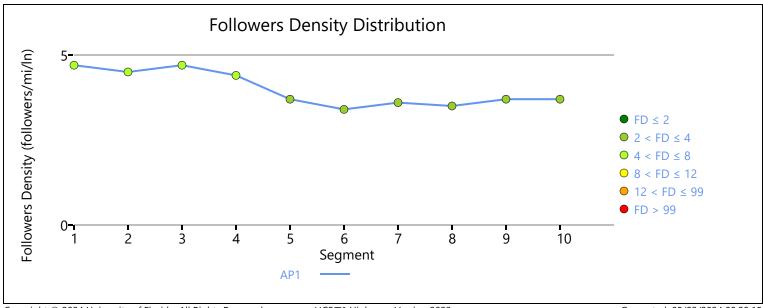
Segment Vertical Class 1		Free-Flow Speed, mi/h 70.0				
	d Slope Coefficient (m)	4.40174		Speed Power Coe		0.50464
	ope Coefficient (m)	-1.19184		PF Power Coefficient (p)		0.82692
In Pa	ssing Lane Effective Length?	No			ensity, veh/mi/ln	3.4
%Improvement to Percent Followers		0.0		%Improvement to	Speed	0.0
Subsegment Data				· · · · ·		
#	Segment Type	Length, ft Rad		lius, ft Superelevation, %		Average Speed, mi/h
1 Tangent 3667 -			-	67.3		
Veh	icle Results					•
Average Speed, mi/h		67.3		Percent Followers	 , %	47.6
	nent Travel Time, minutes	0.62			(FD), followers/mi/ln	3.4
	cle LOS	В		,		
Bicy	ycle Results					
	ent Occupied Parking	0		Pavement Conditi	ion Rating	4
Flow	Rate Outside Lane, veh/h	477		Bicycle Effective Width, ft		24
—— Bicyc	le LOS Score	10.05		Bicycle Effective Speed Factor		5.07
Bicyc	le LOS	F	F			
			Segr	nent 7		
Veh	icle Inputs					
Segment Type Passing Constrained		Length, ft		1846		
		Measured	Measured		mi/h	70.0
Der	nand and Capacity			<u>'</u>		
Direc	tional Demand Flow Rate, veh/h	477	477		d Flow Rate, veh/h	-
Peak	Hour Factor	0.88		Total Trucks, %		18.44
Segn	nent Capacity, veh/h	1700		Demand/Capacity (D/C)		0.28
Inte	ermediate Results					
Segn	nent Vertical Class	1		Free-Flow Speed, mi/h		70.0
Spee	d Slope Coefficient (m)	4.58311	4.58311		fficient (p)	0.41674
PF SI	ope Coefficient (m)	-1.26629	-1.26629		ent (p)	0.77017
In Passing Lane Effective Length?		No	No		ensity, veh/mi/ln	3.6
%lmp	provement to Percent Followers	0.0		%Improvement to Speed		0.0
Sub	segment Data					
# Segment Type Length, ft		Length, ft	Radius, ft		Superelevation, %	Average Speed, mi/h
1	Tangent 1846 -			-	66.9	
Veh	icle Results					
Avera	Average Speed, mi/h 66.9			Percent Followers, %		51.1
Segment Travel Time, minutes			31			

Vehi	icle LOS	В				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Condition Rating		4
Flow	v Rate Outside Lane, veh/h	477		Bicycle Effective W	/idth, ft	24
Bicy	cle LOS Score	10.05		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	F				
		S	egn	nent 8		
Vel	hicle Inputs					
Segi	ment Type	Passing Zone		Length, ft		2174
Mea	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity					
Dire	ctional Demand Flow Rate, veh/h	477		Opposing Demand	d Flow Rate, veh/h	355
Peak	k Hour Factor	0.88		Total Trucks, %		18.44
Segi	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.28
Int	ermediate Results					
Segi	ment Vertical Class	1		Free-Flow Speed, mi/h		70.0
Speed Slope Coefficient (m)		4.38201		Speed Power Coefficient (p)		0.50464
PF Slope Coefficient (m)		-1.22645		PF Power Coefficient (p)		0.81448
In Passing Lane Effective Length? No		No	No		nsity, veh/mi/ln	3.5
%lm	provement to Percent Followers	0.0	0.0		Speed	0.0
Sul	bsegment Data					
#	Segment Type	Length, ft	Rad	dius, ft Superelevation, %		Average Speed, mi/h
1	Tangent	2174	1-	-		67.3
Vel	hicle Results					
Aver	rage Speed, mi/h	67.3		Percent Followers, %		48.9
Segi	ment Travel Time, minutes	0.37		Follower Density (FD), followers/mi/ln		3.5
Vehi	icle LOS	В	В			
Bic	cycle Results	<u>'</u>				
Perc	ent Occupied Parking	0		Pavement Condition Rating		4
Flow Rate Outside Lane, veh/h 477		477	Bicycle Effective			24
Bicycle LOS Score 10.05		10.05	10.05		peed Factor	5.07
Bicy	cle LOS Score	Bicycle LOS F				i
_		F				
_			egn	nent 9		
Bicy			egn	nent 9		
Bicy Vel	cle LOS		egn	nent 9		1277

Demand and Capacity					
Directional Demand Flow Rate, veh/h	ectional Demand Flow Rate, veh/h 477		Opposing Demar	nd Flow Rate, veh/h	-
Peak Hour Factor	0.88		Total Trucks, %	Total Trucks, %	
Segment Capacity, veh/h	1700	Deman		y (D/C)	0.28
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	efficient (p)	0.41674
PF Slope Coefficient (m)	-1.29166	-1.29166		ent (p)	0.76014
In Passing Lane Effective Length?	No		Total Segment De	ensity, veh/mi/ln	3.7
%Improvement to Percent Followers	0.0		%Improvement to	o Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rad	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	1277	-		-	67.0
Vehicle Results					
Average Speed, mi/h	67.0		Percent Followers	Percent Followers, %	
Segment Travel Time, minutes	0.22		Follower Density (FD), followers/mi/ln		3.7
Vehicle LOS	В				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condit	ion Rating	4
Flow Rate Outside Lane, veh/h	477		Bicycle Effective V	Vidth, ft	24
Bicycle LOS Score	10.05	10.05		Speed Factor	5.07
Bicycle LOS	F				
		Segn	nent 10		
Vehicle Inputs					
Segment Type	Passing Constraine	ed	Length, ft		898
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	477		Opposing Demand Flow Rate, veh/h		-
Peak Hour Factor	0.88		Total Trucks, %		18.44
Segment Capacity, veh/h 1700		Demand/Capacity (D/C)		0.28	
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed, mi/h		70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coefficient (p)		0.41674
PF Slope Coefficient (m)	-1.29166		PF Power Coeffici	ent (p)	0.76014
In Passing Lane Effective Length?	No		Total Segment De	ensity, veh/mi/ln	3.7
%Improvement to Percent Followers	0.0		%Improvement to	o Speed	0.0
Subsegment Data					

#	Segment Type	Length, ft	Radius, ft		Superelevation, %	Average Speed, mi/h		
1	Tangent	779	-		-	67.0		
Veh	Vehicle Results							
Average Speed, mi/h		67.0 Per		Percent Followers, %		52.1		
Segn	nent Travel Time, minutes	0.15	Follo	ower Density (	FD), followers/mi/ln	3.7		
Vehic	cle LOS	В						
Bicycle Results								
Perce	ent Occupied Parking	0	Pave	ment Condition	on Rating	4		
Flow Rate Outside Lane, veh/h		477	Bicy	cle Effective W	idth, ft	24		
Bicycle LOS Score		10.05 Bicy		Bicycle Effective Speed Factor		5.07		
Bicycle LOS		F						
Facility Results								
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS		
1	339	0.21			3.9	В		





Copyright © 2024 University of Florida. All Rights Reserved.

HCS™ Highways Version 2023 I90EBRamp466th\_469th\_PM\_2050\_WB.xuf

Generated: 05/02/2024 09:28:15

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	3/7/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build Option 1	Time Analyzed	AM
Project Description	466th St to I90 WB Ramps	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Ca	pacity		
Volume (V) veh/h	769	Heavy Vehicle Adjustment Factor (fHV)	0.980
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	446
Total Trucks, %	2.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.19
Direction 1 Speed and Dens	ity		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	6.4
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	437	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.88
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	acity		
Volume (V) veh/h	436	Heavy Vehicle Adjustment Factor (fHV)	0.833
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	298
Total Trucks, %	20.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.13
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.3
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.0		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	248	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	10.71
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F
Convigant © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	nys Varsian 2022	Congrated: 05/02/2024 00:16:53

Copyright © 2024 University of Florida. All Rights Reserved.

HCSTM Highways Version 2023 466th\_I90WBRamp\_2050\_AM.xuf Generated: 05/02/2024 09:16:53

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	3/7/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build Option 1	Time Analyzed	PM
Project Description	466th St to I90 WB Ramps	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Ca	pacity		
Volume (V) veh/h	450	Heavy Vehicle Adjustment Factor (fHV)	0.917
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	279
Total Trucks, %	9.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12
Direction 1 Speed and Dens	ity		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.0
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	256	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	4.92
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	E

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	acity		
Volume (V) veh/h	910	Heavy Vehicle Adjustment Factor (fHV)	0.971
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	532
Total Trucks, %	3.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.23
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	7.6
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.0		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	517	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.23
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С
Converight © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	avs Varsian 2023	Generated: 05/02/2024 09:17:41

Copyright © 2024 University of Florida. All Rights Reserved.

HCSTM Highways Version 2023 466th\_I90WBRamp\_2050\_PM.xuf Generated: 05/02/2024 09:17:41

	HCS Multilane	e Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build Option 1	Time Analyzed	AM
Project Description	469th to LaMesa	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	ЕВ		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	2.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Ca	pacity		
Volume (V) veh/h	610	Heavy Vehicle Adjustment Factor (fHV)	0.962
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	360
Total Trucks, %	4.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.16
Direction 1 Speed and Dens	ity		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.5
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	5.2
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.5		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	347	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.32
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	1.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.8	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	acity		
Volume (V) veh/h	240	Heavy Vehicle Adjustment Factor (fHV)	0.820
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	166
Total Trucks, %	22.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.07
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.8
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	2.4
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.3		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	136	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	11.74
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F
Convigant © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	nys Varsian 2022	Congrated: 05/02/2024 00:23:11

HCSTM Highways Version 2023 469th\_LaMesaDr\_2050\_AM.xuf Generated: 05/02/2024 09:23:11

	HCS Multilane	e Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build Option1	Time Analyzed	PM
Project Description	469th to LaMesa	Units	U.S. Customary
Direction 1 Geometric Data	·		
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	2.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	366	Heavy Vehicle Adjustment Factor (fHV)	0.917
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	227
Total Trucks, %	9.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.10
Direction 1 Speed and Densi	ty	·	
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.5
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	3.3
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.5		
Direction 1 Bicycle LOS		•	•
Flow Rate in Outside Lane (vOL), veh/h	208	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	4.82
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	E

WB		
2	Terrain Type	Level
Base	Percent Grade, %	-
70.0	Grade Length, mi	-
12	Access Point Density, pts/mi	1.0
TWLTL	Left-Side Lateral Clearance (LCR), ft	6
69.8	Total Lateral Clearance (TLC), ft	12
ors		
All Familiar	Final Speed Adjustment Factor (SAF)	1.000
1.000	Final Capacity Adjustment Factor (CAF)	1.000
1.000		
pacity		
666	Heavy Vehicle Adjustment Factor (fHV)	0.971
0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	390
3.00	Capacity (c), pc/h/ln	2300
-	Adjusted Capacity (cadj), pc/h/ln	2300
-	Volume-to-Capacity Ratio (v/c)	0.17
ty		
0.0	Average Speed (S), mi/h	69.8
0.0	Density (D), pc/mi/ln	5.6
0.0	Level of Service (LOS)	А
0.3		
378	Effective Speed Factor (St)	5.07
18	Bicyle LOS Score (BLOS)	3.07
24	Bicycle Level of Service (LOS)	С
	2 Base 70.0 12 TWLTL 69.8 OFS All Familiar 1.000 1.000 0.88 3.00	2 Terrain Type Base Percent Grade, % 70.0 Grade Length, mi 12 Access Point Density, pts/mi TWLTL Left-Side Lateral Clearance (LCR), ft 69.8 Total Lateral Clearance (TLC), ft  OOTS  All Familiar Final Speed Adjustment Factor (SAF) 1.000 Final Capacity Adjustment Factor (CAF) 1.000  Dacity  666 Heavy Vehicle Adjustment Factor (fHV) 0.88 Flow Rate (Vp), pc/h/ln - Adjusted Capacity (cadj), pc/h/ln - Volume-to-Capacity Ratio (v/c)  (ity  0.0 Average Speed (S), mi/h 0.0 Density (D), pc/mi/ln 0.0 Level of Service (LOS) 0.3  378 Effective Speed Factor (St) 18 Bicyle LOS Score (BLOS) 24 Bicycle Level of Service (LOS)

HCSTM Highways Version 2023 469th\_LaMesaDr\_2050\_PM.xuf Generated: 05/02/2024 09:23:39

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	3/7/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build Option 1	Time Analyzed	AM
Project Description	I90 WB Ramps to I90 EB Ramps	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	745	Heavy Vehicle Adjustment Factor (fHV)	0.971
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	436
Total Trucks, %	3.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.19
Direction 1 Speed and Densi	ty	<u>'</u>	
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	6.2
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	423	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.13
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	273	Heavy Vehicle Adjustment Factor (fHV)	0.877
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	177
Total Trucks, %	14.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.08
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	2.5
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.0		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	155	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	6.98
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

HCSໝ Highways Version 2023 I90WBRamp\_I90EBRamp\_2050\_AM.xuf Generated: 05/02/2024 09:31:33

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	3/7/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build Option 1	Time Analyzed	PM
Project Description	I90 WB Ramps to I90 EB Ramps	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	451	Heavy Vehicle Adjustment Factor (fHV)	0.917
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	280
Total Trucks, %	9.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12
Direction 1 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.0
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	256	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	4.92
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	E

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	455	Heavy Vehicle Adjustment Factor (fHV)	0.877
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	295
Total Trucks, %	14.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.13
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.2
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.0		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	259	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	7.24
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

HCSTM Highways Version 2023 I90WBRamp\_I90EBRamp\_2050\_PM.xuf Generated: 05/02/2024 09:32:05

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	3/7/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build Option 1	Time Analyzed	AM
Project Description	Mickelson Rd to 466th St	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	1.5
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.6	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	ors	<u> </u>	
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Ca	pacity		
Volume (V) veh/h	725	Heavy Vehicle Adjustment Factor (fHV)	0.990
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	416
Total Trucks, %	1.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.18
Direction 1 Speed and Densi	ity		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.6
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	6.0
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.4		
Direction 1 Bicycle LOS	•		•
Flow Rate in Outside Lane (vol.), veh/h	412	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.61
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	2.2
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	425	Heavy Vehicle Adjustment Factor (fHV)	0.885
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	273
Total Trucks, %	13.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.4
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	3.9
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.6		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	241	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	6.70
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

HCSTM Highways Version 2023 Mickelson\_466th\_2050\_AM.xuf Generated: 05/02/2024 09:32:56

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	3/7/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build Option 1	Time Analyzed	PM
Project Description	Mickelson Rd to 466th St	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	1.5
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.6	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	445	Heavy Vehicle Adjustment Factor (fHV)	0.901
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	280
Total Trucks, %	11.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12
Direction 1 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.6
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.0
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.4		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	253	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	5.78
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	2.2
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	acity		
Volume (V) veh/h	913	Heavy Vehicle Adjustment Factor (fHV)	0.980
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	530
Total Trucks, %	2.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.23
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.4
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	7.6
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.6		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	519	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.97
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С
Converight © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	avs Varsian 2023	Ganaratad: 05/02/2024 09:33:30

HCSTM Highways Version 2023 Mickelson\_466th\_2050\_PM.xuf Generated: 05/02/2024 09:33:30

		HCS Two-L	ane	Highway Re	port	
Project Information	n					
Analyst		MJV		Date		5/2/2024
Agency		HRG		Analysis Year		2050 Build Option 1
Jurisdiction		SDDOT		Time Analyzed		AM Peak
Project Description		West of Hartford SD	38 EB	Units		U.S. Customary
			Segn	nent 1		
Vehicle Inputs						
Segment Type		Passing Zone		Length, ft		1069
Measured FFS		Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capac	ity					<b>'</b>
Directional Demand Flow F	Rate, veh/h	243		Opposing Deman	d Flow Rate, veh/h	169
Peak Hour Factor		0.88		Total Trucks, %		5.79
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.14
Intermediate Resul	ts					<b>'</b>
Segment Vertical Class		1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m	1)	4.30713		Speed Power Coe	fficient (p)	0.54838
PF Slope Coefficient (m)		-1.23090	-1.23090		ent (p)	0.80942
In Passing Lane Effective Le	ength?	No		Total Segment De	ensity, veh/mi/ln	1.2
%Improvement to Percent	Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data						•
# Segment Type		Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent		1069	1-		-	68.5
Vehicle Results						_
Average Speed, mi/h		68.5		Percent Followers	, %	32.4
Segment Travel Time, minu	ites	0.18		Follower Density (FD), followers/mi/ln		1.2
Vehicle LOS		Α				
Bicycle Results						-
Percent Occupied Parking		0		Pavement Conditi	ion Rating	4
Flow Rate Outside Lane, veh/h		243		Bicycle Effective Width, ft		24
		3.70		Bicycle Effective Speed Factor		5.07
Bicycle LOS		D				
			Segn	nent 2		
Vehicle Inputs			<u> </u>			
Segment Type		Passing Constrained		Length, ft		664
Measured FFS		Measured		Free-Flow Speed,	mi/h	70.0
				I serious speed,	,	

Demand and Capacity					
Directional Demand Flow Rate, veh/h	243		Opposing Deman	d Flow Rate, veh/h	-
Peak Hour Factor	0.88	0.88			5.79
Segment Capacity, veh/h	1700		Demand/Capacity	/ (D/C)	0.14
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29315		PF Power Coeffici	ent (p)	0.75829
In Passing Lane Effective Length?	No		Total Segment De	ensity, veh/mi/ln	1.3
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	664	-		-	68.0
Vehicle Results					
Average Speed, mi/h	68.0		Percent Followers	, %	35.8
Segment Travel Time, minutes	0.11		Follower Density (FD), followers/mi/ln		1.3
Vehicle LOS	A				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condit	ion Rating	4
Flow Rate Outside Lane, veh/h	243		Bicycle Effective V	Vidth, ft	24
Bicycle LOS Score	3.70		Bicycle Effective Speed Factor		5.07
Bicycle LOS	D				
		Segr	ment 3		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		1871
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	243		Opposing Deman	d Flow Rate, veh/h	169
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.14
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.31694		Speed Power Coe	fficient (p)	0.54838
PF Slope Coefficient (m)	-1.20586		PF Power Coeffici	ent (p)	0.82063
In Passing Lane Effective Length?	No		Total Segment De	ensity, veh/mi/ln	1.1
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
			The second secon		

#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	1871	-		-	68.5
Vel	nicle Results					
Aver	age Speed, mi/h	68.5		Percent Followers	, %	31.5
Segr	ment Travel Time, minutes	0.31		Follower Density (	FD), followers/mi/ln	1.1
Vehi	cle LOS	А				
Bic	ycle Results					
Perce	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow	Rate Outside Lane, veh/h	243		Bicycle Effective V	Vidth, ft	24
Bicyc	cle LOS Score	3.70		Bicycle Effective S	peed Factor	5.07
Bicyc	cle LOS	D				
			Segi	ment 4		·
Vel	nicle Inputs					
Segr	ment Type	Passing Constrain	ned	Length, ft		925
	sured FFS	Measured			Free-Flow Speed, mi/h	
Dei	mand and Capacity	1		<u>'</u>		1
Dire	ctional Demand Flow Rate, veh/h	243		Opposing Deman	d Flow Rate, veh/h	-
Peak	Hour Factor	0.88		Total Trucks, %	Total Trucks, %	
Segr	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
Inte	ermediate Results	•				
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spee	ed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF SI	lope Coefficient (m)	-1.29315		PF Power Coefficient (p)		0.75829
In Pa	assing Lane Effective Length?	No		Total Segment De	Total Segment Density, veh/mi/ln	
%lm	provement to Percent Followers	0.0		%Improvement to Speed		0.0
Suk	osegment Data					
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	925	-		-	68.0
Vel	nicle Results					•
Aver	rage Speed, mi/h	68.0		Percent Followers,	, %	35.8
Segment Travel Time, minutes 0.15			Follower Density (FD), followers/mi/ln		1.3	
Vehicle LOS A						
Bic	ycle Results					,
Perce	ent Occupied Parking	0		Pavement Conditi	on Rating	4
	Rate Outside Lane, veh/h	243		Bicycle Effective V		24
Bicyc	cle LOS Score	3.70		Bicycle Effective S		5.07
Bicycle LOS D						

	Se	egn	ment 5		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		4476
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	243		Opposing Demand	d Flow Rate, veh/h	169
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.35043		Speed Power Coef	fficient (p)	0.54838
PF Slope Coefficient (m)	-1.15155		PF Power Coefficie	ent (p)	0.84082
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.1
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	4476	-		-	68.5
Vehicle Results					
Average Speed, mi/h	68.5		Percent Followers,	. %	29.6
Segment Travel Time, minutes	0.74		Follower Density (FD), followers/mi/ln		1.1
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	243		Bicycle Effective Width, ft		24
Bicycle LOS Score	3.70		Bicycle Effective Speed Factor		5.07
Bicycle LOS	D				
	Se	egn	ment 6		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		896
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	243		Opposing Demand	d Flow Rate, veh/h	-
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
1	1				

Speed Slope Coefficient (m)	4.57372		Speed Power Co	pefficient (n)	0.41674
PF Slope Coefficient (m)			PF Power Coeffic	·	0.75829
In Passing Lane Effective Length?	No			Density, veh/mi/ln	1.3
%Improvement to Percent Followers	0.0		%Improvement		0.0
Subsegment Data	1				
# Segment Type	Length, ft	Rac	lius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	896	-		-	68.0
Vehicle Results					
Average Speed, mi/h	68.0		Percent Followe	rs. %	35.8
Segment Travel Time, minutes	0.15			/ (FD), followers/mi/ln	1.3
Vehicle LOS	Α		Tollower Berisity	(1 <i>D</i> ), 10110We13,1111,111	1.3
Bicycle Results	^				
•	Т.		I		
Percent Occupied Parking	0		Pavement Cond		4
Flow Rate Outside Lane, veh/h	243		Bicycle Effective		24
Bicycle LOS Score	3.70		Bicycle Effective	Speed Factor	5.07
Bicycle LOS	D				
		Segn	nent 7		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		743
Measured FFS	Measured		Free-Flow Speed	d, mi/h	70.0
Demand and Capacity	•				
Directional Demand Flow Rate, veh/h	243		Opposing Dema	and Flow Rate, veh/h	169
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.14
Intermediate Results	•		•		
Segment Vertical Class	1		Free-Flow Speed, mi/h		70.0
Speed Slope Coefficient (m)	4.30713		Speed Power Coefficient (p)		0.54838
PF Slope Coefficient (m)	-1.23090		PF Power Coefficient (p)		0.80942
In Passing Lane Effective Length?	No		Total Segment D	Density, veh/mi/ln	1.2
%Improvement to Percent Followers	0.0		%Improvement to Speed		0.0
Subsegment Data			•		•
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	743	-		-	68.5
Vehicle Results				•	
Average Speed, mi/h	68.5		Percent Followe	rs, %	32.4
Segment Travel Time, minutes	0.12		Follower Density	/ (FD), followers/mi/ln	1.2
Vehicle LOS A					

Bicycle Results					
Percent Occupied Parking	0	Р	Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	243	В	Bicycle Effective W	idth, ft	24
Bicycle LOS Score	3.70	В	Bicycle Effective Sp	peed Factor	5.07
Bicycle LOS	D				
	Se	egme	ent 8		
Vehicle Inputs					
Segment Type	Passing Zone	L	 _ength, ft		2717
Measured FFS	Measured	F	ree-Flow Speed, r	mi/h	70.0
Demand and Capacity	<u>'</u>				
Directional Demand Flow Rate, veh/h	245	C	Opposing Demand	d Flow Rate, veh/h	165
Peak Hour Factor	0.88	Т	Total Trucks, %		3.28
Segment Capacity, veh/h	1700	С	Demand/Capacity	(D/C)	0.14
Intermediate Results					
Segment Vertical Class	1	F	Free-Flow Speed, r	mi/h	70.0
Speed Slope Coefficient (m)			Speed Power Coefficient (p)		0.54983
PF Slope Coefficient (m)	-1.17918		PF Power Coefficient (p)		0.83165
In Passing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		1.1
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	2717	-		-	68.5
Vehicle Results					
Average Speed, mi/h	68.5	Р	Percent Followers,	%	30.7
Segment Travel Time, minutes	0.45	F	Follower Density (FD), followers/mi/ln		1.1
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0	Р	Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	245	В	Bicycle Effective W	idth, ft	24
Bicycle LOS Score	2.93	В	Bicycle Effective Sp	peed Factor	5.07
Bicycle LOS	С				
	Se	egme	ent 9		
Vehicle Inputs					
Segment Type	Passing Constrained	L	 _ength, ft		1013
Measured FFS	Measured		Free-Flow Speed, r	mi/h	70.0
Demand and Capacity	•				'

Directional Demand Flour Bate web/b	245		Onnesina Deman	d Flavy Data wab /b	
Directional Demand Flow Rate, veh/h  Peak Hour Factor			Opposing Demand Flow Rate, veh/h		3.28
	0.88		Total Trucks, %		+
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
Intermediate Results					
Segment Vertical Class	1	1 F		mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29345		PF Power Coefficie	ent (p)	0.75792
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.3
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rad	lius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	1013	1-		-	68.0
Vehicle Results					
Average Speed, mi/h	68.0		Percent Followers,	, %	36.0
Segment Travel Time, minutes	0.17		Follower Density (	FD), followers/mi/ln	1.3
Vehicle LOS	А				
Bicycle Results	•				
Percent Occupied Parking	0		Pavement Condition Rating		4
Flow Rate Outside Lane, veh/h	245		Bicycle Effective Width, ft		24
Bicycle LOS Score	2.93		Bicycle Effective Speed Factor		5.07
Bicycle LOS	С				
	S	Segm	ent 10		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		4569
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity				<u>,                                      </u>	
Directional Demand Flow Rate, veh/h	245		Onnosing Deman	d Flow Rate, veh/h	165
Peak Hour Factor	0.88		Total Trucks, %	a riow Rate, veri, ii	3.28
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.14
Intermediate Results	1100		Demana, capacity	(2, 2)	0.11
	1.		Free-Flow Speed,		1
Segment Vertical Class		1			70.0
Speed Slope Coefficient (m)	4.34958		Speed Power Coefficient (p)		0.54983
PF Slope Coefficient (m)	-1.14981		PF Power Coefficie		0.84100
In Passing Lane Effective Length?	No		Total Segment De		1.1
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rad	lius, ft	Superelevation, %	Average Speed, mi/h

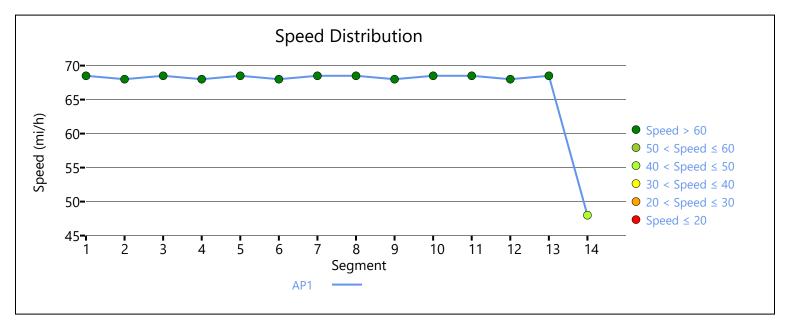
1	Tangant	4560				60 Г
1	Tangent	4569	-		-	68.5
Vehi	icle Results					
Avera	ge Speed, mi/h	68.5		Percent Followers,	, %	29.7
Segm	ent Travel Time, minutes	0.76		Follower Density (	FD), followers/mi/ln	1.1
Vehicl	e LOS	А				
Bicy	cle Results					
Perce	nt Occupied Parking	0		Pavement Conditi	on Rating	4
Flow I	Rate Outside Lane, veh/h	245		Bicycle Effective W	Vidth, ft	24
Bicycl	e LOS Score	2.93		Bicycle Effective S	peed Factor	5.07
Bicycl	e LOS	С				
			Segn	nent 11		
Vehi	icle Inputs					
Segm	ent Type	Passing Zone		Length, ft		5676
Meası	ured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Den	nand and Capacity	<u>'</u>		•		
Direct	ional Demand Flow Rate, veh/h	244		Opposing Demand Flow Rate, veh/h		165
Peak I	Hour Factor	0.88		Total Trucks, %		2.82
Segm	ent Capacity, veh/h	1700		Demand/Capacity (D/C)		0.14
Inte	rmediate Results					
Segm	ent Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed	l Slope Coefficient (m)	4.36055		Speed Power Coefficient (p)		0.54983
PF Slc	pe Coefficient (m)	-1.14222		PF Power Coefficient (p)		0.84066
In Pas	sing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		1.1
%lmp	rovement to Percent Followers	0.0		%Improvement to	0.0	
Sub	segment Data					
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	5676	-		-	68.5
Vehi	icle Results	<u>'</u>				,
Avera	ge Speed, mi/h	68.5		Percent Followers,	, %	29.5
	ent Travel Time, minutes	0.94		Follower Density (FD), followers/mi/ln		1.1
Vehicle LOS A		7. 7. 1. 1. 7.				
Bicy	cle Results	<u>'</u>				
Perce	nt Occupied Parking	0		Pavement Conditi	on Rating	4
Flow I	Rate Outside Lane, veh/h	244		Bicycle Effective W	Vidth, ft	24
Bicycl	e LOS Score	2.80		Bicycle Effective S	peed Factor	5.07
,		C C				
Bicycl	e LOS	10				

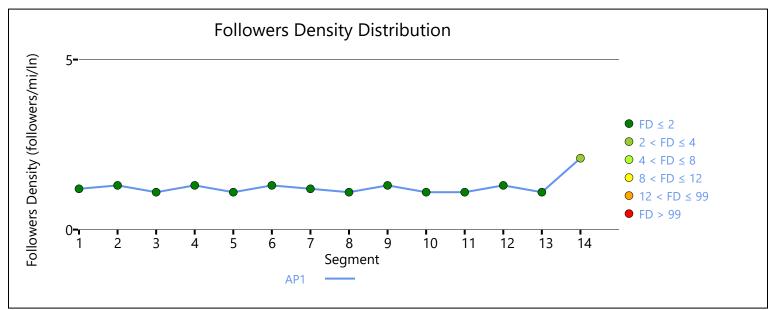
Ve	hicle Inputs					
Seg	gment Type	Passing Constraine	d	Length, ft		657
Me	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	emand and Capacity					
Dir	ectional Demand Flow Rate, veh/h	244		Opposing Deman	d Flow Rate, veh/h	-
Pea	ak Hour Factor	0.88		Total Trucks, %		2.82
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
In	termediate Results					
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	eed Slope Coefficient (m)	4.57372		Speed Power Coef	fficient (p)	0.41674
PF	Slope Coefficient (m)	-1.29350		PF Power Coefficie	ent (p)	0.75785
In F	Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.3
%Ir	mprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Su	ıbsegment Data					
#	Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	657	-		-	68.0
Ve	hicle Results					
Ave	erage Speed, mi/h	68.0		Percent Followers,	%	35.9
Seg	gment Travel Time, minutes	0.11		Follower Density (	FD), followers/mi/ln	1.3
Veł	nicle LOS	А				
Bi	cycle Results					
Per	cent Occupied Parking	0		Pavement Condition	on Rating	4
Flo	w Rate Outside Lane, veh/h	244		Bicycle Effective Width, ft		24
Bic	ycle LOS Score	2.80		Bicycle Effective Speed Factor		5.07
Bic	ycle LOS	С				
			Segm	ent 13		
Ve	hicle Inputs					
Seg	gment Type	Passing Zone		Length, ft		6009
Me	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	emand and Capacity					
Dir	ectional Demand Flow Rate, veh/h	244		Opposing Demand	d Flow Rate, veh/h	165
Pea	ak Hour Factor	0.88		Total Trucks, %		2.82
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
In	termediate Results					
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	eed Slope Coefficient (m)	4.36364		Speed Power Coef	fficient (p)	0.54983
	Slope Coefficient (m)	-1.14089		PF Power Coefficie	ent (p)	0.83997

In Passing Lane Effective Length?		No		Total Segment De	ensity, veh/mi/ln	1.1
%Improvement to Percent Followers		0.0		%Improvement t	o Speed	0.0
Sul	osegment Data					
#	Segment Type	Length, ft	R	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	6009	-		-	68.5
Vel	nicle Results					
Aver	rage Speed, mi/h	68.5		Percent Followers	5, %	29.5
Segi	ment Travel Time, minutes	1.00		Follower Density	(FD), followers/mi/ln	1.1
Vehi	cle LOS	A				
Bic	ycle Results			·		
Perc	ent Occupied Parking	0		Pavement Condit	ion Rating	4
Flow	Rate Outside Lane, veh/h	244		Bicycle Effective	Width, ft	24
Bicy	cle LOS Score	2.80		Bicycle Effective S	Speed Factor	5.07
Bicy	cle LOS	С				
			Segi	ment 14		
Vel	nicle Inputs					
Segi	ment Type	Passing Constrained		Length, ft	Length, ft	
Mea	sured FFS	Measured		Free-Flow Speed	mi/h	50.0
De	mand and Capacity	·				
Dire	ctional Demand Flow Rate, veh/h	244		Opposing Demar	nd Flow Rate, veh/h	-
Peak	Hour Factor	0.88	Total Trucks, %			2.82
Segi	nent Capacity, veh/h	1700		Demand/Capacit	y (D/C)	0.14
Int	ermediate Results					
Segi	ment Vertical Class	1		Free-Flow Speed	mi/h	50.0
Spe	ed Slope Coefficient (m)	4.57372		Speed Power Coefficient (p)		0.41674
PF S	lope Coefficient (m)	-1.47375		PF Power Coefficient (p)		0.71164
In Pa	assing Lane Effective Length?	No		Total Segment De	Total Segment Density, veh/mi/ln	
%lm	provement to Percent Followers	0.0		%Improvement to Speed		0.0
Sul	osegment Data					
#	Segment Type	Length, ft	R	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	891			-	48.0
Vel	nicle Results					
Aver	rage Speed, mi/h	48.0		Percent Followers	5, %	41.8
Segi	ment Travel Time, minutes	0.21		Follower Density	(FD), followers/mi/ln	2.1
Vehi	cle LOS	В				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Condit	ion Rating	4
		1				

Flow Rate Outside Lane, veh/h	244	Bicycle Effective Width, ft	24
Bicycle LOS Score	2.59	Bicycle Effective Speed Factor	4.42
Bicycle LOS	С		
Facility Results			

	T	VMT veh-mi/AP	VHD veh-h/p	Follower Density, followers/ mi/ln	LOS
ĺ	1	327	0.11	1.1	А





	HCS Two	-Lane	Highway Re	eport	
Project Information					
Analyst	MJV		Date		5/2/2024
Agency	HRG		Analysis Year		2050 Build Option 1
Jurisdiction	SDDOT		Time Analyzed		PM Peak
Project Description	West of Hartford	SD 38 EB	Units		U.S. Customary
		Segn	nent 1		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		1069
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					<u>'</u>
Directional Demand Flow Rate, v	eh/h 157		Opposing Demar	nd Flow Rate, veh/h	286
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity	y (D/C)	0.09
Intermediate Results	'		'		<u>'</u>
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.34767		Speed Power Coe	efficient (p)	0.51808
PF Slope Coefficient (m)	-1.25475		PF Power Coeffici	ent (p)	0.80124
In Passing Lane Effective Length?	, No		Total Segment De	ensity, veh/mi/ln	0.6
%Improvement to Percent Follov	vers 0.0		%Improvement to	o Speed	0.0
Subsegment Data	•		,		<u>'</u>
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	1069	-		-	69.0
Vehicle Results	<u>'</u>			1	<u>'</u>
Average Speed, mi/h	69.0		Percent Followers	;, %	24.8
Segment Travel Time, minutes	0.18		Follower Density	(FD), followers/mi/ln	0.6
Vehicle LOS	A		y ( ), 2.12.12.2yy		
Bicycle Results	<u> </u>				
Percent Occupied Parking	0		Pavement Condit	ion Rating	4
Flow Rate Outside Lane, veh/h 157		Bicycle Effective V		30	
Bicycle LOS Score 1.86		Bicycle Effective S		5.07	
Bicycle LOS	В				
		Segn	nent 2		
Vehicle Inputs					
Segment Type	Passing Constrair	ned	Length, ft		664
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
			1, 30,	•	

Demand and Capacity					
Directional Demand Flow Rate, veh/h	157		Opposing Demand Flow Rate, veh/h		-
Peak Hour Factor	0.88	1	Total Trucks, %		5.79
Segment Capacity, veh/h	1700	]	Demand/Capacity	(D/C)	0.09
Intermediate Results					
Segment Vertical Class	1	ı	Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372	9	Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29315	ı	PF Power Coefficie	ent (p)	0.75829
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%Improvement to Percent Followers	0.0	Ç	%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Radiu	us, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	664	-		-	68.6
Vehicle Results					
Average Speed, mi/h	68.6	ı	Percent Followers,	, %	27.2
Segment Travel Time, minutes	0.11	ı	Follower Density (FD), followers/mi/ln		0.6
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0	ı	Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	157	E	Bicycle Effective Width, ft		30
Bicycle LOS Score	1.86	E	Bicycle Effective Speed Factor		5.07
Bicycle LOS	В				
	S	egm	ent 3		
Vehicle Inputs					
Segment Type	Passing Zone	I	Length, ft		1871
Measured FFS	Measured	F	Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	157	(	Opposing Demand Flow Rate, veh/h		286
Peak Hour Factor	0.88	-	Total Trucks, %		5.79
Segment Capacity, veh/h	1700	ı	Demand/Capacity	(D/C)	0.09
Intermediate Results					
Segment Vertical Class	1	ı	Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.35747		Speed Power Coe	fficient (p)	0.51808
PF Slope Coefficient (m)	-1.22915	ı	PF Power Coefficie	ent (p)	0.81213
In Passing Lane Effective Length?	No	-	Total Segment De	nsity, veh/mi/ln	0.5
%Improvement to Percent Followers	0.0	Ç.	%Improvement to	Speed	0.0
Subsegment Data					

#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	1871	-		-	69.0
Veh	nicle Results				·	
Aver	age Speed, mi/h	69.0		Percent Followe	ers, %	23.9
Segn	nent Travel Time, minutes	0.31		Follower Densit	y (FD), followers/mi/ln	0.5
Vehic	cle LOS	А				
Bic	ycle Results					
Perce	ent Occupied Parking	0		Pavement Cond	lition Rating	4
Flow	Rate Outside Lane, veh/h	157		Bicycle Effective	e Width, ft	30
Bicyc	le LOS Score	1.86		Bicycle Effective	e Speed Factor	5.07
Bicyc	ile LOS	В				
			Segi	ment 4		
Veh	icle Inputs					
Segn	nent Type	Passing Constrain	ned	Length, ft		925
Meas	sured FFS	Measured		Free-Flow Spee	d, mi/h	70.0
Der	nand and Capacity					'
Direc	ctional Demand Flow Rate, veh/h	157		Opposing Dem	and Flow Rate, veh/h	-
Peak	Hour Factor	0.88		Total Trucks, %		5.79
Segn	nent Capacity, veh/h	1700		Demand/Capac	ity (D/C)	0.09
Inte	ermediate Results					
Segn	nent Vertical Class	1		Free-Flow Spee	d, mi/h	70.0
Spee	d Slope Coefficient (m)	4.57372		Speed Power C	oefficient (p)	0.41674
PF SI	ope Coefficient (m)	-1.29315	-1.29315		icient (p)	0.75829
In Pa	ssing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.6
%lm <sub>l</sub>	provement to Percent Followers	0.0		%Improvement	to Speed	0.0
Sub	segment Data					
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	925	-		-	68.6
Veh	nicle Results					
Aver	age Speed, mi/h	68.6		Percent Followe	ers, %	27.2
Segn	Segment Travel Time, minutes 0.15		Follower Densit	y (FD), followers/mi/ln	0.6	
Vehicle LOS A						
Bic	ycle Results					
Perce	ent Occupied Parking	0		Pavement Cond	lition Rating	4
Flow	Rate Outside Lane, veh/h	157		Bicycle Effective	e Width, ft	30
Bicyc	le LOS Score	1.86		Bicycle Effective	e Speed Factor	5.07
<u>.                                    </u>	ile LOS	В				

	S	egn	ment 5		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		4476
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					·
Directional Demand Flow Rate, veh/h	157		Opposing Deman	d Flow Rate, veh/h	286
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.09
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.39096		Speed Power Coef	fficient (p)	0.51808
PF Slope Coefficient (m)	-1.17364		PF Power Coefficie	ent (p)	0.83159
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.5
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rad	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	4476	-		-	69.0
Vehicle Results					·
Average Speed, mi/h	69.0		Percent Followers,	. %	22.2
Segment Travel Time, minutes	0.74		Follower Density (	FD), followers/mi/ln	0.5
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition Rating		4
Flow Rate Outside Lane, veh/h	157		Bicycle Effective Width, ft		30
Bicycle LOS Score	1.86		Bicycle Effective Speed Factor		5.07
Bicycle LOS	В				
	S	egn	ment 6		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		896
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity	•				
Directional Demand Flow Rate, veh/h	157		Opposing Demand	d Flow Rate, veh/h	-
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.09
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
			1 ' '		1

Speed Slope Coefficient (m)	4.57372		Speed Power Coo	fficient (n)	0.41674
PF Slope Coefficient (m)			Speed Power Coefficient (p)		0.75829
In Passing Lane Effective Length?			PF Power Coefficient (p)  Total Segment Density, veh/mi/ln		
	0.0				0.6
%Improvement to Percent Followers	0.0		%Improvement to	o speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rac	lius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	896	-		-	68.6
Vehicle Results					
Average Speed, mi/h	68.6		Percent Followers	·, %	27.2
Segment Travel Time, minutes	0.15		Follower Density	(FD), followers/mi/ln	0.6
Vehicle LOS	А		İ		
Bicycle Results	<u>'</u>				
Percent Occupied Parking	0		Pavement Conditi	ion Rating	4
Flow Rate Outside Lane, veh/h	157		Bicycle Effective V	Vidth, ft	30
Bicycle LOS Score	1.86		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	В				
	<u>'</u>	Segn	nent 7		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		743
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity	·				·
Directional Demand Flow Rate, veh/h	157		Opposing Deman	d Flow Rate, veh/h	286
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.09
Intermediate Results	·				·
Segment Vertical Class	1		Free-Flow Speed, mi/h		70.0
Speed Slope Coefficient (m)	4.34767		Speed Power Coefficient (p)		0.51808
PF Slope Coefficient (m)	-1.25475		PF Power Coefficient (p)		0.80124
In Passing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.6
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Length, ft Rad		Superelevation, %	Average Speed, mi/h
1 Tangent	743	-		-	69.0
Vehicle Results					
Average Speed, mi/h	69.0		Percent Followers	, %	24.8
Segment Travel Time, minutes	0.12		Follower Density	(FD), followers/mi/ln	0.6
Vehicle LOS	А				

Bicycle Results					
Percent Occupied Parking	0	П	Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	157 E		Bicycle Effective Width, ft		30
Bicycle LOS Score	1.86		Bicycle Effective Sp	peed Factor	5.07
Bicycle LOS	В				
	Se	egm	ent 8		
Vehicle Inputs					
Segment Type	Passing Zone	П	Length, ft		2717
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity	<u>'</u>				
Directional Demand Flow Rate, veh/h	164	П	Opposing Demand	d Flow Rate, veh/h	289
Peak Hour Factor	0.88		Total Trucks, %		3.28
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Intermediate Results					
Segment Vertical Class	1	П	Free-Flow Speed, mi/h		70.0
Speed Slope Coefficient (m)	4.37072		Speed Power Coefficient (p)		0.51760
PF Slope Coefficient (m)	-1.20338		PF Power Coefficient (p)		0.82225
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Radii	us, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	2717	-		-	68.9
Vehicle Results					
Average Speed, mi/h	68.9	П	Percent Followers, %		23.8
Segment Travel Time, minutes	0.45		Follower Density (FD), followers/mi/ln		0.6
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0	П	Pavement Condition Rating		4
Flow Rate Outside Lane, veh/h	164		Bicycle Effective W	/idth, ft	29
Bicycle LOS Score	1.40		Bicycle Effective Sp	peed Factor	5.07
Bicycle LOS	А				
	Se	egm	ent 9		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		1013
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					

Directional Demand Flow Rate, veh/h	164	Opr	osina Demana	d Flow Rate, veh/h	-
Peak Hour Factor	0.88			a now Nate, veli/II	3.28
Segment Capacity, veh/h	1700		Total Trucks, %  Demand/Capacity (D/C)		0.10
	1700	Den	папи/сарасну	(6/0)	0.10
Intermediate Results					
Segment Vertical Class	1	Free	e-Flow Speed, i	mi/h	70.0
Speed Slope Coefficient (m)	4.57372	Spe	ed Power Coef	ficient (p)	0.41674
PF Slope Coefficient (m)	-1.29345	PF F	Power Coefficie	ent (p)	0.75792
In Passing Lane Effective Length?	No	Tota	ıl Segment Dei	nsity, veh/mi/ln	0.7
%Improvement to Percent Followers	0.0	%Im	nprovement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Radius, ft	t	Superelevation, %	Average Speed, mi/h
1 Tangent	1013	-		-	68.5
Vehicle Results					
Average Speed, mi/h	68.5	Perc	cent Followers,	%	28.0
Segment Travel Time, minutes	0.17	Follo	ower Density (I	FD), followers/mi/ln	0.7
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0	Pave	Pavement Condition Rating		4
Flow Rate Outside Lane, veh/h	164	Bicy	Bicycle Effective Width, ft		29
Bicycle LOS Score	1.40	Bicy	cle Effective Sp	peed Factor	5.07
Bicycle LOS	А				
	Se	gment	: 10		
Vehicle Inputs					
Segment Type	Passing Zone	Len	gth, ft		4569
Measured FFS	Measured	Free	Free-Flow Speed, mi/h		70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	164	Орр	oosing Demand	d Flow Rate, veh/h	289
Peak Hour Factor	0.88	Tota	Total Trucks, %		3.28
Segment Capacity, veh/h	1700	Den	Demand/Capacity (D/C)		0.10
Intermediate Results					
Segment Vertical Class	1		e-Flow Speed, i	mi/h	70.0
Speed Slope Coefficient (m)	4.39263		ed Power Coef	ficient (p)	0.51760
PF Slope Coefficient (m)	-1.17332	PF P	Power Coefficie	ent (p)	0.83118
In Passing Lane Effective Length?	No	Tota	ıl Segment Der	nsity, veh/mi/ln	0.5
-		%Im	provement to	Speed	0.0
, sumprovement to rescent removes					
Subsegment Data					

1 Tanas		4500				C0.0
1 Tange		4569	-		-	68.9
Vehicle F	Results					
Average Speed, mi/h 68.9		Percent Followers,	, %	22.9		
Segment Tra	avel Time, minutes	0.75		Follower Density (	FD), followers/mi/ln	0.5
Vehicle LOS		A				
Bicycle R	Results					
Percent Occ	upied Parking	0		Pavement Conditi	on Rating	4
Flow Rate O	outside Lane, veh/h	164		Bicycle Effective W	Vidth, ft	29
Bicycle LOS	Score	1.40		Bicycle Effective S	peed Factor	5.07
Bicycle LOS		Α				
			Segn	nent 11		
Vehicle I	nputs					
Segment Ty	pe	Passing Zone		Length, ft		5676
Measured F	FS	Measured		Free-Flow Speed,	mi/h	70.0
Demand	and Capacity					
Directional I	Demand Flow Rate, veh/h	164		Opposing Demand Flow Rate, veh/h		280
Peak Hour F	actor	0.88		Total Trucks, %		2.82
Segment Ca	pacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Intermed	diate Results					
Segment Ve	ertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope	e Coefficient (m)	4.40080		Speed Power Coefficient (p)		0.51956
PF Slope Co	efficient (m)	-1.16417		PF Power Coefficient (p)		0.83135
In Passing L	ane Effective Length?	No		Total Segment Density, veh/mi/ln		0.5
%Improvem	ent to Percent Followers	0.0		%Improvement to Speed		0.0
Subsegn	nent Data					
# Segm	ent Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tange	nt	5676	-		-	68.9
Vehicle F	Results					
Average Spe	eed, mi/h	68.9		Percent Followers,	, %	22.8
9 1		0.94		Follower Density (	FD), followers/mi/ln	0.5
Vehicle LOS A						
Bicycle R	Results					
Percent Occ	upied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h 164		Bicycle Effective W	Vidth, ft	29		
		1.28		Bicycle Effective S		5.07
bicycle LO3						+
Bicycle LOS		A				

Ve	ehicle Inputs					
Se	gment Type	Passing Constrained	d k	Length, ft		657
Me	easured FFS	Measured		Free-Flow Speed,	mi/h	70.0
D	emand and Capacity					
Dii	rectional Demand Flow Rate, veh/h	164		Opposing Deman	d Flow Rate, veh/h	-
Pe	ak Hour Factor	0.88		Total Trucks, %		2.82
Se	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
In	termediate Results					
Se	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Sp	eed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF	Slope Coefficient (m)	-1.29350		PF Power Coefficie	ent (p)	0.75785
ln	Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.7
%I	mprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Sı	ubsegment Data					
#	Segment Type	Length, ft	Rad	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	657	-		-	68.5
Ve	ehicle Results					
Av	erage Speed, mi/h	68.5		Percent Followers,	, %	28.0
Se	gment Travel Time, minutes	0.11		Follower Density (	FD), followers/mi/ln	0.7
Ve	hicle LOS	А				
Bi	icycle Results					
Pe	rcent Occupied Parking	0		Pavement Conditi	on Rating	4
Flo	ow Rate Outside Lane, veh/h	164		Bicycle Effective Width, ft		29
Bio	cycle LOS Score	1.28		Bicycle Effective Speed Factor		5.07
Bio	cycle LOS	А				
			Segn	nent 13		
Ve	ehicle Inputs					
Se	gment Type	Passing Zone		Length, ft		6009
М	easured FFS	Measured		Free-Flow Speed,	mi/h	70.0
D	emand and Capacity					
Dii	Pirectional Demand Flow Rate, veh/h 164		Opposing Deman	d Flow Rate, veh/h	280	
Pe	Peak Hour Factor 0.88		Total Trucks, %		2.82	
Segment Capacity, veh/h 1700		Demand/Capacity	(D/C)	0.10		
In	termediate Results					
Se	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Sp	eed Slope Coefficient (m)	4.40389		Speed Power Coe	fficient (p)	0.51956
PF	Slope Coefficient (m)	-1.16281		PF Power Coefficie	ent (p)	0.83065

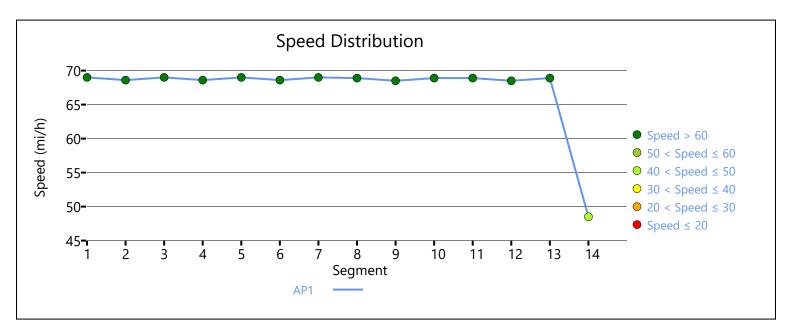
In Passing Lane Effective Length? No		Total Segment De	Total Segment Density, veh/mi/ln			
%lm	provement to Percent Followers	0.0		%Improvement to	o Speed	0.0
Sul	osegment Data					
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	6009	-		-	68.9
Vel	nicle Results					
Aver	age Speed, mi/h	68.9		Percent Followers	5, %	22.8
Segr	ment Travel Time, minutes	0.99		Follower Density	(FD), followers/mi/ln	0.5
Vehi	cle LOS	А				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Condit	ion Rating	4
Flow	Rate Outside Lane, veh/h	164		Bicycle Effective \	Width, ft	29
Bicy	cle LOS Score	1.28		Bicycle Effective S	Speed Factor	5.07
Bicy	cle LOS	А				
			Segr	ment 14		
Vel	nicle Inputs					
Segr	ment Type	Passing Constrain	ed	Length, ft		891
Mea	sured FFS	Measured		Free-Flow Speed,	Free-Flow Speed, mi/h	
De	mand and Capacity					
Dire	ctional Demand Flow Rate, veh/h	164		Opposing Demar	Opposing Demand Flow Rate, veh/h	
Peak	Hour Factor	0.88		Total Trucks, %	Total Trucks, %	
Segr	nent Capacity, veh/h	1700		Demand/Capacity (D/C)		0.10
Int	ermediate Results					
Segr	ment Vertical Class	1		Free-Flow Speed, mi/h		50.0
Spe	ed Slope Coefficient (m)	4.57372		Speed Power Coefficient (p)		0.41674
PF S	lope Coefficient (m)	-1.47375		PF Power Coeffici	PF Power Coefficient (p)	
In Pa	assing Lane Effective Length?	No		Total Segment De	ensity, veh/mi/ln	1.1
%lm	provement to Percent Followers	0.0		%Improvement to	%Improvement to Speed	
Sul	osegment Data					
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	891	-		-	48.5
Vel	nicle Results					
Aver	age Speed, mi/h	48.5		Percent Followers	5, %	33.4
Segr	ment Travel Time, minutes	0.21		Follower Density	(FD), followers/mi/ln	1.1
Vehi	cle LOS	Α				
Bic	ycle Results					•
Perc	ent Occupied Parking	0		Pavement Condit	ion Rating	4
		1				

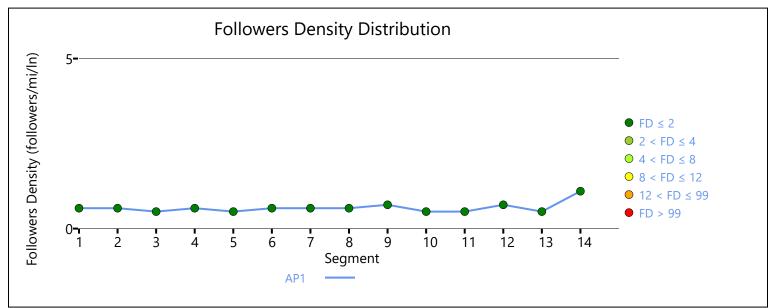
Flow Rate Outside Lane, veh/h	164	Bicycle Effective Width, ft	29
Bicycle LOS Score	1.06	Bicycle Effective Speed Factor	4.42
Bicycle LOS	А		
Facility Results			

Facility	Facility Results										
Т	VMT veh-mi/AP	VHD veh-h/p	Follower Density, followers/ mi/ln	LOS							

0.6

0.05





216

Α

		HCS Two-L	ane	Highway Re	port	
Project Inform	ation					
Analyst		MJV		Date		5/2/2024
Agency		HRG		Analysis Year		2050 Build Option 1
Jurisdiction		SDDOT		Time Analyzed		AM Peak
Project Description		WB 38 West of Hartford		Units		U.S. Customary
			Segn	nent 1		
Vehicle Inputs						
Segment Type Passing Zo		Passing Zone		Length, ft		10549
Measured FFS Measured		Measured	-		mi/h	70.0
Demand and C	Capacity			'		<u>'</u>
Directional Demand Flow Rate, veh/h		165		Opposing Demand Flow Rate, veh/h		244
Peak Hour Factor		0.88		Total Trucks, %		12.50
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.10
Intermediate F	Results	<u>'</u>		<u>'</u>		
Segment Vertical Cla	ass	1		Free-Flow Speed, mi/h		70.0
Speed Slope Coefficient (m)		4.42827		Speed Power Coefficient (p)		0.52768
PF Slope Coefficient (m)		-1.16689		PF Power Coefficient (p)		0.80729
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.6
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0
Subsegment D	ata	•				
# Segment Type		Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent		10549	-		-	69.0
Vehicle Results	5	•				
Average Speed, mi/h		69.0		Percent Followers, %		23.8
Segment Travel Time, minutes		1.74		Follower Density (FD), followers/mi/ln		0.6
Vehicle LOS		А				
Bicycle Results	<u> </u>	<u>'</u>		<u>'</u>		
Percent Occupied Parking		0		Pavement Condition Rating		4
low Rate Outside Lane, veh/h 165		Bicycle Effective Width, ft		29		
Bicycle LOS Score		4.94		Bicycle Effective Speed Factor		5.07
Bicycle LOS		E				
		·	Segn	nent 2		
Vehicle Inputs						
Segment Type Passing Zone		Length, ft		2793		
		Measured		Free-Flow Speed, mi/h		1

Demand and Capacity					
Directional Demand Flow Rate, veh/h	165		Opposing Demand Flow Rate, veh/h		244
Peak Hour Factor	0.88		Total Trucks, %		12.50
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.10
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed, mi/h		70.0
Speed Slope Coefficient (m)	4.35767		Speed Power Coefficient (p)		0.52768
PF Slope Coefficient (m)	-1.19319		PF Power Coefficient (p)		0.82737
In Passing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.6
%Improvement to Percent Followers	0.0		%Improvement to Speed		0.0
Subsegment Data					
# Segment Type	Length, ft Rad		dius, ft Superelevation, %		Average Speed, mi/h
1 Tangent	2793 -		-		69.0
Vehicle Results					
Average Speed, mi/h	69.0		Percent Followers, %		23.5
Segment Travel Time, minutes	0.46		Follower Density (FD), followers/mi/ln		0.6
Vehicle LOS	phicle LOS A				
Bicycle Results					·
Percent Occupied Parking	0		Pavement Condition Rating		4
Flow Rate Outside Lane, veh/h	165		Bicycle Effective Width, ft		29
Bicycle LOS Score 4.94		Bicycle Effective Speed Factor		5.07	
Bicycle LOS	E				
		Segr	ment 3		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		3825
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h 165		Opposing Demand Flow Rate, veh/h		245	
Peak Hour Factor	0.88		Total Trucks, %		2.40
egment Capacity, veh/h 1700		Demand/Capacity (D/C)		0.10	
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed, mi/h		70.0
Speed Slope Coefficient (m)	4.37079		Speed Power Coefficient (p)		0.52741
PF Slope Coefficient (m)	-1.17529		PF Power Coefficient (p)		0.83222
In Passing Lane Effective Length?	ng Lane Effective Length?		Total Segment Density, veh/mi/ln		0.6
6Improvement to Percent Followers 0.0		%Improvement to Speed		0.0	
	-		-		•

#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	3825	-		-	69.0
Vel	nicle Results		•			
Aver	rage Speed, mi/h	69.0		Percent Follower	s, %	23.1
Segr	ment Travel Time, minutes	0.63		Follower Density	(FD), followers/mi/ln	0.6
Vehi	cle LOS	A				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Condi	tion Rating	4
Flow	Rate Outside Lane, veh/h	165		Bicycle Effective	Width, ft	29
Bicy	cle LOS Score	1.17		Bicycle Effective	Speed Factor	5.07
Bicy	cle LOS	А				
			Seg	ment 4		·
Vel	nicle Inputs					
Segr	ment Type	Passing Constrai	ned	Length, ft		791
Mea	sured FFS	Measured		Free-Flow Speed	, mi/h	70.0
De	mand and Capacity	•		•		
Dire	ctional Demand Flow Rate, veh/h	165		Opposing Dema	nd Flow Rate, veh/h	-
Peak	Hour Factor	0.88		Total Trucks, %		2.40
Segr	ment Capacity, veh/h	1700		Demand/Capacit	y (D/C)	0.10
Int	ermediate Results					·
Segr	ment Vertical Class	1	Free-Flow Spee		, mi/h	70.0
Spe	ed Slope Coefficient (m)	4.57372		Speed Power Co	efficient (p)	0.41674
PF S	lope Coefficient (m)	-1.29355		PF Power Coefficient (p)		0.75779
In Pa	assing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.7
%lm	provement to Percent Followers	0.0		%Improvement to Speed		0.0
Sul	osegment Data					
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	791	-		-	68.5
Vel	nicle Results		•			
Aver	rage Speed, mi/h	68.5		Percent Follower	s, %	28.1
Segment Travel Time, minutes 0.13			Follower Density (FD), followers/mi/ln		0.7	
Vehicle LOS A			7, 7,			
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Condi	tion Rating	4
Flow	Rate Outside Lane, veh/h	165		Bicycle Effective	Width, ft	29
Bicy	cle LOS Score	1.17		Bicycle Effective	Speed Factor	5.07
Bicv	cle LOS	Α				

	Se	egn	nent 5		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		3414
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	165		Opposing Deman	d Flow Rate, veh/h	245
Peak Hour Factor	0.88		Total Trucks, %		2.40
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.36595		Speed Power Coef	fficient (p)	0.52741
PF Slope Coefficient (m)	-1.18179		PF Power Coefficie	ent (p)	0.83026
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	3414	-		-	69.0
Vehicle Results					
Average Speed, mi/h	69.0		Percent Followers,	. %	23.2
Segment Travel Time, minutes	0.56		Follower Density (FD), followers/mi/ln		0.6
Vehicle LOS	А				
Bicycle Results					·
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	165		Bicycle Effective Width, ft		29
Bicycle LOS Score	1.17		Bicycle Effective Speed Factor		5.07
Bicycle LOS	А				
	Se	egn	nent 6		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		286
Measured FFS	Measured	-		mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	165		Opposing Demand	d Flow Rate, veh/h	-
Peak Hour Factor	0.88		Total Trucks, %		2.40
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
			<u> </u>		

Speed Slope Coefficient (m)	4.57372		Speed Power Co	efficient (n)	0.41674	
PF Slope Coefficient (m)			PF Power Coefficient (p)		0.75779	
In Passing Lane Effective Length?				vensity, veh/mi/ln	0.7	
%Improvement to Percent Followers	0.0		%Improvement	-	0.0	
Subsegment Data			<u> </u>	'		
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h	
1 Tangent	286	286 -		-	68.5	
Vehicle Results						
Average Speed, mi/h	68.5		Percent Follower		28.1	
Segment Travel Time, minutes	0.05			(FD), followers/mi/ln	0.7	
Vehicle LOS	A			(		
Bicycle Results						
Percent Occupied Parking	0		Pavement Condi	tion Rating	4	
Flow Rate Outside Lane, veh/h	165		Bicycle Effective		29	
Bicycle LOS Score	1.17		Bicycle Effective		5.07	
Bicycle LOS	Α		bicycle Lifective	Speed ractor	3.07	
bicycle 203		Soan	cont 7			
W.1.1.1.		Jegn	nent 7			
Vehicle Inputs						
Segment Type	Passing Constra	ined	Length, ft		463	
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0	
Demand and Capacity						
Directional Demand Flow Rate, veh/h	169		Opposing Demand Flow Rate, veh/h		-	
Peak Hour Factor	0.88		Total Trucks, %		2.60	
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.10	
Intermediate Results						
Segment Vertical Class	1		Free-Flow Speed, mi/h		70.0	
Speed Slope Coefficient (m)	4.57372		Speed Power Co	efficient (p)	0.41674	
PF Slope Coefficient (m)	-1.29353		PF Power Coeffic	cient (p)	0.75782	
In Passing Lane Effective Length?	No		Total Segment D	ensity, veh/mi/ln	0.7	
%Improvement to Percent Followers	0.0		%Improvement to Speed		0.0	
Subsegment Data			·			
# Segment Type	Length, ft	Length, ft Rad		Superelevation, %	Average Speed, mi/h	
1 Tangent	463	-		-	68.5	
Vehicle Results						
Average Speed, mi/h	68.5		Percent Follower	rs, %	28.6	
Segment Travel Time, minutes	0.08			(FD), followers/mi/ln	0.7	
Vehicle LOS A		,	<u> </u>			

Bicycle Results					
Percent Occupied Parking	0	Pavemer	nt Condition	on Rating	4
Flow Rate Outside Lane, veh/h	169	Bicycle E	Bicycle Effective Width, ft		29
Bicycle LOS Score	1.23	Bicycle E	Bicycle Effective Speed Factor		5.07
Bicycle LOS	А				
	Se	egment 8			·
Vehicle Inputs					
Segment Type	Passing Zone	Length, 1	ft		4822
Measured FFS			w Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	169	Opposin	ng Deman	d Flow Rate, veh/h	243
Peak Hour Factor	0.88	Total Tru	ıcks, %		2.60
Segment Capacity, veh/h	1700	Demand	I/Capacity	(D/C)	0.10
Intermediate Results		·			
Segment Vertical Class	1	Free-Flo	Free-Flow Speed, mi/h		70.0
Speed Slope Coefficient (m)	4.38079	Speed Po	Speed Power Coefficient (p)		0.52796
PF Slope Coefficient (m)	-1.16377	PF Powe	r Coefficie	ent (p)	0.83451
In Passing Lane Effective Length?	No	Total Seg	gment De	nsity, veh/mi/ln	0.6
%Improvement to Percent Followers	0.0	%Improv	vement to	Speed	0.0
Subsegment Data		•			
# Segment Type	Length, ft	Radius, ft		Superelevation, %	Average Speed, mi/h
1 Tangent	4822	-		-	68.9
Vehicle Results					
Average Speed, mi/h	68.9	Percent I	Followers,	%	23.2
Segment Travel Time, minutes	0.79	Follower	Follower Density (FD), followers/mi/ln		0.6
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0	Pavemer	nt Conditio	on Rating	4
Flow Rate Outside Lane, veh/h	169	Bicycle E	Effective W	/idth, ft	29
Bicycle LOS Score	1.23	Bicycle E	ffective S <sub>I</sub>	peed Factor	5.07
Bicycle LOS	А				
	Se	egment 9			
Vehicle Inputs					
Segment Type	Passing Constrained	Length, 1	ft		861
Measured FFS	Measured		w Speed,	mi/h	70.0
Demand and Capacity					

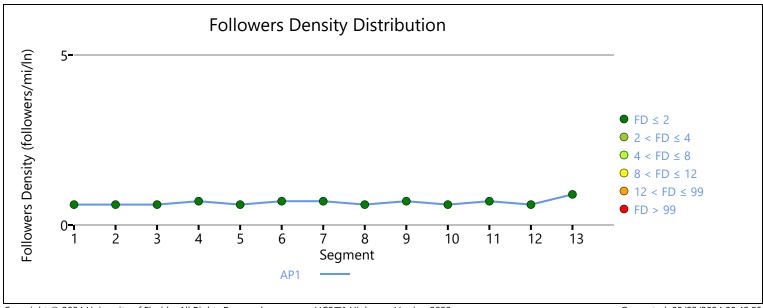
Discretional Development St. 19	169		Opposite	d Class Data and di	
Directional Demand Flow Rate, veh/h				d Flow Rate, veh/h	2.00
Peak Hour Factor	0.88		Total Trucks, %	(D(C)	2.60
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coef	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29353		PF Power Coefficie	ent (p)	0.75782
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.7
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft Radio		adius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	861	-		-	68.5
Vehicle Results					
Average Speed, mi/h	68.5		Percent Followers,	%	28.6
Segment Travel Time, minutes	0.14		Follower Density (	FD), followers/mi/ln	0.7
Vehicle LOS	A				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	169		Bicycle Effective W	/idth, ft	29
Bicycle LOS Score	1.23		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	А				
		Segn	nent 10		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		1556
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity					•
Directional Demand Flow Rate, veh/h	169		Opposing Demand	d Flow Rate, veh/h	243
Peak Hour Factor	0.88		Total Trucks, %		2.60
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Intermediate Results					•
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.33831		Speed Power Coef		0.52796
PF Slope Coefficient (m)	-1.23554		PF Power Coefficie	·	0.80871
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%Improvement to Percent Followers	0.0		%Improvement to		0.0
Subsegment Data					
# Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
9		1.0		1	

1	Tangent	1556	_		_	68.9
		1550	-			00.5
Vehi	cle Results					
Avera	ge Speed, mi/h	68.9		Percent Followers,	. %	25.5
Segm	ent Travel Time, minutes	0.26			FD), followers/mi/ln	0.6
Vehicl	e LOS	A	A			
Bicy	cle Results					
Percer	nt Occupied Parking	0	0 [		on Rating	4
Flow F	Rate Outside Lane, veh/h	169		Bicycle Effective W	/idth, ft	29
Bicycle	e LOS Score	1.23		Bicycle Effective S	peed Factor	5.07
Bicycle	e LOS	А				
			Segn	nent 11		
Vehi	cle Inputs					
Segm	ent Type	Passing Constrain	ned	Length, ft		799
Meası	ired FFS	Measured		Free-Flow Speed,	mi/h	70.0
Dem	and and Capacity	<u>'</u>				
Direct	ional Demand Flow Rate, veh/h	169		Opposing Deman	d Flow Rate, veh/h	-
Peak H	Hour Factor	0.88		Total Trucks, %		2.60
Segm	ent Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Inte	rmediate Results					
Segm	ent Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed	Slope Coefficient (m)	4.57372		Speed Power Coefficient (p)		0.41674
PF Slo	pe Coefficient (m)	-1.29353		PF Power Coefficient (p)		0.75782
In Pas	sing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.7
%lmp	rovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subs	segment Data	·				·
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	799	-		-	68.5
Vehi	cle Results	<u> </u>				
Avera	ge Speed, mi/h	68.5		Percent Followers,	. %	28.6
	ent Travel Time, minutes	0.13			FD), followers/mi/ln	0.7
Vehicle LOS A		2.7, (.2,7,				
Bicy	cle Results					
Percer	nt Occupied Parking	0		Pavement Conditi	on Rating	4
Flow F	Rate Outside Lane, veh/h	169		Bicycle Effective W	/idth, ft	29
Bicycle	e LOS Score	1.23		Bicycle Effective S	peed Factor	5.07
		Α				
Bicycle	e LOS	^				

Ve	ehicle Inputs					
Se	gment Type	Passing Zone		Length, ft		857
Ме	easured FFS	Measured		Free-Flow Speed, mi/h		70.0
D	emand and Capacity	·				
Dir	rectional Demand Flow Rate, veh/h	169		Opposing Deman	d Flow Rate, veh/h	243
Pe	ak Hour Factor	0.88		Total Trucks, %		2.60
Se	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
In	termediate Results					
Se	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Sp	eed Slope Coefficient (m)	4.33390		Speed Power Coe	fficient (p)	0.52796
PF	Slope Coefficient (m)	-1.24754		PF Power Coefficie	ent (p)	0.80350
ln	Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%	mprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Sı	ıbsegment Data					
#	Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	857	1-		-	68.9
Ve	ehicle Results					
Av	erage Speed, mi/h	68.9		Percent Followers,	. %	25.9
Segment Travel Time, minutes		0.14		Follower Density (	FD), followers/mi/ln	0.6
Ve	hicle LOS	А	А			
Bi	cycle Results					
Pe	rcent Occupied Parking	0		Pavement Conditi	on Rating	4
Flo	ow Rate Outside Lane, veh/h	169		Bicycle Effective Width, ft		29
Bio	cycle LOS Score	1.23		Bicycle Effective Speed Factor		5.07
Bio	cycle LOS	A				
		S	egm	ent 13		
Ve	ehicle Inputs					
Se	gment Type	Passing Constrained		Length, ft		1288
Ме	easured FFS	Measured		Free-Flow Speed,	mi/h	60.0
D	emand and Capacity					
Directional Demand Flow Rate, veh/h 169		Opposing Deman	d Flow Rate, veh/h	-		
Peak Hour Factor 0.88		Total Trucks, %		2.60		
Se	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
In	termediate Results					
Se	gment Vertical Class	1		Free-Flow Speed,	mi/h	60.0
Sp	eed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF	Slope Coefficient (m)	-1.39677		PF Power Coefficie	ent (p)	0.73640

In Pas	n Passing Lane Effective Length?			Total Segment De	nsity, veh/mi/ln	0.9
%lmp	rovement to Percent Followers	ent to Percent Followers 0.0 %Improvement to Speed			0.0	
Sub	segment Data					
#	Segment Type	Length, ft	Length, ft Radio		Superelevation, %	Average Speed, mi/h
1	Tangent	1288	-		-	58.5
Vehi	cle Results					
Avera	ge Speed, mi/h	58.5		Percent Followers, %		31.5
Segm	ent Travel Time, minutes	0.25	0.25		FD), followers/mi/ln	0.9
Vehicl	e LOS	А				
Bicy	cle Results					
Percer	nt Occupied Parking	0	0		on Rating	4
Flow F	Rate Outside Lane, veh/h	169	169		Vidth, ft	29
Bicycl	e LOS Score	1.14		Bicycle Effective S	peed Factor	4.79
Bicycl	e LOS	А				
Facil	lity Results					
т	VMT veh-mi/AP	VHD veh-h			ensity, followers/ mi/ln	LOS
1	224	0.05	0.05		0.6	A





Copyright © 2024 University of Florida. All Rights Reserved.

HCSTM Highways Version 2023 WB\_38\_WHartford\_2050AM.xuf Generated: 05/02/2024 09:42:53

		HCS Two-La	ane	Highway Re	port	
Pro	ject Information					
Anal	yst	MJV		Date		5/2/2024
Age	ncy	HRG		Analysis Year		2050 Build Option 1
Juris	diction	SDDOT		Time Analyzed		PM Peak
Proje	ect Description	WB 38 West of Hartfo	ord	Units		U.S. Customary
		S	Segn	nent 1		·
Vel	nicle Inputs					
Segr	ment Type	Passing Zone		Length, ft		10549
Mea	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity					
Dire	ctional Demand Flow Rate, veh/h	280		Opposing Deman	d Flow Rate, veh/h	164
Peak	Hour Factor	0.88		Total Trucks, %		1.94
Segr	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.16
Int	ermediate Results			•		•
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	ed Slope Coefficient (m)	4.39885		Speed Power Coe	fficient (p)	0.55020
PF Slope Coefficient (m) -1.15143		PF Power Coefficie	ent (p)	0.81244		
In Pa	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.4
%lm	provement to Percent Followers	0.0		%Improvement to	Speed	0.0
Sul	osegment Data	·				
#	Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	10549	-		-	68.3
Vel	nicle Results					
Aver	age Speed, mi/h	68.3		Percent Followers	, %	33.6
Segr	ment Travel Time, minutes	1.76		Follower Density (FD), followers/mi/ln		1.4
Vehi	cle LOS	А				
Bic	ycle Results			•		•
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h 280		Bicycle Effective Width, ft		24		
Bicy	cle LOS Score	2.64		Bicycle Effective S		5.07
Bicy	cle LOS	С				
		S	Segn	nent 2		
Vel	nicle Inputs					
	ment Type	Passing Zone		Length, ft		2793
	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0

Demand and Capacity					
Directional Demand Flow Rate, veh/h	280		Opposing Demand Flow Rate, veh/h		164
Peak Hour Factor	0.88		Total Trucks, %		1.94
Segment Capacity, veh/h	1700		Demand/Capacity	/ (D/C)	0.16
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.32824		Speed Power Coe	fficient (p)	0.55020
PF Slope Coefficient (m)	-1.17723		PF Power Coeffici	ent (p)	0.83227
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.4
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rad	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	2793	-		-	68.3
Vehicle Results					
Average Speed, mi/h	68.3		Percent Followers	, %	33.5
Segment Travel Time, minutes	0.46		Follower Density	(FD), followers/mi/ln	1.4
Vehicle LOS	А				
Bicycle Results	•		•		
Percent Occupied Parking	0		Pavement Condit	ion Rating	4
Flow Rate Outside Lane, veh/h	280		Bicycle Effective V	Vidth, ft	24
Bicycle LOS Score	2.64		Bicycle Effective S	Bicycle Effective Speed Factor	
Bicycle LOS	С				
		Segr	ment 3		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		3825
Measured FFS	Measured		Free-Flow Speed,	Free-Flow Speed, mi/h	
Demand and Capacity					
Directional Demand Flow Rate, veh/h	289		Opposing Demand Flow Rate, veh/h		164
Peak Hour Factor	0.88		Total Trucks, %	Total Trucks, %	
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.17
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.34098		Speed Power Coe	fficient (p)	0.55020
PF Slope Coefficient (m)	-1.15833		PF Power Coeffici	ent (p)	0.83897
In Passing Lane Effective Length?	No		Total Segment De	ensity, veh/mi/ln	1.4
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0

#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	3825	-		-	68.3
Vel	nicle Results		•			
Aver	rage Speed, mi/h	68.3		Percent Follower	rs, %	33.5
Segr	ment Travel Time, minutes	0.64		Follower Density	(FD), followers/mi/ln	1.4
Vehi	cle LOS	A				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Condi	tion Rating	4
Flow	Rate Outside Lane, veh/h	289		Bicycle Effective	Width, ft	24
Bicy	cle LOS Score	2.72		Bicycle Effective	Speed Factor	5.07
Bicy	cle LOS	С				
			Seg	ment 4		
Vel	nicle Inputs					
Segr	ment Type	Passing Constrai	ned	Length, ft		791
Mea	sured FFS	Measured		Free-Flow Speed	l, mi/h	70.0
De	mand and Capacity					•
Dire	ctional Demand Flow Rate, veh/h	289		Opposing Dema	nd Flow Rate, veh/h	-
Peak	Hour Factor	0.88		Total Trucks, %		2.19
Segr	ment Capacity, veh/h	1700		Demand/Capaci	ty (D/C)	0.17
Int	ermediate Results					
Segr	ment Vertical Class	1	1 Fre		I, mi/h	70.0
Spe	ed Slope Coefficient (m)	4.57372		Speed Power Co	efficient (p)	0.41674
PF S	lope Coefficient (m)	-1.29358		PF Power Coefficient (p)		0.75776
In Pa	assing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		1.7
%lm	provement to Percent Followers	0.0		%Improvement to Speed		0.0
Sul	osegment Data					
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	791	-		-	67.7
Vel	nicle Results					
Aver	rage Speed, mi/h	67.7		Percent Follower	rs, %	39.6
Segment Travel Time, minutes 0.13			Follower Density	(FD), followers/mi/ln	1.7	
Vehicle LOS A			,,,,,			
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Condi	tion Rating	4
Flow	Rate Outside Lane, veh/h	289		Bicycle Effective	Width, ft	24
Bicy	cle LOS Score	2.72		Bicycle Effective	Speed Factor	5.07
Ricv	cle LOS	С				

	Se	egn	nent 5		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		3414
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					·
Directional Demand Flow Rate, veh/h	289		Opposing Deman	d Flow Rate, veh/h	164
Peak Hour Factor	0.88		Total Trucks, %		2.19
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.33614		Speed Power Coef	fficient (p)	0.55020
PF Slope Coefficient (m)	-1.16472		PF Power Coefficie	ent (p)	0.83695
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.4
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	3414	1-		-	68.3
Vehicle Results					
Average Speed, mi/h	68.3		Percent Followers,	. %	33.7
Segment Travel Time, minutes	0.57		Follower Density (FD), followers/mi/ln		1.4
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	289		Bicycle Effective Width, ft		24
Bicycle LOS Score	2.72		Bicycle Effective Speed Factor		5.07
Bicycle LOS	С				
	S	egn	nent 6		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		286
Measured FFS	Measured	-		mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	289		Opposing Demand	d Flow Rate, veh/h	T-
Peak Hour Factor	0.88		Total Trucks, %		2.19
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
	1		1 2		1

Speed Slope Coefficient (m)	4.57372		Speed Power Co	efficient (n)	0.41674					
PF Slope Coefficient (m)	-1.29358	PF Power Coeffic	•	0.75776						
In Passing Lane Effective Length?	No			ensity, veh/mi/ln	1.7					
%Improvement to Percent Followers	0.0		%Improvement t		0.0					
Subsegment Data	1-1-									
# Segment Type	Length, ft	Rac	 dius, ft	Superelevation, %	Average Speed, mi/h					
1 Tangent	286	-		67.7						
Vehicle Results										
Average Speed, mi/h	67.7		Percent Follower	 s. %	39.6					
Segment Travel Time, minutes	0.05			(FD), followers/mi/ln	1.7					
Vehicle LOS	A		Tollower Belisity	(1 D), Tollowers, This in	1.7					
Bicycle Results	^									
•	T.		l		Ι.					
Percent Occupied Parking	289		Pavement Condi		4					
Flow Rate Outside Lane, veh/h		Bicycle Effective		24						
Bicycle LOS Score		Bicycle Effective	Speed Factor	5.07						
Bicycle LOS										
		Segn	nent 7							
Vehicle Inputs										
Segment Type	Passing Constra	ined	Length, ft		463					
Measured FFS	Measured		Free-Flow Speed	, mi/h	70.0					
Demand and Capacity	•									
Directional Demand Flow Rate, veh/h	286		Opposing Dema	nd Flow Rate, veh/h	-					
Peak Hour Factor	0.88		Total Trucks, %		3.08					
Segment Capacity, veh/h	1700		Demand/Capacit	ry (D/C)	0.17					
Intermediate Results			<u>'</u>							
Segment Vertical Class	1		Free-Flow Speed	, mi/h	70.0					
Speed Slope Coefficient (m)	4.57372		Speed Power Co	efficient (p)	0.41674					
PF Slope Coefficient (m)	-1.29347		PF Power Coeffic	ient (p)	0.75789					
In Passing Lane Effective Length?	No		Total Segment D	ensity, veh/mi/ln	1.7					
%Improvement to Percent Followers	0.0		%Improvement t	o Speed	0.0					
Subsegment Data		,		<u>'</u>						
# Segment Type	Length, ft Rad		dius, ft	Superelevation, %	Average Speed, mi/h					
Tangent 463 -		-		-	67.7					
Vehicle Results										
verage Speed, mi/h 67.7			Percent Follower	s, %	39.4					
Segment Travel Time, minutes	0.08		Follower Density	(FD), followers/mi/ln	1.7					
Vehicle LOS										

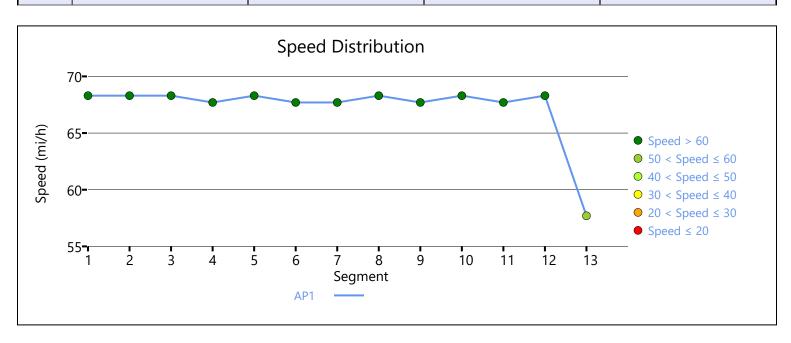
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	286		Bicycle Effective W	/idth, ft	24
Bicycle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	С				
	S	egn	nent 8		·
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		4822
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity	<u>'</u>				
Directional Demand Flow Rate, veh/h	286		Opposing Demand	d Flow Rate, veh/h	157
Peak Hour Factor	0.88		Total Trucks, %		3.08
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.34895		Speed Power Coe	fficient (p)	0.55243
PF Slope Coefficient (m)	-1.14563		PF Power Coefficie	ent (p)	0.84199
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.4
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					·
# Segment Type	Length, ft	Rad	lius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	4822	1-		-	68.3
Vehicle Results	·				
Average Speed, mi/h	68.3		Percent Followers,	. %	33.0
Segment Travel Time, minutes	0.80		Follower Density (	FD), followers/mi/ln	1.4
Vehicle LOS	А				
Bicycle Results	·				•
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	286		Bicycle Effective W	/idth, ft	24
Bicycle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	С				
	S	egn	nent 9		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		861
Measured FFS	Measured		Free-Flow Speed,	70.0	
Demand and Capacity					

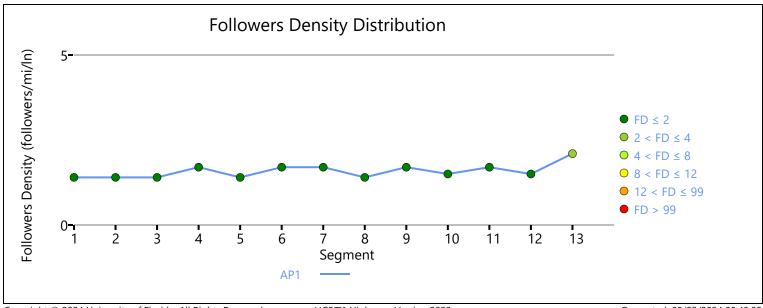
Directional Demand Flow Rate, veh/h   286   Opposing Demand Flow Rate, veh/h   -			
Segnest			
Segment   Vertical Class   1			
Prec			
Speed Slope Coefficient (m)         4.57372         Speed Power Coefficient (p)         0.41674           PF Slope Coefficient (m)         -1.29347         PF Power Coefficient (p)         0.75789           In Pasing Lane Effective Length?         No         Total Segment Density, velv/mi/ln         1.7           Subsequent Deta           # Segment Type         Length, ft         Radius, ft         Superelevation, %         Average Speed           1 alongent         B61         -         -         67.7           Verification Results           Note that the properties of the pr			
PF Slope Coefficient (m)         -1.29347         PF Power Coefficient (p)         0.75789           In Passing Lane Effective Length?         No         Total Segment Density, veh/mi/ln         1.7           %lmprovement to Percent Followers         0.0         %Improvement to Speed         0.0           Support Data           #         Segment Type         Length, ft         Radius, ft         Superelevation, %         Average Speed           1         Tangent         861         -         -         67.7           Vehicle Results           Average Speed, mi/h         67.7         Percent Followers, %         39.4           Segment Travel Time, minutes         0.14         Follower Density (FD), followers/mi/ln         1.7           Vehicle LoS           Biocycle Results           Percent Occupied Parking         0         Pavement Condition Rating         4           8iocycle Effective Width, ft         24         24           8iocycle Effective Width, ft         24         24 <td></td>			
In Passing Lane Effective Length?   No   Total Segment Density, veh/mi/ln   1.7			
Subsequent Data         Mymprovement to Speed         0.0           Subsequent Data           #         Segment Type         Length, ft         Radius, ft         Superelevation, %         Average Speed           1         Tangent         861         -         67.7         67.7         67.7         Center Followers, %         39.4			
Subsegment Data           #         Segment Type         Length, ft         Radius, ft         Superelevation, %         Average Speed           1         Iangent         861          6.7.7           Vehicle Results           Average Speed, mi/h         67.7         Percent Followers, %         39.4           Segment Travel Time, minutes         0.14         Follower Density (FD), followers/mi/ln         1.7           Nevicle LOS         A         Pavement Condition Rating         1.7           Bicycle LOS         Bicycle Effective Width, ft         24           Bicycle LOS Score         2.95         Bicycle Effective Width, ft         24           Bicycle LOS Score         2.95         Bicycle Effective Width, ft         24           Segment 10           Vehicle Inputs           Segment Type         Passing Zone         Length, ft         1556           Measured FFS         Measured         Free-Flow Speed, mi/h         70.0           Directional Demand And Capacity           Directional Demand Flow Rate, veh/h         286         Opposing Demand Flow Rate, veh/h         157           Peak Hour Factor         0.88			
##   Segment Type   Length, ft   Radius, ft   Superelevation, %   Average Speed   1   Tangent   861   -			
Tangent   Results   Res			
Vehicle Results       Average Speed, mi/h     67.7     Percent Followers, %     39.4       Segment Travel Time, minutes     0.14     Follower Density (FD), followers/mi/ln     1.7       Vehicle LOS       Bicycle Results       Percent Occupied Parking     0     Pavement Condition Rating     4       Flow Rate Outside Lane, veh/h     286     Bicycle Effective Width, ft     24       Bicycle LOS Score     2.95     Bicycle Effective Speed Factor     5.07       Bicycle LOS       Cegment 10       Vehicle Inputs       Segment Type     Passing Zone     Length, ft     1556       Measured FFS     Measured     Free-Flow Speed, mi/h     70.0       Demand and Capacity       Directional Demand Flow Rate, veh/h     286     Opposing Demand Flow Rate, veh/h     157       Peak Hour Factor     0.88     Total Trucks, %     3.08       Segment Capacity, veh/h     1700     Demand/Capacity (D/C)     0.17       Intermediate Results       Segment Vertical Class     1     Free-Flow Speed, mi/h     70.0       Speed Power Coefficient (n)     0.55243	, mi/h		
Average Speed, mi/h 67.7 Percent Followers, % 39.4 Segment Travel Time, minutes 0.14 Follower Density (FD), followers/mi/ln 1.7 Vehicle LOS A Pavement Condition Rating 4 Flow Rate Outside Lane, veh/h 286 Bicycle Effective Width, ft 24 Bicycle LOS Core 2.95 Bicycle Effective Speed Factor 5.07 Bicycle LOS C Esument 10  Vehicle Inputs  Segment Type Passing Zone Length, ft 1556 Measured FFS Measured Free-Flow Speed, mi/h 70.0  Demand and Capacity  Directional Demand Flow Rate, veh/h 286 Opposing Demand Flow Rate, veh/h 1700 Demand/Capacity (D/C) 0.17  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0 Speed Slope Coefficient (m) 4.30647 Speed Power Coefficient (p) 0.55243			
Segment Travel Time, minutes  O.14  Follower Density (FD), followers/mi/In  1.7  Vehicle LOS  A  Bicycle Results  Percent Occupied Parking  Percent			
Vehicle LOS     A     Passing Zone     Length, ft     1556       Measured FFS     Measured     Free-Flow Speed, mi/h     157       Demand Flow Rate, veh/h     286     Opposing Demand Flow Rate, veh/h     157       Bicycle LOS     C     Length, ft     1556       Demand and Capacity       Directional Demand Flow Rate, veh/h     286     Opposing Demand Flow Rate, veh/h     157       Peak Hour Factor     0.88     Total Trucks, %     3.08       Segment Capacity, veh/h     1700     Demand/Capacity (D/C)     0.17       Intermediate Results       Segment Vertical Class     1     Free-Flow Speed, mi/h     70.0       Speed Slope Coefficient (m)     4.30647     Speed Power Coefficient (p)     0.55243			
Bicycle Results  Percent Occupied Parking 0 Pavement Condition Rating 4 Flow Rate Outside Lane, veh/h 286 Bicycle Effective Width, ft 24 Bicycle LOS Score 2.95 Bicycle Effective Speed Factor 5.07 Bicycle LOS  Segment 10  Vehicle Inputs  Segment Type Passing Zone Length, ft 1556 Measured FFS Measured FFS Measured Free-Flow Speed, mi/h 70.0  Demand and Capacity  Directional Demand Flow Rate, veh/h 286 Opposing Demand Flow Rate, veh/h 1700 Demand/Capacity, veh/h 1700 Demand/Capacity (D/C) 0.17  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0 Speed Slope Coefficient (m) 4.30647 Speed Power Coefficient (p) 0.55243			
Percent Occupied Parking 0 Pavement Condition Rating 4 Flow Rate Outside Lane, veh/h 286 Bicycle Effective Width, ft 24 Bicycle LOS Score 2.95 Bicycle Effective Speed Factor 5.07 Bicycle LOS C Segment 10  Segment 10  Vehicle Inputs  Segment Type Passing Zone Length, ft 1556 Measured FFS Measured FFS Measured Free-Flow Speed, mi/h 70.0  Demand and Capacity  Directional Demand Flow Rate, veh/h 286 Opposing Demand Flow Rate, veh/h 157 Peak Hour Factor 0.88 Total Trucks, % 3.08 Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.17  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0 Speed Slope Coefficient (m) 4.30647 Speed Power Coefficient (p) 0.55243			
Flow Rate Outside Lane, veh/h Bicycle LOS Score 2.95 Bicycle Effective Speed Factor 5.07 Bicycle LOS  Segment 10  Vehicle Inputs  Segment Type Passing Zone Measured FFS Measured Free-Flow Speed, mi/h Demand and Capacity  Directional Demand Flow Rate, veh/h Peak Hour Factor 0.88 Segment Capacity, veh/h 1700 Demand-Capacity, veh/h Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h Free-Flow Speed, mi/h 70.0  Pree-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.30647 Free-Flow Speed, mi/h 70.0 Speed Power Coefficient (p) 0.55243			
Bicycle LOS Score 2.95 Bicycle Effective Speed Factor 5.07  Bicycle LOS C C Segment 10  Vehicle Inputs  Segment Type Passing Zone Length, ft 1556 Measured FFS Measured Free-Flow Speed, mi/h 70.0  Demand and Capacity  Directional Demand Flow Rate, veh/h 286 Opposing Demand Flow Rate, veh/h 157 Peak Hour Factor 0.88 Total Trucks, % 3.08 Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.17  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0 Speed Slope Coefficient (m) 4.30647 Speed Power Coefficient (p) 0.55243			
Bicycle LOS C Segment 10  Vehicle Inputs  Segment Type Passing Zone Length, ft 1556 Measured FFS Measured Free-Flow Speed, mi/h 70.0  Demand and Capacity  Directional Demand Flow Rate, veh/h 286 Opposing Demand Flow Rate, veh/h 157 Peak Hour Factor 0.88 Total Trucks, % 3.08 Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.17  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0 Speed Slope Coefficient (m) 4.30647 Speed Power Coefficient (p) 0.55243			
Segment 10  Vehicle Inputs  Segment Type Passing Zone Length, ft 1556 Measured FFS Measured Free-Flow Speed, mi/h 70.0  Demand and Capacity  Directional Demand Flow Rate, veh/h Peak Hour Factor 0.88 Total Trucks, % 3.08 Segment Capacity, veh/h 1700 Demand/Capacity (D/C)  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.30647 Speed Power Coefficient (p)  O556  Length, ft 1556 Dength, ft 1556  Opposing Demand Flow Rate, veh/h 157 Demand Flow Rate, veh/h 157 Demand Flow Rate, veh/h 157 Demand/Capacity (D/C) 0.17			
Vehicle InputsSegment TypePassing ZoneLength, ft1556Measured FFSMeasuredFree-Flow Speed, mi/h70.0Demand and CapacityDirectional Demand Flow Rate, veh/h286Opposing Demand Flow Rate, veh/h157Peak Hour Factor0.88Total Trucks, %3.08Segment Capacity, veh/h1700Demand/Capacity (D/C)0.17Intermediate ResultsSegment Vertical Class1Free-Flow Speed, mi/h70.0Speed Slope Coefficient (m)4.30647Speed Power Coefficient (p)0.55243			
Vehicle InputsSegment TypePassing ZoneLength, ft1556Measured FFSMeasuredFree-Flow Speed, mi/h70.0Demand and CapacityDirectional Demand Flow Rate, veh/h286Opposing Demand Flow Rate, veh/h157Peak Hour Factor0.88Total Trucks, %3.08Segment Capacity, veh/h1700Demand/Capacity (D/C)0.17Intermediate ResultsSegment Vertical Class1Free-Flow Speed, mi/h70.0Speed Slope Coefficient (m)4.30647Speed Power Coefficient (p)0.55243			
Segment Type Passing Zone Length, ft 1556  Measured FFS Measured Free-Flow Speed, mi/h 70.0  Demand and Capacity  Directional Demand Flow Rate, veh/h 286 Opposing Demand Flow Rate, veh/h 157  Peak Hour Factor 0.88 Total Trucks, % 3.08  Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.17  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.30647 Speed Power Coefficient (p) 0.55243			
Demand and CapacityDirectional Demand Flow Rate, veh/h286Opposing Demand Flow Rate, veh/h157Peak Hour Factor0.88Total Trucks, %3.08Segment Capacity, veh/h1700Demand/Capacity (D/C)0.17Intermediate ResultsSegment Vertical Class1Free-Flow Speed, mi/h70.0Speed Slope Coefficient (m)4.30647Speed Power Coefficient (p)0.55243			
Directional Demand Flow Rate, veh/h 286 Opposing Demand Flow Rate, veh/h 157  Peak Hour Factor 0.88 Total Trucks, % 3.08  Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.17  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.30647 Speed Power Coefficient (p) 0.55243			
Directional Demand Flow Rate, veh/h 286 Opposing Demand Flow Rate, veh/h 157  Peak Hour Factor 0.88 Total Trucks, % 3.08  Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.17  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.30647 Speed Power Coefficient (p) 0.55243			
Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.17  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.30647 Speed Power Coefficient (p) 0.55243			
Intermediate Results       Segment Vertical Class     1     Free-Flow Speed, mi/h     70.0       Speed Slope Coefficient (m)     4.30647     Speed Power Coefficient (p)     0.55243			
Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.30647 Speed Power Coefficient (p) 0.55243			
Speed Slope Coefficient (m)  4.30647  Speed Power Coefficient (p)  0.55243			
PF Slope Coefficient (m) -1.21611 PF Power Coefficient (p) 0.81541			
In Passing Lane Effective Length? No Total Segment Density, veh/mi/ln 1.5			
%Improvement to Percent Followers 0.0 %Improvement to Speed 0.0	0.0		
Subsegment Data			
# Segment Type Length, ft Radius, ft Superelevation, % Average Speed	, mi/h		

1 Tangant	1556			_	68.3		
1 Tangent	1330	-			00.3		
Vehicle Results							
Average Speed, mi/h	68.3		Percent Followers,	, %	35.5		
Segment Travel Time, minutes	0.26		Follower Density (	FD), followers/mi/ln	1.5		
Vehicle LOS	А						
Bicycle Results							
Percent Occupied Parking	0		Pavement Condition	on Rating	4		
Flow Rate Outside Lane, veh/h	286		Bicycle Effective W	Vidth, ft	24		
Bicycle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07		
Bicycle LOS	С						
		Segn	nent 11				
Vehicle Inputs							
Segment Type	Passing Constraine	d	Length, ft		799		
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0		
Demand and Capacity	<u>'</u>		•				
Directional Demand Flow Rate, veh/h	286		Opposing Demand	d Flow Rate, veh/h	-		
Peak Hour Factor	0.88		Total Trucks, %		3.08		
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17		
Intermediate Results							
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0		
Speed Slope Coefficient (m)	4.57372		Speed Power Coef	fficient (p)	0.41674		
PF Slope Coefficient (m)	-1.29347		PF Power Coefficie	ent (p)	0.75789		
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.7		
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0		
Subsegment Data							
# Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h		
1 Tangent	799	-		-	67.7		
Vehicle Results					•		
Average Speed, mi/h	67.7		Percent Followers,	, %	39.4		
Segment Travel Time, minutes	0.13		Follower Density (	FD), followers/mi/ln	1.7		
Vehicle LOS	А						
Bicycle Results							
Percent Occupied Parking	0		Pavement Condition	on Rating	4		
Flow Rate Outside Lane, veh/h	286		Bicycle Effective W	Vidth, ft	24		
Bicycle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07		
Bicycle LOS	С						
		Segn	nent 12				

Ve	ehicle Inputs										
Se	gment Type	Passing Zone		Length, ft		857					
Ме	easured FFS	Measured		Free-Flow Speed, mi/h 70.0							
D	emand and Capacity										
Dir	rectional Demand Flow Rate, veh/h	286		Opposing Deman	d Flow Rate, veh/h	157					
Pe	ak Hour Factor	0.88		Total Trucks, %		3.08					
Se	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17					
In	termediate Results										
Se	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0					
Sp	eed Slope Coefficient (m)	4.30206		Speed Power Coe	fficient (p)	0.55243					
PF	Slope Coefficient (m)	-1.22789		PF Power Coefficie	ent (p)	0.81007					
ln	Passing Lane Effective Length?		Total Segment De	nsity, veh/mi/ln	1.5						
%I	mprovement to Percent Followers	%Improvement to	Speed	0.0							
Sı	ıbsegment Data										
#	Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h					
1	Tangent	857 -			-	68.3					
Ve	ehicle Results					·					
Average Speed, mi/h 68.3				Percent Followers,	, %	36.0					
Segment Travel Time, minutes 0.14			Follower Density (	FD), followers/mi/ln	1.5						
Ve	hicle LOS	А									
Bi	cycle Results										
Pe	rcent Occupied Parking	0		Pavement Conditi	on Rating	4					
Flo	ow Rate Outside Lane, veh/h	286		Bicycle Effective W	vidth, ft	24					
Bic	cycle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07					
Bic	ycle LOS	С									
		S	egm	ent 13							
Ve	ehicle Inputs										
Se	gment Type	Passing Constrained		Length, ft		1288					
Ме	easured FFS	Measured		Free-Flow Speed,	mi/h	60.0					
D	emand and Capacity										
Dir	rectional Demand Flow Rate, veh/h	286		Opposing Deman	d Flow Rate, veh/h	-					
Pe	ak Hour Factor	0.88		Total Trucks, %		3.08					
Se	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17					
In	termediate Results										
Se	gment Vertical Class	1		Free-Flow Speed,	mi/h	60.0					
Sp	eed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674					
PF	Slope Coefficient (m)	-1.39671		PF Power Coefficie	ent (p)	0.73647					

In Pas	ssing Lane Effective Length?	No	То	tal Segment De	nsity, veh/mi/ln	2.1			
%lmp	provement to Percent Followers	0.0	%I	Improvement to	Speed	0.0			
Sub	segment Data								
#	Segment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h			
1	Tangent	1288	-		-	57.7			
Vehi	icle Results								
Avera	ge Speed, mi/h	57.7	Pe	rcent Followers,	%	42.7			
Segm	ent Travel Time, minutes	Fo	ollower Density (	FD), followers/mi/ln	2.1				
Vehic	le LOS	В							
Bicy	cle Results								
Perce	nt Occupied Parking	0	Pa	vement Condition	on Rating	4			
Flow I	Rate Outside Lane, veh/h	286	Bio	cycle Effective W	/idth, ft	24			
Bicycl	e LOS Score	2.86	Bio	cycle Effective S <sub>l</sub>	peed Factor	4.79			
Bicycl	e LOS	С							
Faci	lity Results								
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS			
1	382	0.15			1.5	А			



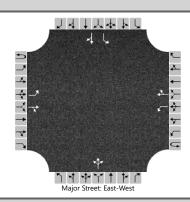


Copyright © 2024 University of Florida. All Rights Reserved.

HCSTM Highways Version 2023 WB\_38\_WHartford\_2050PM.xuf

Generated: 05/02/2024 09:43:39

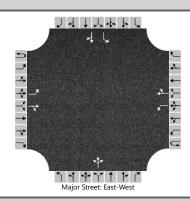
	HCS Two-Way Stop-Control Report											
General Information		Site Information										
Analyst	NM	Intersection	SD 38 & SD 19									
Agency/Co.	HRG	Jurisdiction	SDDOT									
Date Performed	4/30/2024	East/West Street	SD 38									
Analysis Year	2050	North/South Street	SD 19									
Time Analyzed	AM Peak	Peak Hour Factor	0.92									
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25									
Project Description	SD 38 Build Option 1											



Vehicle Volumes and Adju	ıstme	nts														
Approach	Eastbound Westbound						North	bound			South	bound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		1	1	0
Configuration	1	L		TR		L		TR			LTR			L		TR
Volume (veh/h)	1	55	165	0		0	120	50		10	5	10		70	0	95
Percent Heavy Vehicles (%)	1	30				3				3	3	3		9	3	11
Proportion Time Blocked	1															
Percent Grade (%)	1									(	)		0			
Right Turn Channelized	1															
Median Type   Storage	1			Undi	vided	ded										
Critical and Follow-up He	adwa	adways														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.40				4.13				7.13	6.53	6.23		7.19	6.53	6.31
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.47				2.23				3.53	4.03	3.33		3.58	4.03	3.40
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1	60				0					27			76		103
Capacity, c (veh/h)		1238				1390					524			461		865
v/c Ratio		0.05				0.00					0.05			0.16		0.12
95% Queue Length, Q <sub>95</sub> (veh)	1	0.2				0.0					0.2			0.6		0.4
Control Delay (s/veh)		8.1	0.2	0.2		7.6	0.0	0.0			12.2			14.3		9.7
Level of Service (LOS)	1	А	А	А		А	Α	А			В			В		А
Approach Delay (s/veh)		2	.2			0.0			12.2				11.7			
Approach LOS	1	,	Α				4				3				В	

Generated: 5/2/2024 9:52:26 AM

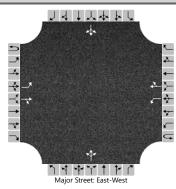
	HCS Two-Way Stop-Control Report											
General Information		Site Information										
Analyst	NM	Intersection	SD 38 & SD 19									
Agency/Co.	HRG	Jurisdiction	SDDOT									
Date Performed	4/30/2024	East/West Street	SD 38									
Analysis Year	2050	North/South Street	SD 19									
Time Analyzed	PM Peak	Peak Hour Factor	0.92									
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25									
Project Description	SD 38 Build Option 1											



Vehicle Volumes and Adju	ıstme	nts															
Approach	Eastbound					Westbound				North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		1	1	0	
Configuration		L		TR		L		TR			LTR			L		TR	
Volume (veh/h)		85	115	0		0	170	80		10	5	10		40	0	50	
Percent Heavy Vehicles (%)		2				3				3	3	3		10	3	14	
Proportion Time Blocked																	
Percent Grade (%)	1									(	)		0				
Right Turn Channelized																	
Median Type   Storage		Undivided															
Critical and Follow-up He	adwa	ys															
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2	
Critical Headway (sec)		4.12				4.13				7.13	6.53	6.23		7.20	6.53	6.34	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.22				2.23				3.53	4.03	3.33		3.59	4.03	3.43	
Delay, Queue Length, and	l Leve	l of Se	ervice														
Flow Rate, v (veh/h)	1	92				0					27			43		54	
Capacity, c (veh/h)		1291				1455					498			395		782	
v/c Ratio		0.07				0.00					0.05			0.11		0.07	
95% Queue Length, Q <sub>95</sub> (veh)		0.2				0.0					0.2			0.4		0.2	
Control Delay (s/veh)		8.0	0.2	0.2		7.5	0.0	0.0			12.6			15.2		9.9	
Level of Service (LOS)		А	А	А		А	А	А			В			С		А	
Approach Delay (s/veh)		3	.5			0	.0		12.6				12.3				
Approach LOS		,	4			,	A				В			1	3		

Generated: 5/2/2024 9:53:17 AM

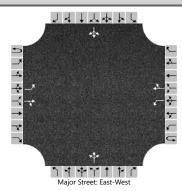
	HCS Two-Way Stop-Control Report											
General Information		Site Information										
Analyst	NM	Intersection	SD 38 & 459th									
Agency/Co.	HRG	Jurisdiction	SDDOT									
Date Performed	4/30/2024	East/West Street	SD 38									
Analysis Year	2050	North/South Street	459th Ave									
Time Analyzed	AM Peak	Peak Hour Factor	0.92									
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25									
Project Description	SD 38 Build Option 1											



		Major Sect. East West															
Vehicle Volumes and Adj	ustme	nts															
Approach		Eastbound				Westbound			Northbound					Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0	
Configuration	1	L		TR		L		TR			LTR				LTR		
Volume (veh/h)	1	0	215	7		2	155	0		15	0	7		9	0	0	
Percent Heavy Vehicles (%)	1	3				3				13	0	0		0	0	0	
Proportion Time Blocked	1																
Percent Grade (%)										(	)			(	0		
Right Turn Channelized																	
Median Type   Storage		Undivided															
Critical and Follow-up Ho																	
Base Critical Headway (sec)	1	4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2	
Critical Headway (sec)		4.13				4.13				7.23	6.50	6.20		7.10	6.50	6.20	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.23				2.23				3.62	4.00	3.30		3.50	4.00	3.30	
Delay, Queue Length, an	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)	1	0				2					24				10		
Capacity, c (veh/h)	1	1403				1319					596				546		
v/c Ratio		0.00				0.00					0.04				0.02		
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.1				0.1		
Control Delay (s/veh)	1	7.6	0.0	0.0		7.7	0.0	0.0			11.3				11.7		
Level of Service (LOS)		Α	А	А		А	Α	Α			В				В		
Approach Delay (s/veh)		0	.0			0.1				11	1.3		11.7				
Approach LOS		,	Ą			,	4				В		В				

Generated: 5/2/2024 9:54:11 AM

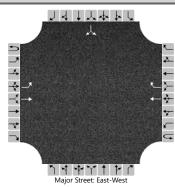
	HCS Two-Way Stop	-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & 459th							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	459th Ave							
Time Analyzed	PM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38 Build Option 1									



Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound			Westl	ound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0	
Configuration		L		TR		L		TR			LTR				LTR		
Volume (veh/h)		0	145	9		15	245	2		15	0	4		2	2	0	
Percent Heavy Vehicles (%)		0				0				13	0	0		0	100	0	
Proportion Time Blocked	]																
Percent Grade (%)	]									(	0		0				
Right Turn Channelized	]																
Median Type   Storage	]			Undi	vided												
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)	T	4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2	
Critical Headway (sec)		4.10				4.10				7.23	6.50	6.20		7.10	7.50	6.20	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.20				2.20				3.62	4.00	3.30		3.50	4.90	3.30	
Delay, Queue Length, an	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)		0				16					21				4		
Capacity, c (veh/h)		1307				1423					534				427		
v/c Ratio		0.00				0.01					0.04				0.01		
95% Queue Length, Q <sub>95</sub> (veh)	1	0.0				0.0					0.1				0.0		
Control Delay (s/veh)		7.8	0.0	0.0		7.6	0.1	0.1			12.0				13.5		
Level of Service (LOS)	1	А	А	А		А	А	А			В				В		
Approach Delay (s/veh)	1	0.0					0.5			12.0				13.5			
Approach LOS	ĺ	,	Ą			,	4				В			ı	В		

Generated: 5/2/2024 9:54:46 AM

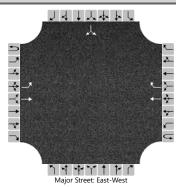
	HCS Two-Way Stop	-Control Report									
General Information		Site Information									
Analyst	NM	Intersection	SD 38 & I-90 Speedway								
Agency/Co.	HRG	Jurisdiction	SDDOT								
Date Performed	4/30/2024	East/West Street	SD 38								
Analysis Year	2050	North/South Street	I-90 Expressway								
Time Analyzed	AM Peak	Peak Hour Factor	0.92								
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25								
Project Description	SD 38 Build Option 1										



					iviaji	or otreet. La	st-vvest											
Vehicle Volumes and Adj	ustme	nts																
Approach		Eastb	ound			Westl	oound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		0	1	0		
Configuration		L	Т				Т	R							LR			
Volume (veh/h)		0	230				165	0						0		0		
Percent Heavy Vehicles (%)		3												3		3		
Proportion Time Blocked																		
Percent Grade (%)														(	0			
Right Turn Channelized						Ν	lo											
Median Type   Storage				Undi	vided													
Critical and Follow-up Ho	eadwa	ys																
Base Critical Headway (sec)		4.1												7.1		6.2		
Critical Headway (sec)		4.13												6.43		6.23		
Base Follow-Up Headway (sec)		2.2												3.5		3.3		
Follow-Up Headway (sec)		2.23												3.53		3.33		
Delay, Queue Length, and	d Leve	l of Se	ervice	)														
Flow Rate, v (veh/h)	]	0													0			
Capacity, c (veh/h)		1390													0			
v/c Ratio		0.00																
95% Queue Length, Q <sub>95</sub> (veh)		0.0			Ì													
Control Delay (s/veh)		7.6	0.0															
Level of Service (LOS)		А	А		Ì													
Approach Delay (s/veh)	0.0																	
Approach LOS		А																

Generated: 5/2/2024 9:55:23 AM

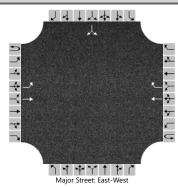
	HCS Two-Way Stop	-Control Report									
General Information		Site Information									
Analyst	NM	Intersection	SD 38 & I-90 Speedway								
Agency/Co.	HRG	Jurisdiction	SDDOT								
Date Performed	4/30/2024	East/West Street	SD 38								
Analysis Year	2050	North/South Street	I-90 Expressway								
Time Analyzed	AM Peak - Event Traffic	Peak Hour Factor	0.92								
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25								
Project Description	SD 38 Build Option 1										



					iviajo	or otreet. La	3t-vvest											
Vehicle Volumes and Adj	ustme	nts																
Approach		Eastb	ound			Westl	oound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		0	1	0		
Configuration		L	Т				Т	R							LR			
Volume (veh/h)		0	412				295	0						0		0		
Percent Heavy Vehicles (%)		3												3		3		
Proportion Time Blocked																		
Percent Grade (%)														(	0			
Right Turn Channelized							lo											
Median Type   Storage	1			Undi	vided													
<b>Critical and Follow-up He</b>	eadwa	ys																
Base Critical Headway (sec)	1	4.1												7.1		6.2		
Critical Headway (sec)		4.13												6.43		6.23		
Base Follow-Up Headway (sec)		2.2												3.5		3.3		
Follow-Up Headway (sec)		2.23												3.53		3.33		
Delay, Queue Length, and	d Leve	l of Se	ervice															
Flow Rate, v (veh/h)	1	0													0			
Capacity, c (veh/h)	1	1234													0			
v/c Ratio	1	0.00																
95% Queue Length, Q <sub>95</sub> (veh)		0.0																
Control Delay (s/veh)	1	7.9	0.0															
Level of Service (LOS)		А	А															
Approach Delay (s/veh)	0.0																	
Approach LOS	А																	

Generated: 5/2/2024 9:57:09 AM

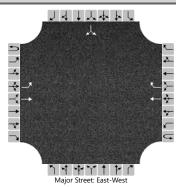
	HCS Two-Way Stop	-Control Report									
General Information		Site Information									
Analyst	NM	Intersection	SD 38 & I-90 Speedway								
Agency/Co.	HRG	Jurisdiction	SDDOT								
Date Performed	4/30/2024	East/West Street	SD 38								
Analysis Year	2050	North/South Street	I-90 Expressway								
Time Analyzed	PM Peak	Peak Hour Factor	0.92								
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25								
Project Description	SD 38 Build Option 1										



					iviajo	or otreet. La	st-vvest											
Vehicle Volumes and Adj	ustme	nts																
Approach		Eastb	ound			Westl	oound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		0	1	0		
Configuration		L	Т				Т	R							LR			
Volume (veh/h)		0	165				260	0						0		0		
Percent Heavy Vehicles (%)		3												3		3		
Proportion Time Blocked																		
Percent Grade (%)														(	0			
Right Turn Channelized							lo											
Median Type   Storage				Undi	vided													
Critical and Follow-up Ho	eadwa	ys																
Base Critical Headway (sec)		4.1												7.1		6.2		
Critical Headway (sec)		4.13												6.43		6.23		
Base Follow-Up Headway (sec)		2.2												3.5		3.3		
Follow-Up Headway (sec)		2.23												3.53		3.33		
Delay, Queue Length, and	d Leve	l of Se	ervice	)														
Flow Rate, v (veh/h)	]	0													0			
Capacity, c (veh/h)		1274													0			
v/c Ratio		0.00																
95% Queue Length, Q <sub>95</sub> (veh)		0.0																
Control Delay (s/veh)		7.8	0.0															
Level of Service (LOS)		А	А		Ì													
Approach Delay (s/veh)	0.0									•	•							
Approach LOS	А																	

Generated: 5/2/2024 9:56:18 AM

	HCS Two-Way Stop	-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & I-90 Speedway							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	I-90 Expressway							
Time Analyzed	PM Peak - Event Traffic	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38 Build Option 1									



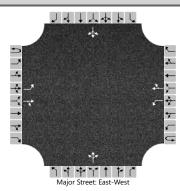
		iviajoi Street. Last-west															
Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		0	1	0	
Configuration	1	L	Т				Т	R							LR		
Volume (veh/h)	1	0	295				465	0						0		0	
Percent Heavy Vehicles (%)	1	3												3		3	
Proportion Time Blocked	1																
Percent Grade (%)															0		
Right Turn Channelized		No															
Median Type   Storage	1			Undi	vided				<u> </u>								
Critical and Follow-up Ho	eadwa	ys															
Base Critical Headway (sec)	1	4.1												7.1		6.2	
Critical Headway (sec)		4.13												6.43		6.23	
Base Follow-Up Headway (sec)		2.2												3.5		3.3	
Follow-Up Headway (sec)		2.23												3.53		3.33	
Delay, Queue Length, and	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)	1	0							П						0		
Capacity, c (veh/h)		1054													0		
v/c Ratio		0.00															
95% Queue Length, Q <sub>95</sub> (veh)		0.0															
Control Delay (s/veh)		8.4	0.0														
Level of Service (LOS)	1	А	А														
Approach Delay (s/veh)	0.0																
Approach LOS	А																

Generated: 5/2/2024 9:57:49 AM

### **HCS Signalized Intersection Results Summary** 144444 Intersection Information **General Information** Agency HRG Duration, h 0.250 CEC Analyst Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & Western Ave File Name (4) SD38&463WesternAve AM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 90 Demand (v), veh/h 9 180 80 60 110 30 65 75 40 80 5 **Signal Information** Cycle, s 50.0 Reference Phase 6 Offset, s 0 Reference Point End Green 30.3 9.7 0.0 0.0 0.0 0.0 Uncoordinated No Simult. Gap E/W On Yellow 4.0 4.0 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 1.0 1.0 0.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL **SBT Assigned Phase** 2 6 8 4 Case Number 6.0 6.0 6.0 6.0 Phase Duration, s 35.3 35.3 14.7 14.7 Change Period, (Y+Rc), s 5.0 5.0 5.0 5.0 Max Allow Headway ( MAH ), s 0.0 0.0 4.3 4.3 Queue Clearance Time ( $g_s$ ), s 7.0 8.7 Green Extension Time ( $g_e$ ), s 0.0 0.0 1.2 1.1 Phase Call Probability 1.00 1.00 0.03 0.07 Max Out Probability **Movement Group Results** EΒ WB NB SB Approach Movement L Т R L Т R L Т R Т L R 12 **Assigned Movement** 5 2 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 10 283 65 152 71 179 43 92 1225 1666 1088 1693 1614 1205 1684 Adjusted Saturation Flow Rate ( s ), veh/h/ln 1180 0.2 4.0 1.5 2.0 2.7 5.0 1.7 2.3 Queue Service Time ( $g_s$ ), s Cycle Queue Clearance Time ( g c ), s 2.2 4.0 5.6 2.0 5.0 5.0 6.7 2.3 Green Ratio (g/C) 0.61 0.61 0.61 0.61 0.19 0.19 0.19 0.19 Capacity (c), veh/h 836 1007 713 1023 320 315 259 329 Volume-to-Capacity Ratio (X) 0.012 0.281 0.092 0.149 0.220 0.569 0.168 0.281 Back of Queue (Q), ft/ln (95 th percentile) 1.6 46.2 13.6 22.3 34.3 80.9 20.5 39.4 Back of Queue (Q), veh/ln (95 th percentile) 0.1 1.8 0.5 0.9 1.2 3.2 8.0 1.5 Queue Storage Ratio (RQ) (95 th percentile) 0.01 0.00 0.05 0.00 0.14 0.00 80.0 0.00 Uniform Delay ( d 1 ), s/veh 4.8 4.7 6.1 4.3 19.2 18.2 21.2 17.1 Incremental Delay ( d 2 ), s/veh 0.0 0.7 0.3 0.3 0.3 1.6 0.3 0.5 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 4.8 5.4 6.3 4.6 19.6 19.8 21.5 17.6 Level of Service (LOS) Α Α Α Α В В С В 5.4 5.1 19.7 18.8 Approach Delay, s/veh / LOS Α Α В В Intersection Delay, s/veh / LOS 11.4 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 1.85 В 1.85 1.91 В 1.91 В В Bicycle LOS Score / LOS 0.97 Α 0.85 Α 0.90 Α 0.71 Α

### **HCS Signalized Intersection Results Summary** 144444 Intersection Information **General Information** Agency HRG Duration, h 0.250 CEC Analyst Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period PM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & Western Ave File Name (4) SD38&463WesternAve PM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 60 Demand (v), veh/h 15 125 55 120 200 70 85 155 55 100 25 **Signal Information** Cycle, s 50.0 Reference Phase 6 Offset, s 0 Reference Point End Green 26.5 0.0 13.5 0.0 0.0 0.0 Uncoordinated No Simult. Gap E/W On Yellow 4.0 4.0 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 1.0 1.0 0.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL **SBT Assigned Phase** 2 6 8 4 Case Number 6.0 6.0 6.0 6.0 Phase Duration, s 31.5 31.5 18.5 18.5 Change Period, (Y+Rc), s 5.0 5.0 5.0 5.0 Max Allow Headway ( MAH ), s 0.0 0.0 4.3 4.3 Queue Clearance Time ( $g_s$ ), s 9.8 12.3 Green Extension Time ( $g_e$ ), s 0.0 0.0 1.5 1.2 Phase Call Probability 1.00 1.00 0.18 0.43 Max Out Probability **Movement Group Results** EΒ WB NB SB Approach Movement L Т R L Т R L Т R Т L R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 16 196 130 283 76 261 60 136 923 1680 1178 1701 1253 1474 1119 1683 Adjusted Saturation Flow Rate ( s ), veh/h/ln 0.5 3.1 3.3 4.7 2.6 7.8 2.5 3.2 Queue Service Time ( $g_s$ ), s 6.5 Cycle Queue Clearance Time ( g c ), s 5.3 3.1 4.7 5.7 7.8 10.3 3.2 0.27 0.27 Green Ratio (g/C) 0.53 0.53 0.53 0.53 0.27 0.27 Capacity (c), veh/h 545 889 693 901 404 399 272 455 Volume-to-Capacity Ratio (X) 0.030 0.220 0.188 0.314 0.188 0.654 0.219 0.298 Back of Queue (Q), ft/ln (95 th percentile) 5 41.2 34.4 63.5 30.4 118.8 27.9 50.3 Back of Queue (Q), veh/ln (95 th percentile) 0.2 1.6 1.3 2.5 1.2 4.4 1.1 2.0 Queue Storage Ratio (RQ) (95 th percentile) 0.02 0.00 0.14 0.00 0.12 0.00 0.11 0.00 Uniform Delay ( d 1 ), s/veh 8.1 6.3 8.0 6.6 16.7 16.2 20.7 14.5 Incremental Delay ( d 2 ), s/veh 0.1 0.6 0.6 0.9 0.2 1.8 0.4 0.4 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 8.2 6.8 8.6 7.5 16.9 18.0 21.1 14.8 Level of Service (LOS) Α Α Α Α В В С В 6.9 7.9 17.8 Approach Delay, s/veh / LOS Α Α В 16.8 В Intersection Delay, s/veh / LOS 12.1 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 1.87 В 1.87 1.90 1.90 В В В Bicycle LOS Score / LOS 0.84 Α 1.17 Α 1.04 Α 0.81 Α

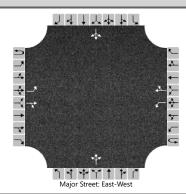
	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & Main Ave
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	Main Ave (9th St)
Time Analyzed	AM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38 Build Option 1		



Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound			Westbound				North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0	
Configuration		L		TR		L		TR			LTR				LTR		
Volume (veh/h)		2	260	30		40	195	20		40	5	85		6	10	4	
Percent Heavy Vehicles (%)		0				11				5	0	2		0	17	0	
Proportion Time Blocked																	
Percent Grade (%)											0			(	0		
Right Turn Channelized													İ				
Median Type   Storage				Left	Only							9	9				
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2	
Critical Headway (sec)		4.10				4.21				7.15	6.50	6.22		7.10	6.67	6.20	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.20				2.30				3.55	4.00	3.32		3.50	4.15	3.30	
Delay, Queue Length, an	d Leve	l of S	ervice														
Flow Rate, v (veh/h)	<u> </u>	2				43					141				22		
Capacity, c (veh/h)		1346				1196					678				459		
v/c Ratio		0.00				0.04					0.21				0.05		
95% Queue Length, Q <sub>95</sub> (veh)	j	0.0			Ì	0.1					0.8				0.1		
Control Delay (s/veh)		7.7				8.1					11.7				13.2		
Level of Service (LOS)		А				А					В				В		
Approach Delay (s/veh)		0	.1			1	.3	•		1	1.7			13	3.2		
Approach LOS	ĺ		Ą			,	4				В			ı	В		

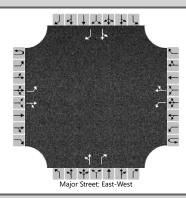
Generated: 5/2/2024 10:01:29 AM

HCS Two-Way Stop-Control Report												
General Information		Site Information										
Analyst	NM	Intersection	SD 38 & Main Ave									
Agency/Co.	HRG	Jurisdiction	SDDOT									
Date Performed	4/30/2024	East/West Street	SD 38									
Analysis Year	2050	North/South Street	Main Ave (9th St)									
Time Analyzed	PM Peak	Peak Hour Factor	0.92									
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25									
Project Description	SD 38 Build Option 1											



Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound		Westbound				Northbound				Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0	
Configuration		L		TR		L		TR			LTR				LTR		
Volume (veh/h)		10	250	45		65	335	60		35	20	55		40	30	7	
Percent Heavy Vehicles (%)		0				0				5	0	0		0	0	0	
Proportion Time Blocked																	
Percent Grade (%)												0					
Right Turn Channelized																	
Median Type   Storage	Left Only 9																
Critical and Follow-up Headways																	
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2	
Critical Headway (sec)		4.10				4.10				7.15	6.50	6.20		7.10	6.50	6.20	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.20				2.20				3.55	4.00	3.30		3.50	4.00	3.30	
Delay, Queue Length, an	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)	<b>T</b>	11				71					120				84		
Capacity, c (veh/h)		1141				1251					467				368		
v/c Ratio		0.01				0.06					0.26				0.23		
95% Queue Length, Q <sub>95</sub> (veh)	j	0.0				0.2				Ì	1.0				0.9		
Control Delay (s/veh)		8.2				8.1					15.3				17.6		
Level of Service (LOS)	j	А				А				Ì	С				С		
Approach Delay (s/veh)		0	.3			1	.1	•		1!	5.3			17	7.6		
Approach LOS	1	,	Ą			,	4			(	С			(	C		

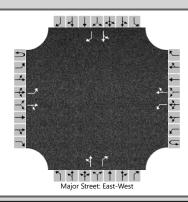
	HCS Two-Way Stop-Control Report													
General Information		Site Information												
Analyst	NM	Intersection	SD 38 & Vandemark Ave											
Agency/Co.	HRG	Jurisdiction	SDDOT											
Date Performed	4/30/2024	East/West Street	SD 38											
Analysis Year	2050	North/South Street	Vandemark Avenue											
Time Analyzed	АМ	Peak Hour Factor	0.92											
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25											
Project Description	SD 38 Build Option 1													



Vehicle Volumes and Adjustments																	
Approach		Eastb	ound		Westbound				Northbound					South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	1	1	0		0	1	1		0	1	1	
Configuration		L		TR		L		TR		LT		R		LT		R	
Volume (veh/h)	25 370 10					8	240	25		9	5	10		40	2	25	
Percent Heavy Vehicles (%)		0				0				40	0	0		0	0	7	
Proportion Time Blocked																	
Percent Grade (%)										(	)			(	0		
Right Turn Channelized										Ν	lo		No				
Median Type   Storage				Undi	vided												
Critical and Follow-up Headways																	
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2	
Critical Headway (sec)		4.10				4.10				7.50	6.50	6.20		7.10	6.50	6.27	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.20				2.20				3.86	4.00	3.30		3.50	4.00	3.36	
Delay, Queue Length, and	Leve	l of Se	ervice														
Flow Rate, v (veh/h)		27				9				15		11		46		27	
Capacity, c (veh/h)		1286				1157				278		648		306		752	
v/c Ratio		0.02				0.01				0.05		0.02		0.15		0.04	
95% Queue Length, Q <sub>95</sub> (veh)		0.1				0.0				0.2		0.1		0.5		0.1	
Control Delay (s/veh)		7.9	0.1	0.1		8.1	0.1	0.1		18.7		10.7		18.8		10.0	
Level of Service (LOS)		Α	А	А		А	Α	А		С		В		С		А	
Approach Delay (s/veh)		0	.6			0	.3		15.4				15.5				
Approach LOS		,	Α			,	4			(	С			(	С		

Generated: 5/2/2024 10:02:45 AM

HCS Two-Way Stop-Control Report												
General Information		Site Information										
Analyst	NM	Intersection	SD 38 & Vandemark Ave									
Agency/Co.	HRG	Jurisdiction	SDDOT									
Date Performed	4/30/2024	East/West Street	SD 38									
Analysis Year	2050	North/South Street	Vandemark Avenue									
Time Analyzed	PM	Peak Hour Factor	0.92									
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25									
Project Description	SD 38 Build Option 1											



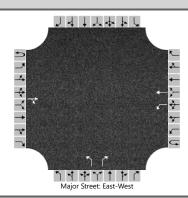
Vehicle Volumes and Adj	ustine	1115															
Approach		Eastb	ound			Westl	oound		Northbound					South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	1	1	0		0	1	1		0	1	1	
Configuration		L		TR		L		TR		LT		R		LT		R	
Volume (veh/h)		20	255	4		5	475	45		0	0	9		30	0	25	
Percent Heavy Vehicles (%)	0					0				0	0	100		0	0	7	
Proportion Time Blocked	1																
Percent Grade (%)	1									(	0			(	0		
Right Turn Channelized										Ν	lo		No				
Median Type   Storage		Undivided															
Critical and Follow-up Headways																	
Base Critical Headway (sec)	1	4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2	
Critical Headway (sec)		4.10				4.10				7.10	6.50	7.20		7.10	6.50	6.27	
Base Follow-Up Headway (sec)	1	2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)	1	2.20				2.20				3.50	4.00	4.20		3.50	4.00	3.36	
Delay, Queue Length, an	d Leve	l of S	ervice														
Flow Rate, v (veh/h)	<u> </u>	22				5				0		10		33		27	
Capacity, c (veh/h)	1	1017				1293				0		574		259		532	
v/c Ratio		0.02				0.00						0.02		0.13		0.05	
95% Queue Length, Q <sub>95</sub> (veh)	1	0.1				0.0						0.1		0.4		0.2	
Control Delay (s/veh)	1	8.6	0.2	0.2		7.8	0.0	0.0				11.4		20.9		12.1	
Level of Service (LOS)	]	А	А	А		А	Α	А				В		С		В	
Approach Delay (s/veh)	]	0	.8			0	.1						16.9				
Approach LOS	]	,	A			,	4							(	С		

Generated: 5/2/2024 10:03:28 AM

HCS Signalized Intersection Results Summary																	
General Inform	nation	V.						$\rightarrow$		ction Inf							
Agency		HRG								ո, h	0.250			Ť			
Analyst		NM		Analysis Date May 8, 2023					Area Ty	ре	Other	-	<i>1</i> ,		. 2		
Jurisdiction		SDDOT		Time F	Period	AM F	eak		PHF		0.92		<b>⊕ ↔</b>	W ∯ E B	<u>-</u>		
Urban Street		SD 38		Analys	is Yea	ır 2050			Analysi	s Period	1> 7:	15	7		*		
Intersection		SD 38 & 2nd Street	į .	File Na	ame	(7) S	D38&2n	d_AM.	xus					7 1			
Project Descrip	tion				THEFTE												
Demand Inform	nation				EB			WI	3		NB		Т	SB			
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R		
Demand ( v ), v		20	325	10	95	20	0 15	5	20	155	35	50	25				
,,																	
Signal Informa	tion							Т		Т					<b>T</b>		
Cycle, s	45.0	Reference Phase		Ħ.	5/1	2					_	♣ .	2	x1x			
Offset, s	0	Reference Point	End	Green	27.9	8.1	0.0	0.0	0.0	0.0		1	Y -	3	4		
Uncoordinated	No	Simult. Gap E/W	On	Yellow		3.5	0.0	0.0					<b>→</b>		KŤ2		
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.0				5	6	7	<b>Y</b> 8		
Timer Results				EBL	<u> </u>	EBT	WB		WBT	NB	1	NBT	SB	<u> </u>	SBT		
Assigned Phase				LDL	-	2	110	_	6	140		8	0.5	-	4		
Case Number				_		8.0	-		6.0	-		6.0			8.0		
Phase Duration					-	32.4	_		32.4	-				_	12.6		
Change Period,		\ 0		_	_	4.5	-			-		12.6		_	4.5		
Max Allow Head		<u>,                                    </u>		_	-	0.0	-	4.5		-	_	3.3	-	-	3.3		
Queue Clearan		· · · · · · · · · · · · · · · · · · ·		_	_	0.0	-	0.0		+				_	7.5		
		, = ,		-	+	0.0	-	0.		-	-	7.8		-	0.5		
Green Extensio		( <i>g</i> e ), s		-	_	0.0	$\vdash$			+	0.5		-	_	0.98		
Phase Call Prol				-	-		-			-		0.96		-			
Max Out Proba	DIIILY				_							0.01			0.00		
Movement Gro	up Res	sults			EB			WB		Т	NB			SB			
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R		
Assigned Move	ment			5	2	12	1	6	16	3	8	18	7	4	14		
Adjusted Flow F	Rate ( v	), veh/h			386		103	234		5	190			120			
Adjusted Satura	ation Flo	ow Rate ( s ), veh/h/l	n		1743		1018	1750		1317	1528			1105			
Queue Service	Time (	g s ), S			0.0		2.5	2.6		0.2	5.2			0.2			
Cycle Queue C	learanc	e Time ( <i>g ε</i> ), s			4.8		7.3	2.6		5.8	5.2			5.5			
Green Ratio ( g					0.62		0.62	0.62		0.18	0.18			0.18			
Capacity ( c ), v					1162		681	1082		236	278			306			
Volume-to-Capa		atio (X)			0.332		0.152	0.216	_	0.023	0.685			0.391			
		/In (95 th percentile	<del>:</del> )														
		eh/ln ( 95 th percenti			1.9		0.8	1.1		0.1	2.9			1.7			
		RQ) (95 th percent			0.00		0.08	0.00		0.02	0.00			0.00			
Uniform Delay (		, ,	,		4.2		6.0	3.8		20.1	17.2			16.3			
Incremental De					0.8		0.5	0.5		0.0	1.1			0.3			
Initial Queue De		0.0		0.0	0.0		0.0	0.0			0.0						
Control Delay (		5.0		6.4	4.2		20.1	18.3			16.6						
Level of Service				A		A	A		C	В			В				
Approach Delay		5.0		Α	4.9		A	18.		В	16.0	16.6 E					
Intersection De				J.,3			3.8			1.5.			A				
	, J, V																
Multimodal Re	sults				EB			WB			NB	NB		SB			
Pedestrian LOS		/ LOS		1.84	-	В	1.62		В	1.9		В	1.68		В		
Bicycle LOS Sc				1.12		A	1.04		A	0.8		A	0.68		A		

	HCS Signalized Intersection Results Summary														
General Inform	nation				Intersection Information										Ja L
Agency	lation	HRG							Duration		0.250			*	
Analyst		NM		Analys	eie Date	e May 8	3 2023		Area Typ		Other		_* _\$		t. A
Jurisdiction		SDDOT		Time I		PM P		_	PHF		0.92		→ 	w. F	<b>.</b> ≥
Urban Street		SD 38			sis Yea		eak		Analysis	Poriod	1> 7:	15	-4		
Intersection		SD 38 & 2nd Street		File N			038&2n			renou	1- 7.	13			<u></u>
Project Descript	tion	3D 36 & Zhu Sheet		File IV	ame	(1) 31	JSOQZIII	u_Fivi.	xu5				-	1 4 4 4	↑ °
1 Toject Descript	lion														
Demand Inforn	nation				EB			WE	3	Т	NB		$\top$	SB	
Approach Move	ment			L	Т	R	L	Т	R	L	T	R	L	Т	R
Demand ( v ), v	eh/h			25	235	9	130	490	) 25	15	25	65	15	30	20
Signal Informa	tion				1.								_		$\perp$
Cycle, s	60.0	Reference Phase	2		Ħ	T 54	2					1	↔ ,	3	x1x
Offset, s	0	Reference Point	End	Green	44.3	6.7	0.0	0.0	0.0	0.0			K		-
Uncoordinated	No	Simult. Gap E/W	On	Yellow		3.5	0.0	0.0	0.0	0.0			<b>₹</b>		Φ
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.0	0.0	0.0		5	6	7	8
							1	-					11		
Timer Results				EBI	-	EBT	WB	L	WBT	NB	L	NBT	SB	L	SBT
Assigned Phase	e			$\vdash$	_	2			6			8			4
Case Number						8.0			6.0			6.0			8.0
Phase Duration						48.8			48.8			11.2	<u> </u>		11.2
Change Period,		· ·				4.5			4.5			4.5			4.5
Max Allow Head	dway( <i>N</i>	<i>MAH</i> ), s		$oxed{oxed}$		0.0			0.0			3.2			3.2
Queue Clearan	ce Time	e ( g s ), s										6.2			5.6
Green Extensio	n Time	( g e ), s				0.0			0.0			0.3			0.3
Phase Call Prob	bability											0.95			0.95
Max Out Probal	bility											0.00			0.00
		14						\A/D			ND			0.0	
Movement Gro		suits		<b>.</b>	EB	T 5		WB	T 5		NB		<b>.</b>	SB	
Approach Move				<u> </u>	T	R	L	T	R	L	T	R	L	T	R
Assigned Move		\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \		5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow F		,·		_	292		141	560	-	16	98		-	71	
		ow Rate ( s ), veh/h/l	n	_	1663		1114	1757	+	1350	1568	-	-	1377	
Queue Service		- ,		_	0.0	-	2.7	7.3	+	0.7	3.5	_	-	0.0	
Cycle Queue Cl		e Time ( g ε ), s		_	3.1	-	5.9	7.3	+	4.2	3.5	-	-	3.6	-
Green Ratio ( g.				_	0.74		0.74	0.74	_	0.11	0.11		-	0.11	_
Capacity ( c ), v				_	1294	_	885	1298	_	190	175		-	227	
Volume-to-Capa			\		0.226		0.160	0.431		0.086	0.560			0.311	
		/In (95 th percentile			1.0		0.0	2.5	+	0.4	2.3		-	1.0	
	• •	eh/ln (95 th percenti	,		1.0		0.8		+	0.4			-	1.6	
		RQ) (95 th percent	iie)		0.00		0.08	0.00		0.07	0.00			0.00	
Uniform Delay ( d 1 ), s/veh Incremental Delay ( d 2 ), s/veh					2.5		3.4	3.0		27.3	25.3			24.7	
		0.4		0.4	1.0		0.1	1.0			0.3				
Initial Queue De		0.0		0.0	0.0		0.0	0.0			0.0				
Control Delay (		2.9		3.8	4.1		27.4	26.3			25.0				
Level of Service	2.0	A	^	A 0	A	^	C	С	<u>C</u>	05.4	C				
Approach Delay				2.9		A 7	4.0		Α	26.5	י ו	С	25.0	J	С
Intersection Del	ay, s/ve	en / LOS				/	'.2						Α		
Multimodal Re	sulte				EB			WB		NB		SB			
Pedestrian LOS		/I OS		1.83		В	1.60		<u>В</u>	1.92		В	1.70		В
Bicycle LOS Sc				0.97		A	1.64	_	В	0.68		A	0.60		A
2.5,5.0 200 00	J. J / LC	-		3.01			1.0		_	0.00			3.30		

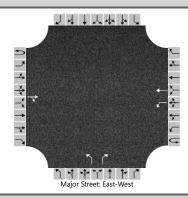
	HCS Two-Way Stop	p-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & West Central HS Entrance							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	West Central HS Entrance							
Time Analyzed	AM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38 Build Option 1									



Vehicle Volumes and Adju	ıstme	nts															
Approach		Eastb	ound			Westk	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	1	1	0		1	0	1		0	0	0	
Configuration				TR		L	Т			L		R					
Volume (veh/h)			425	90		55	285			35		50					
Percent Heavy Vehicles (%)						0				0		0					
Proportion Time Blocked																	
Percent Grade (%)										(	)						
Right Turn Channelized										N	lo						
Median Type   Storage				Left	Only							9	9				
<b>Critical and Follow-up He</b>	adwa	ys															
Base Critical Headway (sec)						4.1				7.1		6.2					
Critical Headway (sec)						4.10				6.40		6.20					
Base Follow-Up Headway (sec)						2.2				3.5		3.3					
Follow-Up Headway (sec)						2.20				3.50		3.30					
Delay, Queue Length, and	l Leve	of Se	ervice														
Flow Rate, v (veh/h)						60				38		54					
Capacity, c (veh/h)	1					1021				576		567					
v/c Ratio	1					0.06				0.07		0.10					
95% Queue Length, Q <sub>95</sub> (veh)						0.2				0.2		0.3					
Control Delay (s/veh)						8.7				11.7		12.0					
Level of Service (LOS)						Α				В		В					
Approach Delay (s/veh)		1.4							11.9								
Approach LOS							A B										

Generated: 5/2/2024 10:04:02 AM

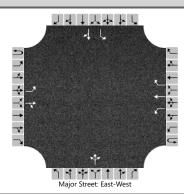
	HCS Two-Way Stop	-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & West Central HS Entrance							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	West Central HS Entrance							
Time Analyzed	PM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38 Build Option 1									



Vehicle Volumes and Adju	ustme	nts															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	1	1	0		1	0	1		0	0	0	
Configuration	1			TR		L	Т			L		R					
Volume (veh/h)	1		305	4		4	620			15		15					
Percent Heavy Vehicles (%)	1					0				0		0					
Proportion Time Blocked	1																
Percent Grade (%)	1									(	)						
Right Turn Channelized	1									Ν	lo						
Median Type   Storage	1			Left	Only	nly							9				
Critical and Follow-up He	eadwa	ys															
Base Critical Headway (sec)	1					4.1				7.1		6.2					
Critical Headway (sec)						4.10				6.40		6.20					
Base Follow-Up Headway (sec)	1					2.2				3.5		3.3					
Follow-Up Headway (sec)	1					2.20				3.50		3.30					
Delay, Queue Length, and	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)	1					4				16		16					
Capacity, c (veh/h)	1					1235				500		713					
v/c Ratio	1					0.00				0.03		0.02					
95% Queue Length, Q <sub>95</sub> (veh)						0.0				0.1		0.1					
Control Delay (s/veh)						7.9				12.4		10.2					
Level of Service (LOS)						А				В		В					
Approach Delay (s/veh)						0.1				11.3							
Approach LOS	1					,	4		В								

Generated: 5/2/2024 10:04:37 AM

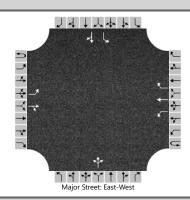
	HCS Two-Way Stop	op-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & Railroad Street							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	Railroad St							
Time Analyzed	AM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38 Build Option 1									



Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	1	1	1		0	1	0		1	1	0	
Configuration		L		TR		L	Т	R			LTR			L		TR	
Volume (veh/h)		4	465	0		15	270	95		2	0	30		145	4	5	
Percent Heavy Vehicles (%)		0				0				0	0	15		0	0	0	
Proportion Time Blocked																	
Percent Grade (%)											0			(	0		
Right Turn Channelized						Ν	lo										
Median Type   Storage				Undi	vided												
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)	1	4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2	
Critical Headway (sec)		4.10				4.10				7.10	6.50	6.35		7.10	6.50	6.20	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.20				2.20				3.50	4.00	3.44		3.50	4.00	3.30	
Delay, Queue Length, an	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)	<u> </u>	4		Π		16			Π		35			158		10	
Capacity, c (veh/h)		1173				1070					505			258		448	
v/c Ratio		0.00				0.02					0.07			0.61		0.02	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.2			3.6		0.1	
Control Delay (s/veh)		8.1	0.0	0.0		8.4	0.1				12.6			38.6		13.2	
Level of Service (LOS)		А	А	Α		А	А				В			E		В	
Approach Delay (s/veh)		0	.1		0.4					12	2.6		37.1				
Approach LOS	i	Α				А				В				E			

Generated: 5/2/2024 10:05:15 AM

	HCS Two-Way Stop	op-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & Railroad Street							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/29/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	Railroad St							
Time Analyzed	PM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38 Build Option 1									



Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	1		0	1	0		1	1	0
Configuration		L		TR		L	Т	R			LTR			L		TR
Volume (veh/h)		4	340	4		15	560	155		2	2	15		85	9	5
Percent Heavy Vehicles (%)		0				40				0	0	15		5	0	0
Proportion Time Blocked																
Percent Grade (%)										(	0			(	)	
Right Turn Channelized						Ν	lo									
Median Type   Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.10				4.50				7.10	6.50	6.35		7.15	6.50	6.20
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.56				3.50	4.00	3.44		3.55	4.00	3.30
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1	4				16					21			92		15
Capacity, c (veh/h)		848				1004					417			197		287
v/c Ratio		0.01				0.02					0.05			0.47		0.05
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.2			2.3		0.2
Control Delay (s/veh)		9.3	0.0	0.0		8.6	0.1				14.1			38.5		18.2
Level of Service (LOS)		А	А	А		А	А				В			Е		С
Approach Delay (s/veh)		0	.2		0.3				14.1				35.7			
Approach LOS		,	A			,	4				В			E		

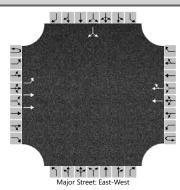
Generated: 5/2/2024 10:05:57 AM

#### **HCS Signalized Intersection Results Summary** Intersection Information **General Information** HRG Duration, h 0.250 Agency Analyst NM Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & Mickelson Roa... File Name (10) SD38&Mickelson AM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 40 190 65 Demand (v), veh/h 135 445 35 195 45 55 215 20 195 **Signal Information** die Cycle, s 70.0 Reference Phase 2 542 Offset, s 0 Reference Point End 35.1 3.1 10.0 0.0 Green 2.9 2.9 Uncoordinated No Simult. Gap E/W On Yellow 4.0 4.0 4.0 0.0 0.0 4.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 0.0 0.0 0.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL SBT **Assigned Phase** 2 6 3 8 4 1 7 Case Number 6.3 1.0 3.0 1.1 4.0 1.1 4.0 Phase Duration, s 39.1 6.9 46.0 7.1 14.0 10.0 16.9 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Change Period, (Y+Rc), s Max Allow Headway ( MAH ), s 0.0 3.1 0.0 3.1 3.3 3.1 3.3 Queue Clearance Time ( $g_s$ ), s 2.8 3.7 7.3 8.0 12.3 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.0 0.0 0.5 0.0 0.6 Phase Call Probability 0.57 0.61 1.00 0.99 1.00 0.03 1.00 0.01 Max Out Probability 0.00 1.00 **Movement Group Results** EΒ WB NB SB Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate (v), veh/h 147 264 258 43 212 207 49 130 234 234 1170 1772 1726 1688 1687 1688 1615 1688 1523 Adjusted Saturation Flow Rate ( s ), veh/h/ln 1323 5.0 0.8 5.2 1.7 5.3 Queue Service Time ( $g_s$ ), s 6.1 6.1 1.9 6.0 10.3 Cycle Queue Clearance Time ( q c ), s 5.0 6.1 6.1 8.0 1.9 5.2 1.7 5.3 6.0 10.3 0.60 Green Ratio (g/C) 0.50 0.50 0.50 0.57 0.60 0.19 0.14 0.24 0.18 Capacity (c), veh/h 690 890 867 536 2024 794 187 231 333 282 Volume-to-Capacity Ratio (X) 0.213 0.296 0.298 0.081 0.105 0.260 0.262 0.565 0.702 0.830 Back of Queue (Q), ft/ln (95 th percentile) Back of Queue (Q), veh/ln (95 th percentile) 2.2 4.0 4.0 0.4 1.0 2.4 1.2 3.5 2.5 6.7 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 Uniform Delay ( d 1 ), s/veh 9.9 10.2 10.2 7.1 6.0 6.6 24.5 28.0 25.5 27.5 Incremental Delay ( d 2 ), s/veh 0.7 8.0 0.9 0.0 0.1 0.8 0.3 8.0 5.5 2.4 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 10.6 11.0 11.1 7.1 6.1 7.4 24.8 28.8 31.1 29.9 Level of Service (LOS) В В В Α Α Α С С С С 11.0 В 6.8 Α 27.7 С 30.5 С Approach Delay, s/veh / LOS Intersection Delay, s/veh / LOS 16.7 В **Multimodal Results** ΕB WB NB SB Pedestrian LOS Score / LOS 1.88 В 1.87 В 2.44 2.28 В В Bicycle LOS Score / LOS 1.04 Α 0.87 Α 0.78 Α 1.26 Α

#### **HCS Signalized Intersection Results Summary** Intersection Information **General Information** HRG Duration, h 0.250 Agency Analyst NM Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & Mickelson Roa... File Name (10) SD38&Mickelson PM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 220 535 10 Demand (v), veh/h 160 20 135 225 20 65 215 15 185 **Signal Information** Ji. Cycle, s 70.0 Reference Phase 2 Offset, s 0 Reference Point End 0.7 30.8 1.3 7.0 Green 5.0 Uncoordinated No Simult. Gap E/W On Yellow 3.5 0.0 4.0 3.5 3.5 4.0 Force Mode Fixed Simult. Gap N/S On Red 1.0 0.0 1.0 1.0 1.0 1.0 **Timer Results EBL EBT WBL** WBT NBL **NBT** SBL SBT **Assigned Phase** 5 2 6 3 8 4 1 7 Case Number 1.1 4.0 1.1 3.0 1.1 4.0 1.1 4.0 Phase Duration, s 10.2 36.5 9.5 35.8 6.2 12.0 12.0 17.8 4.5 5.0 4.5 5.0 4.5 5.0 4.5 5.0 Change Period, (Y+Rc), s Max Allow Headway ( MAH ), s 3.1 0.0 3.1 0.0 3.1 3.3 3.1 3.3 Queue Clearance Time ( $g_s$ ), s 5.8 5.3 2.8 5.1 9.5 11.6 Green Extension Time ( $g_e$ ), s 0.1 0.0 0.1 0.0 0.0 0.3 0.0 0.2 Phase Call Probability 0.97 0.94 0.34 1.00 0.99 1.00 1.00 0.63 1.00 1.00 Max Out Probability 1.00 1.00 **Movement Group Results** EΒ WB NB SB Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate (v), veh/h 174 131 130 147 582 245 22 82 234 217 1688 1772 1720 1688 1687 1323 1688 1730 1688 1519 Adjusted Saturation Flow Rate ( s ), veh/h/ln 3.8 3.1 3.1 3.3 8.2 8.0 7.5 9.6 Queue Service Time ( $g_s$ ), s 8.9 3.1 Cycle Queue Clearance Time ( q c ), s 3.8 3.1 3.1 3.3 8.2 8.9 8.0 3.1 7.5 9.6 Green Ratio (g/C) 0.52 0.45 0.45 0.51 0.44 0.44 0.12 0.10 0.24 0.18 Capacity (c), veh/h 510 798 774 645 1484 582 164 173 356 277 Volume-to-Capacity Ratio (X) 0.341 0.165 0.167 0.228 0.392 0.420 0.132 0.472 0.656 0.785 Back of Queue (Q), ft/ln (95 th percentile) Back of Queue (Q), veh/ln (95 th percentile) 2.2 2.1 2.1 1.9 5.2 4.8 0.6 2.3 6.3 7.1 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 Uniform Delay ( d 1 ), s/veh 9.5 11.4 11.4 9.3 13.3 13.5 27.4 29.8 24.6 27.3 Incremental Delay ( d 2 ), s/veh 0.1 0.4 0.5 0.1 8.0 2.2 0.1 0.7 3.4 8.9 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 9.7 11.9 11.9 9.4 14.0 15.7 27.6 30.5 28.0 36.2 Level of Service (LOS) Α В В Α В В С С С D 11.0 В 13.8 В 29.9 С 32.0 С Approach Delay, s/veh / LOS Intersection Delay, s/veh / LOS 18.2 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 1.89 В В 2.44 2.28 1.89 В В Bicycle LOS Score / LOS 0.85 Α 1.29 Α 0.66 Α 1.23

Generated: 10/31/2023 2:36:18 PM

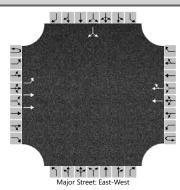
	HCS Two-Way Stop	op-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD38 & 466th Ave							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	466th Ave							
Time Analyzed	AM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38 Build Option 1									



Vehicle Volumes and Ad	1								<b>—</b>	<u>.</u>			Southbound					
Approach	<u> </u>	Eastb	ound			Westl	oound			North	bound			South	bound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R		
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		0	1	0		
Configuration		L	Т				Т	TR							LR			
Volume (veh/h)	0	2	765				430	5						4		0		
Percent Heavy Vehicles (%)	3	0												50		3		
Proportion Time Blocked																		
Percent Grade (%)														(	0			
Right Turn Channelized																		
Median Type   Storage				Left	Only							9	9					
Critical and Follow-up H	eadwa	ys																
Base Critical Headway (sec)		4.1												7.5		6.9		
Critical Headway (sec)		4.10												7.80		6.96		
Base Follow-Up Headway (sec)		2.2												3.5		3.3		
Follow-Up Headway (sec)		2.20												4.00		3.33		
Delay, Queue Length, an	d Leve	l of S	ervice															
Flow Rate, v (veh/h)		2													4			
Capacity, c (veh/h)		1100													457			
v/c Ratio		0.00													0.01			
95% Queue Length, Q <sub>95</sub> (veh)		0.0													0.0			
Control Delay (s/veh)		8.3													12.9			
Level of Service (LOS)		А													В			
Approach Delay (s/veh)		0	.0				•				•	•	12.9					
Approach LOS	1		Ą										В					

Generated: 5/2/2024 10:08:19 AM

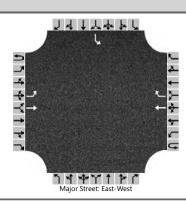
	HCS Two-Way Stop	pp-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD38 & 466th Ave							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	466th Ave							
Time Analyzed	PM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38 Build Option 1									



Vehicle Volumes and Ad	ustme	nts															
Approach		Eastb	ound			Westl	bound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		0	1	0	
Configuration		L	Т				Т	TR							LR		
Volume (veh/h)	0	0	445				910	2						5		2	
Percent Heavy Vehicles (%)	3	0												33		0	
Proportion Time Blocked																	
Percent Grade (%)															0		
Right Turn Channelized																	
Median Type   Storage		Left Only										!	9				
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)		4.1												7.5		6.9	
Critical Headway (sec)		4.10												7.46		6.90	
Base Follow-Up Headway (sec)		2.2												3.5		3.3	
Follow-Up Headway (sec)		2.20												3.83		3.30	
Delay, Queue Length, an	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)	<u> </u>	0		П							Π				8		
Capacity, c (veh/h)		705													299		
v/c Ratio		0.00													0.03		
95% Queue Length, Q <sub>95</sub> (veh)		0.0													0.1		
Control Delay (s/veh)		10.1													17.3		
Level of Service (LOS)		В													С		
Approach Delay (s/veh)		0	.0										17.3				
Approach LOS		,	A										С				

Generated: 5/2/2024 10:09:05 AM

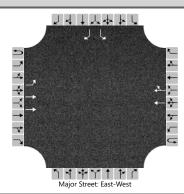
	HCS Two-Way Stop	-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & I-90 WB Terminal							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	12/12/2023	East/West Street	SD 38							
Analysis Year	2050	North/South Street	I-90 WB Terminal							
Time Analyzed	AM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38									



Vehicle Volumes and Adj	justme	nts														
Approach		Eastb	ound			Westl	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		1	0	0
Configuration		L	Т				Т	R						L		
Volume (veh/h)		40	730				255	20						15		
Percent Heavy Vehicles (%)		0												56		
Proportion Time Blocked																
Percent Grade (%)															0	
Right Turn Channelized						١	10									
Median Type   Storage				Left	Only								9			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1												7.1		
Critical Headway (sec)		4.10												6.96		
Base Follow-Up Headway (sec)		2.2												3.5		
Follow-Up Headway (sec)		2.20												4.00		
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		43												16		
Capacity, c (veh/h)		1274												315		
v/c Ratio		0.03												0.05		
95% Queue Length, Q <sub>95</sub> (veh)		0.1												0.2		
Control Delay (s/veh)		7.9	0.2											17.1		
Level of Service (LOS)		А	А											С		
Approach Delay (s/veh)	0.6													17	7.1	
Approach LOS		A							С							

Generated: 12/12/2023 8:58:43 AM

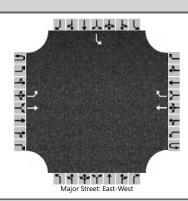
	HCS Two-Way Stop	-Control Report								
General Information		Site Information								
Analyst	CEC	Intersection	SD 38 & I-90 WB Terminal							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	I-90 WB Terminal							
Time Analyzed	AM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38 Build Option 1									



Vehicle Volumes and Ad	ustme	tments																
Approach		Eastb	ound			Westl	bound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		1	0	1		
Configuration		L	Т				Т	TR						L		R		
Volume (veh/h)	0	40	730				255	20						15		190		
Percent Heavy Vehicles (%)	3	0												56		12		
Proportion Time Blocked																		
Percent Grade (%)														(	0			
Right Turn Channelized														Ν	lo			
Median Type   Storage				Left	Only							!	9					
Critical and Follow-up H	eadwa	ys																
Base Critical Headway (sec)	1	4.1												7.5		6.9		
Critical Headway (sec)		4.10												7.92		7.14		
Base Follow-Up Headway (sec)		2.2												3.5		3.3		
Follow-Up Headway (sec)		2.20												4.06		3.42		
Delay, Queue Length, an	d Leve	l of Se	ervice	)														
Flow Rate, v (veh/h)	<u> </u>	43		П										16		207		
Capacity, c (veh/h)		1274												435		839		
v/c Ratio		0.03												0.04		0.25		
95% Queue Length, Q <sub>95</sub> (veh)		0.1												0.1		1.0		
Control Delay (s/veh)		7.9												13.6		10.7		
Level of Service (LOS)		А												В		В		
Approach Delay (s/veh)		0	.4										10.9					
Approach LOS	1	,	Ą										В					

Generated: 5/2/2024 10:10:02 AM

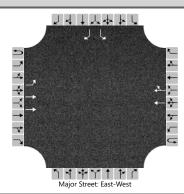
	HCS Two-Way Stop	-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & I-90 WB Terminal							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	5/8/2023	East/West Street	SD 38							
Analysis Year	2050	North/South Street	I-90 WB Terminal							
Time Analyzed	PM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38									



Vehicle Volumes and Adj	justme	nts														
Approach		Eastk	ound			Westl	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		1	0	0
Configuration		L	Т				Т	R						L		
Volume (veh/h)		25	420				415	35						30		
Percent Heavy Vehicles (%)		0												6		
Proportion Time Blocked																
Percent Grade (%)														(	0	
Right Turn Channelized						Ν	10									
Median Type   Storage				Left	Only								9			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	T	4.1												7.1		
Critical Headway (sec)		4.10												6.46		
Base Follow-Up Headway (sec)		2.2												3.5		
Follow-Up Headway (sec)		2.20												3.55		
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		27												33		
Capacity, c (veh/h)		1085												562		
v/c Ratio		0.03												0.06		
95% Queue Length, Q <sub>95</sub> (veh)		0.1												0.2		
Control Delay (s/veh)		8.4	0.2											11.8		
Level of Service (LOS)		А	А											В		
Approach Delay (s/veh)		C	0.6					-			_			11	1.8	_
Approach LOS			A											-	В	

Generated: 10/27/2023 1:49:26 PM

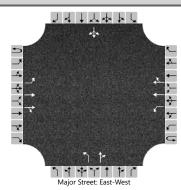
	HCS Two-Way Stop	-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & I-90 WB Terminal							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	I-90 WB Terminal							
Time Analyzed	PM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38 Build Option 1									



Vehicle Volumes and Adj	justme	nts															
Approach		Eastk	oound			West	bound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	T	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		1	0	1	
Configuration		L	Т				Т	TR						L		R	
Volume (veh/h)	0	25	420				415	35						30		495	
Percent Heavy Vehicles (%)	3	0												6		2	
Proportion Time Blocked																	
Percent Grade (%)														(	0		
Right Turn Channelized													No				
Median Type   Storage				Left	Only							9	9				
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)	$\exists$	4.1												7.5		6.9	
Critical Headway (sec)		4.10												6.92		6.94	
Base Follow-Up Headway (sec)		2.2												3.5		3.3	
Follow-Up Headway (sec)		2.20												3.56		3.32	
Delay, Queue Length, an	d Leve	l of S	ervice														
Flow Rate, v (veh/h)	<u> </u>	27												33		538	
Capacity, c (veh/h)		1085												578		756	
v/c Ratio		0.03												0.06		0.71	
95% Queue Length, Q <sub>95</sub> (veh)		0.1												0.2		6.1	
Control Delay (s/veh)		8.4												11.6		20.6	
Level of Service (LOS)		А												В		С	
Approach Delay (s/veh)		0	).5										20.1				
Approach LOS	1		A										С				

Generated: 5/2/2024 10:10:56 AM

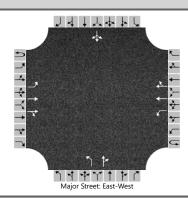
	HCS Two-Way Stop	o-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & I-90 EB Ramp Terminal							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	I-90 EB Ramp Terminal							
Time Analyzed	PM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38 Build Option 1									



	·																
Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	2	0	0	1	2	0		1	1	0		0	1	0	
Configuration		L	Т	TR		L	Т	TR		L		TR			LTR		
Volume (veh/h)	0	190	265	20	0	15	420	30		30	15	25		30	10	35	
Percent Heavy Vehicles (%)	3	10			3	11				20	20	0		8	3	3	
Proportion Time Blocked																	
Percent Grade (%)											0		0				
Right Turn Channelized																	
Median Type   Storage		Undivided															
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9	
Critical Headway (sec)		4.30				4.32				7.90	6.90	6.90		7.66	6.56	6.96	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.30				2.31				3.70	4.20	3.30		3.58	4.03	3.33	
Delay, Queue Length, an	d Leve	l of S	ervice														
Flow Rate, v (veh/h)	]	207				16				33		43			82		
Capacity, c (veh/h)		1016				1185				122		259			193		
v/c Ratio		0.20				0.01				0.27		0.17			0.42		
95% Queue Length, Q <sub>95</sub> (veh)	1	0.8				0.0				1.0		0.6			1.9		
Control Delay (s/veh)		9.4	0.6			8.1	0.1			44.7		21.7			36.6		
Level of Service (LOS)		А	А			А	Α			E		С			E		
Approach Delay (s/veh)		4.1				0.4				31.6				36.6			
Approach LOS	1	,	Ą			,	4		D				E				

Generated: 5/2/2024 10:12:29 AM

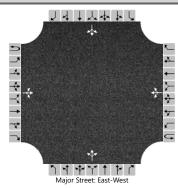
	HCS Two-Way Stop-Control Report												
General Information		Site Information											
Analyst	NM	Intersection	SD38/I-90 EB Ramp Terminal/466th St										
Agency/Co.	HRG	Jurisdiction	SDDOT										
Date Performed	4/30/2024	East/West Street	SD 38										
Analysis Year	2050	North/South Street	I-90 EB Ramp Terminal/466th Street										
Time Analyzed	AM Peak	Peak Hour Factor	0.92										
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25										
Project Description	SD 38 Build Option 1												



Vehicle Volumes and Adju	ıstme	nts															
Approach		Eastb	ound			Westk	ound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	2	0	0	1	2	0		1	1	0		0	1	0	
Configuration		L	T	TR		L	T	TR		L		TR			LTR		
Volume (veh/h)	0	430	300	15	0	20	240	20		15	10	20		3	2	28	
Percent Heavy Vehicles (%)	3	2			3	20				33	33	60		33	0	0	
Proportion Time Blocked																	
Percent Grade (%)										(	)			(	0		
Right Turn Channelized																	
Median Type   Storage				Undi	vided				<u> </u>								
<b>Critical and Follow-up He</b>	adwa	ys															
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9	
Critical Headway (sec)		4.14				4.50				8.16	7.16	8.10		8.16	6.50	6.90	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.22				2.40				3.83	4.33	3.90		3.83	4.00	3.30	
Delay, Queue Length, and	Leve	l of Se	ervice														
Flow Rate, v (veh/h)		467				22				16		33			36		
Capacity, c (veh/h)		1277				1094				40		128			231		
v/c Ratio		0.37				0.02				0.41		0.25			0.16		
95% Queue Length, Q <sub>95</sub> (veh)		1.7				0.1				1.4		1.0			0.5		
Control Delay (s/veh)		9.4	0.6			8.4	0.1			146.1		42.5			23.4		
Level of Service (LOS)	A A					А	А			F		Е			С		
Approach Delay (s/veh)		5.7 0.7								77	7.0		23.4				
Approach LOS		,	4			A	4				F		С				

Generated: 5/2/2024 10:11:43 AM

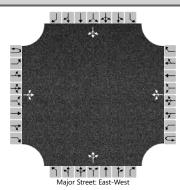
HCS Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	NM	Intersection	SD 38 & 468th Avenue						
Agency/Co.	HRG	Jurisdiction	SDDOT						
Date Performed	4/30/2024	East/West Street	SD 38						
Analysis Year	2050	North/South Street	468th Ave / County Highway 141						
Time Analyzed	AM Peak	Peak Hour Factor	0.92						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description	SD 38 Build Option 1								



					iviajo	or street. La	31-VVE31									
Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume (veh/h)		4	360	0		0	225	50		2	2	0		50	0	7
Percent Heavy Vehicles (%)		0				0				0	100	0		4	0	50
Proportion Time Blocked																
Percent Grade (%)										(	0				0	
Right Turn Channelized	1															
Median Type   Storage	1			Undi	vided											
<b>Critical and Follow-up Ho</b>	eadwa	ys														
Base Critical Headway (sec)	1	4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.10				4.10				7.10	7.50	6.20		7.14	6.50	6.70
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.20				3.50	4.90	3.30		3.54	4.00	3.75
Delay, Queue Length, and	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	<u> </u>	4				0					4				62	
Capacity, c (veh/h)	1	1274				1178					306				383	
v/c Ratio	1	0.00				0.00					0.01				0.16	
95% Queue Length, Q <sub>95</sub> (veh)	1	0.0				0.0					0.0				0.6	
Control Delay (s/veh)	1	7.8	0.0	0.0		8.1	0.0	0.0			16.9				16.2	
Level of Service (LOS)		А	А	А		А	А	Α			С				С	
Approach Delay (s/veh)		0	.1		0.0				16.9				16.2			
Approach LOS	1	,	A			,	Α			(	С				С	

Generated: 5/2/2024 10:13:15 AM

HCS Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	NM	Intersection	SD 38 & 468th Avenue						
Agency/Co.	HRG	Jurisdiction	SDDOT						
Date Performed	4/30/2024	East/West Street	SD 38						
Analysis Year	2050	North/South Street	468th Ave / County Highway 141						
Time Analyzed	PM Peak	Peak Hour Factor	0.92						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description	SD 38 Build Option 1								



Vehicle Volumes and Ad	ustme	nts														
Approach		Eastbound			Westbound				Northbound				Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	1	0
Configuration	<b>-</b>		LTR				LTR				LTR				LTR	
Volume (veh/h)	<b>1</b>	0	310	2		5	420	55		2	2	0		50	4	4
Percent Heavy Vehicles (%)	<b>-</b>	0				0				0	0	0		4	100	50
Proportion Time Blocked																
Percent Grade (%)										(	)			(	0	
Right Turn Channelized																
Median Type   Storage		Undivided														
Critical and Follow-up H	eadwa	adways														
Base Critical Headway (sec)	<u> </u>	4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)	1	4.10				4.10				7.10	6.50	6.20		7.14	7.50	6.70
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.20				3.50	4.00	3.30		3.54	4.90	3.75
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		0				5					4				63	
Capacity, c (veh/h)		1060				1231					285				283	
v/c Ratio		0.00				0.00					0.02				0.22	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.0				0.8	
Control Delay (s/veh)		8.4	0.0	0.0		7.9	0.0	0.0			17.8				21.3	
Level of Service (LOS)		А	А	А		А	Α	А			С				С	
Approach Delay (s/veh)		0	.0			0	.1	•	17.8				21.3			
Approach LOS			Ą			,	4			(	<u> </u>			(	С	

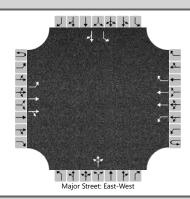
Generated: 5/2/2024 10:13:50 AM

#### **HCS Signalized Intersection Results Summary** 144444 Intersection Information **General Information** Duration, h Agency HRG 0.250 CEC Analyst Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & 469th Ave File Name (16) SD38&469 AM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 5 5 Demand (v), veh/h 5 330 75 75 165 5 110 280 15 5 **Signal Information** Cycle, s 60.0 Reference Phase 2 Offset, s 0 Reference Point End 15.7 26.1 0.0 Green 3.7 0.0 0.0 Uncoordinated No Simult. Gap E/W On Yellow 3.5 4.0 4.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 1.0 1.0 1.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL **SBT Assigned Phase** 2 6 8 1 4 Case Number 7.3 1.0 4.0 6.0 6.0 Phase Duration, s 31.1 8.2 39.3 20.7 20.7 Change Period, (Y+Rc), s 4.5 5.0 5.0 5.0 5.0 Max Allow Headway ( MAH ), s 0.0 3.9 0.0 4.1 4.1 Queue Clearance Time ( $g_s$ ), s 3.5 14.9 15.7 Green Extension Time ( $g_e$ ), s 0.0 0.1 0.0 0.0 0.0 Phase Call Probability 0.74 1.00 1.00 0.20 1.00 1.00 Max Out Probability SB **Movement Group Results** EΒ WB NB Approach Movement Т R L Т R L Т R L Т L R **Assigned Movement** 5 2 12 1 16 3 8 18 7 4 14 6 Adjusted Flow Rate ( v ), veh/h 364 82 82 93 92 120 310 16 11 1755 1490 1647 1730 1712 1426 1374 1061 1613 Adjusted Saturation Flow Rate ( s ), veh/h/ln 0.0 2.0 1.5 12.9 0.9 0.3 Queue Service Time ( $g_s$ ), s 1.5 1.5 4.1 13.7 Cycle Queue Clearance Time ( q c ), s 8.9 2.0 1.5 1.5 1.5 4.3 12.9 0.3 0.53 0.26 Green Ratio (g/C) 0.43 0.43 0.57 0.57 0.26 0.26 0.26 Capacity (c), veh/h 823 647 507 988 978 488 360 172 423 Volume-to-Capacity Ratio (X) 0.442 0.126 0.161 0.094 0.094 0.245 0.860 0.095 0.026 Back of Queue (Q), ft/ln (95 th percentile) 122.6 22.9 14.2 14.6 14.5 50.6 245.6 9.5 4.3 Back of Queue (Q), veh/ln (95 th percentile) 4.8 0.9 0.5 0.6 0.6 2.0 8.9 0.4 0.2 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.06 0.00 0.00 0.20 0.00 0.04 0.00 Uniform Delay ( d 1 ), s/veh 12.1 10.2 8.1 5.8 5.8 18.0 21.1 27.6 16.4 Incremental Delay ( d 2 ), s/veh 1.7 0.4 0.1 0.2 0.2 0.3 18.5 0.2 0.0 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 13.8 10.6 8.2 6.0 6.0 18.3 39.5 27.8 16.5 Level of Service (LOS) В В Α Α Α В D С В 13.2 В 6.7 Α 33.6 С 23.3 С Approach Delay, s/veh / LOS Intersection Delay, s/veh / LOS 19.5 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 1.89 В 1.87 В 2.10 2.10 В В Bicycle LOS Score / LOS 1.22 Α 0.71 Α 1.20 Α 0.53 Α

#### **HCS Signalized Intersection Results Summary** 144444 Intersection Information **General Information** Duration, h Agency HRG 0.250 CEC Analyst Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & 469th Ave File Name (16) SD38&469 PM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 285 5 5 Demand (v), veh/h 5 245 120 380 5 100 120 15 10 **Signal Information** 11 Cycle, s 60.0 Reference Phase 2 Offset, s 0 Reference Point End 28.0 8.5 0.0 Green 9.1 0.0 0.0 Uncoordinated No Simult. Gap E/W On Yellow 3.5 4.0 4.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 1.0 1.0 1.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL **SBT Assigned Phase** 2 6 8 1 4 Case Number 7.3 1.0 4.0 6.0 6.0 Phase Duration, s 33.0 13.6 46.5 13.5 13.5 Change Period, (Y+Rc), s 4.5 5.0 5.0 5.0 5.0 Max Allow Headway ( MAH ), s 0.0 3.9 0.0 4.1 4.1 Queue Clearance Time ( $g_s$ ), s 8.4 7.1 7.9 Green Extension Time ( $g_e$ ), s 0.0 0.7 0.0 0.5 0.6 Phase Call Probability 0.99 0.99 0.99 0.02 0.25 0.03 Max Out Probability SB **Movement Group Results** ΕB WB NB Approach Movement L Т R L Т R L Т R Т L R **Assigned Movement** 5 2 12 16 3 8 18 7 4 14 1 6 Adjusted Flow Rate ( v ), veh/h 272 130 310 210 209 109 136 16 16 1752 1490 1647 1730 1722 1397 1499 1244 1570 Adjusted Saturation Flow Rate ( s ), veh/h/ln 0.0 3.1 6.4 2.6 2.6 5.1 8.0 0.5 Queue Service Time ( $g_s$ ), s 4.4 2.6 Cycle Queue Clearance Time ( g c ), s 5.9 3.1 6.4 2.6 4.9 5.1 5.9 0.5 Green Ratio (g/C) 0.47 0.47 0.65 0.69 0.69 0.14 0.14 0.14 0.14 1196 Capacity (c), veh/h 877 693 369 1191 305 212 190 222 Volume-to-Capacity Ratio (X) 0.310 0.188 0.839 0.175 0.175 0.356 0.640 0.086 0.073 Back of Queue (Q), ft/ln (95 th percentile) 75.7 34.4 81.4 12.3 12.3 58.9 79.5 9.4 8.2 Back of Queue (Q), veh/ln (95 th percentile) 3.0 1.3 3.1 0.5 0.5 2.3 3.1 0.4 0.3 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.33 0.00 0.00 0.24 0.00 0.04 0.00 10.1 Uniform Delay ( d 1 ), s/veh 9.4 14.9 3.2 3.2 24.5 24.3 27.1 22.3 Incremental Delay ( d 2 ), s/veh 0.9 0.6 5.1 0.3 0.3 0.7 3.2 0.2 0.1 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 11.1 10.0 20.1 3.6 3.6 25.2 27.5 27.3 22.5 Level of Service (LOS) В Α С Α Α С С С С 10.7 В 10.6 В 26.5 С 24.9 С Approach Delay, s/veh / LOS Intersection Delay, s/veh / LOS 13.7 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 1.88 В В 1.84 2.11 В 2.11 В Bicycle LOS Score / LOS 1.15 Α 1.09 Α 0.89 Α 0.54

Generated: 5/7/2024 2:10:09 PM

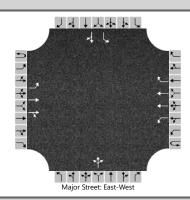
HCS Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & La Mesa							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/29/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	La Mesa							
Time Analyzed	AM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38 Build Option 1									



Approach	1	Fasth	ound			Westk	oound			North	hound			South	hound	
	<b>+</b>	Lasti				VVESII			U	NOITH						
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	1		0	1	0		1	1	0
Configuration		L	T	TR		L	Т	R			LTR			L		TR
Volume (veh/h)	0	30	700	4	0	0	235	15		0	15	5		75	4	30
Percent Heavy Vehicles (%)	3	0			3	0				0	13	0		0	50	0
Proportion Time Blocked																
Percent Grade (%)										(	)			(	)	
Right Turn Channelized						N	lo									
Median Type   Storage		Undivided														
Critical and Follow-up H	eadwa	dways														
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)	]	4.10				4.10				7.50	6.76	6.90		7.50	7.50	6.90
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.20				3.50	4.13	3.30		3.50	4.50	3.30
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	<u> </u>	33				0					22			82		37
Capacity, c (veh/h)		1303				857					229			287		566
v/c Ratio		0.03				0.00					0.09			0.28		0.07
95% Queue Length, Q <sub>95</sub> (veh)		0.1				0.0					0.3			1.1		0.2
Control Delay (s/veh)		7.8	0.2			9.2	0.0				22.3			22.5		11.8
Level of Service (LOS)		А	А			А	А				С			С		В
Approach Delay (s/veh)		0	.5			0	.0		22.3				19.2			
Approach LOS	1	,	4				Α			(	<u> </u>		С			

Generated: 5/2/2024 10:17:45 AM

HCS Two-Way Stop-Control Report									
General Information Site Information									
Analyst	NM	Intersection	SD 38 & La Mesa						
Agency/Co.	HRG	Jurisdiction	SDDOT						
Date Performed	4/29/2024	East/West Street	SD 38						
Analysis Year	2050	North/South Street	La Mesa						
Time Analyzed	PM Peak	Peak Hour Factor	0.92						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description	SD 38 Option 1								



Approach	1	Eacth	ound			Mostk	oound			North	hound			South	hound	
		Easil				vvesti				NOILII						
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	1		0	1	0		1	1	0
Configuration	1	L	Т	TR		L	Т	R			LTR			L		TR
Volume (veh/h)	0	25	325	0	0	9	735	100		4	5	0		80	15	30
Percent Heavy Vehicles (%)	3	0			3	0				0	0	0		9	0	0
Proportion Time Blocked	1															
Percent Grade (%)	1									(	)			(	)	
Right Turn Channelized						N	lo									
Median Type   Storage		Undivided														
Critical and Follow-up H	eadwa	idways														
Base Critical Headway (sec)	1	4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)		4.10				4.10				7.50	6.50	6.90		7.68	6.50	6.90
Base Follow-Up Headway (sec)	1	2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.20				3.50	4.00	3.30		3.59	4.00	3.30
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	<u> </u>	27				10					10			87		49
Capacity, c (veh/h)	1	758				1217					167			158		329
v/c Ratio	1	0.04				0.01					0.06			0.55		0.15
95% Queue Length, Q <sub>95</sub> (veh)	1	0.1				0.0					0.2			2.8		0.5
Control Delay (s/veh)	1	9.9	0.3			8.0	0.1				27.9			52.4		17.8
Level of Service (LOS)	1	А	А			А	А				D			F		С
Approach Delay (s/veh)		1	.0		0.1				27.9				40.0			
Approach LOS	1	,	4				4		D				E			

Generated: 5/2/2024 10:18:20 AM

#### **HCS Signalized Intersection Results Summary** Intersection Information **General Information** HRG Duration, h 0.250 Agency Analyst NM Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & Marion Street File Name (18) SD38&Marion AM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 40 Demand (v), veh/h 165 340 105 50 125 75 110 225 120 45 145 **Signal Information** J. Cycle, s 50.0 Reference Phase 2 Offset, s 0 Reference Point End Green 2.6 3.9 12.9 2.5 2.0 10.0 Uncoordinated No Simult. Gap E/W On Yellow 4.0 0.0 4.0 4.0 0.0 4.0 Force Mode Fixed Simult. Gap N/S On Red 0.0 0.0 0.0 0.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL SBT **Assigned Phase** 5 2 6 3 8 1 7 4 Case Number 2.0 3.0 1.1 4.0 2.0 3.0 2.0 3.0 Phase Duration, s 10.6 20.8 6.6 16.9 8.5 16.0 6.5 14.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Change Period, (Y+Rc), s Max Allow Headway ( MAH ), s 2.9 0.0 2.9 0.0 2.9 2.9 2.9 2.9 Queue Clearance Time ( $g_s$ ), s 7.1 3.1 5.6 8.5 3.5 5.9 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.0 0.0 0.0 0.6 0.0 0.6 Phase Call Probability 0.92 0.53 0.81 1.00 0.49 1.00 1.00 0.04 0.21 1.00 Max Out Probability 1.00 0.15 **Movement Group Results** EΒ WB NB SB Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 179 370 114 54 112 106 120 245 130 49 158 43 Adjusted Saturation Flow Rate ( s ), veh/h/ln 1701 1674 1525 1714 1772 1556 1647 1674 1502 1554 1758 1466 5.1 4.1 2.7 1.1 2.5 2.7 3.6 6.5 3.9 1.2 Queue Service Time ( $g_s$ ), s 3.6 1.5 2.5 Cycle Queue Clearance Time ( q c ), s 5.1 4.1 2.7 1.1 2.7 3.6 6.5 3.6 1.5 3.9 1.2 0.34 0.26 0.24 0.24 0.20 Green Ratio (g/C) 0.13 0.34 0.31 0.26 0.09 0.05 0.20 403 Capacity (c), veh/h 223 1128 514 456 459 403 148 361 77 352 293 Volume-to-Capacity Ratio (X) 0.804 0.328 0.222 0.119 0.243 0.263 0.806 0.607 0.361 0.638 0.448 0.148 Back of Queue (Q), ft/ln (95 th percentile) Back of Queue (Q), veh/ln (95 th percentile) 4.1 2.2 1.4 0.6 1.6 1.6 3.2 3.5 1.7 1.0 2.3 0.6 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 14.7 Uniform Delay ( d 1 ), s/veh 21.1 12.4 11.9 12.3 14.7 22.3 16.9 15.8 23.3 17.6 16.5 Incremental Delay ( d 2 ), s/veh 11.0 8.0 1.0 0.0 1.3 1.6 15.9 8.0 0.2 3.3 0.3 0.1 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 32.1 13.1 12.9 12.3 15.9 16.3 38.2 17.7 16.0 26.6 17.9 16.6 Level of Service (LOS) С В В В В В D В В С В В 18.2 В 15.4 В 22.2 С 19.4 Approach Delay, s/veh / LOS В Intersection Delay, s/veh / LOS 19.1 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 2.08 В 2.09 В 2.26 2.42 В В Bicycle LOS Score / LOS 1.03 Α 0.71 Α 1.30 Α 0.90 Α

#### **HCS Signalized Intersection Results Summary** Intersection Information **General Information** HRG Duration, h 0.250 Agency Analyst NM Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period PM Peak 0.90 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 16:45 SD 38 & Marion Street File Name (18) SD38&Marion PM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 55 Demand (v), veh/h 70 230 105 170 355 180 205 125 85 355 205 **Signal Information** Cycle, s 60.0 Reference Phase 2 Offset, s 0 Reference Point End Green 3.7 0.4 13.3 4.2 2.8 15.5 Uncoordinated No Simult. Gap E/W On Yellow 4.0 4.0 4.0 4.0 0.0 4.0 Force Mode Fixed Simult. Gap N/S On Red 0.0 0.0 0.0 0.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL SBT **Assigned Phase** 5 2 6 3 8 1 7 4 Case Number 2.0 3.0 2.0 3.0 2.0 3.0 2.0 3.0 Phase Duration, s 7.7 17.3 12.2 21.7 11.0 22.3 8.2 19.5 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Change Period, (Y+Rc), s Max Allow Headway ( MAH ), s 2.9 0.0 2.9 0.0 2.9 3.0 2.9 3.0 Queue Clearance Time ( $g_s$ ), s 5.1 8.5 9.0 8.1 5.3 14.7 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.0 0.0 0.0 1.6 0.0 8.0 Phase Call Probability 0.73 0.96 0.96 1.00 0.79 1.00 0.55 0.03 1.00 Max Out Probability 1.00 1.00 0.89 **Movement Group Results** EΒ WB NB SB Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 78 256 117 189 394 61 200 228 139 94 394 228 Adjusted Saturation Flow Rate ( s ), veh/h/ln 1474 1660 1490 1688 1772 1714 1772 1478 1688 1772 1478 1406 3.1 3.9 6.5 12.1 7.0 6.1 4.3 8.1 Queue Service Time ( $g_s$ ), s 4.0 1.9 3.3 12.7 Cycle Queue Clearance Time ( q c ), s 3.1 3.9 4.0 6.5 12.1 1.9 7.0 6.1 4.3 3.3 12.7 8.1 0.22 0.22 0.30 Green Ratio (g/C) 0.06 0.14 0.30 0.12 0.31 0.31 0.07 0.26 0.26 Capacity (c), veh/h 92 735 330 230 523 415 200 541 451 119 459 383 Volume-to-Capacity Ratio (X) 0.845 0.347 0.354 0.822 0.754 0.147 1.000 0.421 0.308 0.797 0.859 0.595 Back of Queue (Q), ft/ln (95 th percentile) Back of Queue (Q), veh/ln (95 th percentile) 2.1 2.5 2.5 6.1 9.1 1.0 10.0 3.6 2.1 2.7 9.5 4.2 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 Uniform Delay ( d 1 ), s/veh 27.8 19.7 19.7 25.2 19.2 15.6 26.5 16.6 16.0 27.5 21.2 19.5 Incremental Delay ( d 2 ), s/veh 7.7 1.3 3.0 18.3 9.7 0.7 63.6 0.2 0.1 10.2 10.8 0.7 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 35.6 21.0 22.7 43.5 28.9 16.3 90.1 16.8 16.1 37.7 31.9 20.2 Level of Service (LOS) D С С D С В F В В D С С 23.9 С 32.0 С 42.5 D 29.0 С Approach Delay, s/veh / LOS Intersection Delay, s/veh / LOS 32.1 С **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 2.10 В 2.10 В 2.26 2.27 В В Bicycle LOS Score / LOS 0.86 Α 1.55 1.42 Α 1.67

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2023
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD 38 Build	Time Analyzed	AM
Project Description	464th_MickelsonRd_2050_ AM	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	55.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	55.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	638	Heavy Vehicle Adjustment Factor (fHV)	0.980
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	370
Total Trucks, %	2.00	Capacity (c), pc/h/ln	2100
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2100
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.18
Direction 1 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	55.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	6.7
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS		ı	
Flow Rate in Outside Lane (vOL), veh/h	362	Effective Speed Factor (St)	4.62
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.66
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	C

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	55.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	8.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	53.0	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	380	Heavy Vehicle Adjustment Factor (fHV)	0.885
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	244
Total Trucks, %	13.00	Capacity (c), pc/h/ln	2060
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2060
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	53.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.6
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	2.0		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	216	Effective Speed Factor (St)	4.62
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	6.14
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

HCS弧 Highways Version 2023 464th\_MickelsonRd\_2050\_AM.xuf Generated: 02/27/2024 14:01:35

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2023
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	PM
Project Description	464th_MickelsonRd_PM	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	55.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	55.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	tors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Ca	pacity		
Volume (V) veh/h	441	Heavy Vehicle Adjustment Factor (fHV)	0.943
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	266
Total Trucks, %	6.00	Capacity (c), pc/h/ln	2100
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2100
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.13
Direction 1 Speed and Dens	ity	<u> </u>	
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	55.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.8
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS		•	•
Flow Rate in Outside Lane (vOL), veh/h	251	Effective Speed Factor (St)	4.62
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.56
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	D

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	55.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	8.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	53.0	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	730	Heavy Vehicle Adjustment Factor (fHV)	0.990
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	419
Total Trucks, %	1.00	Capacity (c), pc/h/ln	2060
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2060
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.20
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	53.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	7.9
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	2.0		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	415	Effective Speed Factor (St)	4.62
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.50
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	В

HCSTM Highways Version 2023 464th\_MickelsonRd\_2050\_PM.xuf Generated: 02/27/2024 13:44:53

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR GREEN INC	Analysis Year	2050
Jurisdiction	SD 38	Time Analyzed	PM
Project Description	2050 Build Analysis - 468th St to 469th St	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	1.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.8	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	410	Heavy Vehicle Adjustment Factor (fHV)	0.952
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	244
Total Trucks, %	5.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.11
Direction 1 Speed and Densi	ty	•	
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.8
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	3.5
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.3		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	233	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.42
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	5.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	68.8	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	280	Heavy Vehicle Adjustment Factor (fHV)	0.862
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	184
Total Trucks, %	16.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.08
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	68.8
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	2.7
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	1.3		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	159	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	8.07
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

HCSTM Highways Version 2023 468th\_469th\_Multilane\_2050\_AM.xuf Generated: 02/27/2024 11:45:56

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR GREEN INC	Analysis Year	2050
Jurisdiction	SD 38	Time Analyzed	PM
Project Description	2050 Build Analysis - 468th St to 469th St	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	1.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.8	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	370	Heavy Vehicle Adjustment Factor (fHV)	0.935
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	225
Total Trucks, %	7.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.10
Direction 1 Speed and Densi	ty	•	
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.8
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	3.2
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.3		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	210	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	4.05
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	D

Direction 2 Geometric Data				
Direction 2	WB			
Number of Lanes (N), In	2	Terrain Type	Level	
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-	
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-	
Lane Width, ft	12	Access Point Density, pts/mi	5.0	
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6	
Free-Flow Speed (FFS), mi/h	68.8	Total Lateral Clearance (TLC), ft	12	
Direction 2 Adjustment Fact	ors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000	
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000	
Driver Population CAF	1.000			
Direction 2 Demand and Cap	pacity			
Volume (V) veh/h	490	Heavy Vehicle Adjustment Factor (fHV)	0.980	
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	284	
Total Trucks, %	2.00	Capacity (c), pc/h/ln	2300	
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300	
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12	
Direction 2 Speed and Densi	ty			
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	68.8	
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.1	
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А	
Access Point Density Adjustment (fA)	1.3			
Direction 2 Bicycle LOS				
Flow Rate in Outside Lane (vOL), veh/h	278	Effective Speed Factor (St)	5.07	
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.65	
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С	

HCSTM Highways Version 2023 468th\_469th\_Multilane\_2050\_PM.xuf Generated: 02/27/2024 11:46:39

	HCS Multilane	e Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	AM
Project Description	469th to LaMesa	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	ЕВ		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	2.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Ca	pacity		
Volume (V) veh/h	610	Heavy Vehicle Adjustment Factor (fHV)	0.962
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	360
Total Trucks, %	4.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.16
Direction 1 Speed and Dens	ity		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.5
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	5.2
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.5		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	347	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.32
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

Direction 2 Geometric Data				
Direction 2	WB			
Number of Lanes (N), In	2	Terrain Type	Level	
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-	
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-	
Lane Width, ft	12	Access Point Density, pts/mi	1.0	
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6	
Free-Flow Speed (FFS), mi/h	69.8	Total Lateral Clearance (TLC), ft	12	
Direction 2 Adjustment Factor	ors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000	
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000	
Driver Population CAF	1.000			
Direction 2 Demand and Cap	acity			
Volume (V) veh/h	240	Heavy Vehicle Adjustment Factor (fHV)	0.820	
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	166	
Total Trucks, %	22.00	Capacity (c), pc/h/ln	2300	
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300	
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.07	
Direction 2 Speed and Densi	ty			
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.8	
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	2.4	
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А	
Access Point Density Adjustment (fA)	0.3			
Direction 2 Bicycle LOS				
Flow Rate in Outside Lane (vol.), veh/h	136	Effective Speed Factor (St)	5.07	
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	11.74	
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F	
Convigant © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	nys Varsian 2022	Generated: 05/01/2024 15:04:26	

HCSTM Highways Version 2023 469th\_LaMesaDr\_2050\_AM.xuf Generated: 05/01/2024 15:04:26

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	PM
Project Description	469th to LaMesa	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	2.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	351	Heavy Vehicle Adjustment Factor (fHV)	0.917
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	218
Total Trucks, %	9.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.09
Direction 1 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.5
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	3.1
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.5		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	199	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	4.80
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	E

Direction 2 Geometric Data				
Direction 2	WB			
Number of Lanes (N), In	2	Terrain Type	Level	
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-	
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-	
Lane Width, ft	12	Access Point Density, pts/mi	1.0	
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6	
Free-Flow Speed (FFS), mi/h	69.8	Total Lateral Clearance (TLC), ft	12	
Direction 2 Adjustment Factor	ors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000	
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000	
Driver Population CAF	1.000			
Direction 2 Demand and Cap	acity			
Volume (V) veh/h	666	Heavy Vehicle Adjustment Factor (fHV)	0.971	
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	390	
Total Trucks, %	3.00	Capacity (c), pc/h/ln	2300	
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300	
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.17	
Direction 2 Speed and Densi	ty			
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.8	
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	5.6	
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А	
Access Point Density Adjustment (fA)	0.3			
Direction 2 Bicycle LOS				
Flow Rate in Outside Lane (vol.), veh/h	378	Effective Speed Factor (St)	5.07	
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.07	
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С	

HCSTM Highways Version 2023 469th\_LaMesaDr\_2050\_PM.xuf Generated: 02/27/2024 11:50:51

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR GREEN INC	Analysis Year	2050
Jurisdiction	SD 38	Time Analyzed	AM
Project Description	2050 Build Analysis - 466th Avenue S/EB Exit Ramp to 468th St	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	4.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	acity		
Volume (V) veh/h	364	Heavy Vehicle Adjustment Factor (fHV)	0.935
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	221
Total Trucks, %	7.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.10
Direction 1 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	3.2
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	1.0		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	207	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	4.05
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	D

Direction 2 Geometric Data				
Direction 2	WB			
Number of Lanes (N), In	2	Terrain Type	Level	
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-	
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-	
Lane Width, ft	12	Access Point Density, pts/mi	2.0	
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6	
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12	
Direction 2 Adjustment Factor	ors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000	
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000	
Driver Population CAF	1.000			
Direction 2 Demand and Cap	acity			
Volume (V) veh/h	260	Heavy Vehicle Adjustment Factor (fHV)	0.901	
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	164	
Total Trucks, %	11.00	Capacity (c), pc/h/ln	2300	
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300	
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.07	
Direction 2 Speed and Densi	ty			
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.5	
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	2.4	
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А	
Access Point Density Adjustment (fA)	0.5			
Direction 2 Bicycle LOS				
Flow Rate in Outside Lane (vol.), veh/h	148	Effective Speed Factor (St)	5.07	
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	5.50	
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	Е	
Converight © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	avs Varsian 2023	Generated: 02/27/2024 11:43:01	

HCS MM Highways Version 2023
I90EBRampS466thSt\_468th\_Multilane\_2050\_AM.xuf

Generated: 02/27/2024 11:43:01

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR GREEN INC	Analysis Year	2050
Jurisdiction	SD 38	Time Analyzed	PM
Project Description	2050 Build Analysis - 466th Avenue S/EB Exit Ramp to 468th St	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	ЕВ		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	4.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Facto	rs		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Capa	acity		
Volume (V) veh/h	312	Heavy Vehicle Adjustment Factor (fHV)	0.909
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	195
Total Trucks, %	10.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.08
Direction 1 Speed and Density	у		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	2.8
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	1.0		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vol), veh/h	177	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	5.16
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	E

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	2.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	acity		
Volume (V) veh/h	420	Heavy Vehicle Adjustment Factor (fHV)	0.935
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	255
Total Trucks, %	7.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.11
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.5
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	3.7
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.5		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	239	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	4.12
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	D
Convigant © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	avs Varsian 2023	Generated: 02/27/2024 11:44:0

HCSTM Highways Version 2023 I90EBRampS466thSt\_468th\_Multilane\_2050\_PM.xuf Generated: 02/27/2024 11:44:01

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	AM
Project Description	I90 WB Ramps to I90 EB Ramps	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	745	Heavy Vehicle Adjustment Factor (fHV)	0.971
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	436
Total Trucks, %	3.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.19
Direction 1 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	6.2
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	423	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.13
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	C C

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	273	Heavy Vehicle Adjustment Factor (fHV)	0.877
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	177
Total Trucks, %	14.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.08
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	2.5
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.0		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	155	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	6.98
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

HCSໝ Highways Version 2023 I90WBRamp\_I90EBRamp\_2050\_AM.xuf Generated: 02/27/2024 13:57:02

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	PM
Project Description	I90 WB Ramps to I90 EB Ramps	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	451	Heavy Vehicle Adjustment Factor (fHV)	0.917
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	280
Total Trucks, %	9.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12
Direction 1 Speed and Densi	ity	<u>'</u>	
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.0
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS			•
Flow Rate in Outside Lane (vol.), veh/h	256	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	4.92
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	E

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	455	Heavy Vehicle Adjustment Factor (fHV)	0.877
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	295
Total Trucks, %	14.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.13
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.2
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.0		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	259	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	7.24
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

HCSTM Highways Version 2023 I90WBRamp\_I90EBRamp\_2050\_PM.xuf Generated: 02/27/2024 13:57:38

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	AM
Project Description	Mickelson Rd to 466th St	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	1.5
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.6	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity	•	
Volume (V) veh/h	725	Heavy Vehicle Adjustment Factor (fHV)	0.990
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	416
Total Trucks, %	1.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.18
Direction 1 Speed and Densi	ty		<u>'</u>
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.6
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	6.0
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.4		
Direction 1 Bicycle LOS			,
Flow Rate in Outside Lane (vOL), veh/h	412	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.61
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	2.2
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	425	Heavy Vehicle Adjustment Factor (fHV)	0.885
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	273
Total Trucks, %	13.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.4
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	3.9
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.6		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	241	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	6.70
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

HCSTM Highways Version 2023 Mickelson\_466th\_2050\_AM.xuf Generated: 02/27/2024 13:52:45

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	PM
Project Description	Mickelson Rd to 466th St	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	1.5
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.6	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Ca	pacity		
Volume (V) veh/h	445	Heavy Vehicle Adjustment Factor (fHV)	0.901
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	280
Total Trucks, %	11.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12
Direction 1 Speed and Dens	ity		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.6
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.0
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.4		
Direction 1 Bicycle LOS		•	
Flow Rate in Outside Lane (vOL), veh/h	253	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	5.78
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	2.2
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	acity		
Volume (V) veh/h	913	Heavy Vehicle Adjustment Factor (fHV)	0.980
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	530
Total Trucks, %	2.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.23
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.4
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	7.6
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.6		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	519	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.97
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С
Convigant © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	avs Varsian 2023	Generated: 02/27/2024 12:52:26

HCSTM Highways Version 2023 Mickelson\_466th\_2050\_PM.xuf Generated: 02/27/2024 13:53:36

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	AM
Project Description	466th St to I90 WB Ramps	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Ca	pacity		
Volume (V) veh/h	769	Heavy Vehicle Adjustment Factor (fHV)	0.980
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	446
Total Trucks, %	2.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.19
Direction 1 Speed and Dens	ity		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	6.4
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS			•
Flow Rate in Outside Lane (vol.), veh/h	437	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.88
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	436	Heavy Vehicle Adjustment Factor (fHV)	0.833
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	298
Total Trucks, %	20.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.13
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.3
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.0		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	248	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	10.71
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

HCSTM Highways Version 2023 466th\_I90WBRamp\_2050\_AM.xuf Generated: 02/27/2024 13:54:45

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	PM
Project Description	466th St to I90 WB Ramps	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Ca	pacity		
Volume (V) veh/h	450	Heavy Vehicle Adjustment Factor (fHV)	0.917
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	279
Total Trucks, %	9.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12
Direction 1 Speed and Dens	ity		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.0
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS			•
Flow Rate in Outside Lane (vol.), veh/h	256	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	4.92
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	E

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	TWLTL	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	acity		
Volume (V) veh/h	910	Heavy Vehicle Adjustment Factor (fHV)	0.971
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	532
Total Trucks, %	3.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.23
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	7.6
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.0		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	517	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.23
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С
Converight © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	nys Varsian 2022	Congrated: 02/27/2024 13:55:36

HCSTM Highways Version 2023 466th\_I90WBRamp\_2050\_PM.xuf Generated: 02/27/2024 13:55:36

		HCS Two-La	ne	Highway Re	port	
Pro	oject Information		_			
Ana	lyst	MJV		Date		5/11/2023
Age	ncy	HRG		Analysis Year		2050 NB
Juri	sdiction	SDDOT		Time Analyzed		AM Peak
Proj	ect Description	West of Hartford SD 3	88 EB	Units		U.S. Customary
		S	egn	nent 1		·
Ve	hicle Inputs					
Seg	ment Type	Passing Zone		Length, ft		1069
Mea	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity					
Dire	ectional Demand Flow Rate, veh/h	243		Opposing Deman	d Flow Rate, veh/h	169
Pea	k Hour Factor	0.88		Total Trucks, %		5.79
Seg	ment Capacity, veh/h	1700		Demand/Capacity	, (D/C)	0.14
Int	ermediate Results	·		'		
Seg	ment Vertical Class	1		Free-Flow Speed, mi/h		70.0
Spe	ed Slope Coefficient (m)	4.30713		Speed Power Coe	fficient (p)	0.54838
PF S	Slope Coefficient (m)	-1.23090	1.23090		ent (p)	0.80942
In P	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.2
%In	nprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Su	bsegment Data					
#	Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	1069	1-		-	68.5
Ve	hicle Results					
Ave	rage Speed, mi/h	68.5		Percent Followers	, %	32.4
	ment Travel Time, minutes	0.18		Follower Density (FD), followers/mi/ln		1.2
Veh	icle LOS	A				
Bio	cycle Results					
	cent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h		243		Bicycle Effective V	Vidth, ft	24
Bicycle LOS Score 3.70		3.70		Bicycle Effective S		5.07
	rcle LOS	D				
		S	egn	nent 2		
Ve	hicle Inputs					
	ment Type	Passing Constrained		Length, ft		664
	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
ivieasureu 113		70.0				

Demand and Capacity					
	242		Opposite Desir	d Flour Data and the	
Directional Demand Flow Rate, veh/h	243		Opposing Demand Flow Rate, veh/h Total Trucks, %		
Peak Hour Factor				(D/C)	5.79
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29315		PF Power Coefficie	ent (p)	0.75829
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.3
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Radi	ius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	664	-		-	68.0
Vehicle Results					
Average Speed, mi/h	68.0		Percent Followers	, %	35.8
Segment Travel Time, minutes	0.11		Follower Density (FD), followers/mi/ln		1.3
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	243		Bicycle Effective V	Vidth, ft	24
Bicycle LOS Score	3.70		Bicycle Effective Speed Factor		5.07
Bicycle LOS	D				
	S	egm	nent 3		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		1871
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	243		Opposing Deman	d Flow Rate, veh/h	169
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity	' (D/C)	0.14
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.31694		Speed Power Coe	fficient (p)	0.54838
PF Slope Coefficient (m)	-1.20586		PF Power Coefficie	ent (p)	0.82063
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.1
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					

#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h	
1	Tangent	1871	-		-	68.5	
Veł	nicle Results	-					
Aver	age Speed, mi/h	68.5		Percent Followers	, %	31.5	
Segr	ment Travel Time, minutes	0.31		Follower Density (	(FD), followers/mi/ln	1.1	
Vehi	cle LOS	A					
Bic	ycle Results						
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4	
Flow	Rate Outside Lane, veh/h	243		Bicycle Effective V	Vidth, ft	24	
Bicy	cle LOS Score	3.70		Bicycle Effective S	peed Factor	5.07	
Bicy	cle LOS	D					
			Segr	ment 4			
Veł	nicle Inputs						
Segr	ment Type	Passing Constrai	ned	Length, ft		925	
Mea	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0	
Dei	mand and Capacity	•					
Dire	ctional Demand Flow Rate, veh/h	243	243		d Flow Rate, veh/h	-	
Peak	Hour Factor	0.88	0.88			5.79	
Segr	nent Capacity, veh/h	1700	Demand/Capacity (D/C)		, (D/C)	0.14	
Int	ermediate Results						
Segr	ment Vertical Class	1		Free-Flow Speed, mi/h		70.0	
Spe	ed Slope Coefficient (m)	4.57372	4.57372		fficient (p)	0.41674	
PF S	lope Coefficient (m)	-1.29315	-1.29315		ent (p)	0.75829	
In Pa	assing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		1.3	
%lm	provement to Percent Followers	0.0		%Improvement to Speed		0.0	
Suk	osegment Data						
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h	
1	Tangent	925	-		-	68.0	
Vel	nicle Results						
Aver	rage Speed, mi/h	68.0		Percent Followers	, %	35.8	
3 1		0.15		Follower Density (	(FD), followers/mi/ln	1.3	
Vehicle LOS A		А					
Bic	ycle Results						
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4	
Flow	Rate Outside Lane, veh/h	243		Bicycle Effective V	Vidth, ft	24	
Bicy	cle LOS Score	3.70		Bicycle Effective S	peed Factor	5.07	
Bicy	cycle LOS D						

			Seg	ment 5		
Ve	hicle Inputs					
Seg	ment Type	Passing Zone		Length, ft		4476
Mea	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity					
Dire	ectional Demand Flow Rate, veh/h	243		Opposing Demand	d Flow Rate, veh/h	169
Pea	k Hour Factor	0.88		Total Trucks, %		5.79
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
Int	termediate Results					
Seg	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	ed Slope Coefficient (m)	4.35043		Speed Power Coef	fficient (p)	0.54838
PF S	Slope Coefficient (m)	-1.15155		PF Power Coefficie	ent (p)	0.84082
In P	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.1
%In	nprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Su	bsegment Data					
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	4476	1-		-	68.5
Ve	hicle Results					
Ave	rage Speed, mi/h	68.5		Percent Followers,	. %	29.6
Seg	ment Travel Time, minutes	0.74		Follower Density (FD), followers/mi/ln		1.1
Veh	icle LOS	А				
Bio	cycle Results					·
Per	cent Occupied Parking	0		Pavement Condition	on Rating	4
Flov	w Rate Outside Lane, veh/h	243		Bicycle Effective Width, ft		24
Bicy	vcle LOS Score	3.70		Bicycle Effective S	peed Factor	5.07
Bicy	rcle LOS	D				
			Seg	ment 6		
Ve	hicle Inputs					
	ment Type	Passing Constrained		Length, ft		896
Mea	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity					
	ectional Demand Flow Rate, veh/h	243		Opposing Demand	d Flow Rate, veh/h	-
Pea	k Hour Factor	0.88		Total Trucks, %		5.79
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
Int	termediate Results					
Sea	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
				1		

Speed Slope Coefficient (m)	4.57372			fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29315	-1.29315		ent (p)	0.75829
In Passing Lane Effective Length?	No	No		nsity, veh/mi/ln	1.3
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	896	-		-	68.0
Vehicle Results					
Average Speed, mi/h	68.0		Percent Followers	, %	35.8
Segment Travel Time, minutes	0.15		Follower Density (	FD), followers/mi/ln	1.3
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	243		Bicycle Effective V	Vidth, ft	24
Bicycle LOS Score	3.70		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	D				
		Segn	nent 7		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		743
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	243		Opposing Demand Flow Rate, veh/h		169
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.14
Intermediate Results					·
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.30713		Speed Power Coe	fficient (p)	0.54838
PF Slope Coefficient (m)	-1.23090		PF Power Coefficie	ent (p)	0.80942
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.2
%Improvement to Percent Followers	0.0		%Improvement to Speed		0.0
Subsegment Data					
# Segment Type	Length, ft	Length, ft Radii		Superelevation, %	Average Speed, mi/h
1 Tangent	743 -			-	68.5
Vehicle Results	<u>'</u>				
Average Speed, mi/h	68.5		Percent Followers, %		32.4
Segment Travel Time, minutes 0.12		Follower Density (FD), followers/mi/ln		1.2	
Vehicle LOS A		2 2 2 3 4 2 7 1 2 7 3 1 3 1 3 1 3 1 3 1 3 1 3 1 3 1 3 1 3			

Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	243		Bicycle Effective W	/idth, ft	24
Bicycle LOS Score	3.70		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	D				
	Se	egn	nent 8		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		2717
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	245		Opposing Demand	d Flow Rate, veh/h	165
Peak Hour Factor	0.88		Total Trucks, %		3.28
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.32768		Speed Power Coefficient (p)		0.54983
PF Slope Coefficient (m)	-1.17918		PF Power Coefficient (p)		0.83165
In Passing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		1.1
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rad	lius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	2717	-		-	68.5
Vehicle Results					
Average Speed, mi/h	68.5		Percent Followers,	%	30.7
Segment Travel Time, minutes	0.45		Follower Density (FD), followers/mi/ln		1.1
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	245		Bicycle Effective W	/idth, ft	24
Bicycle LOS Score	2.93		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	С				
	Se	egn	nent 9		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		1013
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					

Directional Demand Flow Rate, veh/h	245		Opposing Demand	d Flow Rate, yeh/h	-
Peak Hour Factor	0.88			THOW Nate, Vell/II	3.28
Segment Capacity, veh/h			Total Trucks, %  Demand/Capacity (D/C)		0.14
	1700		Беппапа/Сарасіту	(6/0)	0.14
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,		70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coef	ficient (p)	0.41674
PF Slope Coefficient (m)	-1.29345		PF Power Coefficie	nt (p)	0.75792
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.3
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Radi	ius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	1013	-		-	68.0
Vehicle Results					
Average Speed, mi/h	68.0		Percent Followers,	%	36.0
Segment Travel Time, minutes	0.17		Follower Density (	FD), followers/mi/ln	1.3
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition Rating		4
Flow Rate Outside Lane, veh/h	245		Bicycle Effective W	idth, ft	24
Bicycle LOS Score	2.93		Bicycle Effective Speed Factor		5.07
Bicycle LOS	С				
	Se	gm	ent 10		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		4569
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	245		Opposing Demand	d Flow Rate, veh/h	165
Peak Hour Factor	0.88		Total Trucks, %		3.28
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
Intermediate Results					
Segment Vertical Class	1	1		mi/h	70.0
Speed Slope Coefficient (m)	4.34958		Speed Power Coef	ficient (p)	0.54983
PF Slope Coefficient (m)	-1.14981		PF Power Coefficie	ent (p)	0.84100
In Passing Lane Effective Length?	ength? No		Total Segment De	nsity, veh/mi/ln	1.1
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Radi	ius, ft	Superelevation, %	Average Speed, mi/h

1 Tangent	4569	-		-	68.5
Vehicle Results					
Average Speed, mi/h	68.5		Percent Followers,	. %	29.7
Segment Travel Time, minutes	0.76		Follower Density (	FD), followers/mi/ln	1.1
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	245		Bicycle Effective W	/idth, ft	24
Bicycle LOS Score	2.93		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	С				
	\$	Segn	nent 11		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		5676
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	244		Opposing Deman	d Flow Rate, veh/h	165
Peak Hour Factor	0.88		Total Trucks, %		2.82
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.14
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.36055		Speed Power Coefficient (p)		0.54983
PF Slope Coefficient (m)	-1.14222		PF Power Coefficient (p)		0.84066
In Passing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		1.1
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	5676	-		-	68.5
Vehicle Results					·
Average Speed, mi/h	68.5		Percent Followers,	. %	29.5
Segment Travel Time, minutes	0.94		Follower Density (	FD), followers/mi/ln	1.1
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	244		Bicycle Effective W	/idth, ft	24
Bicycle LOS Score	2.80		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	С				
	9	Segn	nent 12		

Ve	ehicle Inputs					
Segment Type Passing Constrained		Length, ft		657		
Me	easured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	emand and Capacity					
Dir	ectional Demand Flow Rate, veh/h	244		Opposing Deman	d Flow Rate, veh/h	-
Pea	ak Hour Factor	0.88		Total Trucks, %		2.82
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
ln <sup>-</sup>	termediate Results					
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	eed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF	Slope Coefficient (m)	-1.29350		PF Power Coefficie	ent (p)	0.75785
In F	Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.3
%Ir	mprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Su	ıbsegment Data					
#	Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	657	-		-	68.0
Ve	hicle Results					
Ave	erage Speed, mi/h	68.0		Percent Followers, %		35.9
Seg	gment Travel Time, minutes	0.11		Follower Density (	FD), followers/mi/ln	1.3
Veł	nicle LOS	A				
Bi	cycle Results					
Per	cent Occupied Parking	0		Pavement Condition Rating		4
Flo	w Rate Outside Lane, veh/h	244	244		/idth, ft	24
Bic	ycle LOS Score	2.80		Bicycle Effective Speed Factor		5.07
Bic	ycle LOS	С				
		S	egm	nent 13		
Ve	hicle Inputs					
Seg	gment Type	Passing Zone		Length, ft		6009
Me	easured FFS	Measured		Free-Flow Speed, mi/h		70.0
De	emand and Capacity					
Dir	ectional Demand Flow Rate, veh/h	244		Opposing Deman	d Flow Rate, veh/h	165
Pea	ak Hour Factor	0.88		Total Trucks, %		2.82
Segment Capacity, veh/h 1700		1700	1700		(D/C)	0.14
ln <sup>-</sup>	termediate Results					
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	eed Slope Coefficient (m)	4.36364		Speed Power Coe	fficient (p)	0.54983
PF	Slope Coefficient (m)	-1.14089		PF Power Coefficie	ent (p)	0.83997

In Passing Lane Effective Length? No				Total Segment D	1.1	
%Improvement to Percent Followers 0.0		0.0		%Improvement	to Speed	0.0
Suk	osegment Data					
#	Segment Type	Length, ft	R	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	6009	-		-	68.5
Vel	nicle Results					
Aver	age Speed, mi/h	68.5		Percent Follower	rs, %	29.5
Segr	ment Travel Time, minutes	1.00		Follower Density	(FD), followers/mi/ln	1.1
Vehi	cle LOS	А				
Bic	ycle Results					
Perce	ent Occupied Parking	0		Pavement Condi	tion Rating	4
Flow	Rate Outside Lane, veh/h	244		Bicycle Effective	Width, ft	24
Bicyc	cle LOS Score	2.80		Bicycle Effective	Speed Factor	5.07
Bicyc	cle LOS	С				
		•	Segr	ment 14		
Vel	nicle Inputs					
Segr	ment Type	Passing Constrain	ned	Length, ft	Length, ft	
Mea	sured FFS	Measured		Free-Flow Speed	l, mi/h	50.0
Der	mand and Capacity					
Dire	ctional Demand Flow Rate, veh/h	244		Opposing Dema	nd Flow Rate, veh/h	-
Peak	Hour Factor	0.88		Total Trucks, %		2.82
Segr	nent Capacity, veh/h	1700		Demand/Capaci	Demand/Capacity (D/C)	
Inte	ermediate Results					
Segr	ment Vertical Class	1	1 Free-Flow Speed,		I, mi/h	50.0
Spee	ed Slope Coefficient (m)	4.57372		Speed Power Co	efficient (p)	0.41674
PF SI	lope Coefficient (m)	-1.47375		PF Power Coeffic	cient (p)	0.71164
In Pa	ssing Lane Effective Length?	No		Total Segment D	ensity, veh/mi/ln	2.1
%lm	provement to Percent Followers	0.0		%Improvement	%Improvement to Speed	
Suk	osegment Data					
#	Segment Type	Length, ft	R	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	891	-		-	48.0
Vel	nicle Results					
Aver	age Speed, mi/h	48.0		Percent Follower	rs, %	41.8
		0.21			Follower Density (FD), followers/mi/ln	
Vehi	cle LOS	В				
Bic	ycle Results					,
	ent Occupied Parking	0		Pavement Condi	tion Rating	4
reitent occupied raiking		1	Pavement Condition Rating 2			

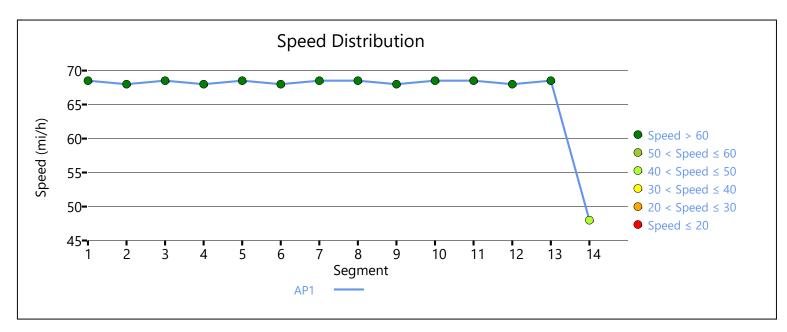
т	VMT	VHD	Follower Density followers/	IOS				
Facility	Facility Results							
Bicycle LC	)S	С						
Bicycle LC	OS Score	2.59	Bicycle Effective Speed Factor	4.42				
Flow Rate	Outside Lane, veh/h	244	Bicycle Effective Width, ft	24				

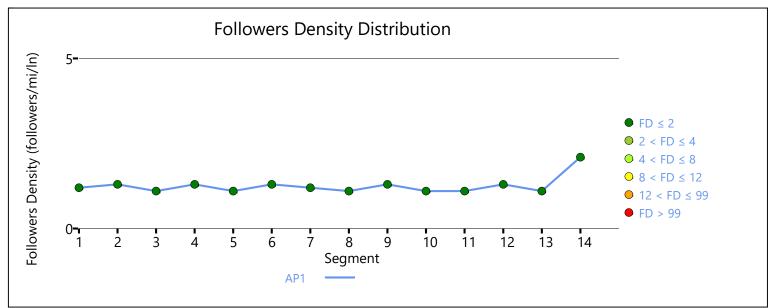
mi/ln

1.1

veh-h/p

0.11





veh-mi/p

327

Α

Analyst MIV Date 5/11/2023 Agency HRG Analysis Year 2050 NB  Arradiction SDOOT Firme Analyzed PM Peak Project Description West of Hartford SD 38 EB Units U.S. Customary  Segment 1  Vehicle Inputs  Segment Type Passing Zone Length, It 1069 Measured FFS Measured Free-Flow Speed, mi/h 70.0  Demand and Capacity Directional Demand Flow Rate, veh/h 170.0  Demand Bemand Flow Rate, veh/h 170.0  Demand Jemand Flow Rate, veh/h 170.0  Demand Capacity Directional Demand Flow Rate, veh/h 170.0  Demand Capacity  Interrediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.1767 Speed Power Coefficient (p) 0.51808  PF Slope Coefficient (m) 4.126475 PF Power Coefficient (p) 0.80124  In Passing Lane Effective Length P No Total Segment to Speed Do.  Subsequent Data  Segment Type Length, It Radius, It Superelevation, Average Speed, mi/h 1069  Vehicle Results  Average Speed, mi/h 69.0  Percent Followers A No Superelevation, Average Speed, mi/h 1069  Vehicle Results  Percent Travel Time, minutes 0.18 Follower Density, IPD, followers/mi/ln 0.6  Vehicle Results  Percent Coccupied Parking 0 Percent Condition Rating 4  Fine Marker Countries Speed Factor 5.07  Free-Flow Speed Factor 5.07  Free-Flow Speed Factor 5.07  Free-Flow Speed Factor 5.07  Free-Flow Speed, mi/h 69.0  Percent Followers, Speed Speed, mi/h 70.0  A North Speed Speed, mi/h 70.0  Percent Followers Speed, mi/h 70.0  Percent Followers Speed, mi/h 70.0  Percent Followers Speed, mi/h 70.0  Percent Followers Speed, mi/h 70.0  Percent Followers Speed Speed, mi/h 70.0  Percent Followers Speed Speed, mi/h 70.0  Percent Followers Speed Speed, mi/h 70.0  Percent Followers Speed Speed, mi/h 70.0  Percent Followers Speed Speed, mi/h 70.0  Percent Followers Speed		HCS Two	-Lane	Highway Re	port	
Agency HRG Analysis Year 2050 NB  Jurisdiction SDOOT Time Analyzed PM Peak  Project Description West of Hartford SD 38 EB Units U.S. Customary  Segment 1  Vehicle Inputs  Segment Type Passing Zone Length, ft 1069  Measured FFS Measured Free-Flow Speed, mi/h 70.0  Demand and Capacity  Directional Demand Flow Rate, veh/h 157 Opposing Demand Flow Rate, veh/h 286  Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.09  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.34767 Speed Power Coefficient (p) 0.51808  PF Slope Coefficient (m) 1.25475 PF Power Coefficient (p) 0.80124 In Passing Lane Effective Length? No Total Segment Density, veh/mi/h 0.6  Subsegment Data  # Segment Type Length? Radius, ft Superelevation, % Average Speed, mi/h 1 Tangent 1069 - 69.0  Vehicle Results  Average Speed, mi/h 69.0 Percent Followers, % 24.8  Segment Travel Time, minutes 0.18 Follower Density (FD), followers/mi/l 0.6  Vehicle LOS A Follower Coefficient Radius, ft Superelevation, % Average Speed, mi/h 1 Tangent 1069 - 69.0  Vehicle Results  Percent Occupied Parking 0 Percent Coefficient Width, ft 30  Biocycle Results  Percent Occupied Parking 0 Percent Coefficient Parking 4 A  Biocycle LOS Score 186 Bicycle Effective Width, ft 30  Bicycle LOS Score 186 Bicycle Effective Width, ft 30  Bicycle LOS Score 186 Bicycle Effective Width, ft 30  Bicycle LOS Score 186 Bicycle Effective Width, ft 30  Bicycle LOS Score 186 Bicycle Effective Width, ft 30  Segment Type Passing Constrained Length, ft 664	Project Information					
Time Analyzed	Analyst	MJV		Date		5/11/2023
Vehicle   Inputs	Agency	HRG	HRG			2050 NB
Segment 1  Vehicle Inputs  Segment Type	Jurisdiction	SDDOT		Time Analyzed		PM Peak
Vehicle Inputs   Segment Type   Passing Zone   Length, ft   1069	Project Description	West of Hartford	SD 38 EB	Units		U.S. Customary
Segment Type			Segn	nent 1		
Measured FFS         Measured         Free-Flow Speed, mi/h         70.0           Demand and Capacity           Directional Demand Flow Rate, veh/h         157         Opposing Demand Flow Rate, veh/h         286           Peak Hour Factor         0.88         Total Trucks, %         5.79           Segment Capacity, veh/h         1700         Demand/Capacity (D/C)         0.09           Intermediate Results           Segment Vertical Class         1         Free-Flow Speed, mi/h         70.0           Speed Slope Coefficient (m)         4.34767         Speed Power Coefficient (p)         0.51808           PF Slope Coefficient (m)         -1.25475         PF Power Coefficient (p)         0.80124           In Passing Lane Effective Length?         No         Total Segment Density, veh/mi/In         0.6           Segment Type         Length, ft         Radius, ft         Superelevation, %         Average Speed, mi/h           Vehicle Results           Average Speed, mi/h         69.0         Percent Followers, %         24.8           Segment Travel Time, minutes         0.18         Follower Density (FD), followers/mi/In         0.6           Vehicle Results           Bicycle	Vehicle Inputs					
Demand and Capacity  Directional Demand Flow Rate, veh/h 157 Opposing Demand Flow Rate, veh/h 286  Peak Hour Factor 0.88 Total Trucks, % 5.79  Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.09  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.34767 Speed Power Coefficient (p) 0.51808  PF Slope Coefficient (m) -1.25475 PF Power Coefficient (p) 0.80124  In Passing Lane Effective Length? No Total Segment Density, veh/mi/ln 0.6  **Improvement to Percent Followers 0.0 **Improvement to Speed 0.0  Subsegment Data  #* Segment Type Length, ft Radius, ft Superelevation, **Average Speed, mi/h 69.0 Percent Followers, **Open Open Open Open Open Open Open Open	Segment Type	Passing Zone		Length, ft		1069
Directional Demand Flow Rate, veh/h Peak Hour Factor  0.88 Total Trucks, % 5.79 Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.09  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0 Speed Slope Coefficient (m) 4.34767 Speed Power Coefficient (p) 0.80124 In Passing Lane Effective Length? No Total Segment Density, veh/mi/ln 0.6 %Improvement to Percent Followers 0.0 %Improvement to Speed 0.0  Subsegment Type Length, ft Radius, ft Superelevation, % Average Speed, mi/h 1 Tangent 0.18 Segment Type Percent Coefficient Superelevation, % Average Speed, mi/h 69.0 Percent Followers, % 24.8 Segment Travel Time, minutes 0.18 Follower Density (FD), followers/mi/ln 0.6 Wehicle LOS A  Bicycle Results  Percent Coupled Parking 0 Pawement Condition Rating 4 Bicycle LOS Score 1.86 Bicycle LOS B Segment 179e Passing Constrained Length, ft 664	Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Peak Hour Factor 0.88 Total Trucks, % 5.79  Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.09  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Segment Vertical Class 2 1 Speed Power Coefficient (p) 0.51808  PF Slope Coefficient (m) 4.34767 Speed Power Coefficient (p) 0.51808  PF Slope Coefficient (m) -1.25475 PF Power Coefficient (p) 0.80124  In Passing Lane Effective Length? No Total Segment Density, veh/mi/ln 0.6  %Improvement to Percent Followers 0.0 %Improvement to Speed 0.0  Subsegment Data  # Segment Type Length, ft Radius, ft Superelevation, % Average Speed, mi/h 1 Tangent 1069 - 59.0  Vehicle Results  Average Speed, mi/h 69.0 Percent Followers, % 24.8  Segment Travel Time, minutes 0.18 Follower Density (FD), followers/mi/ln 0.6  Wehicle LOS A Follower Density (FD), followers/mi/ln 0.6  Bicycle Results  Percent Occupied Parking 0 Pavement Condition Rating 4  Flow Rate Outside Lane, veh/h 157 Bicycle Effective Width, ft 30  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Bicycle LOS Segment 17pe Passing Constrained Length, ft 664	Demand and Capacity			<u> </u>		
Peak Hour Factor 0.88 Total Trucks, % 5.79  Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.09  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.34767 Speed Power Coefficient (p) 0.51808  PF Slope Coefficient (m) -1.25475 PF Power Coefficient (p) 0.80124  In Passing Lane Effective Length? No Total Segment Density, veh/mi/ln 0.6  %Improvement to Percent Followers 0.0 %Improvement to Speed 0.0  Subsegment Data  ## Segment Type Length, ft Radius, ft Superelevation, % Average Speed, mi/h 1 Tangent 1069 - 69.0  Vehicle Results  Average Speed, mi/h 69.0 Percent Followers, % 24.8  Segment Travel Time, minutes 0.18 Follower Density (FD), followers/mi/ln 0.6  Vehicle LOS A Follower Density (FD), followers/mi/ln 0.6  Bicycle Results  Percent Occupied Parking 0 Pavement Condition Rating 4  Flow Rate Outside Lane, veh/h 157 Bicycle Effective Speed Factor 5.07  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Bicycle LOS Segment Type Passing Constrained Length, ft 664	Directional Demand Flow Rate,	veh/h 157		Opposing Deman	d Flow Rate, veh/h	286
Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.34767 Speed Power Coefficient (p) 0.51808  PF Slope Coefficient (m) -1.25475 PF Power Coefficient (p) 0.80124  In Passing Lane Effective Length? No Total Segment Density, veh/mi/ln 0.6  Simprovement to Percent Followers 0.0 Simprovement to Speed 0.0  Subsegment Data  ## Segment Type Length, ft Radius, ft Superelevation, Average Speed, mi/h 1 Tangent 1069 - 69.0  Vehicle Results  Average Speed, mi/h 69.0 Percent Followers, A Segment Travel Time, minutes 0.18 Follower Density (FD), followers/mi/ln 0.6  Bicycle Results  Percent Occupied Parking 0 Pavement Condition Rating 4  Flow Rate Outside Lane, veh/h 157 Bicycle Effective Width, ft 30  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Bicycle LOS  Bregment Type Passing Constrained Length, ft 664	Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Vertical Class  1 Free-Flow Speed, mi/h  70.0  Speed Slope Coefficient (m)  4.34767 Speed Power Coefficient (p)  0.51808  PF Slope Coefficient (m)  -1.25475 PF Power Coefficient (p)  0.80124  In Passing Lane Effective Length?  No Total Segment Density, veh/mi/ln  0.6  Subsegment Data  # Segment Type  Length, ft  Radius, ft  Superelevation, % Average Speed, mi/h  1 Tangent  1069  69.0  Vehicle Results  Average Speed, mi/h  69.0  Percent Followers, % 24.8  Segment Travel Time, minutes  0.18  Follower Density (FD), followers/mi/ln  0.6  Bicycle Results  Percent Occupied Parking  0 Pavement Condition Rating  4  Flow Rate Outside Lane, veh/h  157  Bicycle LOS Score  1.86  Bicycle Effective Speed Factor  5.07  Segment 2  Vehicle Inputs  Segment Type  Passing Constrained  Length, ft  664	Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.09
Speed Slope Coefficient (m) 4.34767 Speed Power Coefficient (p) 0.51808 PF Slope Coefficient (m) -1.25475 PF Power Coefficient (p) 0.80124 In Passing Lane Effective Length? No Total Segment Density, veh/mi/In 0.6 %Improvement to Percent Followers 0.0 %Improvement to Speed 0.0  Subsegment Data ## Segment Type	Intermediate Results			'		<u>'</u>
PF Slope Coefficient (m) -1.25475 PF Power Coefficient (p) 0.80124  In Passing Lane Effective Length? No Total Segment Density, veh/mi/In 0.6  %Improvement to Percent Followers 0.0 %Improvement to Speed 0.0  Subsegment Data  ## Segment Type Length, ft Radius, ft Superelevation, % Average Speed, mi/h 1 Tangent 1069 69.0  Vehicle Results  Average Speed, mi/h 69.0 Percent Followers, % 24.8  Segment Travel Time, minutes 0.18 Follower Density (FD), followers/mi/ln 0.6  Wehicle LOS A Follower Density (FD), followers/mi/ln 0.6  Bicycle Results  Percent Occupied Parking 0 Pavement Condition Rating 4  Flow Rate Outside Lane, veh/h 157 Bicycle Effective Width, ft 30  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Bicycle LOS B  Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664	Segment Vertical Class	1	1		mi/h	70.0
In Passing Lane Effective Length?  No Total Segment Density, veh/mi/ln 0.6 %Improvement to Percent Followers 0.0 %Improvement to Speed 0.0  Subsegment Data  # Segment Type	Speed Slope Coefficient (m)	4.34767	4.34767		fficient (p)	0.51808
%Improvement to Percent Followers  Subsegment Data  # Segment Type	PF Slope Coefficient (m)	-1.25475		PF Power Coefficie	ent (p)	0.80124
# Segment Type   Length, ft   Radius, ft   Superelevation, %   Average Speed, mi/h   1 Tangent   1069   -   -   69.0    Vehicle Results  Average Speed, mi/h   69.0   Percent Followers, %   24.8    Segment Travel Time, minutes   0.18   Follower Density (FD), followers/mi/ln   0.6    Vehicle LOS   A      Bicycle Results  Percent Occupied Parking   0   Pavement Condition Rating   4    Flow Rate Outside Lane, veh/h   157   Bicycle Effective Width, ft   30    Bicycle LOS Score   1.86   Bicycle Effective Speed Factor   5.07    Bicycle LOS Score   1.86   Bicycle Effective Speed Factor   5.07    Segment 2  Vehicle Inputs  Segment Type   Passing Constrained   Length, ft   664	In Passing Lane Effective Length	? No		Total Segment De	nsity, veh/mi/ln	0.6
# Segment Type	%Improvement to Percent Follo	wers 0.0		%Improvement to	Speed	0.0
Tangent 1069 - 69.0  Vehicle Results  Average Speed, mi/h 69.0 Percent Followers, % 24.8  Segment Travel Time, minutes 0.18 Follower Density (FD), followers/mi/ln 0.6  Vehicle LOS A  Bicycle Results  Percent Occupied Parking 0 Pavement Condition Rating 4  Flow Rate Outside Lane, veh/h 157 Bicycle Effective Width, ft 30  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Bicycle LOS  Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664	Subsegment Data			<u>'</u>		
Vehicle Results  Average Speed, mi/h Segment Travel Time, minutes  0.18 Follower Density (FD), followers/mi/ln 0.6 Vehicle LOS  A  Bicycle Results  Percent Occupied Parking 0 Pavement Condition Rating 4 Flow Rate Outside Lane, veh/h Bicycle LOS Score 1.86 Bicycle Effective Width, ft 30 Bicycle LOS  B  Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664	# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
Average Speed, mi/h  Segment Travel Time, minutes  O.18  Follower Density (FD), followers/mi/ln  O.6  Wehicle LOS  A  Bicycle Results  Percent Occupied Parking  O  Pavement Condition Rating  Flow Rate Outside Lane, veh/h  Bicycle LOS Score  1.86  Bicycle Effective Width, ft  30  Bicycle LOS  Bicycle LOS  Segment 2  Vehicle Inputs  Segment Type  Passing Constrained  Length, ft  664	1 Tangent	1069	-		-	69.0
Segment Travel Time, minutes  O.18 Follower Density (FD), followers/mi/In  O.6  Wehicle LOS  A  Bicycle Results  Percent Occupied Parking OPavement Condition Rating 4 Flow Rate Outside Lane, veh/h Bicycle LOS Score I.86 Bicycle Effective Width, ft 30 Bicycle LOS  Bicycle LOS  Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664	Vehicle Results					<u>'</u>
Wehicle LOS  Bicycle Results  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Bicycle Effective Width, ft  30  Bicycle LOS Score  1.86  Bicycle Effective Speed Factor  5.07  Segment 2  Vehicle Inputs  Segment Type  Passing Constrained  Length, ft  664	Average Speed, mi/h	69.0		Percent Followers	, %	24.8
Wehicle LOS  Bicycle Results  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Bicycle Effective Width, ft  30  Bicycle LOS Score  1.86  Bicycle Effective Speed Factor  5.07  Segment 2  Vehicle Inputs  Segment Type  Passing Constrained  Length, ft  664	Segment Travel Time, minutes	0.18				0.6
Percent Occupied Parking 0 Pavement Condition Rating 4  Flow Rate Outside Lane, veh/h 157 Bicycle Effective Width, ft 30  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Bicycle LOS Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664	Vehicle LOS	A		, , , , , , ,		
Percent Occupied Parking 0 Pavement Condition Rating 4  Flow Rate Outside Lane, veh/h 157 Bicycle Effective Width, ft 30  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Bicycle LOS Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664	Bicycle Results					
Flow Rate Outside Lane, veh/h  Bicycle LOS Score  1.86  Bicycle Effective Width, ft  5.07  Bicycle LOS  Segment 2  Vehicle Inputs  Segment Type  Passing Constrained  Length, ft  664	Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Bicycle LOS Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664	Flow Rate Outside Lane, veh/h					
Segment 2  Vehicle Inputs  Segment Type  Passing Constrained  Length, ft  664				-		5.07
Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664	Bicycle LOS	В				
Vehicle Inputs       Segment Type     Passing Constrained     Length, ft     664			Segn	nent 2		
Segment Type Passing Constrained Length, ft 664	Vehicle Inputs		<u> </u>			
	•	Passing Constrain	ned	Length, ft		664
	Measured FFS			-	mi/h	

Demand and Capacity					
	157		Onnesia: D	d Flour Pate 1 "	
Directional Demand Flow Rate, veh/h	157		Opposing Demand Flow Rate, veh/h		-
Peak Hour Factor	0.88		Total Trucks, %	(D/C)	5.79
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.09
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29315		PF Power Coefficie	ent (p)	0.75829
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Radi	ius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	664	-		-	68.6
Vehicle Results					
Average Speed, mi/h	68.6		Percent Followers	, %	27.2
Segment Travel Time, minutes	0.11		Follower Density (FD), followers/mi/ln		0.6
Vehicle LOS	А				
Bicycle Results		<u> </u>			
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	157		Bicycle Effective V	Vidth, ft	30
Bicycle LOS Score	1.86		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	В				
	S	Segm	ent 3		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		1871
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	157		Opposing Deman	d Flow Rate, veh/h	286
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity	' (D/C)	0.09
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.35747		Speed Power Coe	fficient (p)	0.51808
PF Slope Coefficient (m)	-1.22915		PF Power Coefficie	ent (p)	0.81213
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.5
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					

#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	1871	-		-	69.0
Vel	nicle Results	•				
Aver	age Speed, mi/h	69.0		Percent Followers	, %	23.9
Segr	ment Travel Time, minutes	0.31		Follower Density (	FD), followers/mi/ln	0.5
Vehi	cle LOS	A				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow	Rate Outside Lane, veh/h	157		Bicycle Effective V	Vidth, ft	30
Bicy	cle LOS Score	1.86		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	В				
			Segi	ment 4		
Vel	nicle Inputs					
Segr	ment Type	Passing Constrai	ned	Length, ft		925
Mea	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity			•		
Dire	ctional Demand Flow Rate, veh/h	157		Opposing Deman	d Flow Rate, veh/h	-
Peak	: Hour Factor	0.88		Total Trucks, %		5.79
Segr	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.09
Int	ermediate Results					·
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	ed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF S	lope Coefficient (m)	-1.29315	-1.29315		ent (p)	0.75829
In Pa	assing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.6
%lm	provement to Percent Followers	0.0		%Improvement to Speed		0.0
Sul	osegment Data					
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	925	-		-	68.6
Vel	nicle Results					
Aver	age Speed, mi/h	68.6		Percent Followers	, %	27.2
Segment Travel Time, minutes		0.15		Follower Density (FD), followers/mi/ln		0.6
Vehicle LOS A		А				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
	Rate Outside Lane, veh/h	157		Bicycle Effective V		30
Bicy	cle LOS Score	1.86		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	В				

			Segi	ment 5		
Ve	hicle Inputs					
Seg	ment Type	Passing Zone		Length, ft		4476
Mea	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity					
Dire	ectional Demand Flow Rate, veh/h	157		Opposing Deman	d Flow Rate, veh/h	286
Pea	k Hour Factor	0.88		Total Trucks, %		5.79
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.09
Int	termediate Results					
Seg	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	ed Slope Coefficient (m)	4.39096		Speed Power Coe	fficient (p)	0.51808
PF S	Slope Coefficient (m)	-1.17364		PF Power Coefficie	ent (p)	0.83159
In P	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.5
%In	nprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Su	bsegment Data					
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	4476	-		-	69.0
Ve	hicle Results					
Ave	rage Speed, mi/h	69.0		Percent Followers,	. %	22.2
Seg	ment Travel Time, minutes	0.74	).74		FD), followers/mi/ln	0.5
Veh	icle LOS	А				
Bio	cycle Results					
Per	cent Occupied Parking	0		Pavement Conditi	on Rating	4
Flov	w Rate Outside Lane, veh/h	157		Bicycle Effective W	/idth, ft	30
Bicy	/cle LOS Score	1.86		Bicycle Effective Speed Factor		5.07
Bicy	rcle LOS	В				
			Segi	ment 6		
Ve	hicle Inputs					
	ment Type	Passing Constrained		Length, ft		896
Mea	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity					
	ectional Demand Flow Rate, veh/h	157		Opposing Deman	d Flow Rate, veh/h	-
Pea	k Hour Factor	0.88		Total Trucks, %		5.79
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.09
Int	termediate Results					
Sea	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
				1		

Speed Slope Coefficient (m)	4.57372			fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29315			ent (p)	0.75829
In Passing Lane Effective Length?			Total Segment De	nsity, veh/mi/ln	0.6
%Improvement to Percent Followers	0.0	0.0		Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Length, ft Radius		Superelevation, %	Average Speed, mi/h
1 Tangent	896	-		-	68.6
Vehicle Results					
Average Speed, mi/h	68.6		Percent Followers	, %	27.2
Segment Travel Time, minutes	0.15		Follower Density (	FD), followers/mi/ln	0.6
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	157		Bicycle Effective V	Vidth, ft	30
Bicycle LOS Score	1.86		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	В				
		Segn	nent 7		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		743
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	157		Opposing Deman	d Flow Rate, veh/h	286
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.09
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.34767		Speed Power Coefficient (p)		0.51808
PF Slope Coefficient (m)	-1.25475		PF Power Coefficient (p)		0.80124
In Passing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.6
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rac	lius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	743	-		-	69.0
Vehicle Results	•				•
Average Speed, mi/h	69.0		Percent Followers	, %	24.8
Segment Travel Time, minutes	0.12			FD), followers/mi/ln	0.6
Vehicle LOS A					

Bicycle	Results					
Percent Oc	cupied Parking	0		Pavement Condition	on Rating	4
Flow Rate 0	Outside Lane, veh/h	ane, veh/h 157 E		Bicycle Effective W	/idth, ft	30
Bicycle LOS	Bicycle LOS Score 1.86		Bicycle Effective Speed Factor		5.07	
Bicycle LOS		В				
		S	egn	nent 8		·
Vehicle	Inputs					
Segment Ty	ype	Passing Zone		Length, ft		2717
Measured I		Measured		Free-Flow Speed,	mi/h	70.0
Demano	d and Capacity	_				
Directional	Demand Flow Rate, veh/h	164		Opposing Demand	d Flow Rate, veh/h	289
Peak Hour	Factor	0.88		Total Trucks, %		3.28
Segment C	apacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Interme	diate Results	·				
Segment V	ertical Class	1		Free-Flow Speed, mi/h		70.0
Speed Slop	pe Coefficient (m)			Speed Power Coefficient (p)		0.51760
PF Slope Co	oefficient (m)	-1.20338		PF Power Coefficient (p)		0.82225
In Passing I	Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%Improver	ment to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegi	ment Data					
# Segn	nent Type	Length, ft	Rad	lius, ft	Superelevation, %	Average Speed, mi/h
1 Tang	ent	2717	1-		-	68.9
Vehicle	Results					
Average Sp	peed, mi/h	68.9		Percent Followers,	%	23.8
Segment Ti	ravel Time, minutes	0.45		Follower Density (FD), followers/mi/ln		0.6
Vehicle LOS	5	А				
Bicycle	Results					
Percent Oc	cupied Parking	0		Pavement Condition	on Rating	4
Flow Rate 0	Outside Lane, veh/h	164		Bicycle Effective Width, ft		29
Bicycle LOS Score		1.40		Bicycle Effective Speed Factor		5.07
Bicycle LOS		А				
		S	egn	nent 9		
Vehicle	Inputs					
Carrent T	уре	Passing Constrained		Length, ft		1013
Segment Type  Measured FFS		Measured		Free-Flow Speed, mi/h		+

Directional Demand Flow Rate, veh/h Peak Hour Factor Segment Capacity, veh/h	0.88		Total Trucks, %	d Flow Rate, veh/h	
	1	0.88			3.28
	1700		Demand/Capacity	(D/C)	0.10
ntermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29345		PF Power Coefficie	ent (p)	0.75792
n Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.7
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
Tangent	1013	-		-	68.5
Vehicle Results					
Average Speed, mi/h	68.5		Percent Followers	, %	28.0
Segment Travel Time, minutes	0.17		Follower Density (	FD), followers/mi/ln	0.7
/ehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	164		Bicycle Effective V	Vidth, ft	29
Bicycle LOS Score	1.40		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	А				
		Segm	ent 10		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		4569
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity			1		
Directional Demand Flow Rate, veh/h	164		Opposing Deman	d Flow Rate, veh/h	289
Peak Hour Factor	0.88		Total Trucks, %		3.28
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.10
ntermediate Results			<u> </u>		
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.39263		Speed Power Coe		0.51760
PF Slope Coefficient (m)	-1.17332		PF Power Coefficie	·	0.83118
n Passing Lane Effective Length?	No		Total Segment De		0.5
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data	<u>'</u>				
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h

1 Tangent	4569	-		-	68.9
Vehicle Results					
Average Speed, mi/h	68.9		Percent Followers,	%	22.9
Segment Travel Time, minutes	0.75		Follower Density (	FD), followers/mi/ln	0.5
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	164		Bicycle Effective W	/idth, ft	29
Bicycle LOS Score	1.40		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	А				
	Se	egm	ent 11		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		5676
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity	•				
Directional Demand Flow Rate, veh/h	164		Opposing Demand Flow Rate, veh/h		280
Peak Hour Factor	0.88		Total Trucks, %		2.82
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.10
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.40080		Speed Power Coe	fficient (p)	0.51956
PF Slope Coefficient (m)	-1.16417		PF Power Coefficie	ent (p)	0.83135
In Passing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.5
%Improvement to Percent Followers	0.0		%Improvement to Speed		0.0
Subsegment Data					
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	5676	-		-	68.9
Vehicle Results					
Average Speed, mi/h	68.9		Percent Followers, %		22.8
Segment Travel Time, minutes	0.94		Follower Density (FD), followers/mi/ln		0.5
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	164		Bicycle Effective W	/idth, ft	29
Bicycle LOS Score	1.28		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	А				
	Se	egm	ent 12		

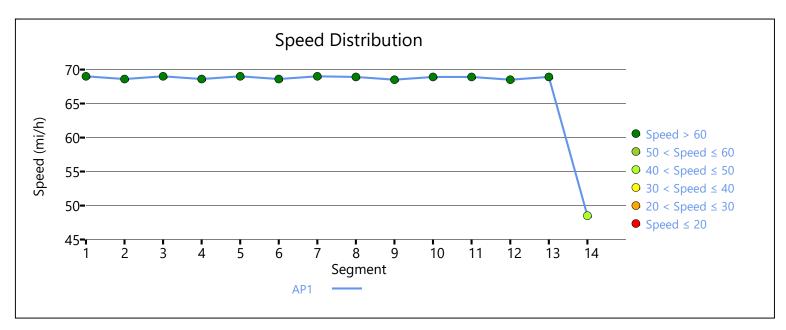
Ve	chicle Inputs					
Seg	gment Type	Passing Constrained		Length, ft		657
Me	easured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	emand and Capacity					
Dir	ectional Demand Flow Rate, veh/h	164		Opposing Deman	d Flow Rate, veh/h	-
Pea	ak Hour Factor	0.88		Total Trucks, %		2.82
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
In	termediate Results					
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	eed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF	Slope Coefficient (m)	-1.29350		PF Power Coefficie	ent (p)	0.75785
In I	Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.7
%lr	mprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Su	ıbsegment Data					
#	Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	657	T-		-	68.5
Ve	hicle Results					
Ave	erage Speed, mi/h	68.5		Percent Followers,	. %	28.0
Seg	gment Travel Time, minutes	0.11		Follower Density (	FD), followers/mi/ln	0.7
Vel	nicle LOS	A				
Bi	cycle Results	·				
Per	cent Occupied Parking	0		Pavement Condition	on Rating	4
Flo	w Rate Outside Lane, veh/h	164		Bicycle Effective Width, ft		29
Bic	ycle LOS Score	1.28		Bicycle Effective S	peed Factor	5.07
Bic	ycle LOS	А				
		S	egm	nent 13		
Ve	hicle Inputs					
Seg	gment Type	Passing Zone		Length, ft		6009
Me	easured FFS	Measured		Free-Flow Speed, mi/h		70.0
De	emand and Capacity					
Dir	ectional Demand Flow Rate, veh/h	164		Opposing Deman	d Flow Rate, veh/h	280
Pea	ak Hour Factor	0.88		Total Trucks, %		2.82
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
In	termediate Results					
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	eed Slope Coefficient (m)	4.40389		Speed Power Coe	fficient (p)	0.51956
PF	Slope Coefficient (m)	-1.16281		PF Power Coefficie	ent (p)	0.83065

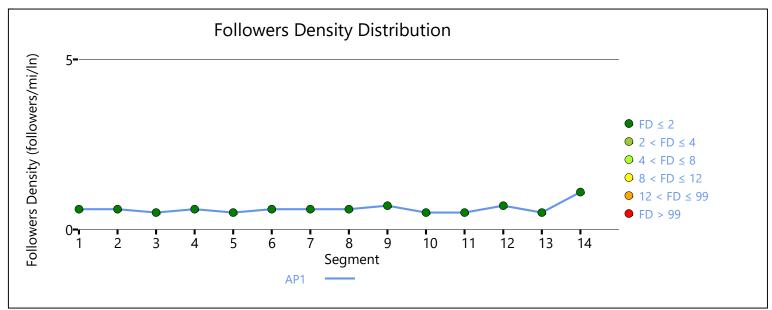
In Passing Lane Effective Length?		No		Total Segment D	ensity, veh/mi/ln	0.5
%Improvement to Percent Followers		0.0		%Improvement	%Improvement to Speed	
Suk	osegment Data					
#	Segment Type	Length, ft	Length, ft Radiu		Superelevation, %	Average Speed, mi/h
1	Tangent	6009	-		-	68.9
Vel	nicle Results					
Aver	age Speed, mi/h	68.9		Percent Follower	rs, %	22.8
Segr	ment Travel Time, minutes	0.99		Follower Density	(FD), followers/mi/ln	0.5
Vehi	cle LOS	А		İ		
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Condi	tion Rating	4
Flow	Rate Outside Lane, veh/h	164		Bicycle Effective	Width, ft	29
Bicy	cle LOS Score	1.28		Bicycle Effective	Speed Factor	5.07
Bicy	cle LOS	A				
		•	Segr	nent 14		
Vel	nicle Inputs					
Segr	ment Type	Passing Constrair	ned	Length, ft		891
Mea	sured FFS	Measured		Free-Flow Speed	l, mi/h	50.0
Dei	mand and Capacity					
Dire	ctional Demand Flow Rate, veh/h	164		Opposing Dema	nd Flow Rate, veh/h	-
Peak	Hour Factor	0.88		Total Trucks, %		2.82
Segr	ment Capacity, veh/h	1700		Demand/Capaci	ty (D/C)	0.10
Inte	ermediate Results					
Segr	ment Vertical Class	1		Free-Flow Speed	I, mi/h	50.0
Spee	ed Slope Coefficient (m)	4.57372		Speed Power Co	Speed Power Coefficient (p)	
PF S	lope Coefficient (m)	-1.47375		PF Power Coeffic	PF Power Coefficient (p)	
In Pa	ssing Lane Effective Length?	No		Total Segment D	Total Segment Density, veh/mi/ln	
%lm	provement to Percent Followers	0.0		%Improvement to Speed		0.0
Suk	segment Data					
#	Segment Type	Length, ft	R	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	891	-		-	48.5
Veł	nicle Results					
Aver	age Speed, mi/h	48.5		Percent Follower	rs, %	33.4
Segr	ment Travel Time, minutes	0.21		Follower Density	Follower Density (FD), followers/mi/ln	
Vehi	cle LOS	A				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Condi	tion Rating	4
	· · ·					

Т	VMT veh-mi/p	VHD veh-h/p	Follower Density, followers/	LOS
Facility	Results			
Bicycle LOS		А		
Bicycle LC	Bicycle LOS Score 1.06		Bicycle Effective Speed Factor	4.42
Flow Rate	Outside Lane, veh/h	164	Bicycle Effective Width, ft	29

0.6

0.05





216

Α

	HCS Two-L	ane	Highway Re	port	
Project Information					
Analyst	MJV	MJV			5/11/2023
Agency	HRG		Analysis Year		2050 NB
Jurisdiction	SDDOT		Time Analyzed		AM Peak
Project Description	WB 38 West of Hartf	ford	Units		U.S. Customary
		Segn	nent 1		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		10549
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity			<u> </u>		
Directional Demand Flow Rate, veh/h	165		Opposing Deman	d Flow Rate, veh/h	244
Peak Hour Factor	0.88		Total Trucks, %		12.50
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Intermediate Results			'		
Segment Vertical Class 1			Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m) 4.42827			Speed Power Coefficient (p)		0.52768
PF Slope Coefficient (m) -1.16689			PF Power Coefficie	ent (p)	0.80729
In Passing Lane Effective Length? No			Total Segment De	nsity, veh/mi/ln	0.6
%Improvement to Percent Followers	%Improvement to Percent Followers 0.0		%Improvement to	Speed	0.0
Subsegment Data			,		
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	10549	-		-	69.0
Vehicle Results					<u>'</u>
Average Speed, mi/h	69.0		Percent Followers, %		23.8
Segment Travel Time, minutes	1.74		Follower Density (FD), followers/mi/ln		0.6
Vehicle LOS	А				
Bicycle Results			1		
Percent Occupied Parking	0		Pavement Condition Rating		4
Flow Rate Outside Lane, veh/h			Bicycle Effective V		29
Bicycle LOS Score 4.94		Bicycle Effective S		5.07	
Bicycle LOS	E				
		Segn	nent 2		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		2793
Segment Type	1		Free-Flow Speed, mi/h		

Demand and Capacity					
Directional Demand Flow Rate, veh/h	165		Opposing Deman	d Flow Rate, veh/h	244
Peak Hour Factor	0.88		Opposing Demand Flow Rate, veh/h Total Trucks, %		12.50
Segment Capacity, veh/h	1700		Demand/Capacity	, (D/C)	0.10
	1700		Demand/Capacity	(,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	0.10
Intermediate Results					_
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.35767		Speed Power Coe	fficient (p)	0.52768
PF Slope Coefficient (m)	-1.19319		PF Power Coefficie	ent (p)	0.82737
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rad	lius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	2793	-		-	69.0
Vehicle Results					
Average Speed, mi/h	69.0		Percent Followers, %		23.5
Segment Travel Time, minutes	0.46		Follower Density (	(FD), followers/mi/ln	0.6
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	165		Bicycle Effective V	Vidth, ft	29
Bicycle LOS Score	4.94		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	E				
	9	Segn	nent 3		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		3825
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity	•				
Directional Demand Flow Rate, veh/h	165		Opposing Demand Flow Rate, veh/h		245
Peak Hour Factor	0.88		Total Trucks, %		2.40
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.37079		Speed Power Coe	fficient (p)	0.52741
PF Slope Coefficient (m)	-1.17529		PF Power Coefficie	ent (p)	0.83222
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
Jabbegineiit Data					

#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	3825 -			-	69.0
Veł	nicle Results	-				
Aver	rage Speed, mi/h	69.0		Percent Followers	, %	23.1
Segr	ment Travel Time, minutes	0.63		Follower Density (	(FD), followers/mi/ln	0.6
Vehi	cle LOS	A				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow	Rate Outside Lane, veh/h	165		Bicycle Effective V	Vidth, ft	29
Bicy	cle LOS Score	1.17		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	А				
			Segr	ment 4		
Veł	nicle Inputs					
Segr	ment Type	Passing Constrai	ned	Length, ft		791
Mea	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Dei	mand and Capacity					
Dire	ctional Demand Flow Rate, veh/h	165	165		d Flow Rate, veh/h	-
Peak Hour Factor 0.8		0.88		Total Trucks, %		2.40
Segment Capacity, veh/h		1700		Demand/Capacity	' (D/C)	0.10
Int	ermediate Results					
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	ed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF S	lope Coefficient (m)	-1.29355		PF Power Coefficient (p)		0.75779
In Pa	assing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.7
%lm	provement to Percent Followers	0.0		%Improvement to Speed		0.0
Suk	osegment Data					
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	791	-		-	68.5
Vel	nicle Results					
Aver	rage Speed, mi/h	68.5		Percent Followers	, %	28.1
Segr	ment Travel Time, minutes	0.13		Follower Density (FD), followers/mi/ln		0.7
Vehicle LOS A						
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow	Rate Outside Lane, veh/h	165		Bicycle Effective V	Vidth, ft	29
Bicy	cle LOS Score	1.17		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	Α				

	Segment 5								
Ve	hicle Inputs								
Seg	ment Type	Passing Zone		Length, ft		3414			
Mea	asured FFS	Measured	Measured		mi/h	70.0			
De	mand and Capacity								
Dire	ectional Demand Flow Rate, veh/h	165		Opposing Demand	d Flow Rate, veh/h	245			
Pea	k Hour Factor	0.88		Total Trucks, %		2.40			
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10			
Int	termediate Results								
Seg	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0			
Spe	ed Slope Coefficient (m)	4.36595		Speed Power Coef	fficient (p)	0.52741			
PF S	Slope Coefficient (m)	-1.18179		PF Power Coefficie	ent (p)	0.83026			
In P	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6			
%In	nprovement to Percent Followers	0.0		%Improvement to	Speed	0.0			
Su	bsegment Data								
#	Segment Type	Length, ft	Ra	ndius, ft	Superelevation, %	Average Speed, mi/h			
1	Tangent	3414	-		-	69.0			
Ve	hicle Results					<u>'</u>			
Ave	rage Speed, mi/h	69.0		Percent Followers,	. %	23.2			
Seg	ment Travel Time, minutes	0.56		Follower Density (	FD), followers/mi/ln	0.6			
Veh	icle LOS	А							
Bio	cycle Results					·			
Per	cent Occupied Parking	0		Pavement Condition Rating		4			
Flov	w Rate Outside Lane, veh/h	165		Bicycle Effective Width, ft		29			
Bicy	/cle LOS Score	1.17		Bicycle Effective S	peed Factor	5.07			
Bicy	rcle LOS	А							
		9	Segi	ment 6					
Ve	hicle Inputs								
Seg	ment Type	Passing Constrained		Length, ft		286			
Mea	asured FFS	Measured	-		mi/h	70.0			
De	mand and Capacity								
Dire	ectional Demand Flow Rate, veh/h	165		Opposing Demand	d Flow Rate, veh/h	-			
Pea	k Hour Factor	0.88		Total Trucks, %		2.40			
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10			
Int	termediate Results			,					
Sea	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0			
		I		1					

Speed Slope C	Coefficient (m)	4.57372		Speed Power Coefficient (p)		0.41674
PF Slope Coef		-1.29355		PF Power Coefficie	·	0.75779
•	ne Effective Length?			Total Segment Density, veh/mi/ln		0.7
	nt to Percent Followers	0.0		%Improvement to		0.0
Subsegme	ent Data			·		
# Segmen		Length, ft	Rad	lius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent		286	-		-	68.5
Vehicle Re		1				
Average Speed	d. mi/h	68.5		Percent Followers,	 . %	28.1
- ,	el Time, minutes	0.05			FD), followers/mi/ln	0.7
Vehicle LOS		A		· energy (	. 2),	
Bicycle Re	esults	1				
Percent Occup		0		Pavement Condition	on Rating	4
<u> </u>	tside Lane, veh/h	165		Bicycle Effective W		29
Bicycle LOS Sc		1.17		Bicycle Effective Speed Factor		5.07
Bicycle LOS		A		bicycle Ellective Speed Factor		
.,			ean	nent 7		
Vehicle In	nute					
	•					1.00
Segment Type		Passing Constrained		Length, ft	· a	463
Measured FFS	5	Measured		Free-Flow Speed,	mı/h	70.0
Demand a	and Capacity					
Directional De	emand Flow Rate, veh/h	169		Opposing Demand Flow Rate, veh/h		-
Peak Hour Fac	ctor	0.88		Total Trucks, %		2.60
Segment Capa	acity, veh/h	1700		Demand/Capacity (D/C)		0.10
Intermedi	ate Results					
Segment Verti	ical Class	1		Free-Flow Speed, mi/h		70.0
Speed Slope C	Coefficient (m)	4.57372		Speed Power Coefficient (p)		0.41674
PF Slope Coef	fficient (m)	-1.29353		PF Power Coefficient (p)		0.75782
In Passing Lan	ne Effective Length?	No		Total Segment Density, veh/mi/ln		0.7
%Improvemer	nt to Percent Followers	0.0		%Improvement to Speed		0.0
Subsegme	ent Data					
	nt Type			lius, ft	Superelevation, %	Average Speed, mi/h
# Segmen			-		-	68.5
-		1.00				
1 Tangent		1.00				
1 Tangent Vehicle Re	esults	68.5		Percent Followers,	, %	28.6
1 Tangent Vehicle Re Average Speed	esults				, % FD), followers/mi/ln	28.6

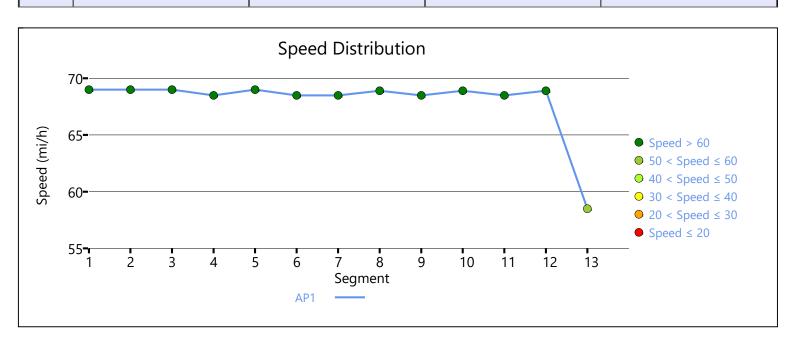
ement Condition Rating rcle Effective Width, ft rcle Effective Speed Factor	4822 70.0 2, veh/h 243 2.60 0.10 70.0 0.52796 0.83451
gth, ft e-Flow Speed, mi/h  cosing Demand Flow Rate al Trucks, % nand/Capacity (D/C) e-Flow Speed, mi/h ed Power Coefficient (p) cover Coefficient (p) al Segment Density, veh/m	4822 70.0 2, veh/h 243 2.60 0.10 70.0 0.52796 0.83451 ni/ln 0.6
gth, ft 2-Flow Speed, mi/h 2-Flow Speed, mi/h 2-Flow Speed, mi/h 2-Flow Speed, mi/h 2-Flow Speed, mi/h 3-Flow  4822 70.0 2, veh/h 243 2.60 0.10 70.0 0.52796 0.83451 ni/ln 0.6	
gth, ft e-Flow Speed, mi/h posing Demand Flow Rate al Trucks, % nand/Capacity (D/C) e-Flow Speed, mi/h ed Power Coefficient (p) Power Coefficient (p)	70.0  243 2.60 0.10  70.0  0.52796 0.83451  ni/ln 0.6
gth, ft e-Flow Speed, mi/h posing Demand Flow Rate al Trucks, % nand/Capacity (D/C) e-Flow Speed, mi/h ed Power Coefficient (p) Power Coefficient (p)	70.0  243 2.60 0.10  70.0  0.52796 0.83451  ni/ln 0.6
e-Flow Speed, mi/h  posing Demand Flow Rate al Trucks, %  nand/Capacity (D/C)  e-Flow Speed, mi/h  ed Power Coefficient (p)  Power Coefficient (p) al Segment Density, veh/m	70.0  243 2.60 0.10  70.0  0.52796 0.83451  ni/ln 0.6
e-Flow Speed, mi/h  posing Demand Flow Rate al Trucks, %  nand/Capacity (D/C)  e-Flow Speed, mi/h  ed Power Coefficient (p)  Power Coefficient (p) al Segment Density, veh/m	70.0  243 2.60 0.10  70.0  0.52796 0.83451  ni/ln 0.6
posing Demand Flow Rate al Trucks, % nand/Capacity (D/C) e-Flow Speed, mi/h ed Power Coefficient (p) Power Coefficient (p)	2, veh/h 243 2.60 0.10 70.0 0.52796 0.83451 ni/ln 0.6
e-Flow Speed, mi/h ed Power Coefficient (p) Power Coefficient (p) al Segment Density, veh/m	2.60 0.10 70.0 0.52796 0.83451 ni/ln 0.6
e-Flow Speed, mi/h ed Power Coefficient (p) Power Coefficient (p) al Segment Density, veh/m	2.60 0.10 70.0 0.52796 0.83451 ni/ln 0.6
e-Flow Speed, mi/h ed Power Coefficient (p) Power Coefficient (p) al Segment Density, veh/m	0.10  70.0  0.52796  0.83451  ni/ln  0.6
e-Flow Speed, mi/h ed Power Coefficient (p) Power Coefficient (p) al Segment Density, veh/m	70.0 0.52796 0.83451 ni/ln 0.6
ed Power Coefficient (p) Power Coefficient (p) al Segment Density, veh/m	0.52796 0.83451 ni/ln 0.6
ed Power Coefficient (p) Power Coefficient (p) al Segment Density, veh/m	0.52796 0.83451 ni/ln 0.6
Power Coefficient (p) al Segment Density, veh/m	0.83451 ni/ln 0.6
al Segment Density, veh/m	ni/ln 0.6
nprovement to Speed	0.0
L	
	·
t Supereleva	ation, % Average Speed, mi/h
-	68.9
	·
cent Followers, %	23.2
ower Density (FD), followe	ers/mi/ln 0.6
	·
ement Condition Rating	4
cle Effective Width, ft	29
cle Effective Speed Factor	r 5.07
t 9	
ath ft	861
gtii, it	70.0
	ement Condition Rating cle Effective Width, ft

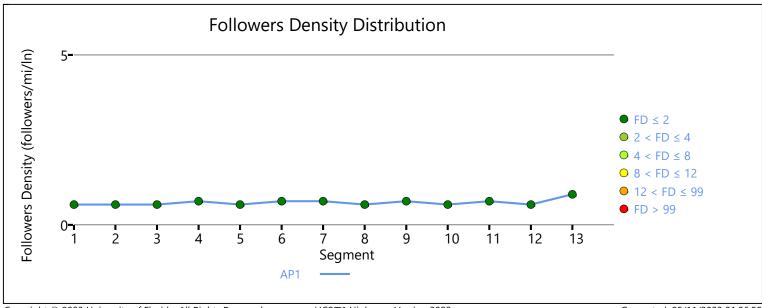
Dire	ctional Demand Flow Rate, veh/h	169		Opposing Demand	d Flow Rate, veh/h	-	
	Hour Factor	0.88		Total Trucks, %	2.12, 1.0.1, 1.	2.60	
Segr	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10	
	ermediate Results				<u> </u>		
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0	
	ed Slope Coefficient (m)	4.57372		Speed Power Coef		0.41674	
-	lope Coefficient (m)	-1.29353		PF Power Coefficie		0.75782	
	assing Lane Effective Length?	No		Total Segment De	<u> </u>	0.7	
%lm	provement to Percent Followers	0.0		%Improvement to	Speed	0.0	
Suk	osegment Data			<u>'</u>			
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h	
1	Tangent	861	-		-	68.5	
Vel	nicle Results					·	
Aver	rage Speed, mi/h	68.5		Percent Followers,	%	28.6	
Segr	ment Travel Time, minutes	0.14		Follower Density (	FD), followers/mi/ln	0.7	
Vehi	cle LOS	А					
Bic	ycle Results	•		<u>'</u>			
Perc	ent Occupied Parking	0		Pavement Condition	on Rating	4	
Flow	Rate Outside Lane, veh/h	169		Bicycle Effective W	/idth, ft	29	
Bicy	cle LOS Score	1.23			peed Factor	5.07	
Bicy	cle LOS	А					
		·	Segr	ment 10			
Veł	nicle Inputs						
Segr	ment Type	Passing Zone		Length, ft		1556	
Mea	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0	
Dei	mand and Capacity						
Dire	ctional Demand Flow Rate, veh/h	169		Opposing Demand	d Flow Rate, veh/h	243	
Peak	Hour Factor	0.88		Total Trucks, %		2.60	
Segr	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10	
Inte	ermediate Results						
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0	
Spee	ed Slope Coefficient (m)	4.33831		Speed Power Coef	ficient (p)	0.52796	
PF S	lope Coefficient (m)	-1.23554		PF Power Coefficie	ent (p)	0.80871	
In Pa	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6	
%lm	provement to Percent Followers	0.0		%Improvement to	Speed	0.0	
Sul	osegment Data						
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h	

1 Tangent	1556	-		-	68.9
Vehicle Results					
Average Speed, mi/h	68.9	Percent Follow		, %	25.5
Segment Travel Time, minutes	0.26	0.26		FD), followers/mi/ln	0.6
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	169		Bicycle Effective W	Vidth, ft	29
Bicycle LOS Score	1.23		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	А				
	Se	egm	nent 11		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		799
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	169		Opposing Deman	d Flow Rate, veh/h	-
Peak Hour Factor	0.88		Total Trucks, %		2.60
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29353		PF Power Coefficient (p)		0.75782
In Passing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.7
%Improvement to Percent Followers	0.0		%Improvement to Speed		0.0
Subsegment Data					
# Segment Type	Length, ft	Rad	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	799	-		-	68.5
Vehicle Results					
Average Speed, mi/h	68.5		Percent Followers, %		28.6
Segment Travel Time, minutes	0.13		Follower Density (FD), followers/mi/ln		0.7
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	169		Bicycle Effective W	Vidth, ft	29
Bicycle LOS Score	1.23		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	А				
	Se	egm	nent 12		

Ve	ehicle Inputs						
Seg	gment Type	Passing Zone		Length, ft		857	
Me	Measured FFS Measured		Free-Flow Speed,	mi/h	70.0		
De	emand and Capacity						
Dir	rectional Demand Flow Rate, veh/h	169		Opposing Demand	d Flow Rate, veh/h	243	
Pea	ak Hour Factor	0.88		Total Trucks, %		2.60	
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10	
ln <sup>.</sup>	termediate Results						
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0	
Spe	eed Slope Coefficient (m)	4.33390		Speed Power Coef	fficient (p)	0.52796	
PF	Slope Coefficient (m)	-1.24754		PF Power Coefficie	ent (p)	0.80350	
In F	Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6	
%Ir	mprovement to Percent Followers	0.0		%Improvement to	Speed	0.0	
Su	ıbsegment Data						
#	Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h	
1	Tangent	857	-	-		68.9	
Ve	ehicle Results						
Ave	erage Speed, mi/h	68.9		Percent Followers,	%	25.9	
Segment Travel Time, minutes		0.14		Follower Density (	FD), followers/mi/ln	0.6	
Veł	hicle LOS	А					
Bi	cycle Results						
Per	cent Occupied Parking	0		Pavement Condition	on Rating	4	
Flo	w Rate Outside Lane, veh/h	169		Bicycle Effective Width, ft		29	
Bic	ycle LOS Score	1.23		Bicycle Effective Speed Factor		5.07	
Bic	ycle LOS	А					
		9	Segm	nent 13			
Ve	ehicle Inputs						
Seg	gment Type	Passing Constrained	d	Length, ft		1288	
Me	easured FFS	Measured		Free-Flow Speed,	Free-Flow Speed, mi/h		
De	emand and Capacity						
Dir	ectional Demand Flow Rate, veh/h	169		Opposing Demand	d Flow Rate, veh/h	-	
Pea	eak Hour Factor 0.88		Total Trucks, %				
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10	
ln <sup>.</sup>	termediate Results						
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	60.0	
Spe	eed Slope Coefficient (m)	4.57372		Speed Power Coef	fficient (p)	0.41674	
PF	Slope Coefficient (m)	-1.39677		PF Power Coefficie	ent (p)	0.73640	

In Pa	ssing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.9
%lmp	provement to Percent Followers	0.0		%Improvement to	o Speed	0.0
Sub	segment Data					
#	Segment Type	Length, ft	Length, ft Radiu		Superelevation, %	Average Speed, mi/h
1	Tangent	1288	-		-	58.5
Veh	icle Results					
Avera	age Speed, mi/h	58.5	58.5 F		5, %	31.5
Segn	Segment Travel Time, minutes 0.25			Follower Density (FD), followers/mi/ln		0.9
Vehic	ile LOS	Α				
Bicy	cle Results					
Perce	ent Occupied Parking	0	0		ion Rating	4
Flow	Rate Outside Lane, veh/h	169		Bicycle Effective \	Width, ft	29
Bicyc	le LOS Score	1.14		Bicycle Effective S	Speed Factor	4.79
Bicycle LOS A		А				
Faci	ility Results					
Т	VMT veh-mi/p	VHI veh-l		Follower D	ensity, followers/ mi/In	LOS
1	224	0.0	0.05		0.6	A





Copyright © 2023 University of Florida. All Rights Reserved.

HCSTM Highways Version 2022 WB\_38\_WHartford\_2050AM.xuf Generated: 05/11/2023 21:06:58

	HCS Two-L	.ane	Highway Re	port	
Project Information					
Analyst	MJV	MJV			5/11/2023
Agency	HRG		Analysis Year		2050 NB
Jurisdiction	SDDOT		Time Analyzed		PM Peak
Project Description	WB 38 West of Hart	ford	Units		U.S. Customary
		Segn	nent 1		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		10549
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity			<u>'</u>		
Directional Demand Flow Rate, veh/h	280		Opposing Deman	d Flow Rate, veh/h	164
Peak Hour Factor	0.88		Total Trucks, %		1.94
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.16
Intermediate Results			'		
Segment Vertical Class 1			Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m) 4.39885			Speed Power Coe	fficient (p)	0.55020
PF Slope Coefficient (m) -1.15143			PF Power Coefficie	ent (p)	0.81244
In Passing Lane Effective Length? No			Total Segment De	nsity, veh/mi/ln	1.4
%Improvement to Percent Followers	%Improvement to Percent Followers 0.0		%Improvement to	Speed	0.0
Subsegment Data			,		
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	10549	-		-	68.3
Vehicle Results					
Average Speed, mi/h	68.3		Percent Followers, %		33.6
Segment Travel Time, minutes	1.76		Follower Density (FD), followers/mi/ln		1.4
Vehicle LOS	A				
Bicycle Results			1		
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h 280		Bicycle Effective V		24	
Bicycle LOS Score 2.64		Bicycle Effective S		5.07	
Bicycle LOS	С				
		Segn	nent 2		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		2793
- ••	1 5		Free-Flow Speed, mi/h		

Demand and Capacity					
	200		Onnacia a Davi	d Flour Data and the	164
Directional Demand Flow Rate, veh/h	280		Opposing Demand Flow Rate, veh/h Total Trucks, %		164
Peak Hour Factor				(D (C)	1.94
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.16
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.32824		Speed Power Coe	fficient (p)	0.55020
PF Slope Coefficient (m)	-1.17723		PF Power Coefficie	ent (p)	0.83227
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.4
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Radi	ius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	2793	-		-	68.3
Vehicle Results					
Average Speed, mi/h	68.3		Percent Followers	, %	33.5
Segment Travel Time, minutes	0.46		Follower Density (FD), followers/mi/ln		1.4
Vehicle LOS	A				
Bicycle Results		<u> </u>			
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	280		Bicycle Effective V	Vidth, ft	24
Bicycle LOS Score	2.64		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	С				
	S	Segm	nent 3		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		3825
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	289		Opposing Demand Flow Rate, veh/h		164
Peak Hour Factor	0.88		Total Trucks, %		2.19
Segment Capacity, veh/h	1700		Demand/Capacity	' (D/C)	0.17
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.34098		Speed Power Coe		0.55020
PF Slope Coefficient (m)	-1.15833		PF Power Coefficie	ent (p)	0.83897
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.4
%Improvement to Percent Followers	0.0		%Improvement to	-	0.0
Subsegment Data					
<b>J</b>					

#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h					
1	Tangent	3825	-		-	68.3					
Veł	nicle Results	•									
Aver	age Speed, mi/h	68.3		Percent Followers	, %	33.5					
Segr	ment Travel Time, minutes	0.64		Follower Density (	(FD), followers/mi/ln	1.4					
Vehi	cle LOS	A									
Bic	ycle Results										
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4					
Flow	Rate Outside Lane, veh/h	289		Bicycle Effective V	Vidth, ft	24					
Bicy	cle LOS Score	2.72		Bicycle Effective S	peed Factor	5.07					
Bicy	cle LOS	С									
		Segment 4									
Vel	nicle Inputs										
Segr	ment Type	Passing Constrai	ned	Length, ft		791					
Mea	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0					
Dei	mand and Capacity										
Dire	ctional Demand Flow Rate, veh/h	289		Opposing Deman	d Flow Rate, veh/h	-					
Peak	Hour Factor	0.88		Total Trucks, %		2.19					
Segr	ment Capacity, veh/h	1700		Demand/Capacity	/ (D/C)	0.17					
Int	ermediate Results			·							
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0					
Spee	ed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674					
PF S	lope Coefficient (m)	-1.29358		PF Power Coefficie	ent (p)	0.75776					
In Pa	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.7					
%lm	provement to Percent Followers	0.0		%Improvement to	Speed	0.0					
Suk	osegment Data										
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h					
1	Tangent	791	-		-	67.7					
Veł	nicle Results										
Aver	age Speed, mi/h	67.7		Percent Followers	, %	39.6					
Segr	ment Travel Time, minutes	0.13		Follower Density (	(FD), followers/mi/ln	1.7					
Vehi	cle LOS	A									
Bic	ycle Results										
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4					
	Rate Outside Lane, veh/h	289		Bicycle Effective V		24					
Bicy	cle LOS Score	2.72		Bicycle Effective S		5.07					
Bicv	cle LOS	С									

		9	Segi	ment 5		
Ve	hicle Inputs					
Seg	ment Type	Passing Zone		Length, ft		3414
Me	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	emand and Capacity					
Dire	ectional Demand Flow Rate, veh/h	289		Opposing Deman	d Flow Rate, veh/h	164
Pea	k Hour Factor	0.88		Total Trucks, %		2.19
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17
Int	termediate Results					
Seg	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	ed Slope Coefficient (m)	4.33614		Speed Power Coe	fficient (p)	0.55020
PF S	Slope Coefficient (m)	-1.16472		PF Power Coefficie	ent (p)	0.83695
In P	Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.4
%In	nprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Su	bsegment Data					
#	Segment Type	Length, ft Ra		dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	3414 -			-	68.3
Ve	hicle Results					
Ave	erage Speed, mi/h	68.3		Percent Followers,	. %	33.7
Seg	ment Travel Time, minutes	0.57		Follower Density (	FD), followers/mi/ln	1.4
Veh	icle LOS	A				
Bio	cycle Results					
Per	cent Occupied Parking	0		Pavement Conditi	on Rating	4
Flov	w Rate Outside Lane, veh/h	289		Bicycle Effective W	/idth, ft	24
Bicy	/cle LOS Score	2.72		Bicycle Effective S	peed Factor	5.07
Bicy	/cle LOS	С				
		9	Segi	ment 6		
Ve	hicle Inputs					
	ment Type	Passing Constrained		Length, ft		286
Me	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	emand and Capacity					
Dire	ectional Demand Flow Rate, veh/h	289		Opposing Deman	d Flow Rate, veh/h	-
Pea	k Hour Factor	0.88		Total Trucks, %		2.19
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17
Int	termediate Results					
Sea	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
9	, <del></del>			1	•	

			1.		1					
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	·	0.41674					
PF Slope Coefficient (m)	-1.29358		PF Power Coefficie	<u> </u>	0.75776					
In Passing Lane Effective Length?	No		Total Segment De		1.7					
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0					
Subsegment Data										
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h					
1 Tangent	286	-		-	67.7					
Vehicle Results										
Average Speed, mi/h	67.7		Percent Followers,	, %	39.6					
Segment Travel Time, minutes	0.05		Follower Density (	FD), followers/mi/ln	1.7					
Vehicle LOS	A									
Bicycle Results										
Percent Occupied Parking	0		Pavement Conditi	on Rating	4					
Flow Rate Outside Lane, veh/h	289		Bicycle Effective V	Vidth, ft	24					
Bicycle LOS Score	2.72		Bicycle Effective S	peed Factor	5.07					
Bicycle LOS	С									
		Segn	nent 7							
Vehicle Inputs										
Segment Type	Passing Constraine	ed	Length, ft		463					
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0					
Demand and Capacity										
Directional Demand Flow Rate, veh/h	286		Opposing Deman	d Flow Rate, veh/h	-					
Peak Hour Factor	0.88		Total Trucks, %		3.08					
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17					
Intermediate Results										
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0					
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674					
PF Slope Coefficient (m)	-1.29347		PF Power Coefficie	ent (p)	0.75789					
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.7					
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0					
Subsegment Data										
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h					
1 Tangent	463	-		-	67.7					
Vehicle Results										
Average Speed, mi/h	67.7		Percent Followers	, %	39.4					
Segment Travel Time, minutes	0.08			FD), followers/mi/ln	1.7					
			1							

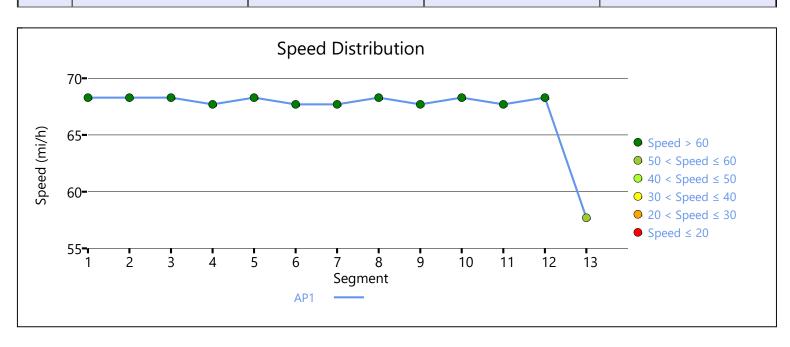
Bicycle Results							
Percent Occupied Parking	0		Pavement Condition	on Rating	4		
Flow Rate Outside Lane, veh/h	286		Bicycle Effective W	/idth, ft	24		
Bicycle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07		
Bicycle LOS	С						
	S	egn	nent 8		<u> </u>		
Vehicle Inputs							
Segment Type	Passing Zone		Length, ft		4822		
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0		
Demand and Capacity							
Directional Demand Flow Rate, veh/h	286		Opposing Demand	d Flow Rate, veh/h	157		
Peak Hour Factor	0.88		Total Trucks, %		3.08		
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17		
Intermediate Results							
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0		
Speed Slope Coefficient (m)			Speed Power Coef		0.55243		
PF Slope Coefficient (m)	-1.14563		PF Power Coefficie	ent (p)	0.84199		
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.4		
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0		
Subsegment Data							
# Segment Type	Length, ft Rad		lius, ft	Superelevation, %	Average Speed, mi/h		
1 Tangent	4822	1-		-	68.3		
Vehicle Results							
Average Speed, mi/h	68.3		Percent Followers,	. %	33.0		
Segment Travel Time, minutes	0.80		Follower Density (	FD), followers/mi/ln	1.4		
Vehicle LOS	А						
Bicycle Results							
Percent Occupied Parking	0		Pavement Condition	on Rating	4		
Flow Rate Outside Lane, veh/h	286		Bicycle Effective W	/idth, ft	24		
Bicycle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07		
Bicycle LOS	С						
	Sc	egn	nent 9				
Vehicle Inputs							
Segment Type	Passing Constrained	g Constrained			861		
Measured FFS	-		Free-Flow Speed,	mi/h	70.0		
Segment Type	Passing Constrained	egn	Length, ft	mi/h			

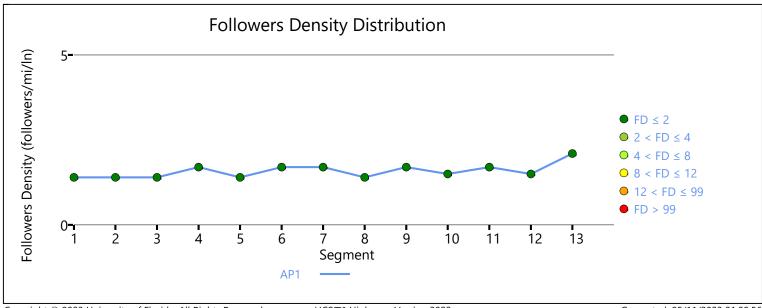
Dire	ctional Demand Flow Rate, veh/h	286	Opposing Demand	Opposing Demand Flow Rate, veh/h					
Peak	Hour Factor	0.88		Total Trucks, %		3.08			
Segr	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17			
Int	ermediate Results								
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0			
Spe	ed Slope Coefficient (m)	4.57372		Speed Power Coef	fficient (p)	0.41674			
PF S	lope Coefficient (m)	-1.29347		PF Power Coefficie	ent (p)	0.75789			
In Pa	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.7			
%lm	provement to Percent Followers	0.0		%Improvement to	Speed	0.0			
Sul	segment Data								
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h			
1	Tangent	861 -			-	67.7			
Vel	nicle Results								
Aver	age Speed, mi/h	67.7		Percent Followers,	. %	39.4			
Segr	nent Travel Time, minutes	0.14		Follower Density (	FD), followers/mi/ln	1.7			
Vehi	cle LOS	А							
Bic	ycle Results								
Perc	ent Occupied Parking	0		Pavement Condition	on Rating	4			
Flow	Rate Outside Lane, veh/h	286		Bicycle Effective W	/idth, ft	24			
Bicy	cle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07			
Bicy	cle LOS	С							
			Segr	ment 10					
Vel	nicle Inputs								
Segr	ment Type	Passing Zone		Length, ft		1556			
Mea	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0			
De	mand and Capacity								
Dire	ctional Demand Flow Rate, veh/h	286		Opposing Demand	d Flow Rate, veh/h	157			
Peak	: Hour Factor	0.88		Total Trucks, %		3.08			
Segr	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17			
Int	ermediate Results								
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0			
Spe	ed Slope Coefficient (m)	4.30647		Speed Power Coef	fficient (p)	0.55243			
PF S	lope Coefficient (m)	-1.21611		PF Power Coefficie	ent (p)	0.81541			
In Pa	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.5			
%lm	provement to Percent Followers	0.0		%Improvement to	0.0				
Sul	osegment Data								
#	Segment Type	Length, ft	Rá	adius, ft	Superelevation, %	Average Speed, mi/h			

1 Tangent	1556	-		-	68.3		
Vehicle Results							
Average Speed, mi/h	68.3		Percent Followers,	. %	35.5		
Segment Travel Time, minutes	0.26		Follower Density (	FD), followers/mi/ln	1.5		
Vehicle LOS	А						
Bicycle Results							
Percent Occupied Parking	0		Pavement Condition	on Rating	4		
Flow Rate Outside Lane, veh/h	286		Bicycle Effective W	/idth, ft	24		
Bicycle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07		
Bicycle LOS	С						
	S	Segn	nent 11				
Vehicle Inputs							
Segment Type	Passing Constrained		Length, ft		799		
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0		
Demand and Capacity							
Directional Demand Flow Rate, veh/h	286		Opposing Deman	d Flow Rate, veh/h	-		
Peak Hour Factor	0.88		Total Trucks, %		3.08		
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17		
Intermediate Results							
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0		
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674		
PF Slope Coefficient (m)	-1.29347		PF Power Coefficie	ent (p)	0.75789		
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.7		
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0		
Subsegment Data							
# Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h		
1 Tangent	799	T-		-	67.7		
Vehicle Results							
Average Speed, mi/h	67.7		Percent Followers,	. %	39.4		
Segment Travel Time, minutes	0.13		Follower Density (	FD), followers/mi/ln	1.7		
Vehicle LOS	А						
Bicycle Results							
Percent Occupied Parking	0		Pavement Condition	on Rating	4		
Flow Rate Outside Lane, veh/h	286		Bicycle Effective W	/idth, ft	24		
Bicycle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07		
Bicycle LOS	С						
	S	Segn	nent 12				

Ve	chicle Inputs							
Seg	gment Type	Passing Zone		Length, ft		857		
Me	easured FFS	Measured		Free-Flow Speed,	70.0			
De	emand and Capacity							
Dir	ectional Demand Flow Rate, veh/h	286		Opposing Demand	d Flow Rate, veh/h	157		
Pea	ak Hour Factor	0.88		Total Trucks, %		3.08		
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17		
In	termediate Results							
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0		
Spe	eed Slope Coefficient (m)	4.30206		Speed Power Coef	fficient (p)	0.55243		
PF	Slope Coefficient (m)	-1.22789		PF Power Coefficie	ent (p)	0.81007		
In I	Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.5		
%lr	mprovement to Percent Followers	0.0		%Improvement to	Speed	0.0		
Su	ıbsegment Data							
#	Segment Type	Length, ft		dius, ft	Superelevation, %	Average Speed, mi/h		
1	Tangent	857 -			-	68.3		
Ve	hicle Results		<u>'</u>			•		
Ave	erage Speed, mi/h	68.3		Percent Followers,	%	36.0		
Seg	gment Travel Time, minutes	0.14		Follower Density (	FD), followers/mi/ln	1.5		
Vel	nicle LOS	А						
Bi	cycle Results					·		
Per	cent Occupied Parking	0		Pavement Condition	on Rating	4		
Flo	w Rate Outside Lane, veh/h	286		Bicycle Effective W	/idth, ft	24		
Bic	ycle LOS Score	2.95		Bicycle Effective S	5.07			
Bic	ycle LOS	С						
			Segm	nent 13				
Ve	chicle Inputs							
Seg	gment Type	Passing Constrained	d	Length, ft		1288		
Me	easured FFS	Measured		Free-Flow Speed,	mi/h	60.0		
De	emand and Capacity							
Dir	ectional Demand Flow Rate, veh/h	286		Opposing Demand	d Flow Rate, veh/h	-		
Pea	ak Hour Factor	0.88		Total Trucks, %		3.08		
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17		
In	termediate Results							
Seg	gment Vertical Class	1		Free-Flow Speed,	60.0			
Spe	eed Slope Coefficient (m)	4.57372		Speed Power Coef	fficient (p)	0.41674		
PF	Slope Coefficient (m)	-1.39671		PF Power Coefficie	ent (p)	0.73647		

In Pa	ssing Lane Effective Length?	No		Total Segment De	ensity, veh/mi/ln	2.1		
%lmp	provement to Percent Followers	0.0		%Improvement to	o Speed	0.0		
Sub	segment Data							
#	Segment Type	Length, ft	Radiu	us, ft	Superelevation, %	Average Speed, mi/h		
1	Tangent	1288	-		-	57.7		
Veh	icle Results							
Avera	age Speed, mi/h	57.7	Т	Percent Followers	5, %	42.7		
Segn	nent Travel Time, minutes	0.25	0.25		(FD), followers/mi/ln	2.1		
Vehic	ile LOS	В						
Bicy	cle Results							
Perce	ent Occupied Parking	0	0		ion Rating	4		
Flow	Rate Outside Lane, veh/h	286		Bicycle Effective V	Width, ft	24		
Bicyc	le LOS Score	2.86		Bicycle Effective S	Speed Factor	4.79		
Bicyc	le LOS	С						
Faci	ility Results							
Т	VMT veh-mi/p		VHD veh-h/p		ensity, followers/ mi/ln	LOS		
1	382	0.1	5		1.5	A		



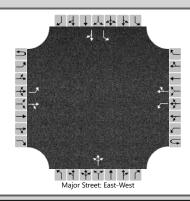


Copyright © 2023 University of Florida. All Rights Reserved.

HCSTM Highways Version 2022 WB\_38\_WHartford\_2050PM.xuf

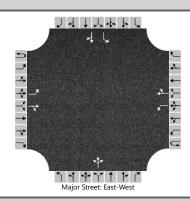
Generated: 05/11/2023 21:09:56

	HCS Two-Way Stop-Control Report												
General Information		Site Information											
Analyst	NM	Intersection	SD 38 & SD 19										
Agency/Co.	HRG	Jurisdiction	SDDOT										
Date Performed	4/30/2024	East/West Street	SD 38										
Analysis Year	2050	North/South Street	SD 19										
Time Analyzed	AM Peak	Peak Hour Factor	0.92										
Intersection Orientation	East-West	Analysis Time Period (hrs) 0.25											
Project Description	SD 38												



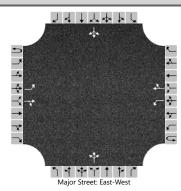
Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound			Westbound				North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		1	1	0	
Configuration	1	L		TR		L		TR			LTR			L		TR	
Volume (veh/h)	1	55	165	0		0	120	50		10	5	10		70	0	95	
Percent Heavy Vehicles (%)	1	30				3				3	3	3		9	3	11	
Proportion Time Blocked	1																
Percent Grade (%)	1									(	0			(	0		
Right Turn Channelized	1																
Median Type   Storage	Undi	vided															
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)	1	4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2	
Critical Headway (sec)		4.40				4.13				7.13	6.53	6.23		7.19	6.53	6.31	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.47				2.23				3.53	4.03	3.33		3.58	4.03	3.40	
Delay, Queue Length, an	d Leve	l of S	ervice														
Flow Rate, v (veh/h)	<u> </u>	60				0					27			76		103	
Capacity, c (veh/h)	1	1238				1390					524			461		865	
v/c Ratio		0.05				0.00					0.05			0.16		0.12	
95% Queue Length, Q <sub>95</sub> (veh)	1	0.2				0.0					0.2			0.6		0.4	
Control Delay (s/veh)	1	8.1	0.2	0.2		7.6	0.0	0.0			12.2			14.3		9.7	
Level of Service (LOS)	1	А	А	А		А	А	А			В			В		А	
Approach Delay (s/veh)		2	.2			0	.0			12.2				11.7			
Approach LOS	1		Ą			,	4				В				В		

	HCS Two-Way Stop-Control Report											
General Information		Site Information										
Analyst	NM	Intersection	SD 38 & SD 19									
Agency/Co.	HRG	Jurisdiction	SDDOT									
Date Performed	4/30/2024	East/West Street	SD 38									
Analysis Year	2050	North/South Street	SD 19									
Time Analyzed	PM Peak	Peak Hour Factor	0.92									
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25									
Project Description	SD 38											



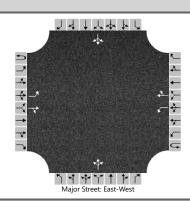
Vehicle Volumes and Adjustments																
Approach		Eastb	ound			Westl	ound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		1	1	0
Configuration		L		TR		L		TR			LTR			L		TR
Volume (veh/h)		85	115	0		0	170	80		10	5	10		40	0	50
Percent Heavy Vehicles (%)		2				3				3	3	3		10	3	14
Proportion Time Blocked																
Percent Grade (%)	1									(	)			(	)	
Right Turn Channelized																
Median Type   Storage	ian Type   Storage Un															
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.12				4.13				7.13	6.53	6.23		7.20	6.53	6.34
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.22				2.23				3.53	4.03	3.33		3.59	4.03	3.43
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1	92				0					27			43		54
Capacity, c (veh/h)		1291				1455					498			395		782
v/c Ratio		0.07				0.00					0.05			0.11		0.07
95% Queue Length, Q <sub>95</sub> (veh)		0.2				0.0					0.2			0.4		0.2
Control Delay (s/veh)		8.0	0.2	0.2		7.5	0.0	0.0			12.6			15.2		9.9
Level of Service (LOS)		А	А	А		А	А	А			В			С		А
Approach Delay (s/veh)		3	.5			0	.0		12.6				12.3			
Approach LOS		,	4			,	A				В			1	3	

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & 459th
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	459th Ave
Time Analyzed	AM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



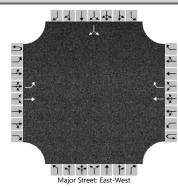
Vehicle Volumes and Ad	iustme	nts														
Approach		Eastk	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0
Configuration		L		TR		L		TR			LTR				LTR	
Volume (veh/h)	<b>1</b>	0	215	7		2	155	0		15	0	7		9	0	0
Percent Heavy Vehicles (%)		3				3				13	0	0		0	0	0
Proportion Time Blocked																
Percent Grade (%)											)			(	0	
Right Turn Channelized	<b>1</b>															
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	1	4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.23	6.50	6.20		7.10	6.50	6.20
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.62	4.00	3.30		3.50	4.00	3.30
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		0				2					24				10	
Capacity, c (veh/h)		1403				1319					596				546	
v/c Ratio		0.00				0.00					0.04				0.02	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.1				0.1	
Control Delay (s/veh)		7.6	0.0	0.0		7.7	0.0	0.0			11.3				11.7	
Level of Service (LOS)		А	А	А		А	А	А			В				В	
Approach Delay (s/veh)		0	.0			0	.1	•		11	1.3			1	1.7	•
Approach LOS			A			,	4			ı	3			l	В	

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & 459th
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	459th Ave
Time Analyzed	PM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



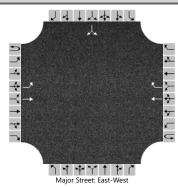
Vehicle Volumes and Adju	ıstme	nents														
Approach		Eastb	ound			West	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0
Configuration		L		TR		L		TR			LTR				LTR	
Volume (veh/h)		0	145	9		15	245	2		15	0	4		2	2	0
Percent Heavy Vehicles (%)		0				0				13	0	0		0	100	0
Proportion Time Blocked																
Percent Grade (%)										(	0			(	0	
Right Turn Channelized																
Median Type   Storage				Undi	vided											
<b>Critical and Follow-up He</b>	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.10				4.10				7.23	6.50	6.20		7.10	7.50	6.20
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.20				3.62	4.00	3.30		3.50	4.90	3.30
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)		0				16					21				4	
Capacity, c (veh/h)		1307				1423					534				427	
v/c Ratio		0.00				0.01					0.04				0.01	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.1				0.0	
Control Delay (s/veh)		7.8	0.0	0.0		7.6	0.1	0.1			12.0				13.5	
Level of Service (LOS)		А	А	А		А	А	А			В				В	
Approach Delay (s/veh)		0.0 0.5							12	2.0		13.5				
Approach LOS		,	4			,	4			ı	В			1	В	

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & I-90 Speedway
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	I-90 Expressway
Time Analyzed	AM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



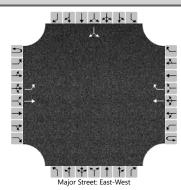
					iviaji	or otreet. La	st-vvest										
Vehicle Volumes and Adj	ustme	stments															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		0	1	0	
Configuration		L	Т				Т	R							LR		
Volume (veh/h)		0	230				165	0						0		0	
Percent Heavy Vehicles (%)		3												3		3	
Proportion Time Blocked																	
Percent Grade (%)														(	0		
Right Turn Channelized		No															
Median Type   Storage				Undi	vided												
Critical and Follow-up Ho	eadwa	ys															
Base Critical Headway (sec)		4.1												7.1		6.2	
Critical Headway (sec)		4.13												6.43		6.23	
Base Follow-Up Headway (sec)		2.2												3.5		3.3	
Follow-Up Headway (sec)		2.23												3.53		3.33	
Delay, Queue Length, and	d Leve	l of Se	ervice	)													
Flow Rate, v (veh/h)	]	0													0		
Capacity, c (veh/h)		1390													0		
v/c Ratio		0.00															
95% Queue Length, Q <sub>95</sub> (veh)		0.0			Ì												
Control Delay (s/veh)		7.6	0.0														
Level of Service (LOS)		А	А		Ì												
Approach Delay (s/veh)		0.0															
Approach LOS		A															

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & I-90 Speedway
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	I-90 Expressway
Time Analyzed	AM Peak - Event Traffic	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



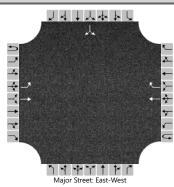
					iviajo	or otreet. La	3t-vvest											
Vehicle Volumes and Adj	ustme	nts																
Approach		Eastb	ound			Westl	oound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		0	1	0		
Configuration		L	Т				Т	R							LR			
Volume (veh/h)		0	412				295	0						0		0		
Percent Heavy Vehicles (%)		3												3		3		
Proportion Time Blocked																		
Percent Grade (%)														(	0			
Right Turn Channelized		No																
Median Type   Storage	1			Undi	vided													
<b>Critical and Follow-up He</b>	eadwa	ys																
Base Critical Headway (sec)	1	4.1												7.1		6.2		
Critical Headway (sec)		4.13												6.43		6.23		
Base Follow-Up Headway (sec)		2.2												3.5		3.3		
Follow-Up Headway (sec)		2.23												3.53		3.33		
Delay, Queue Length, and	d Leve	l of Se	ervice															
Flow Rate, v (veh/h)	1	0													0			
Capacity, c (veh/h)	1	1234													0			
v/c Ratio	1	0.00																
95% Queue Length, Q <sub>95</sub> (veh)		0.0																
Control Delay (s/veh)	1	7.9	0.0															
Level of Service (LOS)		А	А															
Approach Delay (s/veh)	0.0																	
Approach LOS		A																

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & I-90 Speedway
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	I-90 Expressway
Time Analyzed	PM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



Vehicle Volumes and Ad	iuctmo	ntc															
	Justine		ound			\\/oc+l	bound			North	bound			Court	bound		
Approach				<b>—</b>		vvesti		<b>.</b>		NOITH							
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R	
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	] 0	1	1	0	0	0	1	1		0	0	0		0	1	0	
Configuration		L	Т				Т	R							LR		
Volume (veh/h)		0	165				260	0						0		0	
Percent Heavy Vehicles (%)	]	3												3		3	
Proportion Time Blocked	]																
Percent Grade (%)														(	0		
Right Turn Channelized						Ν	10										
Median Type   Storage	<u> </u>			Undi	vided												
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)	1	4.1												7.1		6.2	
Critical Headway (sec)		4.13												6.43		6.23	
Base Follow-Up Headway (sec)	1	2.2												3.5		3.3	
Follow-Up Headway (sec)		2.23												3.53		3.33	
Delay, Queue Length, an	d Leve	l of S	ervice														
Flow Rate, v (veh/h)	<u> </u>	0													0		
Capacity, c (veh/h)	1	1274													0		
v/c Ratio	1	0.00															
95% Queue Length, Q <sub>95</sub> (veh)	1	0.0															
Control Delay (s/veh)		7.8	0.0														
Level of Service (LOS)		А	А														
Approach Delay (s/veh)		0.0															
Approach LOS			A														

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & I-90 Speedway
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	I-90 Expressway
Time Analyzed	PM Peak - Event Traffic	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		

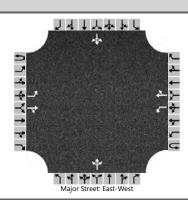


					iviajo	JI JULEU. La	3t-vvest									
Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		0	1	0
Configuration		L	Т				Т	R							LR	
Volume (veh/h)		0	295				465	0						0		0
Percent Heavy Vehicles (%)		3												3		3
Proportion Time Blocked																
Percent Grade (%)														(	0	
Right Turn Channelized		No														
Median Type   Storage	1	Undivided														
Critical and Follow-up Ho	eadwa	ys														
Base Critical Headway (sec)	1	4.1												7.1		6.2
Critical Headway (sec)		4.13												6.43		6.23
Base Follow-Up Headway (sec)		2.2												3.5		3.3
Follow-Up Headway (sec)		2.23												3.53		3.33
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1	0													0	
Capacity, c (veh/h)	1	1054													0	
v/c Ratio	1	0.00														
95% Queue Length, Q <sub>95</sub> (veh)	1	0.0														
Control Delay (s/veh)	1	8.4	0.0													
Level of Service (LOS)		А	А													
Approach Delay (s/veh)	0.0															
Approach LOS	1	A														

### **HCS Signalized Intersection Results Summary** 144444 Intersection Information **General Information** Agency HRG Duration, h 0.250 CEC Analyst Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & Western Ave File Name (4) SD38&463WesternAve AM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 90 Demand (v), veh/h 9 180 80 60 110 30 65 75 40 80 5 **Signal Information** Cycle, s 50.0 Reference Phase 6 Offset, s 0 Reference Point End Green 30.3 9.7 0.0 0.0 0.0 0.0 Uncoordinated No Simult. Gap E/W On Yellow 4.0 4.0 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 1.0 1.0 0.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL **SBT Assigned Phase** 2 6 8 4 Case Number 6.0 6.0 6.0 6.0 Phase Duration, s 35.3 35.3 14.7 14.7 Change Period, (Y+Rc), s 5.0 5.0 5.0 5.0 Max Allow Headway ( MAH ), s 0.0 0.0 4.3 4.3 Queue Clearance Time ( $g_s$ ), s 7.0 8.7 Green Extension Time ( $g_e$ ), s 0.0 0.0 1.2 1.1 Phase Call Probability 1.00 1.00 0.03 0.07 Max Out Probability **Movement Group Results** EΒ WB NB SB Approach Movement L Т R L Т R L Т R Т L R 12 **Assigned Movement** 5 2 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 10 283 65 152 71 179 43 92 1225 1666 1088 1693 1614 1205 1684 Adjusted Saturation Flow Rate ( s ), veh/h/ln 1180 0.2 4.0 1.5 2.0 2.7 5.0 1.7 2.3 Queue Service Time ( $g_s$ ), s Cycle Queue Clearance Time ( g c ), s 2.2 4.0 5.6 2.0 5.0 5.0 6.7 2.3 Green Ratio (g/C) 0.61 0.61 0.61 0.61 0.19 0.19 0.19 0.19 Capacity (c), veh/h 836 1007 713 1023 320 315 259 329 Volume-to-Capacity Ratio (X) 0.012 0.281 0.092 0.149 0.220 0.569 0.168 0.281 Back of Queue (Q), ft/ln (95 th percentile) 1.6 46.2 13.6 22.3 34.3 80.9 20.5 39.4 Back of Queue (Q), veh/ln (95 th percentile) 0.1 1.8 0.5 0.9 1.2 3.2 8.0 1.5 Queue Storage Ratio (RQ) (95 th percentile) 0.01 0.00 0.05 0.00 0.14 0.00 80.0 0.00 Uniform Delay ( d 1 ), s/veh 4.8 4.7 6.1 4.3 19.2 18.2 21.2 17.1 Incremental Delay ( d 2 ), s/veh 0.0 0.7 0.3 0.3 0.3 1.6 0.3 0.5 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 4.8 5.4 6.3 4.6 19.6 19.8 21.5 17.6 Level of Service (LOS) Α Α Α Α В В С В 5.4 5.1 19.7 18.8 Approach Delay, s/veh / LOS Α Α В В Intersection Delay, s/veh / LOS 11.4 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 1.85 В 1.85 1.91 В 1.91 В В Bicycle LOS Score / LOS 0.97 Α 0.85 Α 0.90 Α 0.71 Α

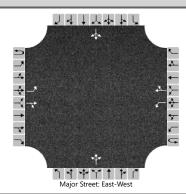
### **HCS Signalized Intersection Results Summary** 144444 Intersection Information **General Information** Agency HRG Duration, h 0.250 CEC Analyst Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period PM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & Western Ave File Name (4) SD38&463WesternAve PM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 60 Demand (v), veh/h 15 125 55 120 200 70 85 155 55 100 25 **Signal Information** Cycle, s 50.0 Reference Phase 6 Offset, s 0 Reference Point End Green 26.5 0.0 13.5 0.0 0.0 0.0 Uncoordinated No Simult. Gap E/W On Yellow 4.0 4.0 0.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 1.0 1.0 0.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL **SBT Assigned Phase** 2 6 8 4 Case Number 6.0 6.0 6.0 6.0 Phase Duration, s 31.5 31.5 18.5 18.5 Change Period, (Y+Rc), s 5.0 5.0 5.0 5.0 Max Allow Headway ( MAH ), s 0.0 0.0 4.3 4.3 Queue Clearance Time ( $g_s$ ), s 9.8 12.3 Green Extension Time ( $g_e$ ), s 0.0 0.0 1.5 1.2 Phase Call Probability 1.00 1.00 0.18 0.43 Max Out Probability **Movement Group Results** EΒ WB NB SB Approach Movement L Т R L Т R L Т R Т L R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 16 196 130 283 76 261 60 136 923 1680 1178 1701 1253 1474 1119 1683 Adjusted Saturation Flow Rate ( s ), veh/h/ln 0.5 3.1 3.3 4.7 2.6 7.8 2.5 3.2 Queue Service Time ( $g_s$ ), s 6.5 Cycle Queue Clearance Time ( g c ), s 5.3 3.1 4.7 5.7 7.8 10.3 3.2 0.27 0.27 Green Ratio (g/C) 0.53 0.53 0.53 0.53 0.27 0.27 Capacity (c), veh/h 545 889 693 901 404 399 272 455 Volume-to-Capacity Ratio (X) 0.030 0.220 0.188 0.314 0.188 0.654 0.219 0.298 Back of Queue (Q), ft/ln (95 th percentile) 5 41.2 34.4 63.5 30.4 118.8 27.9 50.3 Back of Queue (Q), veh/ln (95 th percentile) 0.2 1.6 1.3 2.5 1.2 4.4 1.1 2.0 Queue Storage Ratio (RQ) (95 th percentile) 0.02 0.00 0.14 0.00 0.12 0.00 0.11 0.00 Uniform Delay ( d 1 ), s/veh 8.1 6.3 8.0 6.6 16.7 16.2 20.7 14.5 Incremental Delay ( d 2 ), s/veh 0.1 0.6 0.6 0.9 0.2 1.8 0.4 0.4 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 8.2 6.8 8.6 7.5 16.9 18.0 21.1 14.8 Level of Service (LOS) Α Α Α Α В В С В 6.9 7.9 17.8 Approach Delay, s/veh / LOS Α Α В 16.8 В Intersection Delay, s/veh / LOS 12.1 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 1.87 В 1.87 1.90 1.90 В В В Bicycle LOS Score / LOS 0.84 Α 1.17 Α 1.04 Α 0.81 Α

	HCS Two-Way Stop-Control Report												
General Information		Site Information											
Analyst	NM	Intersection	SD 38 & Main Ave										
Agency/Co.	HRG	Jurisdiction	SDDOT										
Date Performed	5/8/2023	East/West Street	SD 38										
Analysis Year	2050	North/South Street	Main Ave (9th St)										
Time Analyzed	AM Peak	Peak Hour Factor	0.92										
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25										
Project Description	SD 38												



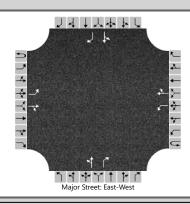
Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound			Westl	oound		Northbound				Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0	
Configuration		L		TR		L		TR			LTR				LTR		
Volume (veh/h)		2	260	30		40	195	20		40	5	85		6	10	4	
Percent Heavy Vehicles (%)		0				11				5	0	2		0	17	0	
Proportion Time Blocked																	
Percent Grade (%)										(	)		0				
Right Turn Channelized																	
Median Type   Storage				Left	Only							!	9				
Critical and Follow-up He	eadwa	ys															
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2	
Critical Headway (sec)		4.10				4.21				7.15	6.50	6.22		7.10	6.67	6.20	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.20				2.30				3.55	4.00	3.32		3.50	4.15	3.30	
Delay, Queue Length, and	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)		2				43					141				22		
Capacity, c (veh/h)		1346				1196					678				459		
v/c Ratio		0.00				0.04					0.21				0.05		
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.1					0.8				0.1		
Control Delay (s/veh)		7.7				8.1					11.7				13.2		
Level of Service (LOS)		А				А					В				В		
Approach Delay (s/veh)		0	.1			1	.3			11	.7			13.2			
Approach LOS		,	4			-	4			ı	3			ı	В		

HCS Two-Way Stop-Control Report												
General Information		Site Information										
Analyst	NM	Intersection	SD 38 & Main Ave									
Agency/Co.	HRG	Jurisdiction	SDDOT									
Date Performed	4/30/2024	East/West Street	SD 38									
Analysis Year	2050	North/South Street	Main Ave (9th St)									
Time Analyzed	PM Peak	Peak Hour Factor	0.92									
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25									
Project Description	SD 38											



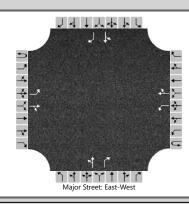
Vehicle Volumes and Ad	iustme	nts															
Approach		Eastb	ound			Westl	oound			North	bound			Southbound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0	
Configuration		L		TR		L		TR			LTR				LTR		
Volume (veh/h)		10	250	45		65	335	60		35	20	55		40	30	7	
Percent Heavy Vehicles (%)		0				0				5	0	0		0	0	0	
Proportion Time Blocked																	
Percent Grade (%)											)		0				
Right Turn Channelized																	
Median Type   Storage		Left Only 9															
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)	_	4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2	
Critical Headway (sec)		4.10				4.10				7.15	6.50	6.20		7.10	6.50	6.20	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.20				2.20				3.55	4.00	3.30		3.50	4.00	3.30	
Delay, Queue Length, an	d Leve	l of S	ervice														
Flow Rate, v (veh/h)	<u> </u>	11				71					120				84		
Capacity, c (veh/h)		1141				1251					467				368		
v/c Ratio		0.01				0.06					0.26				0.23		
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.2					1.0				0.9		
Control Delay (s/veh)		8.2				8.1					15.3				17.6		
Level of Service (LOS)		А		Ì		А					С				С		
Approach Delay (s/veh)		0	.3			1	.1			1!	5.3			1	7.6		
Approach LOS	1		A			,	4			(	C			С			

HCS Two-Way Stop-Control Report												
General Information		Site Information										
Analyst	NM	Intersection	SD 38 & Vandemark Ave									
Agency/Co.	HRG	Jurisdiction	SDDOT									
Date Performed	4/30/2024	East/West Street	SD 38									
Analysis Year	2050	North/South Street	Vandemark Avenue									
Time Analyzed	АМ	Peak Hour Factor	0.92									
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25									
Project Description	SD 38											



Vehicle Volumes and Adju	ıstme	nts															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	1	1	0		0	1	1		0	1	1	
Configuration		L		TR		L		TR		LT		R		LT		R	
Volume (veh/h)		25	370	10		8	240	25		9	5	10		40	2	25	
Percent Heavy Vehicles (%)		0				0				40	0	0		0	0	7	
Proportion Time Blocked																	
Percent Grade (%)										(	0 No						
Right Turn Channelized										Ν	lo			Ν	lo		
Median Type   Storage				Undi	vided												
Critical and Follow-up He	adwa	ys															
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2	
Critical Headway (sec)		4.10				4.10				7.50	6.50	6.20		7.10	6.50	6.27	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.20				2.20				3.86	4.00	3.30		3.50	4.00	3.36	
Delay, Queue Length, and	l Leve	l of Se	ervice														
Flow Rate, v (veh/h)	1	27				9				15		11		46		27	
Capacity, c (veh/h)		1286				1157				278		648		306		752	
v/c Ratio		0.02				0.01				0.05		0.02		0.15		0.04	
95% Queue Length, Q <sub>95</sub> (veh)	1	0.1				0.0				0.2		0.1		0.5		0.1	
Control Delay (s/veh)		7.9	0.1	0.1		8.1	0.1	0.1		18.7		10.7		18.8		10.0	
Level of Service (LOS)		А	А	А		А	А	А		С		В		С		А	
Approach Delay (s/veh)		0	.6			0	.3			15	5.4			1:	5.5		
Approach LOS		,	Α			,	4			(	С				С		

HCS Two-Way Stop-Control Report												
General Information		Site Information										
Analyst	NM	Intersection	SD 38 & Vandemark Ave									
Agency/Co.	HRG	Jurisdiction	SDDOT									
Date Performed	4/30/2024	East/West Street	SD 38									
Analysis Year	2050	North/South Street	Vandemark Avenue									
Time Analyzed	PM	Peak Hour Factor	0.92									
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25									
Project Description	SD 38											

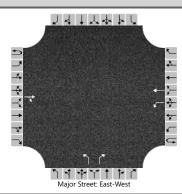


Vehicle Volumes and Adju	ıstme	nts															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	T	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	1	1	0		0	1	1		0	1	1	
Configuration		L		TR		L		TR		LT		R		LT		R	
Volume (veh/h)		20	255	4		5	475	45		0	0	9		30	0	25	
Percent Heavy Vehicles (%)		0				0				0	0	100		0	0	7	
Proportion Time Blocked																	
Percent Grade (%)										(	0		0				
Right Turn Channelized	j									Ν	lo			Ν	lo		
Median Type   Storage				Undi	vided												
<b>Critical and Follow-up He</b>	adwa	ys															
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2	
Critical Headway (sec)		4.10				4.10				7.10	6.50	7.20		7.10	6.50	6.27	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.20				2.20				3.50	4.00	4.20		3.50	4.00	3.36	
Delay, Queue Length, and	Leve	l of Se	ervice														
Flow Rate, v (veh/h)		22				5				0		10		33		27	
Capacity, c (veh/h)		1017				1293				0		574		259		532	
v/c Ratio		0.02				0.00						0.02		0.13		0.05	
95% Queue Length, Q <sub>95</sub> (veh)		0.1				0.0						0.1		0.4		0.2	
Control Delay (s/veh)		8.6	0.2	0.2		7.8	0.0	0.0				11.4		20.9		12.1	
Level of Service (LOS)		Α	Α	А		Α	Α	А				В		С		В	
Approach Delay (s/veh)		0	.8			0	.1							16	5.9		
Approach LOS		,	4			,	4								С		

		HCS	Sigr	nalized	d Int	ersec	tion R	esul	ts Sui	mmary	/				
															No.
General Inform	nation	v						$\rightarrow$		ction Inf	W.				Ja lu
Agency		HRG							Duratio	n, h	0.250	)			
Analyst		NM		Analys	is Dat	e May	8, 2023		Area Ty	ре	Other	r	±,		. 2
Jurisdiction		SDDOT		Time F	Period	AM F	Peak		PHF		0.92		4 4	W <del>↑</del> E 8	<b>~</b>
Urban Street		SD 38		Analys	is Yea	ır 2050			Analysi	s Period	1> 7:	15	7		*
Intersection		SD 38 & 2nd Street		File Na	ame	(7) S	D38&2n	d_AM.	xus					7 1	
Project Descrip	tion													ነ 4 ሰቀጥ	ተሰ
Demand Inform	nation				EB			WI	3		NB			SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Demand ( v ), v				20	325	10	95	20	0 15	5	20	155	35	50	25
,,															
Signal Informa	tion									$\neg \neg$					<b>T</b>
Cycle, s	45.0	Reference Phase	6		Ħ.	54	2					_	Ә∴	2	xtz
Offset, s	0	Reference Point	End	Green	27 9	8.1	0.0	0.0	0.0	0.0		1	X 2	3	4
Uncoordinated	No	Simult. Gap E/W	On	Yellow		3.5	0.0	0.0					<b>→</b>		<b>K</b> D
Force Mode	Fixed	Simult. Gap N/S	ult. Gap N/S On Red 1.0 1.0 0.0 0.0 0.0 0.0					5	6	7	<b>Y</b> 8				
Timer Results				EBL	<u> </u>	EBT	WB	1	WBT	NB	1	NBT	SB	1	SBT
Assigned Phase					-	2	1	-	6	140	_	8	OB	_	4
Case Number				_		8.0			6.0	+		6.0	-		8.0
Phase Duration					-	32.4	_		32.4	-		12.6	-	_	12.6
				_	_		-		4.5	+		4.5	-	_	4.5
	Change Period, ( Y+R c ), s  Max Allow Headway ( MAH ), s				4.5		-	-	0.0	-	_	3.3	-	-	3.3
Queue Clearan				_	_	0.0	-		0.0	+		7.8	-	_	7.5
		, = ,		-	+	0.0	-	-	0.0	+		0.5	-	-	0.5
Green Extensio		( <i>g e</i> ), s		-	_	0.0	$\vdash$		0.0	+		0.98	+	_	0.5
Phase Call Prol				-	-		-			-			-	_	
Max Out Proba	DIIILY				_			_				0.01			0.00
Movement Gro	up Res	sults			EB			WB		т	NB		П	SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	T	R
Assigned Move	ment			5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow F	Rate ( v	), veh/h			386		103	234		5	190			120	
Adjusted Satura	ation Flo	ow Rate ( s ), veh/h/l	n		1743		1018	1750		1317	1528			1105	
Queue Service	Time (	g s ), S			0.0		2.5	2.6		0.2	5.2			0.2	
Cycle Queue C	learanc	e Time ( <i>g c</i> ), s			4.8	1	7.3	2.6		5.8	5.2			5.5	
Green Ratio ( g		, - ,			0.62		0.62	0.62		0.18	0.18			0.18	
Capacity ( c ), v					1162		681	1082		236	278			306	
Volume-to-Capa		itio (X)			0.332		0.152	_	_	0.023				0.391	
		/In (95 th percentile	)												
		eh/ln ( 95 th percenti			1.9		0.8	1.1		0.1	2.9			1.7	1
		RQ) (95 th percent	,		0.00		0.08	0.00		0.02	0.00			0.00	
Uniform Delay (		, ,	,		4.2		6.0	3.8		20.1	17.2			16.3	1
Incremental De					0.8		0.5	0.5		0.0	1.1			0.3	
		•			0.0		0.0	0.0		0.0	0.0			0.0	
	nitial Queue Delay ( <i>d</i> <sub>3</sub> ), s/veh Control Delay ( <i>d</i> ), s/veh				5.0		6.4	4.2		20.1	18.3			16.6	
Level of Service (LOS)					A		A	Α		C	В			В	
	Approach Delay, s/veh / LOS			5.0		A	4.9		A	18.		В	16.		В
Intersection De				0.0			3.8			1 . 3.			A		_
	, 5, 70														
Multimodal Re	sults				EB			WB			NB			SB	
Pedestrian LOS		/ LOS		1.84	-	В	1.62	-	В	1.9		В	1.6		В
	LOS Score / LOS			1.12		A	1.04		A	0.8		A	0.6		A

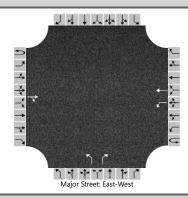
		HCS	Sigr	nalize	d Inte	ersect	ion R	esult	ts Sun	nmary	/				
General Inform	nation							T	Intersec	tion Inf	ormatic	nn		l al alaba t	Ja lj
Agency	lation	HRG							Duration		0.250			*	
Analyst		NM		Analys	eie Date	e May 8	2 2023		Area Typ		Other		_* _\$		t. A
Jurisdiction		SDDOT		Time F		PM P		_	PHF		0.92		→ 	w. He	<b>-</b> ≥}-
Urban Street		SD 38			sis Yea		ean		Analysis	Poriod	1> 7:	15	-4		· •
Intersection		SD 38 & 2nd Street		File Na			038&2n			renou	1- 7.	13			<u></u>
Project Descript	tion	3D 36 & Zhu Sileet		File IN	ame	(1) 31	JOOKZIII	u_FIVI.	xu5				-	1 4 4 4	\$* (*
1 Toject Descript	lion														
Demand Inforn	nation				EB			WE	3	$\top$	NB		$\top$	SB	
Approach Move	ment			L	Т	R	L	Т	R	L	T	R	L	Т	R
Demand ( v ), v	eh/h			25	235	9	130	490	) 25	15	25	65	15	30	20
Signal Informa	tion				1.								_		$\perp$
Cycle, s	60.0	Reference Phase	2		Ħ	T 54	2					1	↔ ,	3	x1x
Offset, s	0	Reference Point	End	Green	44.3	6.7	0.0	0.0	0.0	0.0			K		
Uncoordinated	No	Simult. Gap E/W	On	Yellow		3.5	0.0	0.0	0.0	0.0			<b>₹</b>		₩.
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.0	0.0	0.0		5	6	7	8
							11						11		
Timer Results				EBI	-	EBT	WB	L	WBT	NB	L	NBT	SB	L	SBT
Assigned Phase	e			$\vdash$	_	2		_	6			8			4
Case Number						8.0		_	6.0			6.0			8.0
	Phase Duration, s					48.8			48.8			11.2	<u> </u>		11.2
	Change Period, ( Y+R c ), s					4.5			4.5			4.5			4.5
Max Allow Head	dway( <i>N</i>	<i>MAH</i> ), s				0.0			0.0			3.2			3.2
Queue Clearan	ce Time	e ( g s ), s										6.2			5.6
Green Extensio	n Time	( g e ), s				0.0			0.0			0.3			0.3
Phase Call Prob	bability											0.95			0.95
Max Out Probal	bility											0.00			0.00
		14						\A/D			ND			0.0	
Movement Gro		suits			EB	T 5		WB	T 5		NB		<b>.</b>	SB	
Approach Move				<u> </u>	T	R	L	T	R	L	T	R	L	T	R
Assigned Move		\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \		5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow F		,·		_	292		141	560	-	16	98		-	71	
		ow Rate ( s ), veh/h/l	n	_	1663		1114	1757	+	1350	1568	-	-	1377	
Queue Service		- ,		_	0.0	-	2.7	7.3	+	0.7	3.5	_	-	0.0	
Cycle Queue Cl		e Time ( <i>g ε</i> ), s		_	3.1	-	5.9	7.3	+	4.2	3.5	-	-	3.6	
Green Ratio ( g.				_	0.74		0.74	0.74	_	0.11	0.11		-	0.11	
Capacity ( c ), v				-	1294	_	885	1298	_	190	175		-	227	
Volume-to-Capa			\		0.226		0.160	0.431		0.086	0.560			0.311	
		/In 95 th percentile		_	10	-	0.0	0.5	+	0.4	0.0		-	1.0	
	• •	eh/ln (95 th percenti	,	-	1.0		0.8	2.5	-	0.4	2.3			1.6	
		RQ) (95 th percent	.iie)		0.00		0.08	0.00	-	0.07	0.00			0.00	
Uniform Delay (					2.5		3.4	3.0		27.3	25.3			24.7	
Incremental Del					0.4		0.4	1.0	-	0.1	1.0			0.3	
Initial Queue De					0.0		0.0	0.0		0.0	0.0			0.0	
Control Delay (					2.9		3.8	4.1		27.4	26.3			25.0	
Level of Service				0.0	A	^	A 4.0	Α	^	C	С		05.	C	
Approach Delay				2.9		Α	4.0		A	26.5	0	С	25.0	J	С
Intersection Del	ay, s/ve	en / LOS				7	'.2						A		
Multimodal Re	eulte				EB			WB			NB			SB	
Pedestrian LOS		/LOS		1.83		В	1.60		В	1.92		В	1.70		В
Bicycle LOS Sc				0.97		A	1.64		В	0.68		A	0.60		A
Dioyole LOG GC	JIO / LC	,,		0.97		/1	1.04			0.00		, <b>,</b> ,	0.00		7.

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & West Central HS Entrance
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	West Central HS Entrance
Time Analyzed	AM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



Vehicle Volumes and Adju	ıctma	ntc														
	Journa															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	1	1	0		1	0	1		0	0	0
Configuration	1			TR		L	Т			L		R				
Volume (veh/h)	1		425	90		55	285			35		50				
Percent Heavy Vehicles (%)	1					0				0		0				
Proportion Time Blocked	1															
Percent Grade (%)	1									(	)					
Right Turn Channelized	1									Ν	lo					
Median Type   Storage	Left Only										9	9				
Critical and Follow-up He	adwa															
Base Critical Headway (sec)	1					4.1				7.1		6.2				
Critical Headway (sec)						4.10				6.40		6.20				
Base Follow-Up Headway (sec)						2.2				3.5		3.3				
Follow-Up Headway (sec)	1					2.20				3.50		3.30				
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1					60				38		54				
Capacity, c (veh/h)	1					1021				576		567				
v/c Ratio						0.06				0.07		0.10				
95% Queue Length, Q <sub>95</sub> (veh)	1					0.2				0.2		0.3				
Control Delay (s/veh)						8.7				11.7		12.0				
Level of Service (LOS)	1					А				В		В				
Approach Delay (s/veh)	1	1.4								11	1.9	•			-	-
Approach LOS	j	A A							В							

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & West Central HS Entrance
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	West Central HS Entrance
Time Analyzed	PM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



Vehicle Volumes and Adju	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	1	1	0		1	0	1		0	0	0
Configuration	1			TR		L	Т			L		R				
Volume (veh/h)	1		305	4		4	620			15		15				
Percent Heavy Vehicles (%)	1					0				0		0				
Proportion Time Blocked	1															
Percent Grade (%)	1									(	)					
Right Turn Channelized	1									Ν	lo					
Median Type   Storage	Left Only										!	9				
Critical and Follow-up He	eadwa	·														
Base Critical Headway (sec)	1					4.1				7.1		6.2				
Critical Headway (sec)						4.10				6.40		6.20				
Base Follow-Up Headway (sec)	1					2.2				3.5		3.3				
Follow-Up Headway (sec)	1					2.20				3.50		3.30				
Delay, Queue Length, and	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1					4				16		16				
Capacity, c (veh/h)	1					1235				500		713				
v/c Ratio	1					0.00				0.03		0.02				
95% Queue Length, Q <sub>95</sub> (veh)						0.0				0.1		0.1				
Control Delay (s/veh)						7.9				12.4		10.2				
Level of Service (LOS)						А				В		В				
Approach Delay (s/veh)	0.1								11	1.3						
Approach LOS	1	A A								ı	В					

#### **HCS Signalized Intersection Results Summary** 144444 Intersection Information **General Information** Duration, h Agency HRG 0.250 CEC Analyst Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & Railroad Street File Name (9) SD38&Railroad AM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R R L R 95 30 4 Demand (v), veh/h 4 465 0 15 270 2 0 145 5 **Signal Information** Cycle, s 60.0 Reference Phase 2 Offset, s 0 Reference Point End 0.0 Green 0.3 0.8 34.5 9.8 0.0 Uncoordinated No Simult. Gap E/W On Yellow 3.5 0.0 4.0 4.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 0.0 1.0 1.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL SBT **Assigned Phase** 5 2 6 8 1 4 Case Number 1.1 4.0 1.1 3.0 8.0 6.0 Phase Duration, s 4.8 39.5 5.7 40.3 14.8 14.8 Change Period, (Y+Rc), s 4.5 5.0 4.5 5.0 5.0 5.0 Max Allow Headway ( MAH ), s 4.1 0.0 4.1 0.0 4.2 4.2 Queue Clearance Time ( $g_s$ ), s 2.1 2.2 3.2 9.8 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.0 0.0 0.5 0.4 Phase Call Probability 0.07 0.24 0.97 0.97 0.00 0.00 0.00 Max Out Probability 0.13 **Movement Group Results** EΒ WB NB SB Approach Movement L Т R L Т R L Т R Т L R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 4 0 16 293 103 35 158 10 1688 1750 1688 1772 1502 1510 1376 1611 Adjusted Saturation Flow Rate ( s ), veh/h/ln 0.1 0.0 0.2 4.9 1.8 0.0 6.6 0.3 Queue Service Time ( $g_s$ ), s Cycle Queue Clearance Time ( q c ), s 0.1 0.0 0.2 4.9 1.8 1.2 7.8 0.3 0.59 0.59 Green Ratio (g/C) 0.58 0.43 0.59 0.16 0.16 0.16 Capacity (c), veh/h 643 514 1042 883 312 319 265 Volume-to-Capacity Ratio (X) 0.007 0.000 0.032 0.282 0.117 0.112 0.493 0.037 Back of Queue (Q), ft/ln (95 th percentile) 8.0 0 2.9 69.3 22.2 18.2 95.5 5.1 Back of Queue (Q), veh/ln (95 th percentile) 0.0 0.0 0.1 2.7 0.9 0.7 3.8 0.2 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.01 0.00 0.00 0.00 0.38 0.00 Uniform Delay ( d 1 ), s/veh 5.6 5.9 6.1 5.5 21.4 24.8 21.1 Incremental Delay ( d 2 ), s/veh 0.0 0.0 0.0 0.7 0.3 0.2 1.2 0.1 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 5.6 5.9 6.8 5.7 21.6 26.0 21.1 Level of Service (LOS) Α Α Α Α С С С 9.3 6.5 Α 21.6 С 25.7 С Approach Delay, s/veh / LOS Α Intersection Delay, s/veh / LOS 11.1 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 1.64 В В 2.11 1.92 1.86 В В Bicycle LOS Score / LOS 1.33 Α 1.17 Α 0.54 Α 0.76 Α

#### **HCS Signalized Intersection Results Summary** 144444 Intersection Information **General Information** Duration, h HRG 0.250 Agency CEC Analyst Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period PM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & Railroad Street File Name (9) SD38&Railroad PM.xus Intersection **Project Description Demand Information** EB **WB** NB SB Approach Movement L R L R R R 560 2 9 Demand (v), veh/h 4 340 4 15 155 2 15 85 5 **Signal Information** Cycle, s 60.0 Reference Phase 2 Offset, s 0 Reference Point End 6.2 0.0 Green 0.3 0.8 38.1 0.0 Uncoordinated No Simult. Gap E/W On Yellow 3.5 0.0 4.0 4.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S On Red 0.0 1.0 1.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL **SBT Assigned Phase** 5 2 6 8 1 4 Case Number 1.1 4.0 1.1 3.0 8.0 6.0 Phase Duration, s 4.8 43.1 5.7 44.0 11.2 11.2 Change Period, (Y+Rc), s 4.5 5.0 4.5 5.0 5.0 5.0 Max Allow Headway ( MAH ), s 4.1 0.0 4.1 0.0 4.2 4.2 Queue Clearance Time ( $g_s$ ), s 2.1 2.3 2.7 6.7 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.0 0.0 0.3 0.3 Phase Call Probability 0.07 0.24 0.88 0.88 0.00 0.00 0.00 0.00 Max Out Probability **Movement Group Results** EΒ **WB** NB SB Approach Movement L Т R L Т R L Т R Т L R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 4 374 16 609 168 21 92 15 1688 1768 1179 1772 1502 1536 1361 1665 Adjusted Saturation Flow Rate ( s ), veh/h/ln 0.1 5.9 0.3 11.0 2.7 4.0 0.5 Queue Service Time ( $g_s$ ), s 0.0 Cycle Queue Clearance Time ( q c ), s 0.1 5.9 0.3 11.0 2.7 0.7 4.7 0.5 Green Ratio (g/C) 0.64 0.64 0.66 0.65 0.65 0.10 0.10 0.10 Capacity (c), veh/h 481 1124 522 1151 975 224 244 171 Volume-to-Capacity Ratio (X) 0.009 0.333 0.031 0.529 0.173 0.092 0.379 0.089 Back of Queue (Q), ft/ln (95 th percentile) 0.6 75.5 2.8 141.4 28.5 11.8 58.9 8.8 Back of Queue (Q), veh/ln (95 th percentile) 0.0 3.0 0.1 5.6 1.1 0.5 2.3 0.3 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.01 0.00 0.00 0.00 0.24 0.00 Uniform Delay ( d 1 ), s/veh 5.1 5.1 3.9 5.6 4.1 24.5 26.6 24.4 Incremental Delay ( d 2 ), s/veh 0.0 8.0 0.0 1.7 0.4 0.2 1.0 0.2 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 5.1 5.8 3.9 7.4 4.5 24.6 27.6 24.6 Level of Service (LOS) Α Α Α Α Α С С С 5.8 6.7 Α 24.6 С 27.1 С Approach Delay, s/veh / LOS Α Intersection Delay, s/veh / LOS 8.4 Α **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 1.63 В В 2.12 1.93 1.85 В В Bicycle LOS Score / LOS 1.11 Α 1.80 0.52 Α 0.67 Α

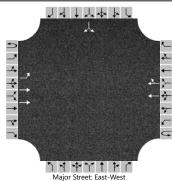
Generated: 5/6/2024 1:04:45 PM

#### **HCS Signalized Intersection Results Summary** Intersection Information **General Information** HRG Duration, h 0.250 Agency Analyst NM Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & Mickelson Roa... File Name (10) SD38&Mickelson AM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 40 190 65 Demand (v), veh/h 135 445 35 195 45 55 215 20 195 **Signal Information** die Cycle, s 70.0 Reference Phase 2 542 Offset, s 0 Reference Point End 35.1 3.1 10.0 0.0 Green 2.9 2.9 Uncoordinated No Simult. Gap E/W On Yellow 4.0 4.0 4.0 0.0 0.0 4.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 0.0 0.0 0.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL SBT **Assigned Phase** 2 6 3 8 4 1 7 Case Number 6.3 1.0 3.0 1.1 4.0 1.1 4.0 Phase Duration, s 39.1 6.9 46.0 7.1 14.0 10.0 16.9 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Change Period, (Y+Rc), s Max Allow Headway ( MAH ), s 0.0 3.1 0.0 3.1 3.3 3.1 3.3 Queue Clearance Time ( $g_s$ ), s 2.8 3.7 7.3 8.0 12.3 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.0 0.0 0.5 0.0 0.6 Phase Call Probability 0.57 0.61 1.00 0.99 1.00 0.03 1.00 0.01 Max Out Probability 0.00 1.00 **Movement Group Results** EΒ **WB** NB SB Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate (v), veh/h 147 264 258 43 212 207 49 130 234 234 1170 1772 1726 1688 1687 1688 1615 1688 1523 Adjusted Saturation Flow Rate ( s ), veh/h/ln 1323 5.0 0.8 5.2 1.7 5.3 Queue Service Time ( $g_s$ ), s 6.1 6.1 1.9 6.0 10.3 Cycle Queue Clearance Time ( q c ), s 5.0 6.1 6.1 8.0 1.9 5.2 1.7 5.3 6.0 10.3 0.60 Green Ratio (g/C) 0.50 0.50 0.50 0.57 0.60 0.19 0.14 0.24 0.18 Capacity (c), veh/h 690 890 867 536 2024 794 187 231 333 282 Volume-to-Capacity Ratio (X) 0.213 0.296 0.298 0.081 0.105 0.260 0.262 0.565 0.702 0.830 Back of Queue (Q), ft/ln (95 th percentile) Back of Queue (Q), veh/ln (95 th percentile) 2.2 4.0 4.0 0.4 1.0 2.4 1.2 3.5 2.5 6.7 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 Uniform Delay ( d 1 ), s/veh 9.9 10.2 10.2 7.1 6.0 6.6 24.5 28.0 25.5 27.5 Incremental Delay ( d 2 ), s/veh 0.7 8.0 0.9 0.0 0.1 0.8 0.3 8.0 5.5 2.4 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 10.6 11.0 11.1 7.1 6.1 7.4 24.8 28.8 31.1 29.9 Level of Service (LOS) В В В Α Α Α С С С С 11.0 В 6.8 Α 27.7 С 30.5 С Approach Delay, s/veh / LOS Intersection Delay, s/veh / LOS 16.7 В **Multimodal Results** ΕB WB NB SB Pedestrian LOS Score / LOS 1.88 В 1.87 В 2.44 2.28 В В Bicycle LOS Score / LOS 1.04 Α 0.87 Α 0.78 Α 1.26 Α

#### **HCS Signalized Intersection Results Summary** Intersection Information **General Information** HRG Duration, h 0.250 Agency Analyst NM Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & Mickelson Roa... File Name (10) SD38&Mickelson PM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 220 535 10 Demand (v), veh/h 160 20 135 225 20 65 215 15 185 **Signal Information** Ji. Cycle, s 70.0 Reference Phase 2 Offset, s 0 Reference Point End 0.7 30.8 1.3 7.0 Green 5.0 Uncoordinated No Simult. Gap E/W On Yellow 3.5 0.0 4.0 3.5 3.5 4.0 Force Mode Fixed Simult. Gap N/S On Red 1.0 0.0 1.0 1.0 1.0 1.0 **Timer Results EBL EBT WBL** WBT NBL **NBT** SBL SBT **Assigned Phase** 5 2 6 3 8 4 1 7 Case Number 1.1 4.0 1.1 3.0 1.1 4.0 1.1 4.0 Phase Duration, s 10.2 36.5 9.5 35.8 6.2 12.0 12.0 17.8 4.5 5.0 4.5 5.0 4.5 5.0 4.5 5.0 Change Period, (Y+Rc), s Max Allow Headway ( MAH ), s 3.1 0.0 3.1 0.0 3.1 3.3 3.1 3.3 Queue Clearance Time ( $g_s$ ), s 5.8 5.3 2.8 5.1 9.5 11.6 Green Extension Time ( $g_e$ ), s 0.1 0.0 0.1 0.0 0.0 0.3 0.0 0.2 Phase Call Probability 0.97 0.94 0.34 1.00 0.99 1.00 1.00 0.63 1.00 1.00 Max Out Probability 1.00 1.00 **Movement Group Results** EΒ **WB** NB SB Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate (v), veh/h 174 131 130 147 582 245 22 82 234 217 1688 1772 1720 1688 1687 1323 1688 1730 1688 1519 Adjusted Saturation Flow Rate ( s ), veh/h/ln 3.8 3.1 3.1 3.3 8.2 8.0 7.5 9.6 Queue Service Time ( $g_s$ ), s 8.9 3.1 Cycle Queue Clearance Time ( q c ), s 3.8 3.1 3.1 3.3 8.2 8.9 8.0 3.1 7.5 9.6 Green Ratio (g/C) 0.52 0.45 0.45 0.51 0.44 0.44 0.12 0.10 0.24 0.18 Capacity (c), veh/h 510 798 774 645 1484 582 164 173 356 277 Volume-to-Capacity Ratio (X) 0.341 0.165 0.167 0.228 0.392 0.420 0.132 0.472 0.656 0.785 Back of Queue (Q), ft/ln (95 th percentile) Back of Queue (Q), veh/ln (95 th percentile) 2.2 2.1 2.1 1.9 5.2 4.8 0.6 2.3 6.3 7.1 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 Uniform Delay ( d 1 ), s/veh 9.5 11.4 11.4 9.3 13.3 13.5 27.4 29.8 24.6 27.3 Incremental Delay ( d 2 ), s/veh 0.1 0.4 0.5 0.1 8.0 2.2 0.1 0.7 3.4 8.9 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 9.7 11.9 11.9 9.4 14.0 15.7 27.6 30.5 28.0 36.2 Level of Service (LOS) Α В В Α В В С С С D 11.0 В 13.8 В 29.9 С 32.0 С Approach Delay, s/veh / LOS Intersection Delay, s/veh / LOS 18.2 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 1.89 В В 2.44 2.28 1.89 В В Bicycle LOS Score / LOS 0.85 Α 1.29 Α 0.66 Α 1.23

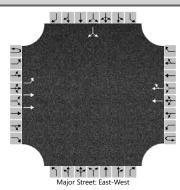
Generated: 10/31/2023 2:36:18 PM

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD38 & 466th Ave
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	466th Ave
Time Analyzed	AM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



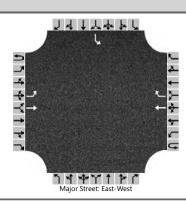
	Major Street: East-West															
Vehicle Volumes and Ad	ustme	nts														
Approach		Eastb	ound			Westl	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		0	1	0
Configuration		L	Т				Т	TR							LR	
Volume (veh/h)	0	2	765				430	5						4		0
Percent Heavy Vehicles (%)	3	0												50		3
Proportion Time Blocked																
Percent Grade (%)															0	
Right Turn Channelized																
Median Type   Storage		Left Only 9														
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1												7.5		6.9
Critical Headway (sec)		4.10												7.80		6.96
Base Follow-Up Headway (sec)		2.2												3.5		3.3
Follow-Up Headway (sec)		2.20												4.00		3.33
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		2													4	
Capacity, c (veh/h)		1100													457	
v/c Ratio		0.00													0.01	
95% Queue Length, Q <sub>95</sub> (veh)		0.0					Ì	Ì					Ì	Ì	0.0	
Control Delay (s/veh)		8.3													12.9	
Level of Service (LOS)		А													В	
Approach Delay (s/veh)		0	.0								•			12	2.9	
Approach LOS	i	,	Ą										В			

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD38 & 466th Ave
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	466th Ave
Time Analyzed	PM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



Vehicle Volumes and Ad	ustme	nts															
Approach		Eastb	ound			Westl	bound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		0	1	0	
Configuration		L	Т				Т	TR							LR		
Volume (veh/h)	0	0	445				910	2						5		2	
Percent Heavy Vehicles (%)	3	0												33		0	
Proportion Time Blocked																	
Percent Grade (%)														0			
Right Turn Channelized																	
Median Type   Storage		Left Only										9					
Critical and Follow-up H	eadwa																
Base Critical Headway (sec)		4.1												7.5		6.9	
Critical Headway (sec)		4.10												7.46		6.90	
Base Follow-Up Headway (sec)		2.2												3.5		3.3	
Follow-Up Headway (sec)		2.20												3.83		3.30	
Delay, Queue Length, an	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)	<u> </u>	0		П							Π				8		
Capacity, c (veh/h)		705													299		
v/c Ratio		0.00													0.03		
95% Queue Length, Q <sub>95</sub> (veh)		0.0													0.1		
Control Delay (s/veh)		10.1													17.3		
Level of Service (LOS)		В													С		
Approach Delay (s/veh)		0	.0											17.3			
Approach LOS		,	A								С						

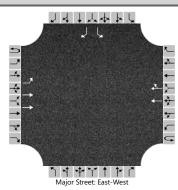
	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & I-90 WB Terminal
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	12/12/2023	East/West Street	SD 38
Analysis Year	2050	North/South Street	I-90 WB Terminal
Time Analyzed	AM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



Vehicle Volumes and Adj	justme	nts														
Approach		Eastb	ound			Westl	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		1	0	0
Configuration		L	Т				Т	R						L		
Volume (veh/h)		40	730				255	20						15		
Percent Heavy Vehicles (%)		0												56		
Proportion Time Blocked																
Percent Grade (%)														0		
Right Turn Channelized	No															
Median Type   Storage				Left	Only								9			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1												7.1		
Critical Headway (sec)		4.10												6.96		
Base Follow-Up Headway (sec)		2.2												3.5		
Follow-Up Headway (sec)		2.20												4.00		
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		43												16		
Capacity, c (veh/h)		1274												315		
v/c Ratio		0.03												0.05		
95% Queue Length, Q <sub>95</sub> (veh)		0.1												0.2		
Control Delay (s/veh)		7.9	0.2											17.1		
Level of Service (LOS)		А	А											С		
Approach Delay (s/veh)		0	.6											17	7.1	
Approach LOS	A C									С						

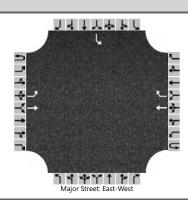
Generated: 12/12/2023 8:58:43 AM

	HCS Two-Way Stop	p-Control Report									
General Information		Site Information									
Analyst	CEC	Intersection	SD 38 & I-90 WB Terminal								
Agency/Co.	HRG	Jurisdiction	SDDOT								
Date Performed	4/30/2024	East/West Street	SD 38								
Analysis Year	2050	North/South Street	I-90 WB Terminal								
Time Analyzed	AM Peak	Peak Hour Factor	0.92								
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25								
Project Description	SD 38										



Vehicle Volumes and Adj	ustme	nts																		
Approach		Eastk	ound			Westl	bound			North	bound			South	bound					
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R				
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12				
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		1	0	1				
Configuration		L	Т				Т	TR						L		R				
Volume (veh/h)	0	40	730				255	20						15		190				
Percent Heavy Vehicles (%)	3	0												56		12				
Proportion Time Blocked																				
Percent Grade (%)														(	0					
Right Turn Channelized														Ν	lo					
Median Type   Storage				Left	Only							9	9							
Critical and Follow-up H	eadwa	ys																		
Base Critical Headway (sec)	<u> </u>	4.1												7.5		6.9				
Critical Headway (sec)		4.10												7.92		7.14				
Base Follow-Up Headway (sec)		2.2												3.5		3.3				
Follow-Up Headway (sec)		2.20												4.06		3.42				
Delay, Queue Length, an	d Leve	l of S	ervice																	
Flow Rate, v (veh/h)		43												16		207				
Capacity, c (veh/h)		1274												435		839				
v/c Ratio		0.03												0.04		0.25				
95% Queue Length, Q <sub>95</sub> (veh)		0.1												0.1		1.0				
Control Delay (s/veh)		7.9												13.6		10.7				
Level of Service (LOS)		А												В		В				
Approach Delay (s/veh)		0	.4							•			10.9							
Approach LOS			A										В							

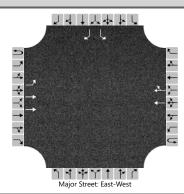
	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & I-90 WB Terminal
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	5/8/2023	East/West Street	SD 38
Analysis Year	2050	North/South Street	I-90 WB Terminal
Time Analyzed	PM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		1	0	0
Configuration		L	Т				Т	R						L		
Volume (veh/h)		25	420				415	35						30		
Percent Heavy Vehicles (%)		0												6		
Proportion Time Blocked																
Percent Grade (%)														(	0	
Right Turn Channelized						Ν	lo									
Median Type   Storage				Left	Only								9			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1												7.1		
Critical Headway (sec)		4.10												6.46		
Base Follow-Up Headway (sec)		2.2												3.5		
Follow-Up Headway (sec)		2.20												3.55		
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		27												33		
Capacity, c (veh/h)		1085												562		
v/c Ratio		0.03												0.06		
95% Queue Length, Q <sub>95</sub> (veh)		0.1												0.2		
Control Delay (s/veh)		8.4	0.2											11.8		
Level of Service (LOS)		А	А											В		
Approach Delay (s/veh)		0	.6									11.8				
Approach LOS		A												-	В	

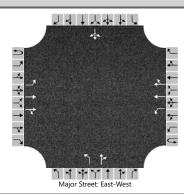
Generated: 10/27/2023 1:49:26 PM

	HCS Two-Way Stop	p-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & I-90 WB Terminal							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	I-90 WB Terminal							
Time Analyzed	PM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38									



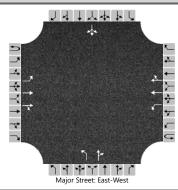
Vehicle Volumes and Adj	justme	nts																
Approach		Eastk	oound			West	bound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	T	R		
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		1	0	1		
Configuration		L	Т				Т	TR						L		R		
Volume (veh/h)	0	25	420				415	35						30		495		
Percent Heavy Vehicles (%)	3	0												6		2		
Proportion Time Blocked																		
Percent Grade (%)														(	0			
Right Turn Channelized														Ν	10			
Median Type   Storage				Left	Only					9								
Critical and Follow-up H	eadwa	ys																
Base Critical Headway (sec)	$\exists$	4.1												7.5		6.9		
Critical Headway (sec)		4.10												6.92		6.94		
Base Follow-Up Headway (sec)		2.2												3.5		3.3		
Follow-Up Headway (sec)		2.20												3.56		3.32		
Delay, Queue Length, an	d Leve	l of S	ervice															
Flow Rate, v (veh/h)	<u> </u>	27												33		538		
Capacity, c (veh/h)		1085												578		756		
v/c Ratio		0.03												0.06		0.71		
95% Queue Length, Q <sub>95</sub> (veh)		0.1												0.2		6.1		
Control Delay (s/veh)		8.4												11.6		20.6		
Level of Service (LOS)		А												В		С		
Approach Delay (s/veh)		0	).5										20.1					
Approach LOS	1		A										С					

	HCS Two-Way Stop	p-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & I-90 EB Ramp Terminal							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	I-90 EB Ramp Terminal							
Time Analyzed	PM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38									



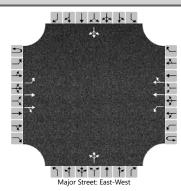
Vehicle Volumes and Adju	ıstme	ents																
Approach		Eastb	ound			Westk	oound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	2	0	0	1	2	0		1	1	0		0	1	0		
Configuration		L	T	TR		L	Т	TR		L		TR			LTR			
Volume (veh/h)	0	190	265	20	0	15	420	30		30	15	25		30	10	35		
Percent Heavy Vehicles (%)	3	10			3	11				20	20	0		8	3	3		
Proportion Time Blocked																		
Percent Grade (%)										(	0			(	0			
Right Turn Channelized																		
Median Type   Storage				Undi	vided													
<b>Critical and Follow-up He</b>	adwa	ys																
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9		
Critical Headway (sec)		4.30				4.32				7.90	6.90	6.90		7.66	6.56	6.96		
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3		
Follow-Up Headway (sec)		2.30				2.31				3.70	4.20	3.30		3.58	4.03	3.33		
Delay, Queue Length, and	l Leve	l of Se	ervice															
Flow Rate, v (veh/h)	1	207				16				33		43			82			
Capacity, c (veh/h)		1016				1185				122		259			193			
v/c Ratio		0.20				0.01				0.27		0.17			0.42			
95% Queue Length, Q <sub>95</sub> (veh)		0.8				0.0				1.0		0.6			1.9			
Control Delay (s/veh)		9.4	0.6			8.1	0.1			44.7		21.7			36.6			
Level of Service (LOS)		Α	Α			А	А			Е		С			Е			
Approach Delay (s/veh)		4.1				0.4			31.6				36.6					
Approach LOS		,	4			A	4			[	)		E					

	HCS Two-Way Stop	p-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD38/I-90 EB Ramp Terminal/466th St							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	I-90 EB Ramp Terminal/466th Street							
Time Analyzed	AM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38									



Vehicle Volumes and Adju	ıstme	nents																
Approach		Eastb	ound			West	oound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	2	0	0	1	2	0		1	1	0		0	1	0		
Configuration		L	Т	TR		L	Т	TR		L		TR			LTR			
Volume (veh/h)	0	430	300	15	0	20	240	20		15	10	20		3	2	28		
Percent Heavy Vehicles (%)	3	2			3	20				33	33	60		33	0	0		
Proportion Time Blocked																		
Percent Grade (%)										(	)			(	0			
Right Turn Channelized																		
Median Type   Storage				Undi	vided													
<b>Critical and Follow-up He</b>	adwa	ys																
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9		
Critical Headway (sec)		4.14				4.50				8.16	7.16	8.10		8.16	6.50	6.90		
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3		
Follow-Up Headway (sec)		2.22				2.40				3.83	4.33	3.90		3.83	4.00	3.30		
Delay, Queue Length, and	l Leve	l of Se	ervice															
Flow Rate, v (veh/h)	1	467				22				16		33			36			
Capacity, c (veh/h)		1277				1094				40		128			231			
v/c Ratio		0.37				0.02				0.41		0.25			0.16			
95% Queue Length, Q <sub>95</sub> (veh)		1.7				0.1				1.4		1.0			0.5			
Control Delay (s/veh)		9.4	0.6			8.4	0.1			146.1		42.5			23.4			
Level of Service (LOS)		Α	Α			А	А			F		E			С			
Approach Delay (s/veh)		5.7				0.7			77.0				23.4					
Approach LOS		,	Α			,	4		F				С					

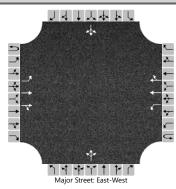
	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & 468th Avenue
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	468th Ave / County Highway 141
Time Analyzed	AM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



	·															
Vehicle Volumes and Adj	ustme	nts														
Approach		Eastk	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	1	0
Configuration		L	Т	TR		L	Т	TR			LTR				LTR	
Volume (veh/h)	0	4	360	0	0	0	225	50		2	2	0		50	0	7
Percent Heavy Vehicles (%)	3	0			3	0				0	100	0		4	0	50
Proportion Time Blocked																
Percent Grade (%)										(	)			(	0	
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	<u> </u>	4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)		4.10				4.10				7.50	8.50	6.90		7.58	6.50	7.90
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.20				3.50	5.00	3.30		3.54	4.00	3.80
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	<u> </u>	4				0					4				62	
Capacity, c (veh/h)		1274				1178					284				483	
v/c Ratio		0.00				0.00					0.02				0.13	
95% Queue Length, Q <sub>95</sub> (veh)		0.0	Ì			0.0					0.0				0.4	
Control Delay (s/veh)		7.8	0.0			8.1	0.0				17.9				13.6	
Level of Service (LOS)		А	А			А	Α				С				В	
Approach Delay (s/veh)		0	.1		0.0				17.9				13.6			
Approach LOS			A			,	Α			(	2		В			

Generated: 5/5/2024 9:02:33 AM

HCS Two-Way Stop-Control Report						
General Information Site Information						
Analyst	NM	Intersection	SD 38 & 468th Avenue			
Agency/Co.	HRG	Jurisdiction	SDDOT			
Date Performed	4/30/2024	East/West Street	SD 38			
Analysis Year	2050	North/South Street	468th Ave / County Highway 141			
Time Analyzed	PM Peak	Peak Hour Factor	0.92			
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25			
Project Description	SD 38					



					iviajo	or street. La	st-vvest									
Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	1	0
Configuration	]	L	Т	TR		L	Т	TR			LTR				LTR	
Volume (veh/h)	0	0	310	2	0	5	420	55		2	2	0		50	4	4
Percent Heavy Vehicles (%)	3	0			3	0				0	0	0		4	100	50
Proportion Time Blocked	]															
Percent Grade (%)										(	0				0	
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	1	4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)		4.10				4.10				7.50	6.50	6.90		7.58	8.50	7.90
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.20				3.50	4.00	3.30		3.54	5.00	3.80
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	<u> </u>	0				5					4				63	
Capacity, c (veh/h)	1	1060				1231					326				324	
v/c Ratio		0.00				0.00					0.01				0.19	
95% Queue Length, Q <sub>95</sub> (veh)	1	0.0				0.0					0.0				0.7	
Control Delay (s/veh)		8.4	0.0			7.9	0.0				16.2				18.8	
Level of Service (LOS)	1	А	А			А	Α				С				С	
Approach Delay (s/veh)		0	.0			0	.1			16	5.2			18	3.8	
Approach LOS		,	Ą			,	4			(	С				С	

Generated: 5/5/2024 9:03:28 AM

#### **HCS Signalized Intersection Results Summary** Intersection Information **General Information** Duration, h Agency HRG 0.250 CEC Analyst Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & 469th Ave File Name (17) SD38&469 AM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R R 5 5 Demand (v), veh/h 5 330 75 75 165 5 110 280 15 5 **Signal Information** Cycle, s 60.0 Reference Phase 2 Offset, s 0 Reference Point End 25.6 16.2 0.0 Green 0.4 3.3 0.0 Uncoordinated No Simult. Gap E/W On Yellow 3.5 0.0 4.0 0.0 4.0 0.0 Force Mode Fixed Simult. Gap N/S On Red 1.0 0.0 1.0 1.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL SBT **Assigned Phase** 5 2 6 8 1 4 Case Number 1.1 4.0 1.1 4.0 6.0 6.0 Phase Duration, s 4.9 30.6 8.2 33.9 21.2 21.2 Change Period, (Y+Rc), s 4.5 5.0 4.5 5.0 5.0 5.0 Max Allow Headway ( MAH ), s 3.9 0.0 3.9 0.0 4.1 4.1 Queue Clearance Time ( $g_s$ ), s 2.1 3.6 14.7 15.6 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.1 0.0 0.7 0.6 Phase Call Probability 0.09 0.74 1.00 1.00 0.01 0.95 1.00 Max Out Probability 0.38 SB **Movement Group Results** EΒ **WB** NB Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 5 2 12 1 16 3 8 18 7 4 14 6 Adjusted Flow Rate ( v ), veh/h 5 225 215 82 93 92 120 310 16 11 1674 1758 1646 1647 1730 1712 1426 1374 1061 1613 Adjusted Saturation Flow Rate ( s ), veh/h/ln 0.1 5.1 5.2 1.6 12.7 0.9 0.3 Queue Service Time ( $g_s$ ), s 1.8 1.8 4.0 Cycle Queue Clearance Time ( q c ), s 0.1 5.1 5.2 1.6 1.8 1.8 4.3 12.7 13.6 0.3 0.43 0.50 0.48 0.27 Green Ratio (g/C) 0.43 0.43 0.48 0.27 0.27 0.27 Capacity (c), veh/h 629 750 702 537 832 824 498 371 182 435 Volume-to-Capacity Ratio (X) 0.009 0.300 0.306 0.152 0.111 0.112 0.240 0.835 0.090 0.025 Back of Queue (Q), ft/ln (95 th percentile) 1.3 70.1 67.9 16.1 21.9 21.9 49.8 219 9.4 4.3 Back of Queue (Q), veh/ln (95 th percentile) 0.0 2.7 2.7 0.6 8.0 8.0 2.0 7.9 0.4 0.2 Queue Storage Ratio (RQ) (95 th percentile) 0.01 0.00 0.00 0.06 0.00 0.00 0.20 0.00 0.04 0.00 Uniform Delay ( d 1 ), s/veh 9.7 11.3 11.4 8.2 8.5 8.5 17.7 20.6 27.0 16.1 Incremental Delay ( d 2 ), s/veh 0.0 1.0 1.1 0.1 0.3 0.3 0.2 11.6 0.2 0.0 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 9.7 12.3 12.5 8.3 8.8 8.8 17.9 32.3 27.2 16.1 Level of Service (LOS) Α В В Α Α Α В С С В 12.4 В 8.7 Α 28.3 С 22.8 С Approach Delay, s/veh / LOS Intersection Delay, s/veh / LOS 17.6 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 1.89 В 1.88 В 2.27 2.27 В В Bicycle LOS Score / LOS 0.86 Α 0.71 Α 1.20 Α 0.53 Α

#### **HCS Signalized Intersection Results Summary** Intersection Information **General Information** Agency HRG Duration, h 0.250 CEC Analyst Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & 469th Ave File Name (17) SD38&469 PM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 285 5 5 Demand (v), veh/h 5 245 120 380 5 100 120 15 10 **Signal Information** Cycle, s 60.0 Reference Phase 2 Offset, s 0 Reference Point End 0.0 Green 0.4 2.7 29.4 8.5 0.0 Uncoordinated No Simult. Gap E/W On Yellow 3.5 3.5 4.0 4.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S On Red 1.0 1.0 1.0 1.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL **SBT Assigned Phase** 5 2 6 8 1 4 Case Number 1.1 4.0 1.1 4.0 6.0 6.0 Phase Duration, s 4.9 34.4 12.1 41.6 13.5 13.5 Change Period, (Y+Rc), s 4.5 5.0 4.5 5.0 5.0 5.0 Max Allow Headway ( MAH ), s 3.9 0.0 3.9 0.0 4.1 4.1 Queue Clearance Time ( $g_s$ ), s 2.1 6.9 7.1 7.9 Green Extension Time ( $g_e$ ), s 0.0 0.0 8.0 0.0 0.7 0.6 Phase Call Probability 0.09 0.99 0.99 0.99 0.00 0.02 0.03 Max Out Probability 0.01 SB **Movement Group Results** EΒ WB NB Approach Movement L Т R L Т R L Т R Т L R **Assigned Movement** 5 2 12 1 16 3 8 18 7 4 14 6 Adjusted Flow Rate ( v ), veh/h 5 206 191 310 210 209 109 136 16 16 1674 1758 1565 1647 1730 1722 1397 1499 1244 1570 Adjusted Saturation Flow Rate ( s ), veh/h/ln 0.1 4.1 4.2 4.9 3.2 3.2 5.1 8.0 0.5 Queue Service Time ( $g_s$ ), s 4.4 Cycle Queue Clearance Time ( q c ), s 0.1 4.1 4.2 4.9 3.2 3.2 4.9 5.1 5.9 0.5 0.49 Green Ratio (g/C) 0.50 0.49 0.65 0.61 0.61 0.14 0.14 0.14 0.14 Capacity (c), veh/h 603 862 767 733 1054 1049 305 212 190 222 Volume-to-Capacity Ratio (X) 0.009 0.239 0.249 0.423 0.199 0.199 0.356 0.640 0.086 0.073 Back of Queue (Q), ft/ln (95 th percentile) 50.2 47.4 26.7 28.4 28.4 58.9 79.5 9.4 8.2 1 Back of Queue (Q), veh/ln (95 th percentile) 0.0 2.0 1.9 1.0 1.1 1.1 2.3 3.1 0.4 0.3 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.00 0.11 0.00 0.00 0.24 0.00 0.04 0.00 Uniform Delay ( d 1 ), s/veh 7.6 8.8 8.9 5.0 5.2 5.2 24.5 24.3 27.1 22.3 Incremental Delay ( d 2 ), s/veh 0.0 0.7 8.0 0.4 0.4 0.4 0.7 3.2 0.2 0.1 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 7.6 9.5 9.7 5.4 5.6 5.6 25.2 27.5 27.3 22.5 Level of Service (LOS) Α Α Α Α Α Α С С С С 9.5 5.5 Α 26.5 С 24.9 С Approach Delay, s/veh / LOS Α Intersection Delay, s/veh / LOS 10.8 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 1.88 В В 2.28 2.28 1.86 В В Bicycle LOS Score / LOS 0.82 Α 1.09 Α 0.89 Α 0.54 Α

#### **HCS Signalized Intersection Results Summary** Intersection Information **General Information** Duration, h Agency HRG 0.250 CEC Analyst Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 Analysis Period 1> 7:15 SD 38 & La Mesa Drive File Name (17) SD38&LaMesa AM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R R L R 4 Demand (v), veh/h 30 700 4 0 235 15 0 15 5 75 30 **Signal Information** JI. Cycle, s 60.0 Reference Phase 2 517 Offset, s 0 Reference Point End Green 2.1 25.4 18.0 0.0 0.0 0.0 Uncoordinated No Simult. Gap E/W On Yellow 3.5 4.0 4.0 0.0 0.0 0.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 1.0 1.0 1.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL **SBT Assigned Phase** 5 2 6 8 1 4 Case Number 1.1 4.0 1.1 3.0 8.0 6.0 Phase Duration, s 6.6 37.0 0.0 30.4 23.0 23.0 Change Period, (Y+Rc), s 4.5 5.0 5.0 5.0 5.0 5.0 Max Allow Headway ( MAH ), s 3.9 0.0 0.0 0.0 4.0 4.0 Queue Clearance Time ( $g_s$ ), s 2.6 2.6 20.0 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.0 0.0 0.3 0.0 Phase Call Probability 0.42 0.90 0.90 1.00 0.00 1.00 Max Out Probability **Movement Group Results** EΒ WB NB SB Approach Movement L Т R L Т R L Т R Т L R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 33 383 382 0 255 16 0 82 37 1674 1758 1754 1674 1674 1323 1434 1412 948 Adjusted Saturation Flow Rate ( s ), veh/h/ln 0.6 7.8 7.8 0.0 2.9 0.4 0.0 2.6 1.7 Queue Service Time ( $g_s$ ), s Cycle Queue Clearance Time ( g c ), s 0.6 7.8 7.8 0.0 2.9 0.4 0.0 3.2 1.7 0.49 0.53 0.42 0.30 Green Ratio (g/C) 0.53 0.34 0.42 0.10 0.30 938 Capacity (c), veh/h 598 936 380 1417 560 530 284 Volume-to-Capacity Ratio (X) 0.055 0.409 0.409 0.000 0.180 0.029 0.000 0.154 0.130 Back of Queue (Q), ft/ln (95 th percentile) 6.3 90.3 90.2 0 35.1 5.1 0 31.4 19.6 Back of Queue (Q), veh/ln (95 th percentile) 0.2 3.5 3.5 0.0 1.4 0.2 0.0 1.3 0.6 Queue Storage Ratio (RQ) (95 th percentile) 0.03 0.00 0.00 0.00 0.00 0.02 0.00 0.13 0.00 Uniform Delay ( d 1 ), s/veh 8.1 8.4 8.4 0.0 10.8 10.1 16.0 15.3 Incremental Delay ( d 2 ), s/veh 0.0 1.3 1.3 0.0 0.3 0.1 0.0 0.1 0.2 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 8.1 9.7 9.7 0.0 11.1 10.2 16.2 15.5 Level of Service (LOS) Α Α Α В В В В 9.6 В 15.0 16.0 Approach Delay, s/veh / LOS Α 11.0 В В Intersection Delay, s/veh / LOS 10.6 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 1.65 В В 2.42 2.26 1.89 В В Bicycle LOS Score / LOS 1.15 Α 0.71 Α 0.52 Α 0.68 Α

#### **HCS Signalized Intersection Results Summary** Intersection Information **General Information** Duration, h HRG 0.250 Agency CEC Analyst Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period PM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & La Mesa Drive File Name (17) SD38&LaMesa PM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R R L R 100 5 Demand (v), veh/h 25 325 0 9 735 4 0 80 15 30 **Signal Information** JI. Ji. Cycle, s 65.0 Reference Phase 2 517 Offset, s 0 Reference Point End 15.3 0.6 24.8 0.0 Green 0.8 4.0 Uncoordinated No Simult. Gap E/W On 3.5 Yellow 4.0 0.0 4.0 0.0 4.0 Force Mode Fixed Simult. Gap N/S On Red 1.0 0.0 1.0 1.0 1.0 0.0 **Timer Results EBL EBT WBL** WBT NBL **NBT** SBL SBT **Assigned Phase** 5 2 6 8 4 1 7 Case Number 1.1 4.0 1.1 3.0 8.3 1.0 4.0 Phase Duration, s 6.4 30.4 5.8 29.8 20.3 8.5 28.7 Change Period, (Y+Rc), s 4.5 5.0 5.0 5.0 5.0 4.5 5.0 Max Allow Headway ( MAH ), s 3.9 0.0 3.9 0.0 4.0 3.9 4.0 Queue Clearance Time ( $g_s$ ), s 2.6 2.2 17.2 4.5 25.7 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Phase Call Probability 0.39 0.16 0.65 0.79 0.93 0.02 1.00 1.00 1.00 Max Out Probability 0.01 **Movement Group Results** EΒ WB NB SB Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 27 353 0 10 799 109 10 87 49 1674 1758 1490 1674 1674 1323 297 1594 1582 Adjusted Saturation Flow Rate ( s ), veh/h/ln 0.6 4.4 0.0 0.2 12.6 3.6 2.5 1.3 Queue Service Time ( $g_s$ ), s 0.1 4.4 Cycle Queue Clearance Time ( q c ), s 0.6 0.0 0.2 12.6 3.6 15.2 2.5 1.3 0.41 0.39 0.53 0.38 0.38 0.37 Green Ratio (g/C) 0.40 0.24 0.33 Capacity (c), veh/h 290 1381 433 1282 507 149 210 576 Volume-to-Capacity Ratio (X) 0.094 0.256 0.000 0.023 0.623 0.214 0.065 0.413 0.085 Back of Queue (Q), ft/ln (95 th percentile) 8.2 62.5 0 3 179.9 47.1 4.8 39.7 17.7 Back of Queue (Q), veh/ln (95 th percentile) 0.3 2.4 0.0 0.1 7.0 1.7 0.2 1.5 0.7 Queue Storage Ratio (RQ) (95 th percentile) 0.03 0.00 0.00 0.01 0.00 0.19 0.00 0.16 0.00 20.4 17.9 Uniform Delay ( d 1 ), s/veh 12.6 13.3 12.2 16.2 13.5 13.6 Incremental Delay ( d 2 ), s/veh 0.1 0.4 0.0 0.0 2.3 1.0 0.2 1.3 0.1 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 12.8 13.8 12.2 18.5 14.4 20.6 19.2 13.6 Level of Service (LOS) В В В В В С В В 13.7 18.0 В 20.6 С 17.2 Approach Delay, s/veh / LOS В В Intersection Delay, s/veh / LOS 16.8 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 1.67 В 1.90 В 2.43 2.26 В В Bicycle LOS Score / LOS 0.80 Α 1.24 Α 0.50 Α 0.71 Α

#### **HCS Signalized Intersection Results Summary** Intersection Information **General Information** HRG Duration, h 0.250 Agency Analyst NM Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & Marion Street File Name (18) SD38&Marion AM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 40 Demand (v), veh/h 165 340 105 50 125 75 110 225 120 45 145 **Signal Information** J. Cycle, s 50.0 Reference Phase 2 Offset, s 0 Reference Point End Green 2.6 3.9 12.9 2.5 2.0 10.0 Uncoordinated No Simult. Gap E/W On Yellow 4.0 0.0 4.0 4.0 0.0 4.0 Force Mode Fixed Simult. Gap N/S On Red 0.0 0.0 0.0 0.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL SBT **Assigned Phase** 5 2 6 3 8 1 7 4 Case Number 2.0 3.0 1.1 4.0 2.0 3.0 2.0 3.0 Phase Duration, s 10.6 20.8 6.6 16.9 8.5 16.0 6.5 14.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Change Period, (Y+Rc), s Max Allow Headway ( MAH ), s 2.9 0.0 2.9 0.0 2.9 2.9 2.9 2.9 Queue Clearance Time ( $g_s$ ), s 7.1 3.1 5.6 8.5 3.5 5.9 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.0 0.0 0.0 0.6 0.0 0.6 Phase Call Probability 0.92 0.53 0.81 1.00 0.49 1.00 1.00 0.04 0.21 1.00 Max Out Probability 1.00 0.15 **Movement Group Results** EΒ **WB** NB SB Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 179 370 114 54 112 106 120 245 130 49 158 43 Adjusted Saturation Flow Rate ( s ), veh/h/ln 1701 1674 1525 1714 1772 1556 1647 1674 1502 1554 1758 1466 5.1 4.1 2.7 1.1 2.5 2.7 3.6 6.5 3.9 1.2 Queue Service Time ( $g_s$ ), s 3.6 1.5 2.5 Cycle Queue Clearance Time ( q c ), s 5.1 4.1 2.7 1.1 2.7 3.6 6.5 3.6 1.5 3.9 1.2 0.34 0.26 0.24 0.24 0.20 Green Ratio (g/C) 0.13 0.34 0.31 0.26 0.09 0.05 0.20 403 Capacity (c), veh/h 223 1128 514 456 459 403 148 361 77 352 293 Volume-to-Capacity Ratio (X) 0.804 0.328 0.222 0.119 0.243 0.263 0.806 0.607 0.361 0.638 0.448 0.148 Back of Queue (Q), ft/ln (95 th percentile) Back of Queue (Q), veh/ln (95 th percentile) 4.1 2.2 1.4 0.6 1.6 1.6 3.2 3.5 1.7 1.0 2.3 0.6 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 14.7 Uniform Delay ( d 1 ), s/veh 21.1 12.4 11.9 12.3 14.7 22.3 16.9 15.8 23.3 17.6 16.5 Incremental Delay ( d 2 ), s/veh 11.0 8.0 1.0 0.0 1.3 1.6 15.9 8.0 0.2 3.3 0.3 0.1 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 32.1 13.1 12.9 12.3 15.9 16.3 38.2 17.7 16.0 26.6 17.9 16.6 Level of Service (LOS) С В В В В В D В В С В В 18.2 В 15.4 В 22.2 С 19.4 Approach Delay, s/veh / LOS В Intersection Delay, s/veh / LOS 19.1 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 2.08 В 2.09 В 2.26 2.42 В В Bicycle LOS Score / LOS 1.03 Α 0.71 Α 1.30 Α 0.90 Α

#### **HCS Signalized Intersection Results Summary** Intersection Information **General Information** HRG Duration, h 0.250 Agency Analyst NM Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period PM Peak 0.90 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 16:45 SD 38 & Marion Street File Name (18) SD38&Marion PM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 55 Demand (v), veh/h 70 230 105 170 355 180 205 125 85 355 205 **Signal Information** Cycle, s 60.0 Reference Phase 2 Offset, s 0 Reference Point End Green 3.7 0.4 13.3 4.2 2.8 15.5 Uncoordinated No Simult. Gap E/W On Yellow 4.0 4.0 4.0 4.0 0.0 4.0 Force Mode Fixed Simult. Gap N/S On Red 0.0 0.0 0.0 0.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL SBT **Assigned Phase** 5 2 6 3 8 1 7 4 Case Number 2.0 3.0 2.0 3.0 2.0 3.0 2.0 3.0 Phase Duration, s 7.7 17.3 12.2 21.7 11.0 22.3 8.2 19.5 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Change Period, (Y+Rc), s Max Allow Headway ( MAH ), s 2.9 0.0 2.9 0.0 2.9 3.0 2.9 3.0 Queue Clearance Time ( $g_s$ ), s 5.1 8.5 9.0 8.1 5.3 14.7 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.0 0.0 0.0 1.6 0.0 8.0 Phase Call Probability 0.73 0.96 0.96 1.00 0.79 1.00 0.55 0.03 1.00 Max Out Probability 1.00 1.00 0.89 **Movement Group Results** EΒ WB NB SB Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 78 256 117 189 394 61 200 228 139 94 394 228 Adjusted Saturation Flow Rate ( s ), veh/h/ln 1474 1660 1490 1688 1772 1714 1772 1478 1688 1772 1478 1406 3.1 3.9 6.5 12.1 7.0 6.1 4.3 8.1 Queue Service Time ( $g_s$ ), s 4.0 1.9 3.3 12.7 Cycle Queue Clearance Time ( q c ), s 3.1 3.9 4.0 6.5 12.1 1.9 7.0 6.1 4.3 3.3 12.7 8.1 0.22 0.22 0.30 Green Ratio (g/C) 0.06 0.14 0.30 0.12 0.31 0.31 0.07 0.26 0.26 Capacity (c), veh/h 92 735 330 230 523 415 200 541 451 119 459 383 Volume-to-Capacity Ratio (X) 0.845 0.347 0.354 0.822 0.754 0.147 1.000 0.421 0.308 0.797 0.859 0.595 Back of Queue (Q), ft/ln (95 th percentile) Back of Queue (Q), veh/ln (95 th percentile) 2.1 2.5 2.5 6.1 9.1 1.0 10.0 3.6 2.1 2.7 9.5 4.2 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 Uniform Delay ( d 1 ), s/veh 27.8 19.7 19.7 25.2 19.2 15.6 26.5 16.6 16.0 27.5 21.2 19.5 Incremental Delay ( d 2 ), s/veh 7.7 1.3 3.0 18.3 9.7 0.7 63.6 0.2 0.1 10.2 10.8 0.7 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 35.6 21.0 22.7 43.5 28.9 16.3 90.1 16.8 16.1 37.7 31.9 20.2 Level of Service (LOS) D С С D С В F В В D С С 23.9 С 32.0 С 42.5 D 29.0 С Approach Delay, s/veh / LOS Intersection Delay, s/veh / LOS 32.1 С **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 2.10 В 2.10 В 2.26 2.27 В В Bicycle LOS Score / LOS 0.86 Α 1.55 1.42 Α 1.67

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	10/17/2023
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD 38 Build	Time Analyzed	2050
Project Description	464th_MickelsonRd_2050_ AM	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	55.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	55.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	638	Heavy Vehicle Adjustment Factor (fHV)	0.980
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	370
Total Trucks, %	2.00	Capacity (c), pc/h/ln	2100
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2100
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.18
Direction 1 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	55.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	6.7
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS			'
Flow Rate in Outside Lane (vOL), veh/h	362	Effective Speed Factor (St)	4.62
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.66
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	55.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	8.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	53.0	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	380	Heavy Vehicle Adjustment Factor (fHV)	0.885
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	244
Total Trucks, %	13.00	Capacity (c), pc/h/ln	2060
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2060
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	53.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.6
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	2.0		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	216	Effective Speed Factor (St)	4.62
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	6.14
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

HCS弧 Highways Version 2023 464th\_MickelsonRd\_2050\_AM.xuf Generated: 02/27/2024 13:47:39

	HCS Multilane	Highway Report				
Project Information						
Analyst	NM	Date	10/17/2023			
Agency	HR Green	Analysis Year	2050			
Jurisdiction	SD38 Build	Time Analyzed	PM			
Project Description	464th_MickelsonRd_PM	Units	U.S. Customary			
Direction 1 Geometric Data						
Direction 1	EB					
Number of Lanes (N), In	2	Terrain Type	Level			
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-			
Base Free-Flow Speed (BFFS), mi/h	55.0	Grade Length, mi	-			
Lane Width, ft	12	Access Point Density, pts/mi	0.0			
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6			
Free-Flow Speed (FFS), mi/h	55.0	Total Lateral Clearance (TLC), ft	12			
Direction 1 Adjustment Fact	tors	<u> </u>				
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000			
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000			
Driver Population CAF	1.000					
Direction 1 Demand and Ca	pacity					
Volume (V) veh/h	441	Heavy Vehicle Adjustment Factor (fHV)	0.943			
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	266			
Total Trucks, %	6.00	Capacity (c), pc/h/ln	2100			
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2100			
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.13			
Direction 1 Speed and Dens	ity	<u> </u>				
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	55.0			
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.8			
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А			
Access Point Density Adjustment (fA)	0.0					
Direction 1 Bicycle LOS						
Flow Rate in Outside Lane (vol.), veh/h	251	Effective Speed Factor (St)	4.62			
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.56			
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	D			

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	55.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	8.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	53.0	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	730	Heavy Vehicle Adjustment Factor (fHV)	0.990
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	419
Total Trucks, %	1.00	Capacity (c), pc/h/ln	2060
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2060
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.20
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	53.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	7.9
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	2.0		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	415	Effective Speed Factor (St)	4.62
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.50
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	В

HCSTM Highways Version 2023 464th\_MickelsonRd\_2050\_PM.xuf Generated: 02/27/2024 11:11:27

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR GREEN INC	Analysis Year	2050
Jurisdiction	SD 38	Time Analyzed	PM
Project Description	2050 Build Analysis - 468th St to 469th St	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	1.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.8	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	410	Heavy Vehicle Adjustment Factor (fHV)	0.952
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	244
Total Trucks, %	5.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.11
Direction 1 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.8
Total Lateral Clearance Adj. (fllc)	0.0	Density (D), pc/mi/ln	3.5
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.3		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	233	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.42
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	5.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	68.8	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	280	Heavy Vehicle Adjustment Factor (fHV)	0.862
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	184
Total Trucks, %	16.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.08
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	68.8
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	2.7
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	1.3		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vol), veh/h	159	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	8.07
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

HCSTM Highways Version 2023 468th\_469th\_Multilane\_2050\_AM.xuf Generated: 02/27/2024 11:45:56

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR GREEN INC	Analysis Year	2050
Jurisdiction	SD 38	Time Analyzed	PM
Project Description	2050 Build Analysis - 468th St to 469th St	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	1.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.8	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	370	Heavy Vehicle Adjustment Factor (fHV)	0.935
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	225
Total Trucks, %	7.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.10
Direction 1 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.8
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	3.2
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.3		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	210	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	4.05
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	D

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	5.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	68.8	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	490	Heavy Vehicle Adjustment Factor (fHV)	0.980
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	284
Total Trucks, %	2.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	68.8
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.1
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	1.3		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	278	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.65
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

HCSTM Highways Version 2023 468th\_469th\_Multilane\_2050\_PM.xuf Generated: 02/27/2024 11:46:39

	HCS Multilane	e Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	AM
Project Description	469th to LaMesa	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	2.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	ors	·	
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Ca	pacity		
Volume (V) veh/h	734	Heavy Vehicle Adjustment Factor (fHV)	0.962
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	434
Total Trucks, %	4.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.19
Direction 1 Speed and Dens	ity	<u>'</u>	
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.5
Total Lateral Clearance Adj. (fllc)	0.0	Density (D), pc/mi/ln	6.2
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.5		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	417	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.41
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

Direction 2 Geometric Data						
Direction 2	WB					
Number of Lanes (N), In	2	Terrain Type	Level			
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-			
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-			
Lane Width, ft	12	Access Point Density, pts/mi	1.0			
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6			
Free-Flow Speed (FFS), mi/h	69.8	Total Lateral Clearance (TLC), ft	12			
Direction 2 Adjustment Factor	ors					
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000			
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000			
Driver Population CAF	1.000					
Direction 2 Demand and Cap	acity					
Volume (V) veh/h	240	Heavy Vehicle Adjustment Factor (fHV)	0.820			
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	166			
Total Trucks, %	22.00	Capacity (c), pc/h/ln	2300			
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300			
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.07			
Direction 2 Speed and Densi	ty					
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.8			
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	2.4			
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А			
Access Point Density Adjustment (fA)	0.3					
Direction 2 Bicycle LOS						
Flow Rate in Outside Lane (vol.), veh/h	136	Effective Speed Factor (St)	5.07			
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	11.74			
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F			
Converight © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	avs Varsian 2023	Congrated: 02/27/2024 11:40:30			

HCSTM Highways Version 2023 469th\_LaMesaDr\_2050\_AM.xuf Generated: 02/27/2024 11:49:39

	HCS Multilane	Highway Report		
Project Information				
Analyst	NM	Date	2/27/2024	
Agency	HR Green	Analysis Year	2050	
Jurisdiction	SD38 Build	Time Analyzed	PM	
Project Description	469th to LaMesa	Units	U.S. Customary	
Direction 1 Geometric Data				
Direction 1	EB			
Number of Lanes (N), In	2	Terrain Type	Level	
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-	
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-	
Lane Width, ft	12	Access Point Density, pts/mi	2.0	
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6	
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12	
Direction 1 Adjustment Factor	ors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000	
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000	
Driver Population CAF	1.000			
Direction 1 Demand and Cap	pacity			
Volume (V) veh/h	351	Heavy Vehicle Adjustment Factor (fHV)	0.917	
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	218	
Total Trucks, %	9.00	Capacity (c), pc/h/ln	2300	
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300	
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.09	
Direction 1 Speed and Densi	ty	·		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.5	
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	3.1	
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А	
Access Point Density Adjustment (fA)	0.5			
Direction 1 Bicycle LOS				
Flow Rate in Outside Lane (vOL), veh/h	199	Effective Speed Factor (St)	5.07	
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	4.80	
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	E	

Direction 2 Geometric Data				
Direction 2	WB			
Number of Lanes (N), In	2	Terrain Type	Level	
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-	
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-	
Lane Width, ft	12	Access Point Density, pts/mi	1.0	
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6	
Free-Flow Speed (FFS), mi/h	69.8	Total Lateral Clearance (TLC), ft	12	
Direction 2 Adjustment Factors				
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000	
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000	
Driver Population CAF	1.000			
Direction 2 Demand and Capacity				
Volume (V) veh/h	666	Heavy Vehicle Adjustment Factor (fHV)	0.971	
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	390	
Total Trucks, %	3.00	Capacity (c), pc/h/ln	2300	
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300	
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.17	
Direction 2 Speed and Density				
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.8	
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	5.6	
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А	
Access Point Density Adjustment (fA)	0.3			
Direction 2 Bicycle LOS				
Flow Rate in Outside Lane (vol.), veh/h	378	Effective Speed Factor (St)	5.07	
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.07	
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С	

HCSTM Highways Version 2023 469th\_LaMesaDr\_2050\_PM.xuf Generated: 02/27/2024 11:50:51

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR GREEN INC	Analysis Year	2050
Jurisdiction	SD 38	Time Analyzed	AM
Project Description	2050 Build Analysis - 466th Avenue S/EB Exit Ramp to 468th St	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	4.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	acity		
Volume (V) veh/h	364	Heavy Vehicle Adjustment Factor (fHV)	0.935
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	221
Total Trucks, %	7.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.10
Direction 1 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	3.2
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	1.0		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	207	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	4.05
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	D

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	2.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	acity		
Volume (V) veh/h	260	Heavy Vehicle Adjustment Factor (fHV)	0.901
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	164
Total Trucks, %	11.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.07
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.5
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	2.4
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.5		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	148	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	5.50
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	Е
Converight © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	avs Varsian 2023	Generated: 02/27/2024 11:43:01

HCS MM Highways Version 2023
I90EBRampS466thSt\_468th\_Multilane\_2050\_AM.xuf

Generated: 02/27/2024 11:43:01

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR GREEN INC	Analysis Year	2050
Jurisdiction	SD 38	Time Analyzed	PM
Project Description	2050 Build Analysis - 466th Avenue S/EB Exit Ramp to 468th St	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	ЕВ		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	4.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Facto	rs		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Capa	acity		
Volume (V) veh/h	312	Heavy Vehicle Adjustment Factor (fHV)	0.909
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	195
Total Trucks, %	10.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.08
Direction 1 Speed and Density	у		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	2.8
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	1.0		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vol), veh/h	177	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	5.16
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	E

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	2.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	acity		
Volume (V) veh/h	420	Heavy Vehicle Adjustment Factor (fHV)	0.935
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	255
Total Trucks, %	7.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.11
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.5
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	3.7
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.5		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	239	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	4.12
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	D
Convigant © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	avs Varsian 2023	Generated: 02/27/2024 11:44:0

HCSTM Highways Version 2023 I90EBRampS466thSt\_468th\_Multilane\_2050\_PM.xuf Generated: 02/27/2024 11:44:01

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	AM
Project Description	I90 WB Ramps to I90 EB Ramps	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	745	Heavy Vehicle Adjustment Factor (fHV)	0.971
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	436
Total Trucks, %	3.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.19
Direction 1 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	6.2
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	423	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.13
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

Direction 2 Geometric Data				
Direction 2	WB			
Number of Lanes (N), In	2	Terrain Type	Level	
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-	
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-	
Lane Width, ft	12	Access Point Density, pts/mi	0.0	
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6	
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12	
Direction 2 Adjustment Factor	ors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000	
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000	
Driver Population CAF	1.000			
Direction 2 Demand and Cap	pacity			
Volume (V) veh/h	273	Heavy Vehicle Adjustment Factor (fHV)	0.877	
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	177	
Total Trucks, %	14.00	Capacity (c), pc/h/ln	2300	
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300	
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.08	
Direction 2 Speed and Densi	ty			
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0	
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	2.5	
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А	
Access Point Density Adjustment (fA)	0.0			
Direction 2 Bicycle LOS				
Flow Rate in Outside Lane (vOL), veh/h	155	Effective Speed Factor (St)	5.07	
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	6.98	
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F	

HCSໝ Highways Version 2023 I90WBRamp\_I90EBRamp\_2050\_AM.xuf Generated: 02/27/2024 11:34:14

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	PM
Project Description	I90 WB Ramps to I90 EB Ramps	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	451	Heavy Vehicle Adjustment Factor (fHV)	0.917
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	280
Total Trucks, %	9.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12
Direction 1 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.0
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	256	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	4.92
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	E

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	acity		
Volume (V) veh/h	455	Heavy Vehicle Adjustment Factor (fHV)	0.877
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	295
Total Trucks, %	14.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.13
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.2
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.0		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	259	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	7.24
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F
Converight © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	nys Varsian 2022	Congrated: 02/27/2024 11:40:50

HCSTM Highways Version 2023 I90WBRamp\_I90EBRamp\_2050\_PM.xuf Generated: 02/27/2024 11:40:50

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	AM
Project Description	Mickelson Rd to 466th St	Units	U.S. Customary
<b>Direction 1 Geometric Data</b>			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	1.5
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.6	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	pacity		
Volume (V) veh/h	725	Heavy Vehicle Adjustment Factor (fHV)	0.990
Peak Hour Factor	0.88	Flow Rate (Vp), pc/h/ln	416
Total Trucks, %	1.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.18
Direction 1 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.6
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	6.0
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.4		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	412	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.61
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	2.2
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	pacity		
Volume (V) veh/h	425	Heavy Vehicle Adjustment Factor (fHV)	0.885
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	273
Total Trucks, %	13.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.4
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	3.9
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.6		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vOL), veh/h	241	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	6.70
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

HCSTM Highways Version 2023 Mickelson\_466th\_2050\_AM.xuf Generated: 02/27/2024 11:27:33

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	PM
Project Description	Mickelson Rd to 466th St	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	1.5
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.6	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Ca	pacity		
Volume (V) veh/h	445	Heavy Vehicle Adjustment Factor (fHV)	0.901
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	280
Total Trucks, %	11.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12
Direction 1 Speed and Dens	ity		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.6
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.0
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	A
Access Point Density Adjustment (fA)	0.4		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	253	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	5.78
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F

Direction 2 Geometric Data			
Direction 2	WB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	2.2
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	69.5	Total Lateral Clearance (TLC), ft	12
Direction 2 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 2 Demand and Cap	acity		
Volume (V) veh/h	913	Heavy Vehicle Adjustment Factor (fHV)	0.980
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	530
Total Trucks, %	2.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.23
Direction 2 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	69.4
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	7.6
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.6		
Direction 2 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	519	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.97
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С
Convigant © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	avs Varsian 2023	Ganarated: 02/27/2024 11:26:53

HCSTM Highways Version 2023 Mickelson\_466th\_2050\_PM.xuf Generated: 02/27/2024 11:26:53

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	AM
Project Description	466th St to I90 WB Ramps	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Fact	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Ca	pacity		
Volume (V) veh/h	769	Heavy Vehicle Adjustment Factor (fHV)	0.980
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	446
Total Trucks, %	2.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.19
Direction 1 Speed and Dens	ity		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	6.4
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS			
Flow Rate in Outside Lane (vol.), veh/h	437	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	2.88
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С

Direction 2 Geometric Data								
Direction 2	WB							
Number of Lanes (N), In	2	Terrain Type	Level					
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-					
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-					
Lane Width, ft	12	Access Point Density, pts/mi	0.0					
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6					
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12					
Direction 2 Adjustment Factors								
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000					
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000					
Driver Population CAF	1.000							
Direction 2 Demand and Cap	acity							
Volume (V) veh/h	436	Heavy Vehicle Adjustment Factor (fHV)	0.833					
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	298					
Total Trucks, %	20.00	Capacity (c), pc/h/ln	2300					
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300					
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.13					
Direction 2 Speed and Densi	ty							
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0					
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.3					
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А					
Access Point Density Adjustment (fA)	0.0							
Direction 2 Bicycle LOS								
Flow Rate in Outside Lane (vol.), veh/h	248	Effective Speed Factor (St)	5.07					
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	10.71					
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	F					
Convigant © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	nys Varsian 2023	Congrated: 02/27/2024 11:20:29					

HCSTM Highways Version 2023 466th\_I90WBRamp\_2050\_AM.xuf Generated: 02/27/2024 11:29:38

	HCS Multilane	Highway Report	
Project Information			
Analyst	NM	Date	2/27/2024
Agency	HR Green	Analysis Year	2050
Jurisdiction	SD38 Build	Time Analyzed	PM
Project Description	466th St to I90 WB Ramps	Units	U.S. Customary
Direction 1 Geometric Data			
Direction 1	EB		
Number of Lanes (N), In	2	Terrain Type	Level
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-
Lane Width, ft	12	Access Point Density, pts/mi	0.0
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12
Direction 1 Adjustment Factor	ors		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000
Driver Population CAF	1.000		
Direction 1 Demand and Cap	acity		
Volume (V) veh/h	450	Heavy Vehicle Adjustment Factor (fHV)	0.917
Peak Hour Factor	0.88	Flow Rate (V <sub>P</sub> ), pc/h/ln	279
Total Trucks, %	9.00	Capacity (c), pc/h/ln	2300
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.12
Direction 1 Speed and Densi	ty		
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	4.0
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	Α
Access Point Density Adjustment (fA)	0.0		
Direction 1 Bicycle LOS			·
Flow Rate in Outside Lane (vOL), veh/h	256	Effective Speed Factor (St)	5.07
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	4.92
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	E

Direction 2 Geometric Data								
Direction 2	WB							
Number of Lanes (N), In	2	Terrain Type	Level					
Measured or Base Free-Flow Speed	Base	Percent Grade, %	-					
Base Free-Flow Speed (BFFS), mi/h	70.0	Grade Length, mi	-					
Lane Width, ft	12	Access Point Density, pts/mi	0.0					
Median Type	Divided	Left-Side Lateral Clearance (LCR), ft	6					
Free-Flow Speed (FFS), mi/h	70.0	Total Lateral Clearance (TLC), ft	12					
Direction 2 Adjustment Factors								
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000					
Driver Population SAF	1.000	Final Capacity Adjustment Factor (CAF)	1.000					
Driver Population CAF	1.000							
Direction 2 Demand and Cap	acity							
Volume (V) veh/h	910	Heavy Vehicle Adjustment Factor (fHV)	0.971					
Peak Hour Factor	0.88	Flow Rate (V <sub>p</sub> ), pc/h/ln	532					
Total Trucks, %	3.00	Capacity (c), pc/h/ln	2300					
Single-Unit Trucks (SUT), %	-	Adjusted Capacity (cadj), pc/h/ln	2300					
Tractor-Trailers (TT), %	-	Volume-to-Capacity Ratio (v/c)	0.23					
Direction 2 Speed and Densi	ty							
Lane Width Adjustment (fLW)	0.0	Average Speed (S), mi/h	70.0					
Total Lateral Clearance Adj. (fLLC)	0.0	Density (D), pc/mi/ln	7.6					
Median Type Adjustment (fM)	0.0	Level of Service (LOS)	А					
Access Point Density Adjustment (fA)	0.0							
Direction 2 Bicycle LOS								
Flow Rate in Outside Lane (vol.), veh/h	517	Effective Speed Factor (St)	5.07					
Effective Width of Volume (Wv), ft	18	Bicyle LOS Score (BLOS)	3.23					
Average Effective Width (We), ft	24	Bicycle Level of Service (LOS)	С					
Converight © 2024 University of Florida, All Rights	Posonyod UCSTM Highwa	avs Varsian 2023	Generated: 02/27/2024 11:21:10					

HCSTM Highways Version 2023 466th\_I90WBRamp\_2050\_PM.xuf Generated: 02/27/2024 11:31:10

		HCS Two-La	ne	Highway Re	port	
Pro	oject Information		_			
Ana	lyst	MJV		Date		5/11/2023
Age	ncy	HRG		Analysis Year		2050 NB
Juri	sdiction	SDDOT		Time Analyzed		AM Peak
Proj	ect Description	West of Hartford SD 3	88 EB	Units		U.S. Customary
		S	egn	nent 1		·
Ve	hicle Inputs					
Seg	ment Type	Passing Zone		Length, ft		1069
Mea	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity					
Dire	ectional Demand Flow Rate, veh/h	243		Opposing Deman	d Flow Rate, veh/h	169
Pea	k Hour Factor	0.88		Total Trucks, %		5.79
Seg	ment Capacity, veh/h	1700		Demand/Capacity	, (D/C)	0.14
Int	ermediate Results	·		'		
Seg	ment Vertical Class	I Class 1			mi/h	70.0
Spe	ed Slope Coefficient (m)	4.30713		Speed Power Coefficient (p)		0.54838
PF S	Slope Coefficient (m)	-1.23090		PF Power Coefficie	ent (p)	0.80942
In P	assing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		1.2
%In	nprovement to Percent Followers	0.0		%Improvement to Speed		0.0
Su	bsegment Data					
#	Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	1069	1-		-	68.5
Ve	hicle Results					
Ave	rage Speed, mi/h	68.5		Percent Followers, %		32.4
	ment Travel Time, minutes	0.18		Follower Density (	(FD), followers/mi/ln	1.2
Veh	icle LOS	A				
Bio	cycle Results					
	cent Occupied Parking	0		Pavement Conditi	on Rating	4
Flov	v Rate Outside Lane, veh/h	243		Bicycle Effective Width, ft		24
Bicy	rcle LOS Score	3.70		Bicycle Effective S		5.07
	cle LOS D					
		S	egn	nent 2		
Ve	hicle Inputs					
	ment Type	Passing Constrained		Length, ft		664
	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0

Demand and Capacity					
	242		Opposite Desir	d Flour Data and the	
Directional Demand Flow Rate, veh/h	243		Opposing Demand Flow Rate, veh/h		
Peak Hour Factor	0.88		Total Trucks, %	(D/C)	5.79
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29315		PF Power Coefficie	ent (p)	0.75829
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.3
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Radi	ius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	664	-		-	68.0
Vehicle Results					
Average Speed, mi/h	68.0		Percent Followers	, %	35.8
Segment Travel Time, minutes	0.11		Follower Density (FD), followers/mi/ln		1.3
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	243		Bicycle Effective V	Vidth, ft	24
Bicycle LOS Score	3.70		Bicycle Effective Speed Factor		5.07
Bicycle LOS	D				
	S	egm	nent 3		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		1871
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	243		Opposing Demand Flow Rate, veh/h		169
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity	' (D/C)	0.14
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.31694		Speed Power Coe	fficient (p)	0.54838
PF Slope Coefficient (m)	-1.20586		PF Power Coefficie	ent (p)	0.82063
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.1
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					

#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	1871 -			-	68.5
Veł	nicle Results	-				
Aver	age Speed, mi/h	68.5		Percent Followers	, %	31.5
Segr	ment Travel Time, minutes	0.31		Follower Density (	(FD), followers/mi/ln	1.1
Vehi	cle LOS	A				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow	Rate Outside Lane, veh/h	243		Bicycle Effective V	Vidth, ft	24
Bicy	cle LOS Score	3.70		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	D				
			Segr	ment 4		
Veł	nicle Inputs					
Segr	ment Type	Passing Constrai	ned	Length, ft		925
Mea	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Dei	mand and Capacity	•				
Dire	ctional Demand Flow Rate, veh/h	243		Opposing Demand Flow Rate, veh/h		-
Peak	Hour Factor	0.88		Total Trucks, %		5.79
Segr	nent Capacity, veh/h	1700		Demand/Capacity (D/C)		0.14
Int	ermediate Results					
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	ed Slope Coefficient (m)	4.57372		Speed Power Coefficient (p)		0.41674
PF S	lope Coefficient (m)	-1.29315		PF Power Coefficient (p)		0.75829
In Pa	assing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		1.3
%lm	provement to Percent Followers	0.0		%Improvement to Speed		0.0
Suk	osegment Data					
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	925	-		-	68.0
Vel	nicle Results					
Aver	rage Speed, mi/h	68.0		Percent Followers	, %	35.8
Segr	ment Travel Time, minutes	0.15		Follower Density (	(FD), followers/mi/ln	1.3
Vehi	cle LOS	A				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow	Rate Outside Lane, veh/h	243		Bicycle Effective V	Vidth, ft	24
Bicy	cle LOS Score	3.70		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	D				

	Segment 5							
Ve	hicle Inputs							
Seg	ment Type	Passing Zone	Passing Zone			4476		
Mea	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0		
De	mand and Capacity							
Dire	ectional Demand Flow Rate, veh/h	243		Opposing Demand	d Flow Rate, veh/h	169		
Pea	k Hour Factor	0.88		Total Trucks, %		5.79		
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14		
Int	termediate Results							
Seg	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0		
Spe	ed Slope Coefficient (m)	4.35043		Speed Power Coef	fficient (p)	0.54838		
PF S	Slope Coefficient (m)	-1.15155		PF Power Coefficie	ent (p)	0.84082		
In P	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.1		
%In	nprovement to Percent Followers	0.0		%Improvement to	Speed	0.0		
Su	bsegment Data							
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h		
1	Tangent	4476	1-		-	68.5		
Ve	hicle Results							
Ave	rage Speed, mi/h	68.5		Percent Followers,	. %	29.6		
Seg	ment Travel Time, minutes	0.74		Follower Density (FD), followers/mi/ln		1.1		
Veh	icle LOS	А						
Bio	cycle Results					·		
Per	cent Occupied Parking	0		Pavement Condition	on Rating	4		
Flov	w Rate Outside Lane, veh/h	243		Bicycle Effective Width, ft		24		
Bicy	vcle LOS Score	3.70		Bicycle Effective S	peed Factor	5.07		
Bicy	rcle LOS	D						
			Seg	ment 6				
Ve	hicle Inputs							
	ment Type	Passing Constrained		Length, ft		896		
Mea	asured FFS	Measured			mi/h	70.0		
De	mand and Capacity							
	ectional Demand Flow Rate, veh/h	243	243		d Flow Rate, veh/h	-		
Pea	k Hour Factor	0.88		Total Trucks, %		5.79		
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14		
Int	termediate Results							
Sea	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0		
				1				

Speed Slope Coefficient (m)	4.57372	4.57372		fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29315		PF Power Coefficie	ent (p)	0.75829
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.3
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	896	-		-	68.0
Vehicle Results					
Average Speed, mi/h	68.0		Percent Followers	, %	35.8
Segment Travel Time, minutes	0.15		Follower Density (	FD), followers/mi/ln	1.3
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	243		Bicycle Effective V	Vidth, ft	24
Bicycle LOS Score	3.70		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	D				
		Segn	nent 7		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		743
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	243		Opposing Demand Flow Rate, veh/h		169
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.14
Intermediate Results					·
Segment Vertical Class	1		Free-Flow Speed, mi/h		70.0
Speed Slope Coefficient (m)	4.30713		Speed Power Coe	fficient (p)	0.54838
PF Slope Coefficient (m)	-1.23090		PF Power Coefficie	ent (p)	0.80942
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.2
%Improvement to Percent Followers	0.0		%Improvement to Speed		0.0
Subsegment Data					
# Segment Type	Length, ft	Length, ft Rad		Superelevation, %	Average Speed, mi/h
1 Tangent	743			-	68.5
Vehicle Results	<u>'</u>				
Average Speed, mi/h	68.5		Percent Followers, %		32.4
Segment Travel Time, minutes	0.12		Follower Density (FD), followers/mi/ln		1.2
Vehicle LOS	А				

Bicycle Results					
Percent Occupied Parking	0 P		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	243		Bicycle Effective W	/idth, ft	24
Bicycle LOS Score	3.70		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	D				
	Se	egn	nent 8		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		2717
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	245		Opposing Demand	d Flow Rate, veh/h	165
Peak Hour Factor	0.88		Total Trucks, %		3.28
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.32768		Speed Power Coefficient (p)		0.54983
PF Slope Coefficient (m)	-1.17918		PF Power Coefficient (p)		0.83165
In Passing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		1.1
%Improvement to Percent Followers	0.0		%Improvement to Speed		0.0
Subsegment Data					
# Segment Type	Length, ft	Rad	lius, ft Superel	Superelevation, %	Average Speed, mi/h
1 Tangent	2717	-		-	68.5
Vehicle Results					
Average Speed, mi/h	68.5		Percent Followers, %		30.7
Segment Travel Time, minutes	0.45		Follower Density (FD), followers/mi/ln		1.1
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition Rating		4
Flow Rate Outside Lane, veh/h	245		Bicycle Effective W	/idth, ft	24
Bicycle LOS Score	2.93		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	С				
	Se	egn	nent 9		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		1013
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					

Directional Demand Flow Rate, veh/h	245		Opposing Demand	d Flow Rate, yeh/h	-
Peak Hour Factor	0.88		Total Trucks, %	THOW Nate, Vell/II	3.28
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.14
	1700		Беппапа/Сарасіту	(6/0)	0.14
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,		70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coef	ficient (p)	0.41674
PF Slope Coefficient (m)	-1.29345		PF Power Coefficie	nt (p)	0.75792
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.3
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Radi	ius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	1013	-		-	68.0
Vehicle Results					
Average Speed, mi/h	68.0		Percent Followers,	%	36.0
Segment Travel Time, minutes	0.17		Follower Density (	FD), followers/mi/ln	1.3
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition Rating		4
Flow Rate Outside Lane, veh/h	245		Bicycle Effective W	idth, ft	24
Bicycle LOS Score	2.93		Bicycle Effective Speed Factor		5.07
Bicycle LOS	С				
	Se	gm	ent 10		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		4569
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	245		Opposing Demand	d Flow Rate, veh/h	165
Peak Hour Factor	0.88		Total Trucks, %		3.28
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.34958		Speed Power Coef	ficient (p)	0.54983
PF Slope Coefficient (m)	-1.14981		PF Power Coefficie	ent (p)	0.84100
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.1
%Improvement to Percent Followers	0.0		%Improvement to Speed 0.0		0.0
Subsegment Data					
# Segment Type	Length, ft	Radi	ius, ft	Superelevation, %	Average Speed, mi/h

1 Tangent	4569	-		-	68.5
Vehicle Results					
Average Speed, mi/h	68.5		Percent Followers,	. %	29.7
Segment Travel Time, minutes	0.76		Follower Density (	FD), followers/mi/ln	1.1
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	245		Bicycle Effective W	/idth, ft	24
Bicycle LOS Score	2.93		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	С				
	\$	Segn	nent 11		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		5676
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	244		Opposing Demand Flow Rate, veh/h		165
Peak Hour Factor	0.88		Total Trucks, %		2.82
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.14
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.36055		Speed Power Coefficient (p)		0.54983
PF Slope Coefficient (m)	-1.14222		PF Power Coefficient (p)		0.84066
In Passing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		1.1
%Improvement to Percent Followers	0.0		%Improvement to Speed		0.0
Subsegment Data					
# Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	5676	-		-	68.5
Vehicle Results					·
Average Speed, mi/h	68.5		Percent Followers, %		29.5
Segment Travel Time, minutes	0.94		Follower Density (FD), followers/mi/ln		1.1
Vehicle LOS	A				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	244			/idth, ft	24
Bicycle LOS Score	2.80		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	С				
	9	Segn	nent 12		

Ve	ehicle Inputs					
Seg	gment Type	Passing Constrained		Length, ft		657
Me	easured FFS	Measured		Free-Flow Speed, mi/h		70.0
De	emand and Capacity					
Dir	ectional Demand Flow Rate, veh/h	244		Opposing Deman	d Flow Rate, veh/h	-
Pea	ak Hour Factor	0.88		Total Trucks, %		2.82
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
ln <sup>-</sup>	termediate Results					
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	eed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF	Slope Coefficient (m)	-1.29350		PF Power Coefficie	ent (p)	0.75785
In F	Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.3
%Ir	mprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Su	ıbsegment Data					
#	Segment Type	Length, ft Radi		dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	657 -			-	68.0
Ve	hicle Results					·
Ave	erage Speed, mi/h	68.0		Percent Followers,	. %	35.9
Seg	gment Travel Time, minutes	0.11		Follower Density (	FD), followers/mi/ln	1.3
Veł	nicle LOS	A				
Bi	cycle Results					
Per	cent Occupied Parking	0		Pavement Condition	on Rating	4
Flo	w Rate Outside Lane, veh/h	244		Bicycle Effective Width, ft		24
Bic	ycle LOS Score	2.80		Bicycle Effective Speed Factor		5.07
Bic	ycle LOS	С				
		S	egm	nent 13		
Ve	hicle Inputs					
Seg	gment Type	Passing Zone		Length, ft		6009
Me	easured FFS	Measured		Free-Flow Speed, mi/h		70.0
De	emand and Capacity					
Dir	ectional Demand Flow Rate, veh/h	244		Opposing Deman	d Flow Rate, veh/h	165
Pea	ak Hour Factor	0.88		Total Trucks, %		2.82
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.14
ln <sup>-</sup>	termediate Results					
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	eed Slope Coefficient (m)	4.36364		Speed Power Coe	fficient (p)	0.54983
PF	Slope Coefficient (m)	-1.14089		PF Power Coefficie	ent (p)	0.83997

In Passing Lane Effective Length?		No		Total Segment D	ensity, veh/mi/ln	1.1
%Improvement to Percent Followers		0.0		%Improvement	%Improvement to Speed	
Suk	osegment Data					
#	Segment Type	Length, ft	Length, ft Radius		Superelevation, %	Average Speed, mi/h
1	Tangent	6009 -			-	68.5
Vel	nicle Results					
Aver	age Speed, mi/h	68.5		Percent Follower	rs, %	29.5
Segr	ment Travel Time, minutes	1.00		Follower Density	(FD), followers/mi/ln	1.1
Vehi	cle LOS	А				
Bic	ycle Results					
Perce	ent Occupied Parking	0		Pavement Condi	tion Rating	4
Flow	Rate Outside Lane, veh/h	244 E		Bicycle Effective	Width, ft	24
Bicyc	cle LOS Score	2.80		Bicycle Effective	Speed Factor	5.07
Bicyc	cle LOS	С				
		•	Segr	ment 14		
Vel	nicle Inputs					
Segr	ment Type	Passing Constrain	ned	Length, ft		891
Mea	sured FFS	Measured		Free-Flow Speed	l, mi/h	50.0
Der	mand and Capacity					
Dire	ctional Demand Flow Rate, veh/h	244		Opposing Dema	nd Flow Rate, veh/h	-
Peak	Hour Factor	0.88	0.88			2.82
Segr	nent Capacity, veh/h	1700		Demand/Capaci	ty (D/C)	0.14
Inte	ermediate Results					
Segr	ment Vertical Class	1		Free-Flow Speed	I, mi/h	50.0
Spee	ed Slope Coefficient (m)	4.57372		Speed Power Co	efficient (p)	0.41674
PF SI	lope Coefficient (m)	-1.47375		PF Power Coeffic	cient (p)	0.71164
In Pa	ssing Lane Effective Length?	No		Total Segment D	Total Segment Density, veh/mi/ln	
%lm	provement to Percent Followers	0.0		%Improvement	%Improvement to Speed	
Suk	osegment Data					
#	Segment Type	Length, ft	R	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	891	-		-	48.0
Vel	nicle Results					
Aver	age Speed, mi/h	48.0		Percent Follower	rs, %	41.8
Segr	ment Travel Time, minutes	0.21			(FD), followers/mi/ln	2.1
Vehi	cle LOS	В				
Bic	ycle Results					,
	ent Occupied Parking	0		Pavement Condi	tion Rating	4
	· J			1	<u> </u>	

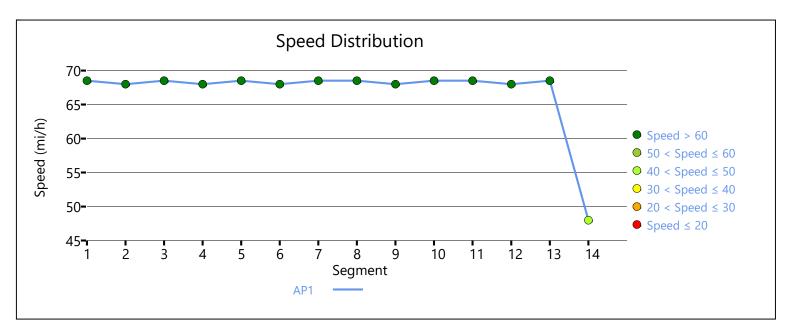
т	VMT	VHD	Follower Density followers/	LOS		
Facility	Facility Results					
Bicycle LC	OS	С				
Bicycle LC	OS Score	2.59	Bicycle Effective Speed Factor	4.42		
Flow Rate Outside Lane, veh/h 244			Bicycle Effective Width, ft	24		

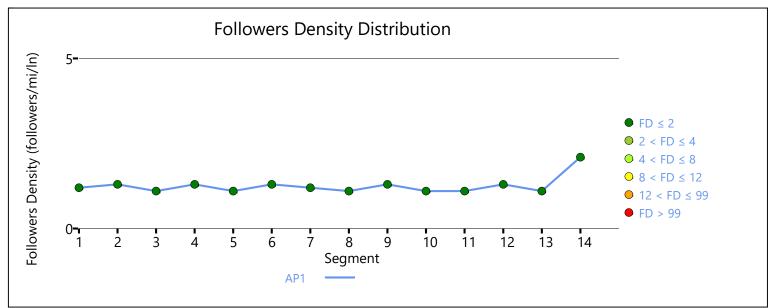
veh-h/p

0.11

mi/ln

1.1





veh-mi/p

327

Α

Analyst MIV Date 5/11/2023 Agency HRG Analysis Year 2050 NB  Arradiction SDOOT Firme Analyzed PM Peak Project Description West of Hartford SD 38 EB Units U.S. Customary  Segment 1  Vehicle Inputs  Segment Type Passing Zone Length, It 1069 Measured FFS Measured Free-Flow Speed, mi/h 70.0  Demand and Capacity Directional Demand Flow Rate, veh/h 170.0  Demand Bemand Flow Rate, veh/h 170.0  Demand Jemand Flow Rate, veh/h 170.0  Demand Capacity Directional Demand Flow Rate, veh/h 170.0  Demand Capacity  Interrediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.1767 Speed Power Coefficient (p) 0.51808  PF Slope Coefficient (m) 4.126475 PF Power Coefficient (p) 0.80124  In Passing Lane Effective Length P No Total Segment to Speed Do.  Subsequent Data  Segment Type Length, It Radius, It Superelevation, Average Speed, mi/h 1069  Vehicle Results  Average Speed, mi/h 69.0  Percent Followers A No Superelevation, Average Speed, mi/h 1069  Vehicle Results  Percent Travel Time, minutes 0.18 Follower Density, IPD, followers/mi/ln 0.6  Vehicle Results  Percent Coccupied Parking 0 Passing Constrained Length, It 30  Bicycle Results  Percent Coulded Lane, veh/h 157 Bicycle Effective Speed Factor 5.07  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Segment 19pe Length Passing Length Results  Segment 19pe 1.86 Bicycle Effective Speed Factor 5.07  Segment 2  Vehicle Inputs  Segment 19pe 1.86 Bicycle Effective Speed Factor 5.07		HCS Two	-Lane	Highway Re	port	
Agency HRG Analysis Year 2050 NB  Jurisdiction SDOOT Time Analyzed PM Peak  Project Description West of Hartford SD 38 EB Units U.S. Customary  Segment 1  Vehicle Inputs  Segment Type Passing Zone Length, ft 1069  Measured FFS Measured Free-Flow Speed, mi/h 70.0  Demand and Capacity  Directional Demand Flow Rate, veh/h 157 Opposing Demand Flow Rate, veh/h 286  Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.09  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.34767 Speed Power Coefficient (p) 0.51808  PF Slope Coefficient (m) 1.25475 PF Power Coefficient (p) 0.80124 In Passing Lane Effective Length? No Total Segment Density, veh/mi/h 0.6  Subsegment Data  # Segment Type Length? Radius, ft Superelevation, % Average Speed, mi/h 1 Tangent 1069 - 69.0  Vehicle Results  Average Speed, mi/h 69.0 Percent Followers, % 24.8  Segment Travel Time, minutes 0.18 Follower Density (FD), followers/mi/l 0.6  Vehicle LOS A Follower Coefficient Radius, ft Superelevation, % Average Speed, mi/h 1 Tangent 1069 - 69.0  Vehicle Results  Percent Occupied Parking 0 Percent Coefficient Width, ft 30  Biocycle Results  Percent Occupied Parking 0 Percent Coefficient Parking 4 A Segment Density (FD), followers/mi/line 0.6  Free-Flow Speed Factor 5.07  Bicycle LOS Score 1.86 Bicycle Effective Width, ft 30  Bicycle LOS Score 1.86 Bicycle Effective Width, ft 30  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Segment Type Passing Constrained Length, ft 664	Project Information					
Time Analyzed	Analyst	MJV		Date		5/11/2023
Vehicle   Inputs	Agency	HRG	HRG			2050 NB
Segment 1  Vehicle Inputs  Segment Type	Jurisdiction	SDDOT		Time Analyzed		PM Peak
Vehicle Inputs   Segment Type   Passing Zone   Length, ft   1069	Project Description	West of Hartford	SD 38 EB	Units		U.S. Customary
Segment Type			Segn	nent 1		
Measured FFS         Measured         Free-Flow Speed, mi/h         70.0           Demand and Capacity           Directional Demand Flow Rate, veh/h         157         Opposing Demand Flow Rate, veh/h         286           Peak Hour Factor         0.88         Total Trucks, %         5.79           Segment Capacity, veh/h         1700         Demand/Capacity (D/C)         0.09           Intermediate Results           Segment Vertical Class         1         Free-Flow Speed, mi/h         70.0           Speed Slope Coefficient (m)         4.34767         Speed Power Coefficient (p)         0.51808           PF Slope Coefficient (m)         -1.25475         PF Power Coefficient (p)         0.80124           In Passing Lane Effective Length?         No         Total Segment Density, veh/mi/In         0.6           Segment Type         Length, ft         Radius, ft         Superelevation, %         Average Speed, mi/h           Vehicle Results           Average Speed, mi/h         69.0         Percent Followers, %         24.8           Segment Travel Time, minutes         0.18         Follower Density (FD), followers/mi/In         0.6           Vehicle Results           Bicycle	Vehicle Inputs					
Demand and Capacity  Directional Demand Flow Rate, veh/h 157 Opposing Demand Flow Rate, veh/h 286  Peak Hour Factor 0.88 Total Trucks, % 5.79  Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.09  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.34767 Speed Power Coefficient (p) 0.51808  PF Slope Coefficient (m) -1.25475 PF Power Coefficient (p) 0.80124  In Passing Lane Effective Length? No Total Segment Density, veh/mi/ln 0.6  **Improvement to Percent Followers 0.0 **Improvement to Speed 0.0  Subsegment Data  # Segment Type Length, ft Radius, ft Superelevation, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Coupied Parking File Speed Followers, * Average Speed, mi/h 69.0 Percent Followers, * Average Speed, mi/h 69.0 Percent Coupied Parking File Speed Followers, * Average Speed, mi/h 69.0 Percent Coupied Parking File Speed Followers, * Average Speed, mi/h 69.0 Percent Coupied Parking File Speed Followers, * Average Speed, mi/h 69.0 Percent Coupied Parking File Speed Followers, * Average Speed, mi/h 69.0 Percent Coupied Parking File Speed Followers, * Average Speed, mi/h 69.0 Percent Coupied Parking File Spee	Segment Type	Passing Zone		Length, ft		1069
Directional Demand Flow Rate, veh/h Peak Hour Factor  0.88 Total Trucks, % 5.79 Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.09  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0 Speed Slope Coefficient (m) 4.34767 Speed Power Coefficient (p) 0.80124 In Passing Lane Effective Length? No Total Segment Density, veh/mi/ln 0.6 %Improvement to Percent Followers 0.0 %Improvement to Speed 0.0  Subsegment Type Length, ft Radius, ft Superelevation, % Average Speed, mi/h 1 Tangent 0.18 Segment Type Percent Coefficient Superelevation, % Average Speed, mi/h 69.0 Percent Followers, % 24.8 Segment Travel Time, minutes 0.18 Follower Density (FD), followers/mi/ln 0.6 Wehicle LOS A  Bicycle Results  Percent Coupled Parking 0 Pawement Condition Rating 4 Bicycle LOS Score 1.86 Bicycle LOS B Segment 179e Passing Constrained Length, ft 664	Measured FFS	Measured	-		mi/h	70.0
Peak Hour Factor 0.88 Total Trucks, % 5.79  Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.09  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Segment Vertical Class 2 1 Speed Power Coefficient (p) 0.51808  PF Slope Coefficient (m) 4.34767 Speed Power Coefficient (p) 0.51808  PF Slope Coefficient (m) -1.25475 PF Power Coefficient (p) 0.80124  In Passing Lane Effective Length? No Total Segment Density, veh/mi/ln 0.6  %Improvement to Percent Followers 0.0 %Improvement to Speed 0.0  Subsegment Data  # Segment Type Length, ft Radius, ft Superelevation, % Average Speed, mi/h 1 Tangent 1069 - 59.0  Vehicle Results  Average Speed, mi/h 69.0 Percent Followers, % 24.8  Segment Travel Time, minutes 0.18 Follower Density (FD), followers/mi/ln 0.6  Wehicle LOS A Follower Density (FD), followers/mi/ln 0.6  Bicycle Results  Percent Occupied Parking 0 Pavement Condition Rating 4  Flow Rate Outside Lane, veh/h 157 Bicycle Effective Width, ft 30  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Bicycle LOS Segment 17pe Passing Constrained Length, ft 664	Demand and Capacity			<u> </u>		
Peak Hour Factor 0.88 Total Trucks, % 5.79  Segment Capacity, veh/h 1700 Demand/Capacity (D/C) 0.09  Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.34767 Speed Power Coefficient (p) 0.51808  PF Slope Coefficient (m) -1.25475 PF Power Coefficient (p) 0.80124  In Passing Lane Effective Length? No Total Segment Density, veh/mi/ln 0.6  %Improvement to Percent Followers 0.0 %Improvement to Speed 0.0  Subsegment Data  ## Segment Type Length, ft Radius, ft Superelevation, % Average Speed, mi/h 1 Tangent 1069 - 69.0  Vehicle Results  Average Speed, mi/h 69.0 Percent Followers, % 24.8  Segment Travel Time, minutes 0.18 Follower Density (FD), followers/mi/ln 0.6  Vehicle LOS A Follower Density (FD), followers/mi/ln 0.6  Bicycle Results  Percent Occupied Parking 0 Pavement Condition Rating 4  Flow Rate Outside Lane, veh/h 157 Bicycle Effective Speed Factor 5.07  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Bicycle LOS Segment Type Passing Constrained Length, ft 664	Directional Demand Flow Rate,	veh/h 157		Opposing Deman	d Flow Rate, veh/h	286
Intermediate Results  Segment Vertical Class 1 Free-Flow Speed, mi/h 70.0  Speed Slope Coefficient (m) 4.34767 Speed Power Coefficient (p) 0.51808  PF Slope Coefficient (m) -1.25475 PF Power Coefficient (p) 0.80124  In Passing Lane Effective Length? No Total Segment Density, veh/mi/ln 0.6  Simprovement to Percent Followers 0.0 Simprovement to Speed 0.0  Subsegment Data  ## Segment Type Length, ft Radius, ft Superelevation, Average Speed, mi/h 1 Tangent 1069 - 69.0  Vehicle Results  Average Speed, mi/h 69.0 Percent Followers, A Segment Travel Time, minutes 0.18 Follower Density (FD), followers/mi/ln 0.6  Bicycle Results  Percent Occupied Parking 0 Pavement Condition Rating 4  Flow Rate Outside Lane, veh/h 157 Bicycle Effective Width, ft 30  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Bicycle LOS  Bregment Type Passing Constrained Length, ft 664	Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Vertical Class  1 Free-Flow Speed, mi/h  70.0  Speed Slope Coefficient (m)  4.34767 Speed Power Coefficient (p)  0.51808  PF Slope Coefficient (m)  -1.25475 PF Power Coefficient (p)  0.80124  In Passing Lane Effective Length?  No Total Segment Density, veh/mi/ln  0.6  Subsegment Data  # Segment Type  Length, ft  Radius, ft  Superelevation, % Average Speed, mi/h  1 Tangent  1069  69.0  Vehicle Results  Average Speed, mi/h  69.0  Percent Followers, % 24.8  Segment Travel Time, minutes  0.18  Follower Density (FD), followers/mi/ln  0.6  Bicycle Results  Percent Occupied Parking  0 Pavement Condition Rating  4  Flow Rate Outside Lane, veh/h  157  Bicycle LOS Score  1.86  Bicycle Effective Speed Factor  5.07  Segment 2  Vehicle Inputs  Segment Type  Passing Constrained  Length, ft  664	Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.09
Speed Slope Coefficient (m) 4.34767 Speed Power Coefficient (p) 0.51808 PF Slope Coefficient (m) -1.25475 PF Power Coefficient (p) 0.80124 In Passing Lane Effective Length? No Total Segment Density, veh/mi/In 0.6 %Improvement to Percent Followers 0.0 %Improvement to Speed 0.0  Subsegment Data ## Segment Type	Intermediate Results			'		-
PF Slope Coefficient (m) -1.25475 PF Power Coefficient (p) 0.80124  In Passing Lane Effective Length? No Total Segment Density, veh/mi/In 0.6  %Improvement to Percent Followers 0.0 %Improvement to Speed 0.0  Subsegment Data  ## Segment Type Length, ft Radius, ft Superelevation, % Average Speed, mi/h 1 Tangent 1069 69.0  Vehicle Results  Average Speed, mi/h 69.0 Percent Followers, % 24.8  Segment Travel Time, minutes 0.18 Follower Density (FD), followers/mi/ln 0.6  Wehicle LOS A Follower Density (FD), followers/mi/ln 0.6  Bicycle Results  Percent Occupied Parking 0 Pavement Condition Rating 4  Flow Rate Outside Lane, veh/h 157 Bicycle Effective Width, ft 30  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664	Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
In Passing Lane Effective Length?  No Total Segment Density, veh/mi/ln 0.6 %Improvement to Percent Followers 0.0 %Improvement to Speed 0.0  Subsegment Data  # Segment Type	Speed Slope Coefficient (m)	4.34767		Speed Power Coe	fficient (p)	0.51808
%Improvement to Percent Followers  Subsegment Data  # Segment Type	PF Slope Coefficient (m)	-1.25475		PF Power Coefficie	ent (p)	0.80124
# Segment Type   Length, ft   Radius, ft   Superelevation, %   Average Speed, mi/h   1 Tangent   1069   -   -   69.0    Vehicle Results  Average Speed, mi/h   69.0   Percent Followers, %   24.8    Segment Travel Time, minutes   0.18   Follower Density (FD), followers/mi/ln   0.6    Vehicle LOS   A      Bicycle Results  Percent Occupied Parking   0   Pavement Condition Rating   4    Flow Rate Outside Lane, veh/h   157   Bicycle Effective Width, ft   30    Bicycle LOS Score   1.86   Bicycle Effective Speed Factor   5.07    Bicycle LOS Score   1.86   Bicycle Effective Speed Factor   5.07    Segment 2  Vehicle Inputs  Segment Type   Passing Constrained   Length, ft   664	In Passing Lane Effective Length	? No		Total Segment De	nsity, veh/mi/ln	0.6
# Segment Type	%Improvement to Percent Follo	wers 0.0		%Improvement to	Speed	0.0
Tangent 1069 - 69.0  Vehicle Results  Average Speed, mi/h 69.0 Percent Followers, % 24.8  Segment Travel Time, minutes 0.18 Follower Density (FD), followers/mi/ln 0.6  Vehicle LOS A  Bicycle Results  Percent Occupied Parking 0 Pavement Condition Rating 4  Flow Rate Outside Lane, veh/h 157 Bicycle Effective Width, ft 30  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Bicycle LOS  Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664	Subsegment Data			<u>'</u>		
Vehicle Results  Average Speed, mi/h Segment Travel Time, minutes  0.18 Follower Density (FD), followers/mi/ln 0.6 Vehicle LOS  A  Bicycle Results  Percent Occupied Parking 0 Pavement Condition Rating 4 Flow Rate Outside Lane, veh/h Bicycle LOS Score 1.86 Bicycle Effective Width, ft 30 Bicycle LOS  B  Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664	# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
Average Speed, mi/h  Segment Travel Time, minutes  O.18  Follower Density (FD), followers/mi/ln  O.6  Wehicle LOS  A  Bicycle Results  Percent Occupied Parking  O Pavement Condition Rating  Flow Rate Outside Lane, veh/h  Bicycle LOS Score  1.86  Bicycle Effective Width, ft  30  Bicycle LOS  Segment 2  Vehicle Inputs  Segment Type  Passing Constrained  Length, ft  664	1 Tangent	1069	-		-	69.0
Segment Travel Time, minutes  O.18 Follower Density (FD), followers/mi/In  O.6  Wehicle LOS  A  Bicycle Results  Percent Occupied Parking OPavement Condition Rating 4 Flow Rate Outside Lane, veh/h Bicycle LOS Score I.86 Bicycle Effective Width, ft 30 Bicycle LOS  Bicycle LOS  Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664	Vehicle Results					<u>'</u>
Wehicle LOS  Bicycle Results  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Bicycle Effective Width, ft  30  Bicycle LOS Score  1.86  Bicycle Effective Speed Factor  5.07  Segment 2  Vehicle Inputs  Segment Type  Passing Constrained  Length, ft  664	Average Speed, mi/h	69.0		Percent Followers	, %	24.8
Wehicle LOS  Bicycle Results  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Percent Occupied Parking  Bicycle Effective Width, ft  30  Bicycle LOS Score  1.86  Bicycle Effective Speed Factor  5.07  Segment 2  Vehicle Inputs  Segment Type  Passing Constrained  Length, ft  664	Segment Travel Time, minutes	0.18				0.6
Percent Occupied Parking 0 Pavement Condition Rating 4  Flow Rate Outside Lane, veh/h 157 Bicycle Effective Width, ft 30  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Bicycle LOS Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664	Vehicle LOS	A				
Percent Occupied Parking 0 Pavement Condition Rating 4  Flow Rate Outside Lane, veh/h 157 Bicycle Effective Width, ft 30  Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Bicycle LOS Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664	Bicycle Results					
Flow Rate Outside Lane, veh/h  Bicycle LOS Score  1.86  Bicycle Effective Width, ft  5.07  Bicycle LOS  Segment 2  Vehicle Inputs  Segment Type  Passing Constrained  Length, ft  664	Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Bicycle LOS Score 1.86 Bicycle Effective Speed Factor 5.07  Bicycle LOS Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664						
Segment 2  Vehicle Inputs  Segment Type  Passing Constrained  Length, ft  664				-		5.07
Segment 2  Vehicle Inputs  Segment Type Passing Constrained Length, ft 664	Bicycle LOS	В				
Vehicle Inputs       Segment Type     Passing Constrained     Length, ft     664			Segn	nent 2		
Segment Type Passing Constrained Length, ft 664	Vehicle Inputs		<u> </u>			
	•	Passing Constrain	ned	Length, ft		664
	Measured FFS			-	mi/h	

Demand and Capacity					
	157		Onnesia: D	d Flour Pate 1 "	
Directional Demand Flow Rate, veh/h	157		Opposing Demand Flow Rate, veh/h		
Peak Hour Factor	0.88		Total Trucks, %	(D/C)	5.79
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.09
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29315		PF Power Coefficie	ent (p)	0.75829
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Radi	ius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	664	-		-	68.6
Vehicle Results					
Average Speed, mi/h	68.6		Percent Followers	, %	27.2
Segment Travel Time, minutes	0.11		Follower Density (FD), followers/mi/ln		0.6
Vehicle LOS	A				
Bicycle Results		<u> </u>			
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	157		Bicycle Effective V	Vidth, ft	30
Bicycle LOS Score	1.86		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	В				
	S	Segm	ent 3		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		1871
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	157		Opposing Deman	d Flow Rate, veh/h	286
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.09
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.35747		Speed Power Coe	fficient (p)	0.51808
PF Slope Coefficient (m)	-1.22915		PF Power Coefficie	ent (p)	0.81213
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.5
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					

#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	1871	-		-	69.0
Vel	nicle Results	•				
Aver	age Speed, mi/h	69.0		Percent Followers	, %	23.9
Segr	ment Travel Time, minutes	0.31		Follower Density (	FD), followers/mi/ln	0.5
Vehi	cle LOS	A				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow	Rate Outside Lane, veh/h	157		Bicycle Effective V	Vidth, ft	30
Bicy	cle LOS Score	1.86		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	В				
			Segi	ment 4		
Vel	nicle Inputs					
Segr	ment Type	Passing Constrai	ned	Length, ft		925
Mea	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity			•		
Dire	ctional Demand Flow Rate, veh/h	157		Opposing Deman	d Flow Rate, veh/h	-
Peak	: Hour Factor	0.88		Total Trucks, %		5.79
Segr	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.09
Int	ermediate Results					·
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	ed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF S	lope Coefficient (m)	-1.29315		PF Power Coefficient (p)		0.75829
In Pa	assing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.6
%lm	provement to Percent Followers	0.0		%Improvement to Speed		0.0
Sul	osegment Data					
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	925	-		-	68.6
Vel	nicle Results					
Aver	age Speed, mi/h	68.6		Percent Followers	, %	27.2
Segment Travel Time, minutes		0.15		Follower Density (	FD), followers/mi/ln	0.6
Vehi	cle LOS	А				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
	Rate Outside Lane, veh/h	157	-		Vidth, ft	30
Bicy	cle LOS Score	1.86		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	В				

			Segi	ment 5		
Ve	hicle Inputs					
Seg	ment Type	Passing Zone		Length, ft		4476
Mea	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity					
Dire	ectional Demand Flow Rate, veh/h	157		Opposing Deman	d Flow Rate, veh/h	286
Pea	k Hour Factor	0.88		Total Trucks, %		5.79
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.09
Int	termediate Results					
Seg	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	ed Slope Coefficient (m)	4.39096		Speed Power Coe	fficient (p)	0.51808
PF S	Slope Coefficient (m)	-1.17364		PF Power Coefficie	ent (p)	0.83159
In P	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.5
%In	nprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Su	bsegment Data					
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	4476	-		-	69.0
Ve	hicle Results					
Ave	rage Speed, mi/h	69.0		Percent Followers,	. %	22.2
Seg	ment Travel Time, minutes	0.74		Follower Density (	FD), followers/mi/ln	0.5
Veh	icle LOS	А				
Bio	cycle Results					
Per	cent Occupied Parking	0		Pavement Conditi	on Rating	4
Flov	w Rate Outside Lane, veh/h	157		Bicycle Effective Width, ft		30
Bicy	/cle LOS Score	1.86		Bicycle Effective Speed Factor		5.07
Bicy	rcle LOS	В				
			Segi	ment 6		
Ve	hicle Inputs					
	ment Type	Passing Constrained		Length, ft		896
Mea	Measured FFS Measured		Free-Flow Speed,	mi/h	70.0	
De	mand and Capacity					
	ectional Demand Flow Rate, veh/h	157		Opposing Deman	d Flow Rate, veh/h	-
Pea	k Hour Factor	0.88		Total Trucks, %		5.79
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.09
Int	termediate Results					
Sea	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
				1		

Speed Slope Coefficient (m)	4.57372		Speed Power Coe		0.41674
PF Slope Coefficient (m)	-1.29315		PF Power Coefficient (p)		0.75829
In Passing Lane Effective Length?			Total Segment De	nsity, veh/mi/ln	0.6
%Improvement to Percent Followers	0.0	0.0		Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rac	lius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	896	896 -		-	68.6
Vehicle Results					
Average Speed, mi/h	68.6		Percent Followers	, %	27.2
Segment Travel Time, minutes	0.15		Follower Density (	FD), followers/mi/ln	0.6
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	157		Bicycle Effective V	Vidth, ft	30
Bicycle LOS Score	1.86		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	В				
		Segn	nent 7		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		743
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	157		Opposing Deman	d Flow Rate, veh/h	286
Peak Hour Factor	0.88		Total Trucks, %		5.79
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.09
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.34767		Speed Power Coefficient (p)		0.51808
PF Slope Coefficient (m)	-1.25475		PF Power Coefficie	ent (p)	0.80124
In Passing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.6
%Improvement to Percent Followers	0.0		%Improvement to Speed		0.0
Subsegment Data					
# Segment Type	Length, ft Radi		lius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent				-	69.0
Vehicle Results	•				•
Average Speed, mi/h	69.0		Percent Followers, %		24.8
Segment Travel Time, minutes	0.12		Follower Density (FD), followers/mi/ln		0.6
Vehicle LOS A					

Bicycle	Results					
Percent Oc	cupied Parking	0		Pavement Condition	on Rating	4
Flow Rate 0	Outside Lane, veh/h	157		Bicycle Effective Width, ft		30
Bicycle LOS	Score	1.86		Bicycle Effective S	peed Factor	5.07
Bicycle LOS		В				
		S	egn	nent 8		·
Vehicle	Inputs					
Segment Ty	ype	Passing Zone		Length, ft		2717
Measured I		Measured		Free-Flow Speed,	mi/h	70.0
Demano	d and Capacity	_				
Directional	Demand Flow Rate, veh/h	164		Opposing Demand	d Flow Rate, veh/h	289
Peak Hour	Factor	0.88		Total Trucks, %		3.28
Segment C	apacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Interme	diate Results	·				
Segment V	ertical Class	1	1		mi/h	70.0
Speed Slop	pe Coefficient (m)			Speed Power Coefficient (p)		0.51760
PF Slope Co	oefficient (m)	-1.20338	-1.20338		ent (p)	0.82225
In Passing I	Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%Improver	ment to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegi	ment Data					
# Segn	nent Type	Length, ft	Rad	lius, ft	Superelevation, %	Average Speed, mi/h
1 Tang	ent	2717	1-		-	68.9
Vehicle	Results					
Average Sp	peed, mi/h	68.9		Percent Followers,	%	23.8
Segment Ti	ravel Time, minutes	0.45		Follower Density (FD), followers/mi/ln		0.6
Vehicle LOS	5	А				
Bicycle	Results					
Percent Oc	cupied Parking	0		Pavement Condition	on Rating	4
Flow Rate 0	Outside Lane, veh/h	164		Bicycle Effective W	/idth, ft	29
Bicycle LOS Score		1.40		Bicycle Effective S	peed Factor	5.07
Bicycle LOS		А				
		S	egn	nent 9		
Vehicle	Inputs					
Carrent T	уре	Passing Constrained		Length, ft		1013
Segment Type P		Measured		Free-Flow Speed, mi/h		+

Directional Demand Flow Rate, veh/h Peak Hour Factor Segment Capacity, veh/h	0.88		Total Trucks, %	d Flow Rate, veh/h	
	1	0.88			3.28
	1700		Demand/Capacity (D/C)		0.10
ntermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29345		PF Power Coefficie	ent (p)	0.75792
n Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.7
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
Tangent	1013	-		-	68.5
Vehicle Results					
Average Speed, mi/h	68.5		Percent Followers	, %	28.0
Segment Travel Time, minutes	0.17		Follower Density (	FD), followers/mi/ln	0.7
/ehicle LOS	А	A			
Bicycle Results					
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	164		Bicycle Effective V	Vidth, ft	29
Bicycle LOS Score	1.40		Bicycle Effective Speed Factor		5.07
Bicycle LOS	А				
		Segm	ent 10		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		4569
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity			1		
Directional Demand Flow Rate, veh/h	164		Opposing Deman	d Flow Rate, veh/h	289
Peak Hour Factor	0.88		Total Trucks, %		3.28
Segment Capacity, veh/h	1700		Demand/Capacity (D/C)		0.10
ntermediate Results			<u> </u>		
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.39263		Speed Power Coe		0.51760
PF Slope Coefficient (m)	-1.17332		PF Power Coefficie	·	0.83118
n Passing Lane Effective Length?	No		Total Segment De		0.5
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data	<u>'</u>				
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h

1 Tangent	4569	-		-	68.9
Vehicle Results					
Average Speed, mi/h	68.9		Percent Followers,	%	22.9
Segment Travel Time, minutes	0.75		Follower Density (	FD), followers/mi/ln	0.5
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0	Pa		on Rating	4
Flow Rate Outside Lane, veh/h	164		Bicycle Effective W	/idth, ft	29
Bicycle LOS Score	1.40		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	А				
	Se	egm	ent 11		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		5676
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity	•				
Directional Demand Flow Rate, veh/h	164	164		d Flow Rate, veh/h	280
Peak Hour Factor	0.88	0.88			2.82
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.40080		Speed Power Coefficient (p)		0.51956
PF Slope Coefficient (m)	-1.16417		PF Power Coefficient (p)		0.83135
In Passing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.5
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	5676	-		-	68.9
Vehicle Results					
Average Speed, mi/h	68.9		Percent Followers,	%	22.8
Segment Travel Time, minutes	0.94		Follower Density (FD), followers/mi/ln		0.5
Vehicle LOS A					
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	164		Bicycle Effective W	/idth, ft	29
Bicycle LOS Score	1.28		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	А				
	Se	egm	ent 12		

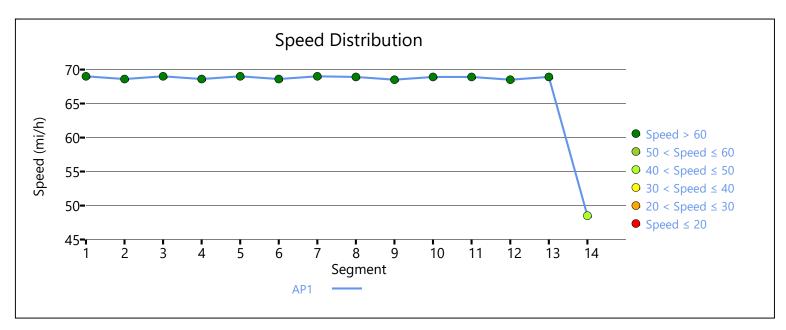
Ve	chicle Inputs					
Seg	gment Type	Passing Constrained		Length, ft		657
Measured FFS Measured			Free-Flow Speed,	mi/h	70.0	
De	emand and Capacity					
Dir	ectional Demand Flow Rate, veh/h	164		Opposing Deman	d Flow Rate, veh/h	-
Pea	ak Hour Factor	0.88		Total Trucks, %		2.82
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
In	termediate Results					
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	eed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF	Slope Coefficient (m)	-1.29350		PF Power Coefficie	ent (p)	0.75785
In I	Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.7
%lr	mprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Su	ıbsegment Data					
#	Segment Type	Length, ft Radi		dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	657 -			-	68.5
Ve	hicle Results					
Ave	erage Speed, mi/h	68.5		Percent Followers,	. %	28.0
Seg	gment Travel Time, minutes	0.11		Follower Density (	FD), followers/mi/ln	0.7
Vel	nicle LOS	A				
Bi	cycle Results	·				
Per	cent Occupied Parking	0		Pavement Condition	on Rating	4
Flo	w Rate Outside Lane, veh/h	164		Bicycle Effective Width, ft		29
Bic	ycle LOS Score	1.28		Bicycle Effective Speed Factor		5.07
Bic	ycle LOS	А				
		S	egm	nent 13		
Ve	hicle Inputs					
Seg	gment Type	Passing Zone		Length, ft		6009
Me	easured FFS	Measured		Free-Flow Speed, mi/h		70.0
De	emand and Capacity					
Dir	ectional Demand Flow Rate, veh/h	164		Opposing Deman	d Flow Rate, veh/h	280
Pea	ak Hour Factor	0.88		Total Trucks, %		2.82
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
In	termediate Results					
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	eed Slope Coefficient (m)	4.40389		Speed Power Coe	fficient (p)	0.51956
PF	Slope Coefficient (m)	-1.16281		PF Power Coefficie	ent (p)	0.83065

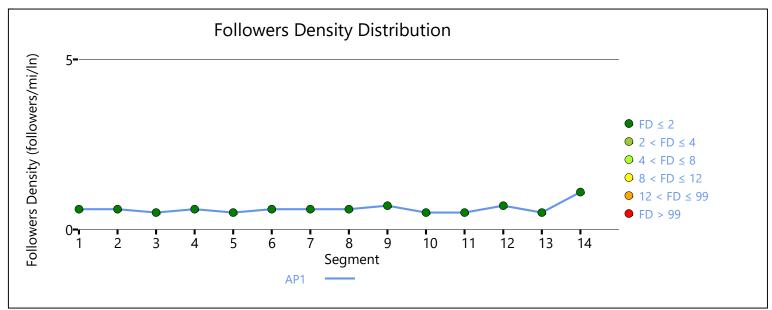
In Passing Lane Effective Length?		No	No		Total Segment Density, veh/mi/ln	
%Improvement to Percent Followers		0.0		%Improvement	to Speed	0.0
Suk	osegment Data					
#	Segment Type	Length, ft	Length, ft Radi		Superelevation, %	Average Speed, mi/h
1	Tangent	6009	-		-	68.9
Vel	nicle Results					
Aver	age Speed, mi/h	68.9		Percent Follower	rs, %	22.8
Segr	ment Travel Time, minutes	0.99		Follower Density	(FD), followers/mi/ln	0.5
Vehi	cle LOS	А		İ		
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Condi	Pavement Condition Rating	
Flow	Rate Outside Lane, veh/h	164		Bicycle Effective	Width, ft	29
Bicy	cle LOS Score	1.28		Bicycle Effective	Speed Factor	5.07
Bicy	cle LOS	A				
		•	Segr	nent 14		
Vel	nicle Inputs					
Segr	ment Type	Passing Constrair	ned	Length, ft		891
Mea	sured FFS	Measured		Free-Flow Speed	l, mi/h	50.0
Dei	mand and Capacity					
Dire	ctional Demand Flow Rate, veh/h	164	164		nd Flow Rate, veh/h	-
Peak	Hour Factor	0.88		Total Trucks, %		2.82
Segr	ment Capacity, veh/h	1700		Demand/Capaci	ty (D/C)	0.10
Inte	ermediate Results					
Segr	ment Vertical Class	1	1		I, mi/h	50.0
Spee	ed Slope Coefficient (m)	4.57372	4.57372		efficient (p)	0.41674
PF S	lope Coefficient (m)	-1.47375		PF Power Coefficient (p)		0.71164
In Pa	ssing Lane Effective Length?	No		Total Segment D	ensity, veh/mi/ln	1.1
%lm	provement to Percent Followers	0.0		%Improvement	to Speed	0.0
Suk	segment Data					
#	Segment Type	Length, ft	R	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	891	-		-	48.5
Veł	nicle Results					
Aver	age Speed, mi/h	48.5		Percent Follower	rs, %	33.4
Segr	ment Travel Time, minutes	0.21		Follower Density	(FD), followers/mi/ln	1.1
Vehi	cle LOS	A				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Condi	tion Rating	4
	· · ·					

Т	VMT veh-mi/p	VHD veh-h/p	Follower Density, followers/	LOS		
Facility	Facility Results					
Bicycle LC	icycle LOS A					
Bicycle LC	OS Score	1.06	Bicycle Effective Speed Factor	4.42		
Flow Rate	Flow Rate Outside Lane, veh/h 164		Bicycle Effective Width, ft	29		

0.6

0.05





216

Α

	HCS Two-L	ane	Highway Re	port	
Project Information					
Analyst	MJV	MJV			5/11/2023
Agency	HRG	HRG			2050 NB
Jurisdiction	SDDOT		Time Analyzed		AM Peak
Project Description	WB 38 West of Hartf	ford	Units		U.S. Customary
		Segn	nent 1		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		10549
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity			<u> </u>		
Directional Demand Flow Rate, veh/h	165		Opposing Deman	d Flow Rate, veh/h	244
Peak Hour Factor	0.88		Total Trucks, %		12.50
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Intermediate Results			'		
Segment Vertical Class	1	1		mi/h	70.0
Speed Slope Coefficient (m)	4.42827	4.42827		fficient (p)	0.52768
PF Slope Coefficient (m)	-1.16689		PF Power Coefficie	ent (p)	0.80729
In Passing Lane Effective Length? No			Total Segment De	nsity, veh/mi/ln	0.6
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data			,		
# Segment Type	Length, ft	Rac	dius, ft Superelevation, %		Average Speed, mi/h
1 Tangent	10549	-		-	69.0
Vehicle Results					<u>'</u>
Average Speed, mi/h	69.0		Percent Followers	. %	23.8
Segment Travel Time, minutes	1.74		Follower Density (FD), followers/mi/ln		0.6
Vehicle LOS	А		7. 7. 2. 2. 7		
Bicycle Results			1		
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	165		Bicycle Effective V		29
Bicycle LOS Score	4.94		Bicycle Effective S		5.07
Bicycle LOS	E				
		Segn	nent 2		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		2793
Segment Type	1	Passing Zone			

Demand and Capacity					
Directional Demand Flow Rate, veh/h	165		Opposing Deman	d Flow Rate, veh/h	244
Peak Hour Factor	0.88		Opposing Demand Flow Rate, veh/h Total Trucks, %		12.50
Segment Capacity, veh/h	1700		Demand/Capacity	, (D/C)	0.10
	1700		Demand/Capacity	(,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	0.10
Intermediate Results					_
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.35767		Speed Power Coe	fficient (p)	0.52768
PF Slope Coefficient (m)	-1.19319		PF Power Coefficie	ent (p)	0.82737
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Rad	lius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	2793	-		-	69.0
Vehicle Results					
Average Speed, mi/h	69.0		Percent Followers, %		23.5
Segment Travel Time, minutes	0.46		Follower Density (FD), followers/mi/ln		0.6
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow Rate Outside Lane, veh/h	165		Bicycle Effective V	Vidth, ft	29
Bicycle LOS Score	4.94		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	E				
	9	Segn	nent 3		
Vehicle Inputs					
Segment Type	Passing Zone		Length, ft		3825
Measured FFS	Measured		Free-Flow Speed, mi/h		70.0
Demand and Capacity	•				
Directional Demand Flow Rate, veh/h	165		Opposing Deman	d Flow Rate, veh/h	245
Peak Hour Factor	0.88		Total Trucks, %		2.40
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.37079		Speed Power Coe	fficient (p)	0.52741
PF Slope Coefficient (m)	-1.17529		PF Power Coefficie	ent (p)	0.83222
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
Jabbegineiit Data					

#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	3825	-		-	69.0
Veł	nicle Results	-				
Aver	rage Speed, mi/h	69.0	69.0		, %	23.1
Segr	ment Travel Time, minutes	0.63		Follower Density (	(FD), followers/mi/ln	0.6
Vehi	cle LOS	A				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow	Rate Outside Lane, veh/h	165		Bicycle Effective V	Vidth, ft	29
Bicy	cle LOS Score	1.17		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	А				
			Segr	ment 4		
Veł	nicle Inputs					
Segr	ment Type	Passing Constrai	ned	Length, ft		791
Mea	sured FFS	Measured	-		mi/h	70.0
Dei	mand and Capacity					
Dire	ctional Demand Flow Rate, veh/h	165		Opposing Deman	d Flow Rate, veh/h	-
Peak	Hour Factor	0.88		Total Trucks, %		2.40
Segment Capacity, veh/h		1700		Demand/Capacity	' (D/C)	0.10
Int	ermediate Results					
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	ed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF S	lope Coefficient (m)	-1.29355		PF Power Coefficient (p)		0.75779
In Pa	assing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.7
%lm	provement to Percent Followers	0.0	0.0		Speed	0.0
Suk	osegment Data					
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	791	-		-	68.5
Vel	nicle Results					
Aver	rage Speed, mi/h	68.5		Percent Followers	, %	28.1
Segr	ment Travel Time, minutes	0.13		Follower Density (	(FD), followers/mi/ln	0.7
Vehi	cle LOS	A				
Bic	ycle Results					
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4
Flow	Rate Outside Lane, veh/h	165		Bicycle Effective V	Vidth, ft	29
Bicy	cle LOS Score	1.17		Bicycle Effective S	peed Factor	5.07
Bicy	cle LOS	Α				

		9	Segi	ment 5		
Ve	hicle Inputs					
Seg	ment Type	Passing Zone		Length, ft		3414
Mea	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity					
Dire	ectional Demand Flow Rate, veh/h	165		Opposing Demand	d Flow Rate, veh/h	245
Pea	k Hour Factor	0.88		Total Trucks, %		2.40
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Int	termediate Results					
Seg	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	ed Slope Coefficient (m)	4.36595		Speed Power Coef	fficient (p)	0.52741
PF S	Slope Coefficient (m)	-1.18179		PF Power Coefficie	ent (p)	0.83026
In P	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%In	nprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Su	bsegment Data					
#	Segment Type	Length, ft	Ra	ndius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	3414	-		-	69.0
Ve	hicle Results					<u>'</u>
Ave	rage Speed, mi/h	69.0		Percent Followers,	. %	23.2
Seg	ment Travel Time, minutes	0.56		Follower Density (	FD), followers/mi/ln	0.6
Veh	icle LOS	А				
Bio	cycle Results					·
Per	cent Occupied Parking	0		Pavement Condition Rating		4
Flov	w Rate Outside Lane, veh/h	165		Bicycle Effective W	/idth, ft	29
Bicy	/cle LOS Score	1.17		Bicycle Effective S	peed Factor	5.07
Bicy	rcle LOS	А				
		9	Segi	ment 6		
Ve	hicle Inputs					
Seg	ment Type	Passing Constrained		Length, ft		286
Mea	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	mand and Capacity					
Dire	ectional Demand Flow Rate, veh/h	165		Opposing Demand	d Flow Rate, veh/h	-
Pea	k Hour Factor	0.88		Total Trucks, %		2.40
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Int	termediate Results			,		
Sea	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
		I		1		

Speed Slope C	Coefficient (m)	4.57372	<i>4</i> 57372		fficient (p)	0.41674	
PF Slope Coef		-1.29355		PF Power Coefficie	·	0.75779	
•	ne Effective Length?			Total Segment Density, veh/mi/ln		0.7	
	nt to Percent Followers	0.0		%Improvement to		0.0	
Subsegme	ent Data			·			
# Segmen		Length, ft	Rad	lius, ft	Superelevation, %	Average Speed, mi/h	
1 Tangent		286 -			-	68.5	
Vehicle Re		1					
Average Speed	d. mi/h	68.5		Percent Followers,	. %	28.1	
- ,	el Time, minutes	0.05			FD), followers/mi/ln	0.7	
Vehicle LOS		A		· energy (	. 2),		
Bicycle Re	esults	1					
Percent Occup		0		Pavement Condition	on Rating	4	
<u> </u>	tside Lane, veh/h	165		Bicycle Effective Width, ft		29	
Bicycle LOS Sc		1.17		Bicycle Effective Speed Factor		5.07	
Bicycle LOS		A		bicycle Effective Speed Factor			
Segment					ent 7		
Vehicle In	nute						
	•					1.00	
Segment Type		Passing Constrained		Length, ft Free-Flow Speed, mi/h		463	
Measured FFS	5	Measured		Free-Flow Speed,	mı/h	70.0	
Demand a	and Capacity						
Directional De	emand Flow Rate, veh/h	169		Opposing Demand Flow Rate, veh/h		-	
Peak Hour Fac	ctor	0.88		Total Trucks, %		2.60	
Segment Capa	acity, veh/h	1700		Demand/Capacity (D/C)		0.10	
Intermedi	ate Results						
Segment Verti	ical Class	1		Free-Flow Speed, mi/h		70.0	
Speed Slope C	Coefficient (m)	4.57372		Speed Power Coefficient (p)		0.41674	
PF Slope Coef	fficient (m)	-1.29353		PF Power Coefficient (p)		0.75782	
In Passing Lan	ne Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.7	
%Improvemer	nt to Percent Followers	0.0		%Improvement to	Speed	0.0	
Subsegme	ent Data						
	nt Type	Length, ft	Rad	lius, ft	Superelevation, %	Average Speed, mi/h	
# Segmen			-		-	68.5	
-		1.00					
1 Tangent		1.00					
1 Tangent Vehicle Re	esults	68.5		Percent Followers,	, %	28.6	
1 Tangent Vehicle Re Average Speed	esults				, % FD), followers/mi/ln	28.6	

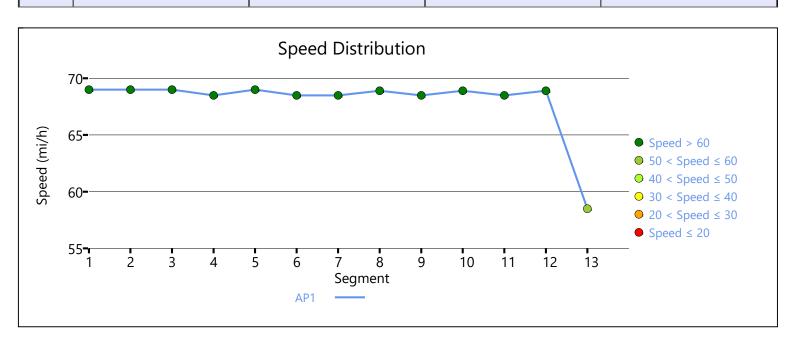
ement Condition Rating rcle Effective Width, ft rcle Effective Speed Factor	4822 70.0 2, veh/h 243 2.60 0.10 70.0 0.52796 0.83451
gth, ft e-Flow Speed, mi/h  cosing Demand Flow Rate al Trucks, % nand/Capacity (D/C) e-Flow Speed, mi/h ed Power Coefficient (p) cover Coefficient (p) al Segment Density, veh/m	4822 70.0 2, veh/h 243 2.60 0.10 70.0 0.52796 0.83451 ni/ln 0.6
gth, ft 2-Flow Speed, mi/h 2-Flow Speed, mi/h 2-Flow Speed, mi/h 2-Flow Speed, mi/h 2-Flow Speed, mi/h 3-Flow  4822 70.0 2, veh/h 243 2.60 0.10 70.0 0.52796 0.83451 ni/ln 0.6	
gth, ft e-Flow Speed, mi/h posing Demand Flow Rate al Trucks, % nand/Capacity (D/C) e-Flow Speed, mi/h ed Power Coefficient (p) Power Coefficient (p)	70.0  243 2.60 0.10  70.0  0.52796 0.83451  ni/ln 0.6
gth, ft e-Flow Speed, mi/h posing Demand Flow Rate al Trucks, % nand/Capacity (D/C) e-Flow Speed, mi/h ed Power Coefficient (p) Power Coefficient (p)	70.0  243 2.60 0.10  70.0  0.52796 0.83451  ni/ln 0.6
e-Flow Speed, mi/h  posing Demand Flow Rate al Trucks, %  nand/Capacity (D/C)  e-Flow Speed, mi/h  ed Power Coefficient (p)  Power Coefficient (p) al Segment Density, veh/m	70.0  243 2.60 0.10  70.0  0.52796 0.83451  ni/ln 0.6
e-Flow Speed, mi/h  posing Demand Flow Rate al Trucks, %  nand/Capacity (D/C)  e-Flow Speed, mi/h  ed Power Coefficient (p)  Power Coefficient (p) al Segment Density, veh/m	70.0  243 2.60 0.10  70.0  0.52796 0.83451  ni/ln 0.6
posing Demand Flow Rate al Trucks, % nand/Capacity (D/C) e-Flow Speed, mi/h ed Power Coefficient (p) Power Coefficient (p)	2, veh/h 243 2.60 0.10 70.0 0.52796 0.83451 ni/ln 0.6
e-Flow Speed, mi/h ed Power Coefficient (p) Power Coefficient (p) al Segment Density, veh/m	2.60 0.10 70.0 0.52796 0.83451 ni/ln 0.6
e-Flow Speed, mi/h ed Power Coefficient (p) Power Coefficient (p) al Segment Density, veh/m	2.60 0.10 70.0 0.52796 0.83451 ni/ln 0.6
e-Flow Speed, mi/h ed Power Coefficient (p) Power Coefficient (p) al Segment Density, veh/m	0.10  70.0  0.52796  0.83451  ni/ln  0.6
e-Flow Speed, mi/h ed Power Coefficient (p) Power Coefficient (p) al Segment Density, veh/m	70.0 0.52796 0.83451 ni/ln 0.6
ed Power Coefficient (p) Power Coefficient (p) al Segment Density, veh/m	0.52796 0.83451 ni/ln 0.6
ed Power Coefficient (p) Power Coefficient (p) al Segment Density, veh/m	0.52796 0.83451 ni/ln 0.6
Power Coefficient (p) al Segment Density, veh/m	0.83451 ni/ln 0.6
al Segment Density, veh/m	ni/ln 0.6
nprovement to Speed	0.0
L	
	·
t Supereleva	ation, % Average Speed, mi/h
-	68.9
	·
cent Followers, %	23.2
ower Density (FD), followe	ers/mi/ln 0.6
	·
ement Condition Rating	4
cle Effective Width, ft	29
cle Effective Speed Factor	r 5.07
t 9	
ath ft	861
gtii, it	70.0
	ement Condition Rating cle Effective Width, ft

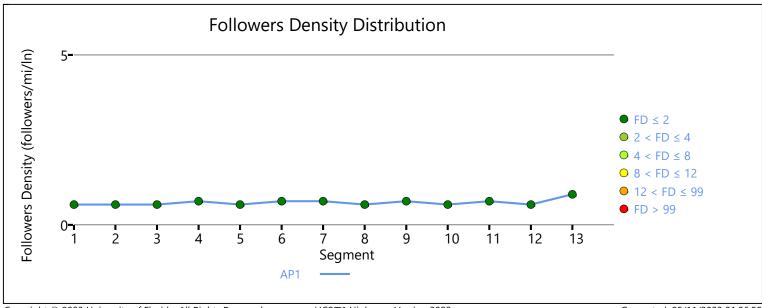
Dire	ctional Demand Flow Rate, veh/h	169		Opposing Demand	d Flow Rate, veh/h	-
	Hour Factor	0.88		Total Trucks, %	2.12, 1.0.1, 1.	2.60
Segr	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
	ermediate Results				<u> </u>	
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
	ed Slope Coefficient (m)	4.57372		Speed Power Coef		0.41674
-	lope Coefficient (m)	-1.29353		PF Power Coefficie		0.75782
	assing Lane Effective Length?	No		Total Segment De	<u> </u>	0.7
%lm	provement to Percent Followers	0.0		%Improvement to	Speed	0.0
Suk	osegment Data			<u>'</u>		
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	861	-		-	68.5
Vel	nicle Results					·
Aver	rage Speed, mi/h	68.5		Percent Followers,	%	28.6
Segr	ment Travel Time, minutes	0.14		Follower Density (FD), followers/mi/ln		0.7
Vehi	cle LOS	А				
Bic	ycle Results	•		<u>'</u>		
Perc	ent Occupied Parking	0		Pavement Condition	on Rating	4
Flow	Rate Outside Lane, veh/h	169		Bicycle Effective W	/idth, ft	29
Bicy	cle LOS Score	1.23		Bicycle Effective S <sub>I</sub>	peed Factor	5.07
Bicy	cle LOS	А				
		·	Segr	ment 10		
Veł	nicle Inputs					
Segr	ment Type	Passing Zone		Length, ft		1556
Mea	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Dei	mand and Capacity					
Dire	ctional Demand Flow Rate, veh/h	169		Opposing Demand	d Flow Rate, veh/h	243
Peak	Hour Factor	0.88		Total Trucks, %		2.60
Segr	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Inte	ermediate Results					
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spee	ed Slope Coefficient (m)	4.33831		Speed Power Coef	ficient (p)	0.52796
PF S	lope Coefficient (m)	-1.23554		PF Power Coefficie	ent (p)	0.80871
In Pa	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%lm	provement to Percent Followers	0.0		%Improvement to	Speed	0.0
Sul	osegment Data					
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h

1 Tangent	1556	-		-	68.9
Vehicle Results					
Average Speed, mi/h	68.9		Percent Followers,	, %	25.5
Segment Travel Time, minutes	0.26		Follower Density (	FD), followers/mi/ln	0.6
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	169		Bicycle Effective W	Vidth, ft	29
Bicycle LOS Score	1.23		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	А				
	Se	egm	nent 11		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		799
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	169		Opposing Deman	d Flow Rate, veh/h	-
Peak Hour Factor	0.88	0.88			2.60
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coefficient (p)		0.41674
PF Slope Coefficient (m)	-1.29353		PF Power Coefficient (p)		0.75782
In Passing Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.7
%Improvement to Percent Followers	0.0		%Improvement to Speed		0.0
Subsegment Data					
# Segment Type	Length, ft	Rad	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	799	-		-	68.5
Vehicle Results					
Average Speed, mi/h	68.5		Percent Followers, %		28.6
Segment Travel Time, minutes	0.13		Follower Density (	FD), followers/mi/ln	0.7
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	169		Bicycle Effective W	Vidth, ft	29
Bicycle LOS Score	1.23		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	Α				
	Se	egm	nent 12		

Ve	ehicle Inputs					
Seg	gment Type	Passing Zone		Length, ft		857
Me	Measured FFS Measured		Free-Flow Speed,	mi/h	70.0	
De	emand and Capacity					
Dir	rectional Demand Flow Rate, veh/h	169		Opposing Demand	d Flow Rate, veh/h	243
Pea	ak Hour Factor	0.88		Total Trucks, %		2.60
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
ln <sup>.</sup>	termediate Results					
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Spe	eed Slope Coefficient (m)	4.33390		Speed Power Coef	fficient (p)	0.52796
PF	Slope Coefficient (m)	-1.24754		PF Power Coefficie	ent (p)	0.80350
In F	Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%Ir	mprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Su	ıbsegment Data					
#	Segment Type	Length, ft	Length, ft Rad		Superelevation, %	Average Speed, mi/h
1	Tangent	857	-		-	68.9
Ve	ehicle Results					
Ave	erage Speed, mi/h	68.9		Percent Followers,	%	25.9
Segment Travel Time, minutes		0.14		Follower Density (	FD), followers/mi/ln	0.6
Vehicle LOS		A				
Bi	cycle Results					
Per	cent Occupied Parking	0	0		on Rating	4
Flo	w Rate Outside Lane, veh/h	169		Bicycle Effective W	/idth, ft	29
Bic	ycle LOS Score	1.23		Bicycle Effective S	Bicycle Effective Speed Factor	
Bic	ycle LOS	А				
		9	Segm	nent 13		
Ve	ehicle Inputs					
Seg	gment Type	Passing Constrained	d	Length, ft		1288
Me	easured FFS	Measured		Free-Flow Speed,	mi/h	60.0
De	emand and Capacity					
Dir	ectional Demand Flow Rate, veh/h	169		Opposing Demand	d Flow Rate, veh/h	-
Pea	ak Hour Factor	0.88		Total Trucks, %		2.60
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.10
ln <sup>.</sup>	termediate Results					
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	60.0
Spe	eed Slope Coefficient (m)	4.57372		Speed Power Coef	fficient (p)	0.41674
PF	Slope Coefficient (m)	-1.39677		PF Power Coefficie	ent (p)	0.73640

In Passing Lane Effective Length?		No	No To		ensity, veh/mi/ln	0.9
%lmp	provement to Percent Followers	0.0		%Improvement to	o Speed	0.0
Sub	segment Data					
#	Segment Type	Length, ft	Length, ft Radiu		Superelevation, %	Average Speed, mi/h
1	Tangent	1288	-	-		58.5
Veh	icle Results					
Avera	age Speed, mi/h	58.5	58.5		5, %	31.5
Segment Travel Time, minutes		0.25	0.25		(FD), followers/mi/ln	0.9
Vehicle LOS A		А	A			
Bicy	cle Results					
Perce	ent Occupied Parking	0	0		ion Rating	4
Flow	Rate Outside Lane, veh/h	169		Bicycle Effective \	Width, ft	29
Bicyc	le LOS Score	1.14		Bicycle Effective Speed Factor		4.79
Bicyc	le LOS	А				
Faci	ility Results					
Т	VMT veh-mi/p	VHI veh-l		Follower D	ensity, followers/ mi/In	LOS
1	224	0.0	0.05		0.6	A





Copyright © 2023 University of Florida. All Rights Reserved.

HCSTM Highways Version 2022 WB\_38\_WHartford\_2050AM.xuf Generated: 05/11/2023 21:06:58

	HCS Two-L	.ane	Highway Re	port		
Project Information						
Analyst	MJV		Date		5/11/2023	
Agency	HRG		Analysis Year		2050 NB	
Jurisdiction	SDDOT		Time Analyzed		PM Peak	
Project Description	WB 38 West of Hart	ford	Units		U.S. Customary	
		Segn	nent 1			
Vehicle Inputs						
Segment Type	Passing Zone		Length, ft		10549	
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0	
Demand and Capacity			<u>'</u>			
Directional Demand Flow Rate, veh/h	280		Opposing Deman	d Flow Rate, veh/h	164	
Peak Hour Factor	0.88		Total Trucks, %		1.94	
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.16	
Intermediate Results			'			
Segment Vertical Class 1			Free-Flow Speed,	mi/h	70.0	
Speed Slope Coefficient (m)	4.39885		Speed Power Coe	fficient (p)	0.55020	
PF Slope Coefficient (m)	-1.15143			ent (p)	0.81244	
In Passing Lane Effective Length?	No			nsity, veh/mi/ln	1.4	
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0	
Subsegment Data			,			
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h	
1 Tangent	10549	-		-	68.3	
Vehicle Results						
Average Speed, mi/h	68.3		Percent Followers	, %	33.6	
Segment Travel Time, minutes	1.76		Follower Density (	FD), followers/mi/ln	1.4	
Vehicle LOS	A					
Bicycle Results			1			
Percent Occupied Parking	0		Pavement Conditi	on Rating	4	
Flow Rate Outside Lane, veh/h	280		Bicycle Effective V		24	
Bicycle LOS Score	2.64		Bicycle Effective S		5.07	
Bicycle LOS	С					
		Segn	nent 2			
Vehicle Inputs						
Segment Type	Passing Zone		Length, ft		2793	
- ••	1 9		1 -			

Demand and Capacity						
	200		Onnacia a Davi	d Flour Data and the	164	
Directional Demand Flow Rate, veh/h	280		,, ,	d Flow Rate, veh/h	164	
Peak Hour Factor	0.88		Total Trucks, %	(D (C)	1.94	
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.16	
Intermediate Results						
Segment Vertical Class	1	1		mi/h	70.0	
Speed Slope Coefficient (m)	4.32824		Speed Power Coe	fficient (p)	0.55020	
PF Slope Coefficient (m)	-1.17723		PF Power Coefficie	ent (p)	0.83227	
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.4	
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0	
Subsegment Data						
# Segment Type	Length, ft	Radi	ius, ft	Superelevation, %	Average Speed, mi/h	
1 Tangent	2793	-		-	68.3	
Vehicle Results						
Average Speed, mi/h	68.3	68.3		, %	33.5	
Segment Travel Time, minutes	0.46	0.46		(FD), followers/mi/ln	1.4	
Vehicle LOS	А					
Bicycle Results		<u> </u>				
Percent Occupied Parking	0		Pavement Conditi	on Rating	4	
Flow Rate Outside Lane, veh/h	280		Bicycle Effective V	Vidth, ft	24	
Bicycle LOS Score	2.64		Bicycle Effective S	peed Factor	5.07	
Bicycle LOS	С					
	S	Segm	nent 3			
Vehicle Inputs						
Segment Type	Passing Zone		Length, ft		3825	
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0	
Demand and Capacity						
Directional Demand Flow Rate, veh/h	289		Opposing Deman	d Flow Rate, veh/h	164	
Peak Hour Factor	0.88		Total Trucks, %		2.19	
Segment Capacity, veh/h	1700		Demand/Capacity	' (D/C)	0.17	
Intermediate Results						
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0	
Speed Slope Coefficient (m)	4.34098		Speed Power Coe		0.55020	
PF Slope Coefficient (m)	-1.15833		PF Power Coefficie	ent (p)	0.83897	
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.4	
%Improvement to Percent Followers	0.0		%Improvement to	-	0.0	
Subsegment Data						
<b>J</b>						

#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h			
1	Tangent	3825	-		-	68.3			
Veł	nicle Results	•							
Aver	age Speed, mi/h	68.3		Percent Followers	, %	33.5			
Segr	ment Travel Time, minutes	0.64		Follower Density (	(FD), followers/mi/ln	1.4			
Vehi	cle LOS	A							
Bic	ycle Results								
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4			
Flow	Rate Outside Lane, veh/h	289		Bicycle Effective V	Vidth, ft	24			
Bicy	cle LOS Score	2.72		Bicycle Effective S	peed Factor	5.07			
Bicy	cle LOS	С							
			Segi	ment 4					
Vel	nicle Inputs								
Segr	ment Type	Passing Constrai	ned	Length, ft		791			
Mea	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0			
Dei	mand and Capacity								
Directional Demand Flow Rate, veh/h 289			Opposing Deman	d Flow Rate, veh/h	-				
Peak	Hour Factor	0.88		Total Trucks, %		2.19			
Segr	ment Capacity, veh/h	1700		Demand/Capacity	/ (D/C)	0.17			
Int	ermediate Results			·					
Segr	ment Vertical Class	1		Free-Flow Speed, mi/h 70.0					
Spee	ed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674			
PF S	lope Coefficient (m)	-1.29358		PF Power Coefficie	ent (p)	0.75776			
In Pa	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.7			
%lm	provement to Percent Followers	0.0		%Improvement to	Speed	0.0			
Suk	osegment Data								
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h			
1	Tangent	791	-		-	67.7			
Veł	nicle Results								
Aver	age Speed, mi/h	67.7		Percent Followers	, %	39.6			
Segr	ment Travel Time, minutes	0.13		Follower Density (	(FD), followers/mi/ln	1.7			
Vehi	cle LOS	A							
Bic	ycle Results								
Perc	ent Occupied Parking	0		Pavement Conditi	on Rating	4			
	Rate Outside Lane, veh/h	289		Bicycle Effective V		24			
Bicy	cle LOS Score	2.72		Bicycle Effective S		5.07			
Bicv	cle LOS	С							

		9	Segi	ment 5		
Ve	hicle Inputs					
Seg	ment Type	Passing Zone		Length, ft		3414
Me	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	emand and Capacity					
Dire	ectional Demand Flow Rate, veh/h	289		Opposing Deman	d Flow Rate, veh/h	164
Pea	k Hour Factor	0.88	0.88 T			2.19
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17
Int	termediate Results					
Segment Vertical Class 1				Free-Flow Speed,	mi/h	70.0
Spe	ed Slope Coefficient (m)	4.33614	4.33614		fficient (p)	0.55020
PF S	Slope Coefficient (m)	-1.16472		PF Power Coefficie	ent (p)	0.83695
In P	Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.4
%In	nprovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Su	bsegment Data					
#	Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	3414	1-		-	68.3
Ve	hicle Results					
Ave	erage Speed, mi/h	68.3		Percent Followers,	. %	33.7
Seg	ment Travel Time, minutes	0.57		Follower Density (	FD), followers/mi/ln	1.4
Veh	icle LOS	А				
Bio	cycle Results					
Per	cent Occupied Parking	0		Pavement Conditi	on Rating	4
Flov	w Rate Outside Lane, veh/h	289		Bicycle Effective W	/idth, ft	24
Bicy	/cle LOS Score	2.72		Bicycle Effective S	peed Factor	5.07
Bicy	/cle LOS	С				
		9	Segi	ment 6		
Ve	hicle Inputs					
	ment Type	Passing Constrained		Length, ft		286
Me	asured FFS	Measured		Free-Flow Speed,	mi/h	70.0
De	emand and Capacity					
Dire	ectional Demand Flow Rate, veh/h	289		Opposing Deman	d Flow Rate, veh/h	-
Pea	k Hour Factor	0.88		Total Trucks, %		2.19
Seg	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17
Int	termediate Results					
Sea	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
9	, <del></del>			1	•	

			1.		1			
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	·	0.41674			
PF Slope Coefficient (m)	-1.29358		PF Power Coefficie	<u> </u>	0.75776			
In Passing Lane Effective Length?	No		Total Segment De		1.7			
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0			
Subsegment Data								
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h			
1 Tangent	286	-		-	67.7			
Vehicle Results								
Average Speed, mi/h	67.7		Percent Followers,	, %	39.6			
Segment Travel Time, minutes	0.05		Follower Density (	FD), followers/mi/ln	1.7			
Vehicle LOS	A	А						
Bicycle Results								
Percent Occupied Parking	0		Pavement Conditi	on Rating	4			
Flow Rate Outside Lane, veh/h	289		Bicycle Effective V	Vidth, ft	24			
Bicycle LOS Score	2.72		Bicycle Effective S	peed Factor	5.07			
Bicycle LOS	С							
		Segn	nent 7					
Vehicle Inputs								
Segment Type	Passing Constraine	ed	Length, ft		463			
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0			
Demand and Capacity								
Directional Demand Flow Rate, veh/h	286		Opposing Deman	d Flow Rate, veh/h	-			
Peak Hour Factor	0.88		Total Trucks, %		3.08			
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17			
Intermediate Results								
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0			
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674			
PF Slope Coefficient (m)	-1.29347		PF Power Coefficie	ent (p)	0.75789			
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.7			
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0			
Subsegment Data								
# Segment Type	Length, ft	Rac	dius, ft	Superelevation, %	Average Speed, mi/h			
1 Tangent	463	-		-	67.7			
Vehicle Results								
Average Speed, mi/h	67.7		Percent Followers	, %	39.4			
Segment Travel Time, minutes	0.08			FD), followers/mi/ln	1.7			
	e LOS A				+			

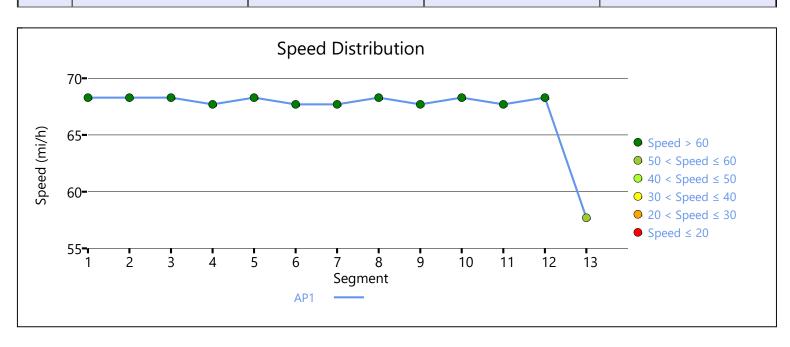
Bicycle Results						
Percent Occupied Parking	0		Pavement Condition	on Rating	4	
Flow Rate Outside Lane, veh/h	286		Bicycle Effective Width, ft		24	
Bicycle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07	
Bicycle LOS	С					
	S	egn	nent 8		<u> </u>	
Vehicle Inputs						
Segment Type	Passing Zone		Length, ft		4822	
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0	
Demand and Capacity						
Directional Demand Flow Rate, veh/h	286		Opposing Demand	d Flow Rate, veh/h	157	
Peak Hour Factor	0.88		Total Trucks, %		3.08	
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17	
Intermediate Results						
Segment Vertical Class	1		Free-Flow Speed, mi/h		70.0	
Speed Slope Coefficient (m)	4.34895	4.34895		fficient (p)	0.55243	
PF Slope Coefficient (m)	-1.14563		PF Power Coefficie	ent (p)	0.84199	
In Passing Lane Effective Length? No			Total Segment De	nsity, veh/mi/ln	1.4	
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0	
Subsegment Data						
# Segment Type	Length, ft	Rad	ius, ft Superelevation, %		Average Speed, mi/h	
1 Tangent	4822	1-		-	68.3	
Vehicle Results						
Average Speed, mi/h	68.3		Percent Followers,	. %	33.0	
Segment Travel Time, minutes	0.80		Follower Density (	FD), followers/mi/ln	1.4	
Vehicle LOS	А					
Bicycle Results						
Percent Occupied Parking	0		Pavement Condition	on Rating	4	
Flow Rate Outside Lane, veh/h	286		Bicycle Effective W	/idth, ft	24	
Bicycle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07	
Bicycle LOS	С					
	Sc	egn	nent 9			
Vehicle Inputs						
Segment Type	Passing Constrained		Length, ft		861	
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0	
Segment Type	Passing Constrained	egn	Length, ft	mi/h		

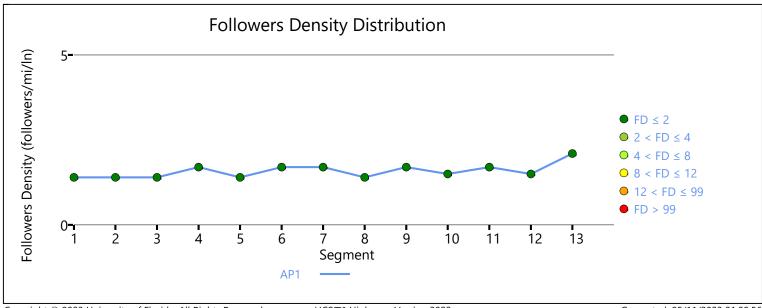
Dire	ctional Demand Flow Rate, veh/h	286		Opposing Demand	d Flow Rate, veh/h	-			
Peak	Hour Factor	0.88		Total Trucks, %		3.08			
Segr	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17			
Int	ermediate Results								
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0			
Spe	ed Slope Coefficient (m)	4.57372		Speed Power Coef	fficient (p)	0.41674			
PF S	lope Coefficient (m)	-1.29347		PF Power Coefficie	ent (p)	0.75789			
In Pa	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.7			
%lm	provement to Percent Followers	0.0		%Improvement to	Speed	0.0			
Sul	segment Data								
#	Segment Type	Length, ft	Ra	adius, ft	Superelevation, %	Average Speed, mi/h			
1	Tangent	861	-		-	67.7			
Vel	nicle Results								
Average Speed, mi/h 67.7				Percent Followers,	. %	39.4			
Segr	nent Travel Time, minutes	0.14	0.14 F		FD), followers/mi/ln	1.7			
Vehi	cle LOS	A							
Bic	ycle Results								
Perc	ent Occupied Parking	0		Pavement Condition	on Rating	4			
Flow	Rate Outside Lane, veh/h	286		Bicycle Effective W	/idth, ft	24			
Bicy	cle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07			
Bicy	cle LOS	С							
			Segr	ment 10					
Vel	nicle Inputs								
Segr	ment Type	Passing Zone		Length, ft		1556			
Mea	sured FFS	Measured		Free-Flow Speed,	mi/h	70.0			
De	mand and Capacity								
Dire	ctional Demand Flow Rate, veh/h	286		Opposing Demand	d Flow Rate, veh/h	157			
Peak	: Hour Factor	0.88		Total Trucks, %		3.08			
Segr	ment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17			
Int	ermediate Results								
Segr	ment Vertical Class	1		Free-Flow Speed,	mi/h	70.0			
Spe	ed Slope Coefficient (m)	4.30647		Speed Power Coef	fficient (p)	0.55243			
PF S	lope Coefficient (m)	-1.21611		PF Power Coefficie	ent (p)	0.81541			
In Pa	assing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.5			
%lm	provement to Percent Followers	0.0		%Improvement to	Speed	0.0			
Sul	osegment Data								
#	Segment Type	Length, ft	Rá	adius, ft	Superelevation, %	Average Speed, mi/h			

1 Tangent	1556	-		-	68.3
Vehicle Results					
Average Speed, mi/h	68.3		Percent Followers,	. %	35.5
Segment Travel Time, minutes	0.26		Follower Density (	FD), followers/mi/ln	1.5
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	286		Bicycle Effective W	/idth, ft	24
Bicycle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	С				
	S	Segn	nent 11		
Vehicle Inputs					
Segment Type	Passing Constrained		Length, ft		799
Measured FFS	Measured		Free-Flow Speed,	mi/h	70.0
Demand and Capacity					
Directional Demand Flow Rate, veh/h	286		Opposing Deman	d Flow Rate, veh/h	-
Peak Hour Factor	0.88	0.88			3.08
Segment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17
Intermediate Results					
Segment Vertical Class	1		Free-Flow Speed,	mi/h	70.0
Speed Slope Coefficient (m)	4.57372		Speed Power Coe	fficient (p)	0.41674
PF Slope Coefficient (m)	-1.29347		PF Power Coefficie	ent (p)	0.75789
In Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.7
%Improvement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subsegment Data					
# Segment Type	Length, ft	Ra	dius, ft	Superelevation, %	Average Speed, mi/h
1 Tangent	799	T-		-	67.7
Vehicle Results					
Average Speed, mi/h	67.7		Percent Followers,	. %	39.4
Segment Travel Time, minutes	0.13		Follower Density (	FD), followers/mi/ln	1.7
Vehicle LOS	А				
Bicycle Results					
Percent Occupied Parking	0		Pavement Condition	on Rating	4
Flow Rate Outside Lane, veh/h	286		Bicycle Effective W	/idth, ft	24
Bicycle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07
Bicycle LOS	С				
	S	Segn	nent 12		

Ve	chicle Inputs						
Seg	gment Type	Passing Zone		Length, ft		857	
Me	easured FFS	Measured		Free-Flow Speed,	mi/h	70.0	
De	emand and Capacity						
Dir	ectional Demand Flow Rate, veh/h	286		Opposing Demand	d Flow Rate, veh/h	157	
Pea	ak Hour Factor	0.88		Total Trucks, %		3.08	
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17	
In	termediate Results						
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	70.0	
Spe	Speed Slope Coefficient (m) 4.30206		Speed Power Coef	fficient (p)	0.55243		
PF	Slope Coefficient (m)	-1.22789		PF Power Coefficie	ent (p)	0.81007	
In I	Passing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	1.5	
%lr	mprovement to Percent Followers	0.0		%Improvement to	Speed	0.0	
Su	ıbsegment Data						
#	Segment Type	Length, ft	Length, ft Radiu		Superelevation, %	Average Speed, mi/h	
1	Tangent	857	-		-	68.3	
Ve	hicle Results		<u>'</u>			•	
Ave	erage Speed, mi/h	68.3		Percent Followers,	%	36.0	
Seg	gment Travel Time, minutes	0.14		Follower Density (	FD), followers/mi/ln	1.5	
Vel	nicle LOS	А					
Bi	cycle Results					·	
Per	cent Occupied Parking	0		Pavement Condition	on Rating	4	
Flo	w Rate Outside Lane, veh/h	286		Bicycle Effective W	/idth, ft	24	
Bic	ycle LOS Score	2.95		Bicycle Effective S	peed Factor	5.07	
Bic	ycle LOS	С					
			Segm	nent 13			
Ve	chicle Inputs						
Seg	gment Type	Passing Constrained	d	Length, ft		1288	
Me	easured FFS	Measured		Free-Flow Speed,	mi/h	60.0	
De	emand and Capacity						
Dir	ectional Demand Flow Rate, veh/h	286		Opposing Demand	d Flow Rate, veh/h	-	
Pea	ak Hour Factor	0.88		Total Trucks, %		3.08	
Seg	gment Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.17	
In	termediate Results						
Seg	gment Vertical Class	1		Free-Flow Speed,	mi/h	60.0	
Spe	eed Slope Coefficient (m)	4.57372		Speed Power Coef	fficient (p)	0.41674	
PF	Slope Coefficient (m)	-1.39671		PF Power Coefficie	ent (p)	0.73647	

In Pa	ssing Lane Effective Length?	No		Total Segment De	ensity, veh/mi/ln	2.1		
%lmp	provement to Percent Followers	0.0		%Improvement to	o Speed	0.0		
Sub	segment Data							
#	Segment Type	Length, ft	Radiu	us, ft Superelevation, %		Average Speed, mi/h		
1	Tangent	1288	-		-	57.7		
Veh	icle Results							
Average Speed, mi/h 57.7			Т	Percent Followers	5, %	42.7		
Segn	nent Travel Time, minutes	0.25		Follower Density	(FD), followers/mi/ln	2.1		
Vehic	ile LOS	В						
Bicy	cle Results							
Perce	ent Occupied Parking	0		Pavement Condit	ion Rating	4		
Flow	Rate Outside Lane, veh/h	286		Bicycle Effective V	Width, ft	24		
Bicyc	le LOS Score	2.86		Bicycle Effective S	Speed Factor	4.79		
Bicyc	le LOS	С						
Faci	ility Results							
Т	VMT veh-mi/p	VHI veh-l		Follower D	ensity, followers/ mi/ln	LOS		
1	382	0.1	5		1.5	А		



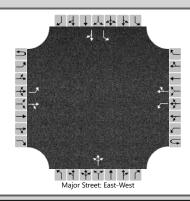


Copyright © 2023 University of Florida. All Rights Reserved.

HCSTM Highways Version 2022 WB\_38\_WHartford\_2050PM.xuf

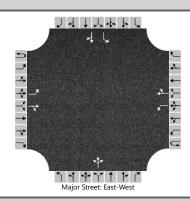
Generated: 05/11/2023 21:09:56

	HCS Two-Way Stop-Control Report										
General Information		Site Information									
Analyst	NM	Intersection	SD 38 & SD 19								
Agency/Co.	HRG	Jurisdiction	SDDOT								
Date Performed	4/30/2024	East/West Street	SD 38								
Analysis Year	2050	North/South Street	SD 19								
Time Analyzed	AM Peak	Peak Hour Factor	0.92								
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25								
Project Description	SD 38										



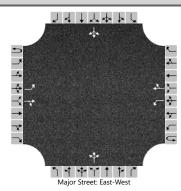
Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		1	1	0
Configuration	1	L		TR		L		TR			LTR			L		TR
Volume (veh/h)	1	55	165	0		0	120	50		10	5	10		70	0	95
Percent Heavy Vehicles (%)	1	30				3				3	3	3		9	3	11
Proportion Time Blocked	1															
Percent Grade (%)	1									(	0		0			
Right Turn Channelized	1															
Median Type   Storage	1	Undivided														
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	1	4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.40				4.13				7.13	6.53	6.23		7.19	6.53	6.31
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.47				2.23				3.53	4.03	3.33		3.58	4.03	3.40
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	<u> </u>	60				0					27			76		103
Capacity, c (veh/h)	1	1238				1390					524			461		865
v/c Ratio		0.05				0.00					0.05			0.16		0.12
95% Queue Length, Q <sub>95</sub> (veh)	1	0.2				0.0					0.2			0.6		0.4
Control Delay (s/veh)	1	8.1	0.2	0.2		7.6	0.0	0.0			12.2			14.3		9.7
Level of Service (LOS)	1	А	А	А		А	А	А			В			В		А
Approach Delay (s/veh)		2	.2			0	.0			12	2.2			1	1.7	
Approach LOS	1		Ą			,	4				В				В	

	HCS Two-Way Stop	p-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & SD 19							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	SD 19							
Time Analyzed	PM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38									



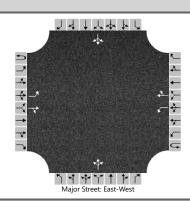
Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			Westl	ound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		1	1	0
Configuration		L		TR		L		TR			LTR			L		TR
Volume (veh/h)		85	115	0		0	170	80		10	5	10		40	0	50
Percent Heavy Vehicles (%)		2				3				3	3	3		10	3	14
Proportion Time Blocked																
Percent Grade (%)	1									(	)			(	)	
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.12				4.13				7.13	6.53	6.23		7.20	6.53	6.34
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.22				2.23				3.53	4.03	3.33		3.59	4.03	3.43
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1	92				0					27			43		54
Capacity, c (veh/h)		1291				1455					498			395		782
v/c Ratio		0.07				0.00					0.05			0.11		0.07
95% Queue Length, Q <sub>95</sub> (veh)		0.2				0.0					0.2			0.4		0.2
Control Delay (s/veh)		8.0	0.2	0.2		7.5	0.0	0.0			12.6			15.2		9.9
Level of Service (LOS)		А	А	А		А	А	А			В			С		А
Approach Delay (s/veh)		3.5				0.0				12.6				12.3		
Approach LOS		,	4			,	A				В			1	3	

	HCS Two-Way Stop	pp-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & 459th							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	459th Ave							
Time Analyzed	AM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38									



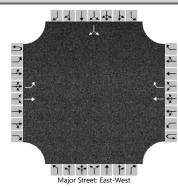
Vehicle Volumes and Ad	ustme	nts														
Approach		Eastk	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0
Configuration		L		TR		L		TR			LTR				LTR	
Volume (veh/h)	1	0	215	7		2	155	0		15	0	7		9	0	0
Percent Heavy Vehicles (%)		3				3				13	0	0		0	0	0
Proportion Time Blocked																
Percent Grade (%)										. (	)			(	0	
Right Turn Channelized	1															
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	1	4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.23	6.50	6.20		7.10	6.50	6.20
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.62	4.00	3.30		3.50	4.00	3.30
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)		0				2					24				10	
Capacity, c (veh/h)		1403				1319					596				546	
v/c Ratio		0.00				0.00					0.04				0.02	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.1				0.1	
Control Delay (s/veh)		7.6	0.0	0.0		7.7	0.0	0.0			11.3				11.7	
Level of Service (LOS)		А	А	А		А	А	А			В				В	
Approach Delay (s/veh)	0.0				0.1				11.3				11.7			
Approach LOS			A			,	4		В				В			

	HCS Two-Way Stop	p-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & 459th							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	459th Ave							
Time Analyzed	PM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38									



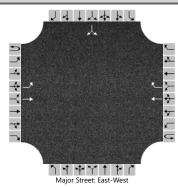
Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0
Configuration		L		TR		L		TR			LTR				LTR	
Volume (veh/h)		0	145	9		15	245	2		15	0	4		2	2	0
Percent Heavy Vehicles (%)		0				0				13	0	0		0	100	0
Proportion Time Blocked																
Percent Grade (%)										(	0		0			
Right Turn Channelized																
Median Type   Storage				Undi	vided											
<b>Critical and Follow-up He</b>	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.10				4.10				7.23	6.50	6.20		7.10	7.50	6.20
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.20				3.62	4.00	3.30		3.50	4.90	3.30
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)		0				16					21				4	
Capacity, c (veh/h)		1307				1423					534				427	
v/c Ratio		0.00				0.01					0.04				0.01	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.1				0.0	
Control Delay (s/veh)		7.8	0.0	0.0		7.6	0.1	0.1			12.0				13.5	
Level of Service (LOS)		А	А	А		А	А	А			В				В	
Approach Delay (s/veh)		0.0 0.5								12	2.0		13.5			
Approach LOS		,	4			,	4			ı	В		В			

	HCS Two-Way Stop	pp-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & I-90 Speedway							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	I-90 Expressway							
Time Analyzed	AM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38									



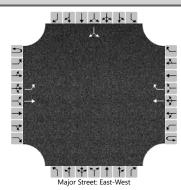
		iviajoi street. Last-west																
Vehicle Volumes and Adj	ustme	nts																
Approach		Eastb	ound			Westl	oound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		0	1	0		
Configuration		L	Т				Т	R							LR			
Volume (veh/h)		0	230				165	0						0		0		
Percent Heavy Vehicles (%)		3												3		3		
Proportion Time Blocked																		
Percent Grade (%)														(	0			
Right Turn Channelized						Ν	lo											
Median Type   Storage				Undi	vided													
Critical and Follow-up Ho	eadwa	ys																
Base Critical Headway (sec)		4.1												7.1		6.2		
Critical Headway (sec)		4.13												6.43		6.23		
Base Follow-Up Headway (sec)		2.2												3.5		3.3		
Follow-Up Headway (sec)		2.23												3.53		3.33		
Delay, Queue Length, and	d Leve	l of Se	ervice	)														
Flow Rate, v (veh/h)	]	0													0			
Capacity, c (veh/h)		1390													0			
v/c Ratio		0.00																
95% Queue Length, Q <sub>95</sub> (veh)		0.0			Ì													
Control Delay (s/veh)		7.6	0.0															
Level of Service (LOS)		А	А		Ì													
Approach Delay (s/veh)	0.0																	
Approach LOS		А																

	HCS Two-Way Stop	p-Control Report									
General Information		Site Information									
Analyst	NM	Intersection	SD 38 & I-90 Speedway								
Agency/Co.	HRG	Jurisdiction	SDDOT								
Date Performed	4/30/2024	East/West Street	SD 38								
Analysis Year	2050	North/South Street	I-90 Expressway								
Time Analyzed	AM Peak - Event Traffic	Peak Hour Factor	0.92								
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25								
Project Description	SD 38										



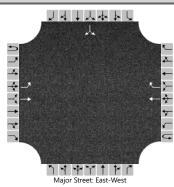
Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		0	1	0	
Configuration		L	Т				Т	R							LR		
Volume (veh/h)		0	412				295	0						0		0	
Percent Heavy Vehicles (%)		3												3		3	
Proportion Time Blocked																	
Percent Grade (%)														(	0		
Right Turn Channelized						Ν	lo										
Median Type   Storage	1			Undi	vided												
<b>Critical and Follow-up He</b>	eadwa	ys															
Base Critical Headway (sec)	1	4.1												7.1		6.2	
Critical Headway (sec)		4.13												6.43		6.23	
Base Follow-Up Headway (sec)		2.2												3.5		3.3	
Follow-Up Headway (sec)		2.23												3.53		3.33	
Delay, Queue Length, and	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)	1	0													0		
Capacity, c (veh/h)	1	1234													0		
v/c Ratio	1	0.00															
95% Queue Length, Q <sub>95</sub> (veh)		0.0															
Control Delay (s/veh)	1	7.9	0.0														
Level of Service (LOS)		А	А														
Approach Delay (s/veh)	0.0																
Approach LOS		А															

	HCS Two-Way Stop	p-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & I-90 Speedway							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	I-90 Expressway							
Time Analyzed	PM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38									



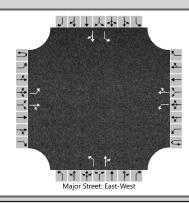
Vehicle Volumes and Ad	iuctmo	ntc																
	Justine		ound			\\/oc+l	bound			North	bound			Court	bound			
Approach				<b>—</b>		vvesti		<b>.</b>		NOITH								
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R		
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	] 0	1	1	0	0	0	1	1		0	0	0		0	1	0		
Configuration		L	Т				Т	R							LR			
Volume (veh/h)		0	165				260	0						0		0		
Percent Heavy Vehicles (%)	]	3												3		3		
Proportion Time Blocked	]																	
Percent Grade (%)														(	0			
Right Turn Channelized						Ν	10											
Median Type   Storage	<u> </u>			Undi	vided													
Critical and Follow-up H	eadwa	ys																
Base Critical Headway (sec)	1	4.1												7.1		6.2		
Critical Headway (sec)		4.13												6.43		6.23		
Base Follow-Up Headway (sec)	1	2.2												3.5		3.3		
Follow-Up Headway (sec)		2.23												3.53		3.33		
Delay, Queue Length, an	d Leve	l of S	ervice															
Flow Rate, v (veh/h)	<u> </u>	0													0			
Capacity, c (veh/h)	1	1274													0			
v/c Ratio		0.00																
95% Queue Length, Q <sub>95</sub> (veh)	1	0.0																
Control Delay (s/veh)		7.8	0.0															
Level of Service (LOS)		А	А															
Approach Delay (s/veh)		0.0																
Approach LOS			A															

	HCS Two-Way Stop-Control Report													
General Information		Site Information												
Analyst	NM	Intersection	SD 38 & I-90 Speedway											
Agency/Co.	HRG	Jurisdiction	SDDOT											
Date Performed	4/30/2024	East/West Street	SD 38											
Analysis Year	2050	North/South Street	I-90 Expressway											
Time Analyzed	PM Peak - Event Traffic	Peak Hour Factor	0.92											
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25											
Project Description	SD 38													



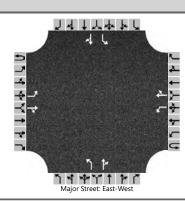
					iviajo	JI JULEU. La	3t-vvest										
Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound			Westl	bound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		0	1	0	
Configuration		L	Т				Т	R							LR		
Volume (veh/h)		0	295				465	0						0		0	
Percent Heavy Vehicles (%)		3												3		3	
Proportion Time Blocked																	
Percent Grade (%)														(	0		
Right Turn Channelized		No															
Median Type   Storage	1	Undivided															
Critical and Follow-up Ho	eadwa	ys															
Base Critical Headway (sec)	1	4.1												7.1		6.2	
Critical Headway (sec)		4.13												6.43		6.23	
Base Follow-Up Headway (sec)		2.2												3.5		3.3	
Follow-Up Headway (sec)		2.23												3.53		3.33	
Delay, Queue Length, and	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)	1	0													0		
Capacity, c (veh/h)	1	1054													0		
v/c Ratio	1	0.00															
95% Queue Length, Q <sub>95</sub> (veh)		0.0															
Control Delay (s/veh)	1	8.4	0.0														
Level of Service (LOS)		А	А														
Approach Delay (s/veh)		0.0															
Approach LOS	А																

	HCS Two-Way Stop-Control Report												
General Information		Site Information											
Analyst	NM	Intersection	SD 38 & 463rd Ave / Western Ave										
Agency/Co.	HRG	Jurisdiction	SDDOT										
Date Performed	4/29/2024	East/West Street	SD 38										
Analysis Year	2050	North/South Street	463rd Ave / Western Ave										
Time Analyzed	AM Peak	Peak Hour Factor	0.92										
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25										
Project Description	SD 38												



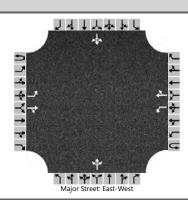
Vehicle Volumes and Adju	ıstme	nts																
Approach		Eastb	ound			Westl	oound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	1	0	0	1	1	0		1	1	0		1	1	0		
Configuration		L		TR		L		TR		L		TR		L		TR		
Volume (veh/h)		9	180	80		60	110	30		65	75	90		40	80	5		
Percent Heavy Vehicles (%)		3				3				14	2	6		0	7	33		
Proportion Time Blocked																		
Percent Grade (%)									(	)			(	0				
Right Turn Channelized																		
Median Type   Storage				Undi	vided													
<b>Critical and Follow-up He</b>	r-up Headways																	
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2		
Critical Headway (sec)		4.13				4.13				7.24	6.52	6.26		7.10	6.57	6.53		
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3		
Follow-Up Headway (sec)		2.23				2.23				3.63	4.02	3.35		3.50	4.06	3.60		
Delay, Queue Length, and	l Leve	l of Se	ervice	1														
Flow Rate, v (veh/h)	1	10				65				71		179		43		92		
Capacity, c (veh/h)		1422				1274				307		566		274		414		
v/c Ratio		0.01				0.05				0.23		0.32		0.16		0.22		
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.2				0.9		1.4		0.6		0.8		
Control Delay (s/veh)		7.5	0.0	0.0		8.0	0.2	0.2		20.2		14.3		20.6		16.2		
Level of Service (LOS)		А	А	А		А	А	А		С		В		С		С		
Approach Delay (s/veh)	0.3			2.5			16.0				17.6							
Approach LOS		1	Α			,	4			(	2		С					

	HCS Two-Way Stop	-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & 463rd Ave / Western Ave							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	5/8/2023	East/West Street	SD 38							
Analysis Year	2050	North/South Street	463rd Ave / Western Ave							
Time Analyzed	PM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38									



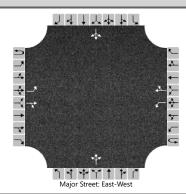
<b>Vehicle Volumes and Adj</b>	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		1	1	0		1	1	0
Configuration		L		TR		L		TR		L		TR		L		TR
Volume (veh/h)		15	125	55		120	200	60		70	85	155		55	100	25
Percent Heavy Vehicles (%)		22				3				0	11	4		0	4	0
Proportion Time Blocked																
Percent Grade (%)										(	)			(	0	
Right Turn Channelized																
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.32				4.13				7.10	6.61	6.24		7.10	6.54	6.20
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.40				2.23				3.50	4.10	3.34		3.50	4.04	3.30
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	Τ	16				130				76		261		60		136
Capacity, c (veh/h)		1173				1371				183		518		148		349
v/c Ratio		0.01				0.10				0.42		0.50		0.41		0.39
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.3				1.9		2.8		1.8		1.8
Control Delay (s/veh)		8.1	0.1	0.1		7.9	0.3	0.3		38.1		18.8		45.1		21.8
Level of Service (LOS)	ĺ	А	Α	Α		А	Α	А		E		С		Е		С
Approach Delay (s/veh)		0	.7			2	.7	•		23	3.1		28.9			
Approach LOS		,	A			,	4			(	2			[	)	

	HCS Two-Way Stop	-Control Report							
General Information		Site Information							
Analyst	NM	Intersection	SD 38 & Main Ave						
Agency/Co.	HRG	Jurisdiction	SDDOT						
Date Performed	5/8/2023	East/West Street	SD 38						
Analysis Year	2050	North/South Street	Main Ave (9th St)						
Time Analyzed	AM Peak	Peak Hour Factor	0.92						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description	SD 38								



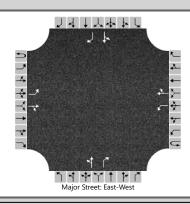
Vehicle Volumes and Adj	ustme	nts															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0	
Configuration		L		TR		L		TR			LTR				LTR		
Volume (veh/h)		2	260	30		40	195	20		40	5	85		6	10	4	
Percent Heavy Vehicles (%)		0				11				5	0	2		0	17	0	
Proportion Time Blocked																	
Percent Grade (%)										(	)		0				
Right Turn Channelized																	
Median Type   Storage		Left Only									!	9					
Critical and Follow-up He	eadwa	dways															
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2	
Critical Headway (sec)		4.10				4.21				7.15	6.50	6.22		7.10	6.67	6.20	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.20				2.30				3.55	4.00	3.32		3.50	4.15	3.30	
Delay, Queue Length, and	d Leve	l of Se	ervice														
Flow Rate, v (veh/h)		2				43					141				22		
Capacity, c (veh/h)		1346				1196					678				459		
v/c Ratio		0.00				0.04					0.21				0.05		
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.1					0.8				0.1		
Control Delay (s/veh)		7.7				8.1					11.7				13.2		
Level of Service (LOS)		А				Α					В				В		
Approach Delay (s/veh)		0	.1		1.3			11.7				13.2					
Approach LOS		,	4			-	4			ı	3		В				

	HCS Two-Way Stop-Control Report												
General Information		Site Information											
Analyst	NM	Intersection	SD 38 & Main Ave										
Agency/Co.	HRG	Jurisdiction	SDDOT										
Date Performed	4/30/2024	East/West Street	SD 38										
Analysis Year	2050	North/South Street	Main Ave (9th St)										
Time Analyzed	PM Peak	Peak Hour Factor	0.92										
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25										
Project Description	SD 38												



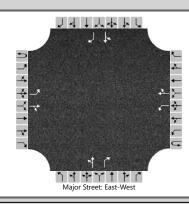
Vehicle Volumes and Ad	iustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0
Configuration		L		TR		L		TR			LTR				LTR	
Volume (veh/h)		10	250	45		65	335	60		35	20	55		40	30	7
Percent Heavy Vehicles (%)		0				0				5	0	0		0	0	0
Proportion Time Blocked																
Percent Grade (%)											)				0	
Right Turn Channelized																
Median Type   Storage		Left Only 9														
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	_	4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.10				4.10				7.15	6.50	6.20		7.10	6.50	6.20
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.20				3.55	4.00	3.30		3.50	4.00	3.30
Delay, Queue Length, an	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	<u> </u>	11				71					120				84	
Capacity, c (veh/h)		1141				1251					467				368	
v/c Ratio		0.01				0.06					0.26				0.23	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.2					1.0				0.9	
Control Delay (s/veh)		8.2				8.1					15.3				17.6	
Level of Service (LOS)		А		Ì		А					С				С	
Approach Delay (s/veh)		0	.3			1	.1			1!	5.3		17.6			
Approach LOS	1		A			,	4			(	C		С			

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & Vandemark Ave
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	Vandemark Avenue
Time Analyzed	АМ	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastbound Westbound U L T R U L T R								North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		0	1	1		0	1	1
Configuration		L		TR		L		TR		LT		R		LT		R
Volume (veh/h)		25	370	10		8	240	25		9	5	10		40	2	25
Percent Heavy Vehicles (%)		0				0				40	0	0		0	0	7
Proportion Time Blocked																
Percent Grade (%)										(	0				0	
Right Turn Channelized										Ν	lo			Ν	lo	
Median Type   Storage		Undivided														
Critical and Follow-up He	adwa	dways														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.10				4.10				7.50	6.50	6.20		7.10	6.50	6.27
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.20				3.86	4.00	3.30		3.50	4.00	3.36
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1	27				9				15		11		46		27
Capacity, c (veh/h)		1286				1157				278		648		306		752
v/c Ratio		0.02				0.01				0.05		0.02		0.15		0.04
95% Queue Length, Q <sub>95</sub> (veh)	1	0.1				0.0				0.2		0.1		0.5		0.1
Control Delay (s/veh)		7.9	0.1	0.1		8.1	0.1	0.1		18.7		10.7		18.8		10.0
Level of Service (LOS)		А	А	А		А	А	А		С		В		С		А
Approach Delay (s/veh)	0.6 0.3								15	5.4		15.5				
Approach LOS		A A							(	С		С				

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & Vandemark Ave
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	Vandemark Avenue
Time Analyzed	PM	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		

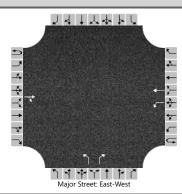


Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastbound Westbound								North	bound			South	bound	
Movement	U	L	T	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		0	1	1		0	1	1
Configuration		L		TR		L		TR		LT		R		LT		R
Volume (veh/h)		20	255	4		5	475	45		0	0	9		30	0	25
Percent Heavy Vehicles (%)		0				0				0	0	100		0	0	7
Proportion Time Blocked																
Percent Grade (%)										(	)				0	
Right Turn Channelized										Ν	lo			Ν	lo	
Median Type   Storage		Undivided														
<b>Critical and Follow-up He</b>	adwa	dways														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.10				4.10				7.10	6.50	7.20		7.10	6.50	6.27
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.20				3.50	4.00	4.20		3.50	4.00	3.36
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)		22				5				0		10		33		27
Capacity, c (veh/h)		1017				1293				0		574		259		532
v/c Ratio		0.02				0.00						0.02		0.13		0.05
95% Queue Length, Q <sub>95</sub> (veh)		0.1				0.0						0.1		0.4		0.2
Control Delay (s/veh)		8.6	0.2	0.2		7.8	0.0	0.0				11.4		20.9		12.1
Level of Service (LOS)		А	Α	А		Α	Α	А	В					С		В
Approach Delay (s/veh)	0.8 0.1							16.9								
Approach LOS		A A						С								

		HCS	Sigr	nalized	d Int	ersec	tion R	esul	ts Sui	mmary	/				
															No.
General Inform	nation	v						$\rightarrow$		ction Inf	W.				Ja lu
Agency		HRG							Duratio	n, h	0.250	)			
Analyst		NM		Analys	is Dat	e May	8, 2023		Area Ty	ре	Other	r	±, →		. 2
Jurisdiction		SDDOT		Time F	Period	AM F	Peak		PHF		0.92		4 4	W <del>↑</del> E 8	<b>~</b>
Urban Street		SD 38		Analys	is Yea	ır 2050			Analysi	s Period	1> 7:	15	7		*
Intersection		SD 38 & 2nd Street		File Na	ame	(7) S	D38&2n	d_AM.	xus					7 1	
Project Descrip	tion													ነ 4 ሰቀጥ	ተሰ
Demand Inform	nation				EB			WI	3		NB			SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	Т	R
Demand ( v ), v				20	325	10	95	20	0 15	5	20	155	35	50	25
,,															
Signal Informa	tion									$\neg \neg$					<b>T</b>
Cycle, s	45.0	Reference Phase	6		Ħ.	54	2					_	Ә∴	2	xtz
Offset, s	0	Reference Point	End	Green	27 9	8.1	0.0	0.0	0.0	0.0		1	X 2	3	4
Uncoordinated	No	Simult. Gap E/W	On	Yellow		3.5	0.0	0.0					<b>→</b>		<b>K</b> D
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.0				5	6	7	<b>Y</b> 8
Timer Results				EBL	<u> </u>	EBT	WB	1	WBT	NB	1	NBT	SB	1	SBT
Assigned Phase					-	2	1	-	6	140	_	8	OB	_	4
	ase Number					8.0	-		6.0	+		6.0	-		8.0
	hase Duration, s					32.4	_		32.4	-		12.6	-	_	12.6
	Phase Duration, s Change Period, ( Y+R c ), s					4.5	-		4.5	+		4.5	-	_	4.5
Max Allow Head		<u>,                                      </u>		_	-	0.0	-	-	0.0	-	_	3.3	-	-	3.3
Queue Clearan						0.0	-		0.0	+		7.8	-	_	7.5
		, = ,				0.0	-			+		0.5	-	-	0.5
Green Extensio		( <i>g e</i> ), s				0.0			0.0	+		0.98	+	_	0.98
Phase Call Prol				-	-		-			-		0.98		_	
Max Out Proba	DIIILY				_			_				0.01			0.00
Movement Gro	up Res	sults			EB			WB		т	NB		П	SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т	R	L	T	R
Assigned Move	ment			5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow F	Rate ( v	), veh/h			386		103	234		5	190			120	
Adjusted Satura	ation Flo	ow Rate ( s ), veh/h/l	n		1743		1018	1750		1317	1528			1105	
Queue Service	Time (	g s ), S			0.0		2.5	2.6		0.2	5.2			0.2	
Cycle Queue C	learanc	e Time ( <i>g c</i> ), s			4.8	1	7.3	2.6		5.8	5.2			5.5	
Green Ratio ( g		, - ,			0.62		0.62	0.62		0.18	0.18			0.18	
Capacity ( c ), v					1162		681	1082		236	278			306	
Volume-to-Capa		itio (X)			0.332	_	0.152	_	_	0.023				0.391	
		/In (95 th percentile	)												
		eh/ln ( 95 th percenti			1.9		0.8	1.1		0.1	2.9			1.7	1
		RQ) (95 th percent	,		0.00		0.08	0.00		0.02	0.00			0.00	
Uniform Delay (		, ,	,		4.2		6.0	3.8		20.1	17.2			16.3	1
					0.8		0.5	0.5		0.0	1.1			0.3	
	Incremental Delay ( d 2 ), s/veh Initial Queue Delay ( d 3 ), s/veh						0.0	0.0		0.0	0.0			0.0	
Control Delay (			0.0 5.0		6.4	4.2		20.1	18.3			16.6			
Level of Service			A		A	Α		C	В			В			
		5.0		A	4.9		A	18.		В	16.		В		
Approach Delay, s/veh / LOS Intersection Delay, s/veh / LOS				0.0			3.8			1 . 3.			A		_
	intersection belay, s/ven / LOS												A		
Multimodal Re	sults				EB			WB			NB			SB	
		/ LOS		1.84	-	В	1.62	-	В	1.9		В	1.6		В
	destrian LOS Score / LOS ycle LOS Score / LOS					A	1.04		A	0.8		A	0.6		A

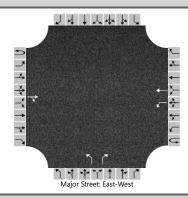
		HCS	Sigr	nalize	d Inte	ersect	ion R	esult	ts Sun	nmary	/				
General Inform	nation							T	Intersec	tion Inf	ormatic	nn		l al alaba t	Ja k
Agency	lation	HRG							Duration		0.250			*	
Analyst		NM		Analys	eie Date	e May 8	2 2023		Area Typ		Other		_* _\$		t. A
Jurisdiction		SDDOT		Time F		PM P		_	PHF		0.92		→ 	w. F	<b>-</b> ≥}-
Urban Street		SD 38			sis Yea		ean		Analysis	Poriod	1> 7:	15	-4		· •
Intersection		SD 38 & 2nd Street		File Na			038&2n			renou	1- 7.	13			<u></u>
Project Descript	tion	3D 36 & Zhu Sileet		File IN	ame	(1) 31	JOOKZIII	u_FIVI.	xu5				-	1 4 4 4	\$* (*
1 Toject Descript	lion														
Demand Inforn	nation				EB			WE	3	Т	NB		$\top$	SB	
Approach Move	ment			L	Т	R	L	Т	R	L	T	R	L	Т	R
Demand ( v ), v	eh/h			25	235	9	130	490	) 25	15	25	65	15	30	20
Signal Informa	tion				1.								_		$\perp$
Cycle, s	60.0	Reference Phase	2		Ħ	T 54	2					1	↔ ,	3	x1x
Offset, s	0	Reference Point	End	Green	44.3	6.7	0.0	0.0	0.0	0.0			K		
Uncoordinated	No	Simult. Gap E/W	On	Yellow		3.5	0.0	0.0	0.0	0.0			<b>₹</b>		₩.
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	0.0	0.0	0.0	0.0		5	6	7	8
				EBI			11						11		
Timer Results						EBT	WB	L	WBT	NB	L	NBT	SB	L	SBT
Assigned Phase	-					2		_	6			8			4
	Case Number					8.0		_	6.0			6.0			8.0
	Phase Duration, s					48.8			48.8			11.2			11.2
Change Period,						4.5			4.5			4.5			4.5
Max Allow Head	dway( <i>N</i>	<i>MAH</i> ), s				0.0			0.0			3.2			3.2
Queue Clearan	ce Time	e ( g s ), s										6.2			5.6
Green Extensio	n Time	( g e ), s				0.0			0.0			0.3			0.3
Phase Call Prob	bability											0.95			0.95
Max Out Probal	bility											0.00			0.00
		14						\A/D			ND			0.0	
Movement Gro		suits			EB	T 5		WB	T 5		NB		<b>.</b>	SB	
Approach Move				<u> </u>	T	R	L	T	R	L	T	R	L	T	R
Assigned Move		\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \		5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow F		,·		_	292		141	560	-	16	98		-	71	
		ow Rate ( s ), veh/h/l	n	_	1663		1114	1757	+	1350	1568	-	-	1377	
Queue Service		- ,		_	0.0	-	2.7	7.3	+	0.7	3.5	_	-	0.0	
Cycle Queue Cl		e Time ( <i>g ε</i> ), s		_	3.1	-	5.9	7.3	+	4.2	3.5	-	-	3.6	
Green Ratio ( g.				_	0.74		0.74	0.74	_	0.11	0.11		-	0.11	
Capacity ( c ), v				-	1294	_	885	1298	_	190	175		-	227	
Volume-to-Capa			\		0.226		0.160	0.431		0.086	0.560			0.311	
		/In 95 th percentile		_	10	-	0.0	0.5	+	0.4	0.0		-	1.0	
	• •	eh/ln (95 th percenti	,	-	1.0		0.8	2.5	-	0.4	2.3			1.6	
		RQ) (95 th percent	.iie)		0.00		0.08	0.00	-	0.07	0.00			0.00	
Uniform Delay (					2.5		3.4	3.0		27.3	25.3			24.7	
	Incremental Delay ( d 2 ), s/veh				0.4		0.4	1.0	-	0.1	1.0			0.3	
	nitial Queue Delay ( d 3 ), s/veh				0.0		0.0	0.0		0.0	0.0			0.0	
	Control Delay ( d ), s/veh				2.9		3.8	4.1		27.4	26.3			25.0	
	Level of Service (LOS)				A	^	A 4.0	Α	^	C	С		05.	C	
	Approach Delay, s/veh / LOS					Α	4.0		A	26.5	0	С	25.0	J	С
Intersection Del	ntersection Delay, s/veh / LOS					7	'.2						A		
Multimodal Ba	lultimodal Results				EB			WB			NB			SB	
Pedestrian LOS		/LOS		1.83		В	1.60		В	1.92		В	1.70		В
Bicycle LOS Sc				0.97		A	1.64		В	0.68		A	0.60		A
Dioyole LOG GC	JIO / LC	,,		0.97		/1	1.04			0.00		, <b>,</b> ,	0.00		7.

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & West Central HS Entrance
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	West Central HS Entrance
Time Analyzed	AM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



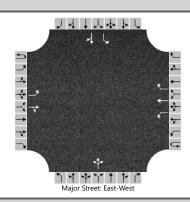
Vehicle Volumes and Adjustments																
	Journa															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	1	1	0		1	0	1		0	0	0
Configuration	1			TR		L	Т			L		R				
Volume (veh/h)	1		425	90		55	285			35		50				
Percent Heavy Vehicles (%)	1					0				0		0				
Proportion Time Blocked	1															
Percent Grade (%)	1									(	)					
Right Turn Channelized	1									Ν	lo					
Median Type   Storage	1	Left Only										9	9			
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)	1					4.1				7.1		6.2				
Critical Headway (sec)						4.10				6.40		6.20				
Base Follow-Up Headway (sec)						2.2				3.5		3.3				
Follow-Up Headway (sec)	1					2.20				3.50		3.30				
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1					60				38		54				
Capacity, c (veh/h)	1					1021				576		567				
v/c Ratio						0.06				0.07		0.10				
95% Queue Length, Q <sub>95</sub> (veh)	1					0.2				0.2		0.3				
Control Delay (s/veh)						8.7				11.7		12.0				
Level of Service (LOS)	1					А				В		В				
Approach Delay (s/veh)	1	1.4								11.9					-	-
Approach LOS	j	A A							В							

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & West Central HS Entrance
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	West Central HS Entrance
Time Analyzed	PM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



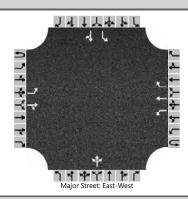
Vehicle Volumes and Adju	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	1	1	0		1	0	1		0	0	0
Configuration	1			TR		L	Т			L		R				
Volume (veh/h)	1		305	4		4	620			15		15				
Percent Heavy Vehicles (%)	1					0				0		0				
Proportion Time Blocked	1															
Percent Grade (%)	1									(	)					
Right Turn Channelized	1	1.60.1								Ν	lo					
Median Type   Storage	1	Left Only										!	9			
Critical and Follow-up He	eadwa	ys														
Base Critical Headway (sec)	1					4.1				7.1		6.2				
Critical Headway (sec)						4.10				6.40		6.20				
Base Follow-Up Headway (sec)	1					2.2				3.5		3.3				
Follow-Up Headway (sec)	1					2.20				3.50		3.30				
Delay, Queue Length, and	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1					4				16		16				
Capacity, c (veh/h)	1					1235				500		713				
v/c Ratio	1					0.00				0.03		0.02				
95% Queue Length, Q <sub>95</sub> (veh)						0.0				0.1		0.1				
Control Delay (s/veh)						7.9				12.4		10.2				
Level of Service (LOS)						А				В		В				
Approach Delay (s/veh)		0.1							11.3							
Approach LOS	1	0.1 A							ı	В						

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & Railroad Street
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/30/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	Railroad St
Time Analyzed	AM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



Vehicle Volumes and Adjustments       Approach     Eastbound     Westbound     Northbound     Southbound																
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	1		0	1	0		1	1	0
Configuration		L		TR		L	Т	R			LTR			L		TR
Volume (veh/h)	1	4	465	0		15	270	95		2	0	30		145	4	5
Percent Heavy Vehicles (%)	1	0				0				0	0	15		0	0	0
Proportion Time Blocked	1															
Percent Grade (%)	1									(	)			(	0	
Right Turn Channelized	1	No														
Median Type   Storage	1	Undivided														
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.10				4.10				7.10	6.50	6.35		7.10	6.50	6.20
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.20				3.50	4.00	3.44		3.50	4.00	3.30
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1	4				16					35			158		10
Capacity, c (veh/h)		1173				1070					505			258		448
v/c Ratio		0.00				0.02					0.07			0.61		0.02
95% Queue Length, Q <sub>95</sub> (veh)	1	0.0				0.0					0.2			3.6		0.1
Control Delay (s/veh)		8.1	0.0	0.0		8.4	0.1				12.6			38.6		13.2
Level of Service (LOS)	1	А	Α	А		А	А		В					Е		В
Approach Delay (s/veh)	0.1 0.4							12.6				37.1				
Approach LOS	1	A A					Α		В Е							

	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & Railroad Street
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	4/29/2024	East/West Street	SD 38
Analysis Year	2050	North/South Street	Railroad St
Time Analyzed	PM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



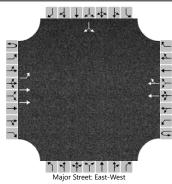
Vehicle Volumes and Adj	justme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	1		0	1	0		1	1	0
Configuration		L		TR		L	Т	R			LTR			L		TR
Volume (veh/h)		4	340	4		15	560	155		2	2	15		85	9	5
Percent Heavy Vehicles (%)		0				40				0	0	15		5	0	0
Proportion Time Blocked																
Percent Grade (%)										(	)			(	0	
Right Turn Channelized						Ν	lo									
Median Type   Storage				Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.10				4.50				7.10	6.50	6.35		7.15	6.50	6.20
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.56				3.50	4.00	3.44		3.55	4.00	3.30
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		4				16					21			92		15
Capacity, c (veh/h)		848				1004					417			197		287
v/c Ratio		0.01				0.02					0.05			0.47		0.05
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.0					0.2			2.3		0.2
Control Delay (s/veh)		9.3	0.0	0.0		8.6	0.1				14.1			38.5		18.2
Level of Service (LOS)		А	Α	Α		А	Α				В			Е		С
Approach Delay (s/veh)		0	.2			0	.3			14	1.1	•		35	5.7	
Approach LOS			4			1	4				В				E	

#### **HCS Signalized Intersection Results Summary** Intersection Information **General Information** HRG Duration, h 0.250 Agency Analyst NM Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & Mickelson Roa... File Name (10) SD38&Mickelson AM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 40 190 65 Demand (v), veh/h 135 445 35 195 45 55 215 20 195 **Signal Information** die Cycle, s 70.0 Reference Phase 2 542 Offset, s 0 Reference Point End 35.1 3.1 10.0 0.0 Green 2.9 2.9 Uncoordinated No Simult. Gap E/W On Yellow 4.0 4.0 4.0 0.0 0.0 4.0 Force Mode Fixed Simult. Gap N/S 0.0 On Red 0.0 0.0 0.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL SBT **Assigned Phase** 2 6 3 8 4 1 7 Case Number 6.3 1.0 3.0 1.1 4.0 1.1 4.0 Phase Duration, s 39.1 6.9 46.0 7.1 14.0 10.0 16.9 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Change Period, (Y+Rc), s Max Allow Headway ( MAH ), s 0.0 3.1 0.0 3.1 3.3 3.1 3.3 Queue Clearance Time ( $g_s$ ), s 2.8 3.7 7.3 8.0 12.3 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.0 0.0 0.5 0.0 0.6 Phase Call Probability 0.57 0.61 1.00 0.99 1.00 0.03 1.00 0.01 Max Out Probability 0.00 1.00 **Movement Group Results** EΒ **WB** NB SB Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 147 264 258 43 212 207 49 130 234 234 1170 1772 1726 1688 1687 1688 1615 1688 1523 Adjusted Saturation Flow Rate ( s ), veh/h/ln 1323 5.0 0.8 5.2 1.7 5.3 Queue Service Time ( $g_s$ ), s 6.1 6.1 1.9 6.0 10.3 Cycle Queue Clearance Time ( q c ), s 5.0 6.1 6.1 8.0 1.9 5.2 1.7 5.3 6.0 10.3 0.60 Green Ratio (g/C) 0.50 0.50 0.50 0.57 0.60 0.19 0.14 0.24 0.18 Capacity (c), veh/h 690 890 867 536 2024 794 187 231 333 282 Volume-to-Capacity Ratio (X) 0.213 0.296 0.298 0.081 0.105 0.260 0.262 0.565 0.702 0.830 Back of Queue (Q), ft/ln (95 th percentile) Back of Queue (Q), veh/ln (95 th percentile) 2.2 4.0 4.0 0.4 1.0 2.4 1.2 3.5 2.5 6.7 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 Uniform Delay ( d 1 ), s/veh 9.9 10.2 10.2 7.1 6.0 6.6 24.5 28.0 25.5 27.5 Incremental Delay ( d 2 ), s/veh 0.7 8.0 0.9 0.0 0.1 0.8 0.3 8.0 5.5 2.4 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 10.6 11.0 11.1 7.1 6.1 7.4 24.8 28.8 31.1 29.9 Level of Service (LOS) В В В Α Α Α С С С С 11.0 В 6.8 Α 27.7 С 30.5 С Approach Delay, s/veh / LOS Intersection Delay, s/veh / LOS 16.7 В **Multimodal Results** ΕB WB NB SB Pedestrian LOS Score / LOS 1.88 В 1.87 В 2.44 2.28 В В Bicycle LOS Score / LOS 1.04 Α 0.87 Α 0.78 Α 1.26 Α

#### **HCS Signalized Intersection Results Summary** Intersection Information **General Information** HRG Duration, h 0.250 Agency Analyst NM Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & Mickelson Roa... File Name (10) SD38&Mickelson PM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 220 535 10 Demand (v), veh/h 160 20 135 225 20 65 215 15 185 **Signal Information** Ji. Cycle, s 70.0 Reference Phase 2 Offset, s 0 Reference Point End 0.7 30.8 1.3 7.0 Green 5.0 Uncoordinated No Simult. Gap E/W On Yellow 3.5 0.0 4.0 3.5 3.5 4.0 Force Mode Fixed Simult. Gap N/S On Red 1.0 0.0 1.0 1.0 1.0 1.0 **Timer Results EBL EBT WBL** WBT NBL **NBT** SBL SBT **Assigned Phase** 5 2 6 3 8 4 1 7 Case Number 1.1 4.0 1.1 3.0 1.1 4.0 1.1 4.0 Phase Duration, s 10.2 36.5 9.5 35.8 6.2 12.0 12.0 17.8 4.5 5.0 4.5 5.0 4.5 5.0 4.5 5.0 Change Period, (Y+Rc), s Max Allow Headway ( MAH ), s 3.1 0.0 3.1 0.0 3.1 3.3 3.1 3.3 Queue Clearance Time ( $g_s$ ), s 5.8 5.3 2.8 5.1 9.5 11.6 Green Extension Time ( $g_e$ ), s 0.1 0.0 0.1 0.0 0.0 0.3 0.0 0.2 Phase Call Probability 0.97 0.94 0.34 1.00 0.99 1.00 1.00 0.63 1.00 1.00 Max Out Probability 1.00 1.00 **Movement Group Results** EΒ WB NB SB Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 174 131 130 147 582 245 22 82 234 217 1688 1772 1720 1688 1687 1323 1688 1730 1688 1519 Adjusted Saturation Flow Rate ( s ), veh/h/ln 3.8 3.1 3.1 3.3 8.2 8.0 7.5 9.6 Queue Service Time ( $g_s$ ), s 8.9 3.1 Cycle Queue Clearance Time ( q c ), s 3.8 3.1 3.1 3.3 8.2 8.9 8.0 3.1 7.5 9.6 Green Ratio (g/C) 0.52 0.45 0.45 0.51 0.44 0.44 0.12 0.10 0.24 0.18 Capacity (c), veh/h 510 798 774 645 1484 582 164 173 356 277 Volume-to-Capacity Ratio (X) 0.341 0.165 0.167 0.228 0.392 0.420 0.132 0.472 0.656 0.785 Back of Queue (Q), ft/ln (95 th percentile) Back of Queue (Q), veh/ln (95 th percentile) 2.2 2.1 2.1 1.9 5.2 4.8 0.6 2.3 6.3 7.1 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 Uniform Delay ( d 1 ), s/veh 9.5 11.4 11.4 9.3 13.3 13.5 27.4 29.8 24.6 27.3 Incremental Delay ( d 2 ), s/veh 0.1 0.4 0.5 0.1 8.0 2.2 0.1 0.7 3.4 8.9 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 9.7 11.9 11.9 9.4 14.0 15.7 27.6 30.5 28.0 36.2 Level of Service (LOS) Α В В Α В В С С С D 11.0 В 13.8 В 29.9 С 32.0 С Approach Delay, s/veh / LOS Intersection Delay, s/veh / LOS 18.2 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 1.89 В В 2.44 2.28 1.89 В В Bicycle LOS Score / LOS 0.85 Α 1.29 Α 0.66 Α 1.23

Generated: 10/31/2023 2:36:18 PM

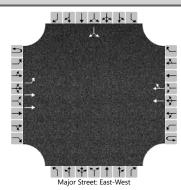
	HCS Two-Way Stop	p-Control Report									
General Information		Site Information									
Analyst	NM	Intersection	SD38 & 466th Ave								
Agency/Co.	HRG	Jurisdiction	SDDOT								
Date Performed	4/30/2024	East/West Street	SD 38								
Analysis Year	2050	North/South Street	466th Ave								
Time Analyzed	AM Peak	Peak Hour Factor	0.92								
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25								
Project Description	SD 38										



					Мај	Major Street: East-West										
Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	T	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		0	1	0
Configuration	1	L	Т				Т	TR							LR	
Volume (veh/h)	0	2	765				430	5						4		0
Percent Heavy Vehicles (%)	3	0												50		3
Proportion Time Blocked	1															
Percent Grade (%)	1														0	
Right Turn Channelized	1															
Median Type   Storage	1			Undi	vided											
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)	ī	4.1												7.5		6.9
Critical Headway (sec)		4.10												7.80		6.96
Base Follow-Up Headway (sec)		2.2												3.5		3.3
Follow-Up Headway (sec)		2.20												4.00		3.33
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	<del></del>	2													4	
Capacity, c (veh/h)	1	1100													206	
v/c Ratio	1	0.00													0.02	
95% Queue Length, Q <sub>95</sub> (veh)	1	0.0													0.1	
Control Delay (s/veh)	1	8.3	0.0												22.9	
Level of Service (LOS)	1	А	А		Ì		Ì	Ì					Ì	Ì	С	
Approach Delay (s/veh)	1	0.0											22.9			
Approach LOS	i	A											С			

Generated: 5/2/2024 2:18:14 PM

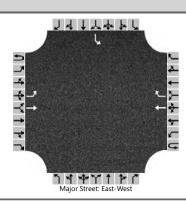
	HCS Two-Way Stop	p-Control Report									
General Information		Site Information									
Analyst	NM	Intersection	SD38 & 466th Ave								
Agency/Co.	HRG	Jurisdiction	SDDOT								
Date Performed	4/30/2024	East/West Street	SD 38								
Analysis Year	2050	North/South Street	466th Ave								
Time Analyzed	PM Peak	Peak Hour Factor	0.92								
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25								
Project Description	SD 38										



Vehicle Volumes and Adj	ustme	nts															
Approach		Eastk	ound			Westl	bound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		0	1	0	
Configuration		L	Т				Т	TR							LR		
Volume (veh/h)	0	0	445				910	2						5		2	
Percent Heavy Vehicles (%)	3	0												33		0	
Proportion Time Blocked																	
Percent Grade (%)														(	0		
Right Turn Channelized																	
Median Type   Storage				Undi	vided												
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)	<u> </u>	4.1												7.5		6.9	
Critical Headway (sec)	1	4.10												7.46		6.90	
Base Follow-Up Headway (sec)		2.2												3.5		3.3	
Follow-Up Headway (sec)		2.20												3.83		3.30	
Delay, Queue Length, an	d Leve	l of S	ervice														
Flow Rate, v (veh/h)	<u> </u>	0					Π				Π	П			8		
Capacity, c (veh/h)		705													167		
v/c Ratio	_	0.00													0.05		
95% Queue Length, Q <sub>95</sub> (veh)		0.0													0.1		
Control Delay (s/veh)		10.1	0.0												27.6		
Level of Service (LOS)		В	А												D		
Approach Delay (s/veh)	0.0												27.6				
Approach LOS		A												D			

Generated: 5/2/2024 2:17:11 PM

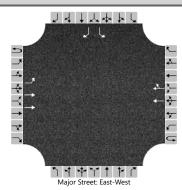
	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & I-90 WB Terminal
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	12/12/2023	East/West Street	SD 38
Analysis Year	2050	North/South Street	I-90 WB Terminal
Time Analyzed	AM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



Vehicle Volumes and Adj	justme	nts														
Approach		Eastb	ound			Westl	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		1	0	0
Configuration		L	Т				Т	R						L		
Volume (veh/h)		40	730				255	20						15		
Percent Heavy Vehicles (%)		0												56		
Proportion Time Blocked																
Percent Grade (%)															0	
Right Turn Channelized						١	10									
Median Type   Storage				Left	Only								9			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1												7.1		
Critical Headway (sec)		4.10												6.96		
Base Follow-Up Headway (sec)		2.2												3.5		
Follow-Up Headway (sec)		2.20												4.00		
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		43												16		
Capacity, c (veh/h)		1274												315		
v/c Ratio		0.03												0.05		
95% Queue Length, Q <sub>95</sub> (veh)		0.1												0.2		
Control Delay (s/veh)		7.9	0.2											17.1		
Level of Service (LOS)		А	А											С		
Approach Delay (s/veh)		0	.6										17.1			
Approach LOS		A C										С				

Generated: 12/12/2023 8:58:43 AM

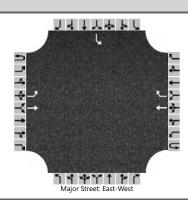
	HCS Two-Way Stop	p-Control Report									
General Information		Site Information									
Analyst	CEC	Intersection	SD 38 & I-90 WB Terminal								
Agency/Co.	HRG	Jurisdiction	SDDOT								
Date Performed	4/30/2024	East/West Street	SD 38								
Analysis Year	2050	North/South Street	I-90 WB Terminal								
Time Analyzed	AM Peak	Peak Hour Factor	0.92								
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25								
Project Description	SD 38										



Vehicle Volumes and Ad																	
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		1	0	1	
Configuration		L	Т				Т	TR						L		R	
Volume (veh/h)	0	40	730				255	20						15		190	
Percent Heavy Vehicles (%)	3	0												56		12	
Proportion Time Blocked																	
Percent Grade (%)														(	)		
Right Turn Channelized														N	lo		
Median Type   Storage				Undi	vided												
Critical and Follow-up H	eadwa	ys															
Base Critical Headway (sec)	<u> </u>	4.1												7.5		6.9	
Critical Headway (sec)		4.10												7.92		7.14	
Base Follow-Up Headway (sec)		2.2												3.5		3.3	
Follow-Up Headway (sec)		2.20												4.06		3.42	
Delay, Queue Length, an	d Leve	l of S	ervice														
Flow Rate, v (veh/h)	<u> </u>	43												16		207	
Capacity, c (veh/h)		1274												235		839	
v/c Ratio		0.03												0.07		0.25	
95% Queue Length, Q <sub>95</sub> (veh)		0.1												0.2		1.0	
Control Delay (s/veh)		7.9	0.2											21.5		10.7	
Level of Service (LOS)		А	А											С		В	
Approach Delay (s/veh)		0.6											11.5				
Approach LOS		A												В			

Generated: 5/1/2024 3:36:18 PM

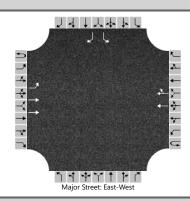
	HCS Two-Way Stop	-Control Report	
General Information		Site Information	
Analyst	NM	Intersection	SD 38 & I-90 WB Terminal
Agency/Co.	HRG	Jurisdiction	SDDOT
Date Performed	5/8/2023	East/West Street	SD 38
Analysis Year	2050	North/South Street	I-90 WB Terminal
Time Analyzed	PM Peak	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	SD 38		



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	0	1	1		0	0	0		1	0	0
Configuration		L	Т				Т	R						L		
Volume (veh/h)		25	420				415	35						30		
Percent Heavy Vehicles (%)		0												6		
Proportion Time Blocked																
Percent Grade (%)														(	0	
Right Turn Channelized						Ν	lo									
Median Type   Storage				Left	Only								9			
Critical and Follow-up H	eadwa	ys														
Base Critical Headway (sec)		4.1												7.1		
Critical Headway (sec)		4.10												6.46		
Base Follow-Up Headway (sec)		2.2												3.5		
Follow-Up Headway (sec)		2.20												3.55		
Delay, Queue Length, an	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)		27												33		
Capacity, c (veh/h)		1085												562		
v/c Ratio		0.03												0.06		
95% Queue Length, Q <sub>95</sub> (veh)		0.1												0.2		
Control Delay (s/veh)		8.4	0.2											11.8		
Level of Service (LOS)		А	А											В		
Approach Delay (s/veh)		0	.6											1	1.8	
Approach LOS		A B									В					

Generated: 10/27/2023 1:49:26 PM

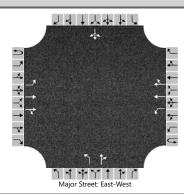
	HCS Two-Way Stop	-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & I-90 WB Terminal							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	I-90 WB Terminal							
Time Analyzed	PM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38									



Vehicle Volumes and Adj	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		1	0	1
Configuration	1	L	Т				Т	TR						L		R
Volume (veh/h)	0	25	420				415	35						30		495
Percent Heavy Vehicles (%)	3	0												6		2
Proportion Time Blocked	1															
Percent Grade (%)	1													(	)	
Right Turn Channelized	1													Ν	lo	
Median Type   Storage	1			Undi	vided											
Critical and Follow-up He	eadwa	ys														
Base Critical Headway (sec)	1	4.1												7.5		6.9
Critical Headway (sec)	1	4.10												6.92		6.94
Base Follow-Up Headway (sec)	1	2.2												3.5		3.3
Follow-Up Headway (sec)	1	2.20												3.56		3.32
Delay, Queue Length, and	d Leve	l of S	ervice													
Flow Rate, v (veh/h)	1	27												33		538
Capacity, c (veh/h)	1	1085												329		756
v/c Ratio	1	0.03												0.10		0.71
95% Queue Length, Q <sub>95</sub> (veh)		0.1												0.3		6.1
Control Delay (s/veh)	1	8.4	0.2											17.1		20.6
Level of Service (LOS)	1	А	А											С		С
Approach Delay (s/veh)	1	0.6											20.4			
Approach LOS	1	А											С			

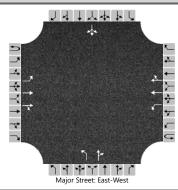
Generated: 5/1/2024 3:37:12 PM

	HCS Two-Way Stop-Control Report											
General Information		Site Information										
Analyst	NM	Intersection	SD 38 & I-90 EB Ramp Terminal									
Agency/Co.	HRG	Jurisdiction	SDDOT									
Date Performed	4/30/2024	East/West Street	SD 38									
Analysis Year	2050	North/South Street	I-90 EB Ramp Terminal									
Time Analyzed	PM Peak	Peak Hour Factor	0.92									
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25									
Project Description	SD 38											



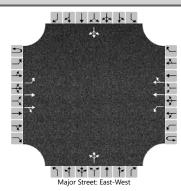
Vehicle Volumes and Adju	ıstme	nts																
Approach		Eastb	ound			Westk	oound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	2	0	0	1	2	0		1	1	0		0	1	0		
Configuration		L	T	TR		L	Т	TR		L		TR			LTR			
Volume (veh/h)	0	190	265	20	0	15	420	30		30	15	25		30	10	35		
Percent Heavy Vehicles (%)	3	10			3	11				20	20	0		8	3	3		
Proportion Time Blocked																		
Percent Grade (%)										(	0		0					
Right Turn Channelized																		
Median Type   Storage				Undi	vided													
<b>Critical and Follow-up He</b>	adwa	ys																
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9		
Critical Headway (sec)		4.30				4.32				7.90	6.90	6.90		7.66	6.56	6.96		
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3		
Follow-Up Headway (sec)		2.30				2.31				3.70	4.20	3.30		3.58	4.03	3.33		
Delay, Queue Length, and	l Leve	l of Se	ervice															
Flow Rate, v (veh/h)	1	207				16				33		43			82			
Capacity, c (veh/h)		1016				1185				122		259			193			
v/c Ratio		0.20				0.01				0.27		0.17			0.42			
95% Queue Length, Q <sub>95</sub> (veh)		0.8				0.0				1.0		0.6			1.9			
Control Delay (s/veh)		9.4	0.6			8.1	0.1			44.7		21.7			36.6			
Level of Service (LOS)		A A				A A			E C						Е			
Approach Delay (s/veh)	4.1				0.4			31.6				36.6						
Approach LOS		A				А				[	)		E					

	HCS Two-Way Stop	-Control Report								
General Information		Site Information								
Analyst	NM	Intersection	SD38/I-90 EB Ramp Terminal/466th St							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/30/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	I-90 EB Ramp Terminal/466th Street							
Time Analyzed	AM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38									



Vehicle Volumes and Adju	ıstme	nts																
Approach		Eastb	ound			Westbound				Northbound				South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	2	0	0	1	2	0		1	1	0		0	1	0		
Configuration		L	Т	TR		L	Т	TR		L		TR			LTR			
Volume (veh/h)	0	430	300	15	0	20	240	20		15	10	20		3	2	28		
Percent Heavy Vehicles (%)	3	2			3	20				33	33	60		33	0	0		
Proportion Time Blocked																		
Percent Grade (%)										(	)		0					
Right Turn Channelized																		
Median Type   Storage				Undi	vided													
<b>Critical and Follow-up He</b>	adwa	ys																
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9		
Critical Headway (sec)		4.14				4.50				8.16	7.16	8.10		8.16	6.50	6.90		
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3		
Follow-Up Headway (sec)		2.22				2.40				3.83	4.33	3.90		3.83	4.00	3.30		
Delay, Queue Length, and	l Leve	l of Se	ervice															
Flow Rate, v (veh/h)	1	467				22				16		33			36			
Capacity, c (veh/h)		1277				1094				40		128			231			
v/c Ratio		0.37				0.02				0.41		0.25			0.16			
95% Queue Length, Q <sub>95</sub> (veh)		1.7				0.1				1.4		1.0			0.5			
Control Delay (s/veh)		9.4	0.6			8.4	0.1			146.1		42.5			23.4			
Level of Service (LOS)		A A				A A			F E						С			
Approach Delay (s/veh)	5.7				0.7			77.0				23.4						
Approach LOS		A				А					F		С					

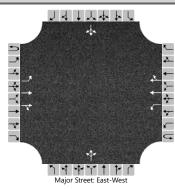
	HCS Two-Way Stop-Control Report											
General Information		Site Information										
Analyst	NM	Intersection	SD 38 & 468th Avenue									
Agency/Co.	HRG	Jurisdiction	SDDOT									
Date Performed	4/30/2024	East/West Street	SD 38									
Analysis Year	2050	North/South Street	468th Ave / County Highway 141									
Time Analyzed	AM Peak	Peak Hour Factor	0.92									
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25									
Project Description	SD 38											



ustme	nts															
匸	Eastb	ound			Westk	oound			North	bound			South	bound		
U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	T	R	
1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
0	1	2	0	0	1	2	0		0	1	0		0	1	0	
1	L	Т	TR		L	Т	TR			LTR				LTR		
0	4	360	0	0	0	225	50		2	2	0		50	0	7	
3	0			3	0				0	100	0		4	0	50	
1																
<u> </u>									(	)		0				
1																
1			Undi	vided												
eadwa	ys															
<b>T</b>	4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9	
	4.10				4.10				7.50	8.50	6.90		7.58	6.50	7.90	
1	2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
	2.20				2.20				3.50	5.00	3.30		3.54	4.00	3.80	
d Leve	l of Sc	ervice	,													
	4				0					4				62		
	1274				1178					284				483		
1	0.00				0.00					0.02				0.13		
	0.0				0.0					0.0				0.4		
1	7.8	0.0			8.1	0.0				17.9				13.6		
	А	А			Α	А				С				В		
1	0.1				0.0				17.9				13.6			
i	,	A		A				С				В				
	1U 0 0 3 3	eadways  A 11  A 10  A 10  A 10  A 10  A 10  A 10  A 10  A 10  A 10  A 10  A 1274  A 10  A 1274  A 10  A 1274  A 10  A 1274  A 10  A 1274  A 10	Eastbound  U L T  1U 1 2  0 1 2  L T  0 4 360  3 0 0  4.1  4.10  2.2  2.20  d Level of Service  4 1274  0.00  0.00  7.8 0.0	Eastbound  U L T R  1U 1 2 3  0 1 2 0  L T TR  0 4 360 0  3 0 0  3 0 0  Undi  eadways  4.1	Eastbound    U	Company   Comp	Eastbound   Westbound	Eastbound   Westbound	Eastbound   Westbound	Eastbound   Westbound   North	Eastbound   Westbound   Northbound     U	Eastbound   Westbound   Northbound	Eastbound   Westbound   Northbound	Lastbound   Westbound   Northbound   South	Company	

Generated: 5/5/2024 8:51:49 AM

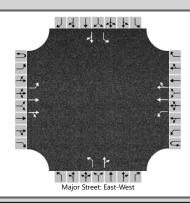
	HCS Two-Way Stop-Control Report												
General Information		Site Information											
Analyst	NM	Intersection	SD 38 & 468th Avenue										
Agency/Co.	HRG	Jurisdiction	SDDOT										
Date Performed	4/30/2024	East/West Street	SD 38										
Analysis Year	2050	North/South Street	468th Ave / County Highway 141										
Time Analyzed	PM Peak	Peak Hour Factor	0.92										
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25										
Project Description	SD 38												



					iviajo	or street. La	st-vvest											
Vehicle Volumes and Adj	ustme	nts																
Approach		Eastb	ound			Westl	oound			North	bound			South	bound			
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R		
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	1	0		
Configuration	]	L	Т	TR		L	Т	TR			LTR				LTR			
Volume (veh/h)	0	0	310	2	0	5	420	55		2	2	0		50	4	4		
Percent Heavy Vehicles (%)	3	0			3	0				0	0	0		4	100	50		
Proportion Time Blocked	]																	
Percent Grade (%)										(	0				0			
Right Turn Channelized																		
Median Type   Storage				Undi	vided				<u> </u>									
Critical and Follow-up H	eadwa	ys																
Base Critical Headway (sec)	1	4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9		
Critical Headway (sec)		4.10				4.10				7.50	6.50	6.90		7.58	8.50	7.90		
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3		
Follow-Up Headway (sec)		2.20				2.20				3.50	4.00	3.30		3.54	5.00	3.80		
Delay, Queue Length, an	d Leve	l of S	ervice															
Flow Rate, v (veh/h)	<u> </u>	0				5					4				63			
Capacity, c (veh/h)	1	1060				1231					326				324			
v/c Ratio		0.00				0.00					0.01				0.19			
95% Queue Length, Q <sub>95</sub> (veh)	1	0.0				0.0					0.0				0.7			
Control Delay (s/veh)		8.4	0.0			7.9	0.0				16.2				18.8			
Level of Service (LOS)	1	А	А			А	Α				С				С			
Approach Delay (s/veh)	0.0				0.1				16.2				18.8					
Approach LOS		А			А					(	С		С					

Generated: 5/5/2024 8:49:56 AM

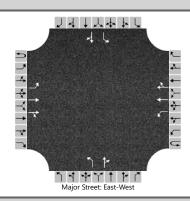
	HCS Two-Way Stop-Control Report												
General Information		Site Information											
Analyst	NM	Intersection	SD 38 & 469th Ave										
Agency/Co.	HRG	Jurisdiction	SDDOT										
Date Performed	4/26/2023	East/West Street	SD 38										
Analysis Year	2050	North/South Street	469th Ave / Co Hwy 139										
Time Analyzed	AM Peak	Peak Hour Factor	0.92										
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25										
Project Description	SD 38												



Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		1	1	0		1	1	0
Configuration		L	Т	TR		L	Т	TR		L		TR		L		TR
Volume (veh/h)	0	5	330	75	0	75	165	5		110	5	280		15	5	5
Percent Heavy Vehicles (%)	3	3			3	5				13	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)										(	)			(	0	
Right Turn Channelized																
Median Type   Storage	1			Undi	vided											
<b>Critical and Follow-up He</b>	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)		4.16				4.20				7.76	6.56	6.96		7.56	6.56	6.96
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.25				3.63	4.03	3.33		3.53	4.03	3.33
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1	5				82				120		310		16		11
Capacity, c (veh/h)		1380				1095				291		760		219		446
v/c Ratio		0.00				0.07				0.41		0.41		0.07		0.02
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.2				1.9		2.0		0.2		0.1
Control Delay (s/veh)		7.6	0.0			8.6	0.3			25.8		13.0		22.7		13.3
Level of Service (LOS)	A A				A A			D B					С		В	
Approach Delay (s/veh)	0.1				2.8			16.5				19.0				
Approach LOS	А				А				С				С			

Generated: 5/5/2024 8:53:24 AM

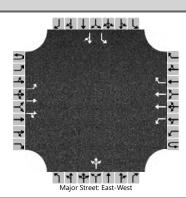
HCS Two-Way Stop-Control Report									
General Information		Site Information							
Analyst	NM	Intersection	SD 38 & 469th Ave						
Agency/Co.	HRG	Jurisdiction	SDDOT						
Date Performed	4/26/2024	East/West Street	SD 38						
Analysis Year	2050	North/South Street	469th Ave / Co Hwy 139						
Time Analyzed	PM Peak	Peak Hour Factor	0.92						
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25						
Project Description	SD 38								



Approach	$\overline{}$	Facth	ound			Westk	ound			North	hound			South	bound			
Movement	U	Laste	T	R	U	West	Т	R	U L T R			U						
	<u>+                                    </u>								U				U					
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12		
Number of Lanes	0	1	2	0	0	1	2	0		1	1	0		1	1	0		
Configuration		L	T	TR		L	Т	TR		L		TR		L		TR		
Volume (veh/h)	0	5	245	120	0	285	380	5		100	5	120		15	5	10		
Percent Heavy Vehicles (%)	3	3			3	5				2	3	15		3	3	3		
Proportion Time Blocked	1																	
Percent Grade (%)	]									(	)		0					
Right Turn Channelized	1																	
Median Type   Storage	1			Undi	vided													
Critical and Follow-up H	eadwa	ys																
Base Critical Headway (sec)	1	4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9		
Critical Headway (sec)	]	4.16				4.20				7.54	6.56	7.20		7.56	6.56	6.96		
Base Follow-Up Headway (sec)	1	2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3		
Follow-Up Headway (sec)		2.23				2.25				3.52	4.03	3.45		3.53	4.03	3.33		
Delay, Queue Length, an	d Leve	l of Se	ervice															
Flow Rate, v (veh/h)	<u> </u>	5				310				109		136		16		16		
Capacity, c (veh/h)	1	1130				1137				101		611		74		228		
v/c Ratio	1	0.00				0.27				1.08		0.22		0.22		0.07		
95% Queue Length, Q <sub>95</sub> (veh)	]	0.0				1.1				6.9		0.8		0.8		0.2		
Control Delay (s/veh)	]	8.2	0.0			9.3	0.8			192.1		12.6		66.4		22.0		
Level of Service (LOS)	1	А	Α			Α	Α			F		В		F		С		
Approach Delay (s/veh)		0	.1			4	.4		92.3				44.2					
Approach LOS	1	,	Α			-	4				F			[	E	E		

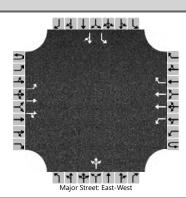
Generated: 5/5/2024 8:54:38 AM

HCS Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & La Mesa							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/29/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	La Mesa							
Time Analyzed	AM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38									



Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	1		0	1	0		1	1	0
Configuration		L	Т	TR		L	T	R			LTR			L		TR
Volume (veh/h)	0	30	700	4	0	0	235	15		0	15	5		75	4	30
Percent Heavy Vehicles (%)	3	0			3	0				0	13	0		0	50	0
Proportion Time Blocked																
Percent Grade (%)										(	)			(	0	
Right Turn Channelized						Ν	lo									
Median Type   Storage	ian Type   Storage Undivided															
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)		4.10				4.10				7.50	6.76	6.90		7.50	7.50	6.90
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.20				3.50	4.13	3.30		3.50	4.50	3.30
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)		33				0					22			82		37
Capacity, c (veh/h)		1303				857					229			287		566
v/c Ratio		0.03				0.00					0.09			0.28		0.07
95% Queue Length, Q <sub>95</sub> (veh)		0.1				0.0					0.3			1.1		0.2
Control Delay (s/veh)		7.8	0.2			9.2	0.0				22.3			22.5		11.8
Level of Service (LOS)		А	А			А	А				С			С		В
Approach Delay (s/veh)		0	.5			0	.0		22.3				19.2			
Approach LOS		/	4			/	4			(	2			(	С	

HCS Two-Way Stop-Control Report										
General Information		Site Information								
Analyst	NM	Intersection	SD 38 & La Mesa							
Agency/Co.	HRG	Jurisdiction	SDDOT							
Date Performed	4/29/2024	East/West Street	SD 38							
Analysis Year	2050	North/South Street	La Mesa							
Time Analyzed	PM Peak	Peak Hour Factor	0.92							
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25							
Project Description	SD 38									



Vehicle Volumes and Adju	stme	nts														
Approach		Eastb	ound			Westl	ound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	1		0	1	0		1	1	0
Configuration		L	Т	TR		L	T	R			LTR			L		TR
Volume (veh/h)	0	25	325	0	0	9	735	100		4	5	0		80	15	30
Percent Heavy Vehicles (%)	3	0			3	0				0	0	0		9	0	0
Proportion Time Blocked																
Percent Grade (%)										(	)			(	0	
Right Turn Channelized						Ν	lo									
Median Type   Storage				Undi	vided											
Critical and Follow-up He	adwa	ys														
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)		4.10				4.10				7.50	6.50	6.90		7.68	6.50	6.90
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20				2.20				3.50	4.00	3.30		3.59	4.00	3.30
Delay, Queue Length, and	Leve	of Se	ervice													
Flow Rate, v (veh/h)		27				10					10			87		49
Capacity, c (veh/h)		758				1217					167			158		329
v/c Ratio		0.04				0.01					0.06			0.55		0.15
95% Queue Length, Q <sub>95</sub> (veh)		0.1				0.0					0.2			2.8		0.5
Control Delay (s/veh)		9.9	0.3			8.0	0.1				27.9			52.4		17.8
Level of Service (LOS)		Α	Α			Α	Α				D			F		С
Approach Delay (s/veh)		1	.0			0	.1		27.9				40.0			
Approach LOS			4			,	4			[	)				E	

#### **HCS Signalized Intersection Results Summary** 144444 Intersection Information **General Information** HRG Duration, h 0.250 Agency Analyst NM Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period AM Peak 0.92 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 7:15 SD 38 & Marion Street File Name (18) SD38&Marion AM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 40 Demand (v), veh/h 165 340 105 50 125 75 110 225 120 45 145 **Signal Information** J. Cycle, s 50.0 Reference Phase 2 Offset, s 0 Reference Point End Green 2.6 3.9 12.9 2.5 2.0 10.0 Uncoordinated No Simult. Gap E/W On Yellow 4.0 0.0 4.0 4.0 0.0 4.0 Force Mode Fixed Simult. Gap N/S On Red 0.0 0.0 0.0 0.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL SBT **Assigned Phase** 5 2 6 3 8 1 7 4 Case Number 2.0 3.0 1.1 4.0 2.0 3.0 2.0 3.0 Phase Duration, s 10.6 20.8 6.6 16.9 8.5 16.0 6.5 14.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Change Period, (Y+Rc), s Max Allow Headway ( MAH ), s 2.9 0.0 2.9 0.0 2.9 2.9 2.9 2.9 Queue Clearance Time ( $g_s$ ), s 7.1 3.1 5.6 8.5 3.5 5.9 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.0 0.0 0.0 0.6 0.0 0.6 Phase Call Probability 0.92 0.53 0.81 1.00 0.49 1.00 1.00 0.04 0.21 1.00 Max Out Probability 1.00 0.15 **Movement Group Results** EΒ WB NB SB Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 179 370 114 54 112 106 120 245 130 49 158 43 Adjusted Saturation Flow Rate ( s ), veh/h/ln 1701 1674 1525 1714 1772 1556 1647 1674 1502 1554 1758 1466 5.1 4.1 2.7 1.1 2.5 2.7 3.6 6.5 3.9 1.2 Queue Service Time ( $g_s$ ), s 3.6 1.5 2.5 Cycle Queue Clearance Time ( q c ), s 5.1 4.1 2.7 1.1 2.7 3.6 6.5 3.6 1.5 3.9 1.2 0.34 0.26 0.24 0.24 0.20 Green Ratio (g/C) 0.13 0.34 0.31 0.26 0.09 0.05 0.20 403 Capacity (c), veh/h 223 1128 514 456 459 403 148 361 77 352 293 Volume-to-Capacity Ratio (X) 0.804 0.328 0.222 0.119 0.243 0.263 0.806 0.607 0.361 0.638 0.448 0.148 Back of Queue (Q), ft/ln (95 th percentile) Back of Queue (Q), veh/ln (95 th percentile) 4.1 2.2 1.4 0.6 1.6 1.6 3.2 3.5 1.7 1.0 2.3 0.6 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 14.7 Uniform Delay ( d 1 ), s/veh 21.1 12.4 11.9 12.3 14.7 22.3 16.9 15.8 23.3 17.6 16.5 Incremental Delay ( d 2 ), s/veh 11.0 8.0 1.0 0.0 1.3 1.6 15.9 8.0 0.2 3.3 0.3 0.1 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 32.1 13.1 12.9 12.3 15.9 16.3 38.2 17.7 16.0 26.6 17.9 16.6 Level of Service (LOS) С В В В В В D В В С В В 18.2 В 15.4 В 22.2 С 19.4 Approach Delay, s/veh / LOS В Intersection Delay, s/veh / LOS 19.1 В **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 2.08 В 2.09 В 2.26 2.42 В В Bicycle LOS Score / LOS 1.03 Α 0.71 Α 1.30 Α 0.90 Α

#### **HCS Signalized Intersection Results Summary** 144444 Intersection Information **General Information** HRG Duration, h 0.250 Agency Analyst NM Analysis Date May 8, 2023 Area Type Other PHF Jurisdiction SDDOT Time Period PM Peak 0.90 **Urban Street** SD 38 Analysis Year 2050 **Analysis Period** 1> 16:45 SD 38 & Marion Street File Name (18) SD38&Marion PM.xus Intersection **Project Description** WB **Demand Information** EB NB SB Approach Movement L R L R L R L R 55 Demand (v), veh/h 70 230 105 170 355 180 205 125 85 355 205 **Signal Information** Cycle, s 60.0 Reference Phase 2 Offset, s 0 Reference Point End Green 3.7 0.4 13.3 4.2 2.8 15.5 Uncoordinated No Simult. Gap E/W On Yellow 4.0 4.0 4.0 4.0 0.0 4.0 Force Mode Fixed Simult. Gap N/S On Red 0.0 0.0 0.0 0.0 0.0 0.0 **Timer Results EBL EBT WBL WBT** NBL **NBT** SBL SBT **Assigned Phase** 5 2 6 3 8 1 7 4 Case Number 2.0 3.0 2.0 3.0 2.0 3.0 2.0 3.0 Phase Duration, s 7.7 17.3 12.2 21.7 11.0 22.3 8.2 19.5 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Change Period, (Y+Rc), s Max Allow Headway ( MAH ), s 2.9 0.0 2.9 0.0 2.9 3.0 2.9 3.0 Queue Clearance Time ( $g_s$ ), s 5.1 8.5 9.0 8.1 5.3 14.7 Green Extension Time ( $g_e$ ), s 0.0 0.0 0.0 0.0 0.0 1.6 0.0 8.0 Phase Call Probability 0.73 0.96 0.96 1.00 0.79 1.00 0.55 0.03 1.00 Max Out Probability 1.00 1.00 0.89 **Movement Group Results** EΒ WB NB SB Approach Movement L Т R L Т R L Т R L Т R **Assigned Movement** 5 2 12 1 6 16 3 8 18 7 4 14 Adjusted Flow Rate ( v ), veh/h 78 256 117 189 394 61 200 228 139 94 394 228 Adjusted Saturation Flow Rate ( s ), veh/h/ln 1474 1660 1490 1688 1772 1714 1772 1478 1688 1772 1478 1406 3.1 3.9 6.5 12.1 7.0 6.1 4.3 8.1 Queue Service Time ( $g_s$ ), s 4.0 1.9 3.3 12.7 Cycle Queue Clearance Time ( q c ), s 3.1 3.9 4.0 6.5 12.1 1.9 7.0 6.1 4.3 3.3 12.7 8.1 0.22 0.22 0.30 Green Ratio (g/C) 0.06 0.14 0.30 0.12 0.31 0.31 0.07 0.26 0.26 Capacity (c), veh/h 92 735 330 230 523 415 200 541 451 119 459 383 Volume-to-Capacity Ratio (X) 0.845 0.347 0.354 0.822 0.754 0.147 1.000 0.421 0.308 0.797 0.859 0.595 Back of Queue (Q), ft/ln (95 th percentile) Back of Queue (Q), veh/ln (95 th percentile) 2.1 2.5 2.5 6.1 9.1 1.0 10.0 3.6 2.1 2.7 9.5 4.2 Queue Storage Ratio (RQ) (95 th percentile) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 Uniform Delay ( d 1 ), s/veh 27.8 19.7 19.7 25.2 19.2 15.6 26.5 16.6 16.0 27.5 21.2 19.5 Incremental Delay ( d 2 ), s/veh 7.7 1.3 3.0 18.3 9.7 0.7 63.6 0.2 0.1 10.2 10.8 0.7 Initial Queue Delay ( d 3 ), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Control Delay ( d ), s/veh 35.6 21.0 22.7 43.5 28.9 16.3 90.1 16.8 16.1 37.7 31.9 20.2 Level of Service (LOS) D С С D С В F В В D С С 23.9 С 32.0 С 42.5 D 29.0 С Approach Delay, s/veh / LOS Intersection Delay, s/veh / LOS 32.1 С **Multimodal Results** ΕB WB NB Pedestrian LOS Score / LOS 2.10 В 2.10 В 2.26 2.27 В В Bicycle LOS Score / LOS 0.86 Α 1.55 1.42 Α 1.67



Appendix B - IHSDM Output

# Interactive Highway Safety Design Model

# **Crash Prediction Evaluation Report**

#### Disclaimer

The Interactive Highway Design Model (IHSDM) software is disseminated under the sponsorship of the Department of Transportation in the interest of information exchange. The United States Government assumes no liability for its content or use thereof. This document does not constitute a standard, specification, or regulation.

The United States Government does not endorse products or manufacturers. Trade and manufacturers' names may appear in this software and documentation only because they are considered essential to the objective of the software.

#### **Limited Warranty and Limitations of Remedies**

This software product is provided "as-is," without warranty of any kind-either expressed or implied (but not limited to the implied warranties of merchantability and fitness for a particular purpose). The FHWA do not warrant that the functions contained in the software will meet the end-user's requirements or that the operation of the software will be uninterrupted and error-free.

Under no circumstances will the FHWA be liable to the end-user for any damages or claimed lost profits, lost savings, or other incidental or consequential damages rising out of the use or inability to use the software (even if these organizations have been advised of the possibility of such damages), or for any claim by any other party.

#### **Notice**

The use of the IHSDM software is being done strictly on a voluntary basis. In exchange for provision of IHSDM, the user agrees that the Federal Highway Administration (FHWA), U.S. Department of Transportation and any other agency of the Federal Government shall not be responsible for any errors, damage or other liability that may result from any and all use of the software, including installation and testing of the software. The user further agrees to hold the FHWA and the Federal Government harmless from any resulting liability. The user agrees that this hold harmless provision shall flow to any person to whom or any entity to which the user provides the IHSDM software. It is the user's full responsibility to inform any person to whom or any entity to which it provides the IHSDM software of this hold harmless provision.

# **Table of Contents**

Report Overview	1
Disclaimer Regarding Crash Prediction Method	2
Section Types	3
Section 1 Evaluation	3
List of Tables	
Table Observed Crashes Used in the Evaluation (Section 1)	5
Table Evaluation Highway - Homogeneous Segments (Section 1)	6
Table Crash History Highway - Homogeneous Segments (Section 1)	17
Table Evaluation Intersection - Section 1	23
Table Crash History Intersection - Section 1	24
Table Expected Highway Crash Rates and Frequencies Summary (Section 1)	25
Table Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 1)	26
Table Expected Crash Frequencies and Rates by Horizontal Design Element (Section 1)	30
Table Predicted Crash Frequencies by Year (Section 1)	31
Table Expected Crash Frequencies by Year (Section 1)	32
Table Comparing Predicted and Expected Crashes for the Evaluation Period (Section 1)	33
Table Expected Crash Type Distribution (Section 1)	34
Table Evaluation Message	35
List of Figures	
Figure Crash Prediction Summary (Section 1)	4

# **Report Overview**

Report Generated: Jun 1, 2024 3:23 PM

Report Template: System: Single Page, 508 Compliant [System] (mlcpm5, Dec 5, 2019 2:16 PM)

**Evaluation Date:** Sat Jun 01 15:07:09 CDT 2024 **IHSDM Version:** v17.0.0 (Sep 22, 2021)

Crash Prediction Module: v12.0.0 (Sep 22, 2021)

User Name: naveen.mallipaddi

**Organization Name:** 

Phone: E-Mail:

Project Title: SD-38\_Build\_Option1\_I90EBRamp\_I

Project Comment: Created Mon Mar 27 16:47:43 CDT 2023

Project Unit System: U.S. Customary

**Highway Title: SD-38** 

Highway Comment: Created Mon Mar 27 16:49:47 CDT 2023

**Highway Version: 22** 

**Evaluation Title:** Evaluation 56

Evaluation Comment: Created Sat Jun 01 15:06:37 CDT 2024

**Minimum Location:** 171+44.000 **Maximum Location:** 580+10.000

Policy for Superelevation: AASHTO 2011 U.S. Customary

Calibration: HSM Configuration

Crash Distribution: HSM Configuration Model/CMF: HSM Configuration

First Year of Analysis: 2025 Last Year of Analysis: 2050

**Empirical-Bayes Analysis:** Site-Specific **Highway with Crash History:** SD-38

Highway with Crash History Comment: Created Mon Mar 27 16:49:47 CDT 2023

Highway with Crash History Version: 22 First Year of Observed Crashes: 2019 Last Year of Observed Crashes: 2023

# **Disclaimer Regarding Crash Prediction Method**

IMPORTANT NOTICE ABOUT COMPARING RESULTS FROM HIGHWAY SAFETY MANUAL FIRST EDITION (2010) MODELS TO RESULTS FROM NEW MODELS DEVELOPED UNDER NCHRP PROJECTS 17-70, 17-58, AND 17-68

Since the publication of the Highway Safety Manual - First Edition (HSM-1), in 2010 by the American Association of State Highway and Transportation Officials (AASHTO), multiple research efforts have been undertaken through the National Cooperative Highway Research Program (NCHRP) to develop safety performance models for road segment and intersection facility types that were not initially reflected in the HSM-1, in order to expand the breadth and depth of the HSM in the future.

The IHSDM Crash Prediction Module (CPM) is intended as a faithful implementation of HSM Part C predictive methods. As NCHRP projects to develop new predictive methods for the HSM are completed, FHWA works to incorporate the new methods into IHSDM, sometimes in advance of publication in the HSM. The following new crash predictive methods have been accepted by NCHRP project panels and incorporated into IHSDM, while pending AASHTO's approval for incorporation into a future edition of the HSM:

- Roundabouts: completed in 2018 under NCHRP Project 17-70, the new methods will provide improved outcomes for the safety analysis of roundabouts.
- 6+ lane and one-way urban/suburban arterials (including models for segments and intersections): completed under NCHRP Project 17-58.
- Intersection crash prediction methods for some intersection configurations and traffic control types not currently addressed in the HSM (e.g., all-way stop; rural 3-leg signalized; 3-leg stop-controlled where the major leg turns; urban 5-leg signalized; urban high-speed intersections): completed in 2021 under NCHRP Project 17-68.

However, in the absence of local calibration factors (see HSM-1 Part C, Appendix A for guidance on calibration of the predictive models), it is neither appropriate nor advisable to directly compare the results from new models (from NCHRP Projects 17-58, 17-68, and 17-70) to results from HSM-1 models, as the models were not calibrated to the same base state data sets, and consequently can produce unexpected results. If local calibration factors are available and applied to both new models and HSM-1 models, then it may be appropriate to directly compare the results. [Note: Work being performed under NCHRP Project 17-72 (Update of Crash Modification Factors for the Highway Safety Manual) is expected to re-calibrate many of the old (HSM-1) and new (e.g., NCHRP 17-70) models to data from a single (or small number of) states, that would allow results from all models to be directly compared.]

The models produced for NCHRP Project 17-70 have independent value in terms of informing the design of a roundabout and assessing the effects of different design characteristics on the expected safety performance of a roundabout.

The HSM-1 interim method previously included in IHSDM for evaluating roundabouts on urban/suburban arterials (i.e., evaluating an existing intersection and then applying a Crash Modification Factor for replacing the existing intersection with a roundabout) has been deactivated in IHSDM, to minimize any confusion with the new roundabout methodology.

# **Section Types**

## **Section 1 Evaluation**

Section: Section 1

**Evaluation Start Location:** 171+44.000 **Evaluation End Location:** 580+10.000

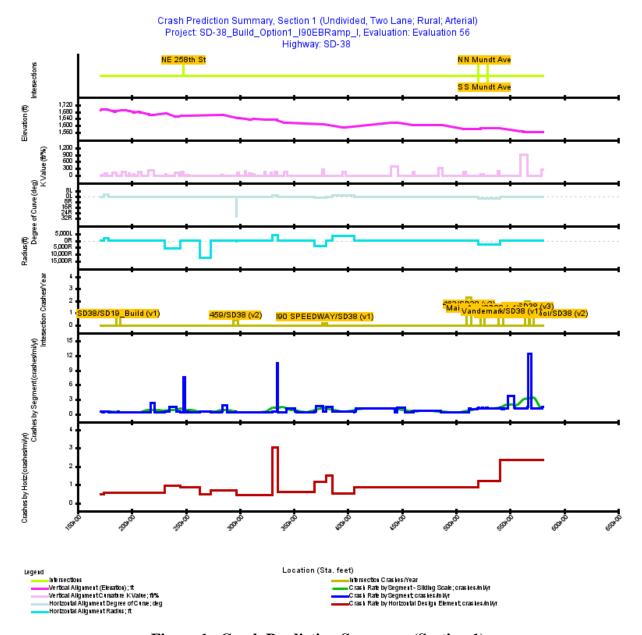
Area Type: Rural

Functional Class: Arterial

Type of Alignment: Undivided, Two Lane

Model Category: Rural, Two Lane

Calibration Factor: 2U=1.0; 3ST=1.0; 4ST=1.0;



**Figure 1. Crash Prediction Summary (Section 1)** 

**Table 1. Observed Crashes Used in the Evaluation (Section 1)** 

Year	Observed Crashes	Total Crashes Used	FI Crashes	FI no/C Crashes	PDO Crashes
2019	5	4	1	0	3
2020	9	9	5	1	4
2021	8	7	3	1	4
2022	6	6	3	1	3
2023	0	0	0	0	0
All Years	28 <sup>[1]</sup>	26	12	3	14

## **Footnotes**

<sup>[1]</sup> Note: Observed crash data that does not comply with the associated CPM model requirements may not be used in EB processing.

 Table 2. Evaluation Highway - Homogeneous Segments (Section 1)

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	rd	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
1	Rural Two-Lane Segment Two-lane Undivided	171+44. 000	172+42. 000	98.00	0.018 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,362; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	4.25	6.2	3	false	0	false	false	false				
2	Rural Two-Lane Segment Two-lane Undivided	172+42. 000	174+52. 690	210.6 9	0.039 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.17	6.2	3	false	0	false	false	false				
3	Rural Two-Lane Segment Two-lane Undivided	174+52. 690	176+25. 000	172.3 1	0.032 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.17	6.2	3	false	0	false	false	false	2,074. 80	2.0	true	40
4	Rural Two-Lane Segment Two-lane Undivided	176+25. 000	178+85. 250	260.2 5		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.88	6.2	3	false	0	false	false	false	2,074. 80	2.0	true	40
5	Rural Two-Lane Segment Two-lane Undivided	178+85. 250	183+75. 370	490.1 2	0.092	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,000; 2041: 3,122; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.88	6.2	3	false	0	false	false	false				
6	Rural Two-Lane Segment Two-lane Undivided	183+75. 370	184+00. 000	24.63	0.004 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,000; 2041: 3,122; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
7	Rural Two-Lane Segment Two-lane Undivided	184+00. 000	184+45. 000	45.00	0.008	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
8	Rural Two-Lane Segment Two-lane Undivided	184+45. 000	185+20. 000	75.00	0.014	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
9	Rural Two-Lane Segment Two-lane Undivided	185+20. 000	186+60. 000	140.0 0		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
10	Rural Two-Lane Segment Two-lane Undivided	186+60. 000	187+20. 000	60.00	0.011	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
11	Rural Two-Lane Segment Two-lane Undivided	187+20. 000	187+60. 000	40.00	0.007 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	1.13	6.2	3	false	0	false	false	false				
12	Rural Two-Lane Segment Two-lane Undivided	187+60. 000	190+00. 000	240.0 0	0.045 5	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
13	Rural Two-Lane Segment Two-lane Undivided	190+00. 000	192+00. 000	200.0	0.037 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
14	Rural Two-Lane Segment Two-lane Undivided	192+00. 000	192+39. 270	39.27	0.007 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,362; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
15	Rural Two-Lane Segment Two-lane Undivided	192+39. 270	193+60. 000	120.7 3	0.022 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.94	6.2	3	false	0	false	false	false				
16	Rural Two-Lane Segment Two-lane Undivided	193+60. 000	197+65. 000	405.0 0	0.076 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 0.94	6.2	3	false	0	false	false	false				
17	Rural Two-Lane Segment Two-lane Undivided	197+65. 000	199+00. 000		0.025 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 1.94	6.2	3	false	0	false	false	false				
18	Rural Two-Lane Segment Two-lane Undivided	199+00. 000	201+63. 750		0.050 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 1.94	6.2	3	false	0	false	false	false				
19	Rural Two-Lane Segment Two-lane Undivided	201+63. 750	202+00. 000	36.25	0.006 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
20	Rural Two-Lane Segment Two-lane Undivided	202+00. 000	207+00. 000	500.0 0	0.094 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,362; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
21	Rural Two-Lane Segment Two-lane Undivided	207+00. 000	207+49. 760	49.76	0.009 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
22	Rural Two-Lane Segment Two-lane Undivided	207+49. 760	217+74. 250		0.194 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 1.70	6.2	3	false	0	false	false	false				
23	Rural Two-Lane Segment Two-lane Undivided	217+74. 250	221+00. 000	325.7 5	0.061 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
24	Rural Two-Lane Segment Two-lane Undivided	221+00. 000	226+00. 000	500.0	0.094 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
25	Rural Two-Lane Segment Two-lane Undivided	226+00. 000	230+66. 250	466.2 5	0.088	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
26	Rural Two-Lane Segment Two-lane Undivided	230+66. 250	231+39. 700	73.45	0.013 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.77	6.2	3	false	0	false	false	false	5,644. 64	2.0	true	70
27	Rural Two-Lane Segment Two-lane Undivided	231+39. 700	235+00. 000		0.068 2	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	2.00	6.2	3	false	0	false	false	false	5,644. 64	2.0	true	70
28	Rural Two-Lane Segment Two-lane Undivided	235+00. 000	241+61. 390	661.3 9	0.125	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,096; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	2.00	6.2	3	false	0	false	false	false	5,644. 64	2.0	true	70
29	Rural Two-Lane Segment Two-lane Undivided	241+61. 390	242+00. 000	38.61	0.007	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	1.16	6.2	3	false	0	false	false	false	5,644. 64	2.0	true	70

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
30	Rural Two-Lane Segment Two-lane Undivided	242+00. 000	245+14. 280	314.2 8		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,096; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.16	6.2	3	false	0	false	false	false	5,644. 64	2.0	true	70
31	Rural Two-Lane Segment Two-lane Undivided	245+14. 280	246+55. 100	140.8 2		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.16	6.2	3	false	0	false	false	false				
32	Rural Two-Lane Segment Two-lane Undivided	246+55. 100	248+00. 000	144.9 0		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2044: 3,046; 2044: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.42	6.2	3	false	0	false	false	false				
33	Rural Two-Lane Segment Two-lane Undivided	248+00. 000	249+00. 000	100.0 0		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	0.42	6.2	3	false	0	false	false	false				
34	Rural Two-Lane Segment Two-lane Undivided	249+00. 000	251+21. 980	221.9 8		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.42	6.2	3	false	0	false	false	false				
35	Rural Two-Lane Segment Two-lane Undivided	251+21. 980	252+40. 240	118.2 6	0.022 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	3.43	6.2	3	false	0	false	false	false				
36	Rural Two-Lane Segment Two-lane Undivided	252+40. 240	263+22. 600	1,082. 36		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
37	Rural Two-Lane Segment Two-lane Undivided	263+22. 600	272+66. 740	944.1 4		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	false	false	false	12,237 .00	2.0	true	70
38	Rural Two-Lane Segment Two-lane Undivided	272+66. 740	280+00. 000	733.2 6		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
39	Rural Two-Lane Segment Two-lane Undivided	280+00. 000	283+15. 050	315.0 5		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
40	Rural Two-Lane Segment Two-lane Undivided	283+15. 050	284+08. 540	93.49	0.017 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	4.47	6.2	3	false	0	false	false	false				
41	Rural Two-Lane Segment Two-lane Undivided	284+08. 540	288+50. 000	441.4 6		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.47	6.2	3	false	0	false	false	false				
42	Rural Two-Lane Segment Two-lane Undivided	288+50. 000	289+00. 000	50.00		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.47	6.2	3	false	0	false	false	false				
43	Rural Two-Lane Segment Two-lane Undivided	289+00. 000	295+90. 000	690.0 0		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,096; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.47	6.2	3	false	0	false	false	false				
44	Rural Two-Lane Segment Two-lane Undivided	295+90. 000	296+00. 000	10.00		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	1.47	6.2	3	false	0	false	false	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
45	Rural Two-Lane Segment Two-lane Undivided	296+00. 000	296+10. 000	10.00	0.001 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,334; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	1.47	6.2	3	false	0	false	false	false				
46	Rural Two-Lane Segment Two-lane Undivided	296+10. 000	296+96. 520	86.52	0.016 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.47	6.2	3	false	0	false	false	false				
47	Rural Two-Lane Segment Two-lane Undivided	296+96. 520	298+33. 660	137.1 4	0.026 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.47	6.2	3	false	0	false	false	false				
48	Rural Two-Lane Segment Two-lane Undivided	298+33. 660	303+50. 000	516.3 4	0.097 8	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.61	6.2	3	false	0	false	false	false				
49	Rural Two-Lane Segment Two-lane Undivided	303+50. 000	304+50. 000		0.018 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 2,324; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.61	6.2	3	false	0	false	false	false				
50	Rural Two-Lane Segment Two-lane Undivided	304+50. 000	305+02. 039	52.04	0.009 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.61	6.2	3	false	0	false	false	false				
51	Rural Two-Lane Segment Two-lane Undivided	305+02. 039	309+35. 490	433.4 5	0.082 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.15	6.2	3	false	0	false	false	false				
52	Rural Two-Lane Segment Two-lane Undivided	309+35. 490	311+70. 000	234.5 1	0.044 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.24	6.2	3	false	0	false	false	false				
53	Rural Two-Lane Segment Two-lane Undivided	311+70. 000	313+25. 000	155.0 0	0.029 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.24	6.2	3	false	0	false	false	false				
54	Rural Two-Lane Segment Two-lane Undivided	313+25. 000	323+00. 000		0.184 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.33	6.2	3	false	0	false	false	false				
55	Rural Two-Lane Segment Two-lane Undivided	323+00. 000	323+26. 980	26.98	0.005 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.33	6.2	3	false	0	false	false	false				
56	Rural Two-Lane Segment Two-lane Undivided	323+26. 980	328+89. 230	562.2 5	0.106 5	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.26	6.2	3	false	0	false	false	false				
57	Rural Two-Lane Segment Two-lane Undivided	328+89. 230	329+81. 740	92.51	0.017 5	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,343; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.52	6.2	3	false	0	false	false	false				
58	Rural Two-Lane Segment Two-lane Undivided	329+81. 740	333+24. 920	343.1 8	0.065 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,343; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.52	6.2	3	false	0	false	false	false	4,010. 13	2.0	true	70
59	Rural Two-Lane Segment Two-lane Undivided	333+24. 920	334+00. 000	75.08	0.014 2	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,343; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	2.17	6.2	3	false	0	false	false	false	4,010. 13	2.0	true	70

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
60	Rural Two-Lane Segment Two-lane Undivided	334+00. 000	335+39. 960		0.026 5	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,096; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	2.17	6.2	3	false	0	false	false	false	4,010. 13	2.0	true	70
61	Rural Two-Lane Segment Two-lane Undivided	335+39. 960	342+39. 000	699.0 4	0.132 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	2.17	6.2	3	false	0	false	false	false				
62	Rural Two-Lane Segment Two-lane Undivided	342+39. 000	343+00. 000	61.00	0.011 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.24	6.2	3	false	0	false	false	false				
63	Rural Two-Lane Segment Two-lane Undivided	343+00. 000	351+20. 000		0.155 3	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.24	6.2	3	false	0	false	false	false				
64	Rural Two-Lane Segment Two-lane Undivided	351+20. 000	352+00. 000	80.00	0.015 2	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	0.24	6.2	3	false	0	false	false	false				
65	Rural Two-Lane Segment Two-lane Undivided	352+00. 000	352+20. 000	20.00	0.003	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	0.24	6.2	3	false	0	false	false	false				
66	Rural Two-Lane Segment Two-lane Undivided	352+20. 000	362+50. 000	1,030. 00	0.195 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.24	6.2	3	false	0	false	false	false				
67	Rural Two-Lane Segment Two-lane Undivided	362+50. 000	369+14. 990	664.9 9	0.125 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.24	6.2	3	false	0	false	false	false				
68	Rural Two-Lane Segment Two-lane Undivided	369+14. 990	370+30. 000		0.021 8	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.24	6.2	3	false	0	false	false	false	4,023. 18	2.0	true	70
69	Rural Two-Lane Segment Two-lane Undivided	370+30. 000	370+60. 000	30.00	0.005 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.24	6.2	3	false	0	false	false	false	4,023. 18	2.0	true	70
70	Rural Two-Lane Segment Two-lane Undivided	370+60. 000	376+83. 610	623.6 1	0.118 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.24	6.2	3	false	0	false	false	false	4,023. 18	2.0	true	70
71	Rural Two-Lane Segment Two-lane Undivided	376+83. 610	378+00. 000	116.3 9	0.022 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	4,023. 18	2.0	true	70
72	Rural Two-Lane Segment Two-lane Undivided	378+00. 000	378+40. 000	40.00	0.007 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	8.00	1.04	6.2	3	false	0	false	false	false	4,023. 18	2.0	true	70
73	Rural Two-Lane Segment Two-lane Undivided	378+40. 000	378+60. 000	20.00	0.003	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	8.00	1.04	6.2	3	false	0	false	false	false	4,023. 18	2.0	true	70
74	Rural Two-Lane Segment Two-lane Undivided	378+60. 000	379+00. 000	40.00	0.007	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	8.00	1.04	6.2	3	false	0	false	false	false	4,023. 18	2.0	true	70

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	y Density	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
75	Rural Two-Lane Segment Two-lane Undivided	379+00. 000	379+62. 690	62.69	0.011 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	4,023. 18	2.0	true	e 70
76	Rural Two-Lane Segment Two-lane Undivided	379+62. 690	385+22. 970	560.2 8	0.106 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false				
77	Rural Two-Lane Segment Two-lane Undivided	385+22. 970	386+60. 000	137.0 3	0.026 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
78	Rural Two-Lane Segment Two-lane Undivided	386+60. 000	389+50. 000	290.0 0	0.054 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
79	Rural Two-Lane Segment Two-lane Undivided	389+50. 000	394+00. 000	450.0 0	0.085 2	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
80	Rural Two-Lane Segment Two-lane Undivided	394+00. 000	396+46. 150	246.1 5	0.046 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
81	Rural Two-Lane Segment Two-lane Undivided	396+46. 150	397+00. 000	53.85	0.010 2	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
82	Rural Two-Lane Segment Two-lane Undivided	397+00. 000	399+00. 000	200.0 0	0.037 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
83	Rural Two-Lane Segment Two-lane Undivided	399+00. 000	405+75. 410	675.4 1	0.127 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	70
84	Rural Two-Lane Segment Two-lane Undivided	405+75. 410	406+00. 000	24.59	0.004 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.72	6.2	3	false	0	false	false	false				
85	Rural Two-Lane Segment Two-lane Undivided	406+00. 000	407+00. 000	100.0	0.018 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	8.00	0.72	6.2	3	false	0	false	false	false				
86	Rural Two-Lane Segment Two-lane Undivided	407+00. 000	443+25. 000	3,625. 00	0.686 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.72	6.2	3	false	0	false	false	false				
87	Rural Two-Lane Segment Two-lane Undivided	443+25. 000	445+50. 000	225.0 0	0.042 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 0.96	6.2	3	false	0	false	false	false				
88	Rural Two-Lane Segment Two-lane Undivided	445+50. 000	452+50. 000	700.0 0	0.132 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,096; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 0.96	6.2	3	false	0	false	false	false				
89	Rural Two-Lane Segment Two-lane Undivided	452+50. 000	459+00. 000	650.0 0	0.123 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
90	Rural Two-Lane Segment Two-lane Undivided	459+00. 000	460+00. 000	100.0	0.018 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	0.00	0.96	6.2	3	false	0	false	false	false				
91	Rural Two-Lane Segment Two-lane Undivided	460+00. 000	460+58. 580	58.58	0.011 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
92	Rural Two-Lane Segment Two-lane Undivided	460+58. 580	485+61. 230	2,502. 65		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.01	6.2	3	false	0	false	false	false				
93	Rural Two-Lane Segment Two-lane Undivided	485+61. 230	503+00. 000	1,738. 77	0.329 3	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.07	6.2	3	false	0	false	false	false				
94	Rural Two-Lane Segment Two-lane Undivided	503+00. 000	507+00. 000		0.075 8	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.07	6.2	3	false	0	false	false	false				
95	Rural Two-Lane Segment Two-lane Undivided	507+00. 000	508+00. 000		0.018 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.07	6.2	3	false	0	false	true	false				
96	Rural Two-Lane Segment Two-lane Undivided	508+00. 000	508+08. 240	8.24	0.001 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.07	6.2	3	false	0	false	true	false				
97	Rural Two-Lane Segment Two-lane Undivided	508+08. 240	510+30. 000	221.7 6	0.042 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.21	6.2	3	false	0	false	true	false				
98	Rural Two-Lane Segment Two-lane Undivided	510+30. 000	512+00. 000	170.0 0	0.032	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.21	6.2	3	false	0	false	false	false				
99	Rural Two-Lane Segment Two-lane Undivided	512+00. 000	513+00. 000	100.0 0	0.018 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.21	6.2	3	false	0	false	true	false				
100	Rural Two-Lane Segment Two-lane Undivided	513+00. 000	515+00. 000		0.037 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.21	6.2	3	false	0	false	true	false				
101	Rural Two-Lane Segment Two-lane Undivided	515+00. 000	520+00. 000	500.0 0	0.094 7	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.21	6.2	3	false	0	true	true	false				
102	Rural Two-Lane Segment Two-lane Undivided	520+00. 000	520+49. 150	49.15	0.009	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.21	6.2	3	false	0	false	true	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
103	Rural Two-Lane Segment Two-lane Undivided	520+49. 150	521+00. 000	50.85	0.009 6	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0	0.00	0.00	0.21	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
104	Rural Two-Lane Segment Two-lane Undivided	521+00. 000	523+38. 600	238.6 0	0.045 2	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.21	6.2	3	false	0	true	true	false	2,458. 49	2.0	true	e 45
105	Rural Two-Lane Segment Two-lane Undivided	523+38. 600	524+00. 000	61.40	0.011 6	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	1.90	6.2	3	false	0	true	true	false	2,458. 49	2.0	true	е 45
106	Rural Two-Lane Segment Two-lane Undivided	524+00. 000			0.018 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	1.90	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
107	Rural Two-Lane Segment Two-lane Undivided	525+00. 000	525+18. 580	18.58	0.003 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	1.90	6.2	3	false	0	true	true	false	2,458. 49	2.0	true	e 45
108	Rural Two-Lane Segment Two-lane Undivided	525+18. 580	528+00. 000		0.053 3	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	true	true	false	2,458. 49	2.0	true	е 45
109	Rural Two-Lane Segment Two-lane Undivided	528+00. 000	529+00. 000	100.0 0	0.018 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.02	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
110	Rural Two-Lane Segment Two-lane Undivided	529+00. 000	539+00. 000	1,000. 00	0.189 4	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	true	true	false	2,458. 49	2.0	true	e 45
111	Rural Two-Lane Segment Two-lane Undivided	539+00. 000	539+50. 000	50.00	0.009 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
112	Rural Two-Lane Segment Two-lane Undivided	539+50. 000	540+00. 000	50.00	0.009 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
113	Rural Two-Lane Segment Two-lane Undivided	540+00. 000	540+50. 000	50.00	0.009 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
114	Rural Two-Lane Segment Two-lane Undivided	540+50. 000	540+74. 370	24.37	0.004 6	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0 0	0.00	0.00	0.96	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	y Density	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
115	Rural Two-Lane Segment Two-lane Undivided	540+74. 370	541+00. 000	25.63	0.004 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0	0.00	0.00	0.96	6.2	3	false	0	false	true	false				
116	Rural Two-Lane Segment Two-lane Undivided	541+00. 000	541+50. 000	50.00	0.009 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.96	6.2	3	false	0	false	true	false				
117	Rural Two-Lane Segment Two-lane Undivided	541+50. 000		20.00	0.003	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015: 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.96	6.2	3	false	0	false	true	false				
118	Rural Two-Lane Segment Two-lane Undivided	541+70. 000	542+30. 000	60.00	0.011 4	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015: 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.96	6.2	3	false	0	false	true	false				
119	Rural Two-Lane Segment Two-lane Undivided	542+30. 000	542+64. 000	34.00	0.006 4	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015: 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	true	false				
120	Rural Two-Lane Segment Two-lane Undivided	542+64. 000	543+34. 000	70.00	0.013 3	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	true	false				
121	Rural Two-Lane Segment Two-lane Undivided	543+34. 000	544+00. 000	66.00	0.012 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015: 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	true	false				
122	Rural Two-Lane Segment Two-lane Undivided	544+00. 000			0.018 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
123	Rural Two-Lane Segment Two-lane Undivided	545+00. 000	548+23. 000		0.061	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	- 0.96	6.2	3	false	0	false	false	false				
124	Rural Two-Lane Segment Two-lane Undivided	548+23. 000			0.103 6	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
125	Rural Two-Lane Segment Two-lane Undivided	553+70. 000		30.00	0.005 7	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.96	6.2	3	false	0	false	false	false				
126	Rural Two-Lane Segment Two-lane Undivided	554+00. 000	554+20. 000	20.00	0.003	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0	0.00	0.00	0.96	6.2	3	false	0	false	false	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
127	Rural Two-Lane Segment Two-lane Undivided	554+20. 000	560+00. 000	580.0 0	0.109	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
128	Rural Two-Lane Segment Two-lane Undivided	560+00. 000	562+58. 560	258.5 6	0.049	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
129	Rural Two-Lane Segment Two-lane Undivided	562+58. 560	564+00. 000	141.4 4	0.026	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				
130	Rural Two-Lane Segment Two-lane Undivided	564+00. 000	565+00. 000	100.0 0	0.018	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,833; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				
131	Rural Two-Lane Segment Two-lane Undivided	565+00. 000	565+77. 000	77.00	0.014 6	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				
132	Rural Two-Lane Segment Two-lane Undivided	565+77. 000	566+10. 000	33.00	0.006	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	0.00	0.00	0.20	6.2	3	false	0	false	false	false				
133	Rural Two-Lane Segment Two-lane Undivided	566+10. 000	566+50. 000	40.00	0.007 6	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,771; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	0.00	0.00	0.20	6.2	3	false	0	false	false	false				
134	Rural Two-Lane Segment Two-lane Undivided	566+50. 000	569+37. 000	287.0 0	0.054 4	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				
135	Rural Two-Lane Segment Two-lane Undivided	569+37. 000	569+70. 000	33.00	0.006	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,3394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	8.00	0.00	0.20	6.2	3	false	0	false	false	false				
136	Rural Two-Lane Segment Two-lane Undivided	569+70. 000	570+00. 000	30.00	0.005	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,3394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				
137	Rural Two-Lane Segment Two-lane Undivided	570+00. 000	575+00. 000	500.0 0	0.094	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,3394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	true	false	false				
138	Rural Two-Lane Segment Two-lane Undivided	575+00. 000	579+50. 000	450.0 0	0.085	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,3394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				

Se No	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)		AADT	Lan e Wid th	Lan	Left Shoulde r Width (ft)	r Width		Drivewa y Density (drivewa ys/mi)	rd	Centerline	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt		Adver se	Desig n Spee d (mph
13	Rural Two-Lane Segment Two-lane Undivided	579+50. 000	579+70. 000	20.00	0.003	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0	12.0	0.00	0.00	0.20	6.2	3	false	0	false	false	false			
14	Rural Two-Lane Segment Two-lane Undivided	579+70. 000	580+10. 000		0.007	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0	12.0 0	0.00	0.00	0.20	6.2	3	false	0	false	false	false			

**Table 3. Crash History Highway - Homogeneous Segments (Section 1)** 

wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided	(Sta. ft)  171+44.00 0 172+42.00 0 174+52.69 0 176+25.00 0 178+85.25 0 183+75.37 0 184+00.00 0 184+45.00 185+20.00	(Sta. ft)  172+42.00 0  174+52.69 0  176+25.00 0  178+85.25 0  183+75.37 0  184+00.00 0  184+45.00 0	98.00 210.69 172.31 260.25 490.12	0.0399 0.0326 0.0493	2019-2022: 2,085; 2023: 2,134 2019-2022: 2,085; 2023: 2,134 2019-2022: 2,085; 2023: 2,134 2019-2022: 2,085; 2023: 2,134 2019-2022: 2,085; 2023: 2,134	Widt h (ft) 12.00 12.00 12.00 12.00	12.00	8.00	8.00 8.00	4.25 0.17	(driveways/ mi) 6.2	Rating 3	Strip false	Lanes 0	false	false	<b>Enforcement</b> false	(ft)	(%)		(mph)
Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane	0 172+42.00 0 174+52.69 0 176+25.00 0 178+85.25 0 183+75.37 0 184+00.00 0 184+45.00	0 174+52.69 0 176+25.00 0 178+85.25 0 183+75.37 0 184+00.00 0	210.69 172.31 260.25 490.12	0.0399 0.0326 0.0493	2019-2022: 2,085; 2023: 2,134 2019-2022: 2,085; 2023: 2,134 2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00					false	0	false	false	false				
Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided	0 174+52.69 0 176+25.00 0 178+85.25 0 183+75.37 0 184+00.00 0	0 176+25.00 0 178+85.25 0 183+75.37 0 184+00.00 0	172.31 260.25 490.12	0.0326	2019-2022: 2,085; 2023: 2,134 2019-2022: 2,085; 2023: 2,134	12.00			8.00	0.17	6.2	,									L
Undivided wo-Lane Segment Two-lane Undivided vo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided vo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided	0 176+25.00 0 178+85.25 0 183+75.37 0 184+00.00 0	0 178+85.25 0 183+75.37 0 184+00.00 0 184+45.00	260.25 490.12	0.0493	2019-2022: 2,085; 2023: 2,134		12.00	8.00				3	false	0	false	false	false				
Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided vo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided	0 178+85.25 0 183+75.37 0 184+00.00 0 184+45.00	0 183+75.37 0 184+00.00 0 184+45.00 0	490.12			12.00	1		8.00	0.17	6.2	3	false	0	false	false	false	2,074.80	2.0	true	40
Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided vo-Lane Segment Two-lane Undivided	0 183+75.37 0 184+00.00 0 184+45.00	0 184+00.00 0 184+45.00 0		0.0928	2010 2022 2 005 2022 2 124		12.00	8.00	8.00	-1.88	6.2	3	false	0	false	false	false	2,074.80	2.0	true	40
Undivided wo-Lane Segment Two-lane Undivided vo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided	0 184+00.00 0 184+45.00 0	0 184+45.00 0	24.63		2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.88	6.2	3	false	0	false	false	false				
Undivided wo-Lane Segment Two-lane Undivided wo-Lane Segment Two-lane Undivided	0 184+45.00 0	0		0.0047	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
Undivided wo-Lane Segment Two-lane Undivided	0		45.00	0.0085	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
Undivided	185   20.00	185+20.00 0	75.00	0.0142	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
vo-Lane Segment Two-lane	185+20.00	186+60.00 0	140.00	0.0265	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
Undivided	186+60.00 0	187+20.00 0	60.00	0.0114	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
wo-Lane Segment Two-lane Undivided	187+20.00 0	187+60.00 0	40.00	0.0076	2019-2022: 2,085; 2023: 2,134	12.00	12.00	0.00	0.00	1.13	6.2	3	false	0	false	false	false				
wo-Lane Segment Two-lane Undivided	187+60.00 0	190+00.00 0	240.00	0.0455	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
wo-Lane Segment Two-lane Undivided	190+00.00 0	192+00.00 0	200.00	0.0379	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
wo-Lane Segment Two-lane Undivided	192+00.00 0	192+39.27 0	39.27	0.0074	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
wo-Lane Segment Two-lane Undivided	192+39.27 0	193+60.00 0	120.73	0.0229	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.94	6.2	3	false	0	false	false	false				
wo-Lane Segment Two-lane Undivided	193+60.00 0	197+65.00 0	405.00	0.0767	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.94	6.2	3	false	0	false	false	false				
wo-Lane Segment Two-lane Undivided	197+65.00 0	199+00.00 0	135.00	0.0256	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.94	6.2	3	false	0	false	false	false				
wo-Lane Segment Two-lane Undivided	199+00.00 0	201+63.75 0	263.75	0.0500	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.94	6.2	3	false	0	false	false	false				
wo-Lane Segment Two-lane Undivided	201+63.75 0	202+00.00 0	36.25	0.0069	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
wo-Lane Segment Two-lane Undivided	202+00.00 0	207+00.00 0	500.00	0.0947	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
wo-Lane Segment Two-lane Undivided	207+00.00 0	207+49.76 0	49.76	0.0094	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
vo-Lane Segment Two-lane	207+49.76 0	217+74.25 0	1,024.4 9	0.1940	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.70	6.2	3	false	0	false	false	false				
Undivided	217+74.25 0	221+00.00 0	325.75	0.0617	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
Undivided wo-Lane Segment Two-lane Undivided	221 00 00	226+00.00 0	500.00	0.0947	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
wo-Lane Segment Two-lane	221+00.00						12.00														
	Undivided  ro-Lane Segment Two-lane Undivided  ro-Lane Segment Two-lane Undivided  ro-Lane Segment Two-lane Undivided  ro-Lane Segment Two-lane Undivided  ro-Lane Segment Two-lane Undivided  ro-Lane Segment Two-lane Undivided  ro-Lane Segment Two-lane Undivided  ro-Lane Segment Two-lane Undivided	Undivided 0  ro-Lane Segment Two-lane Undivided 0  ro-Lane Segment Two-lane Undivided 0  ro-Lane Segment Two-lane Undivided 0  ro-Lane Segment Two-lane Undivided 0  ro-Lane Segment Two-lane Undivided 0  ro-Lane Segment Two-lane Undivided 0  ro-Lane Segment Two-lane Undivided 0  ro-Lane Segment Two-lane Undivided 0  ro-Lane Segment Two-lane Undivided 0  ro-Lane Segment Two-lane Undivided 0  ro-Lane Segment Two-lane Undivided 0  ro-Lane Segment Two-lane Undivided 0  ro-Lane Segment Two-lane Undivided 0  ro-Lane Segment Two-lane Undivided 0  ro-Lane Segment Two-lane 221+00.00	Undivided 0 0 0 ro-Lane Segment Two-lane Undivided 0 199+00.00 0 ro-Lane Segment Two-lane Undivided 0 0 0 0 ro-Lane Segment Two-lane Undivided 0 0 0 0 ro-Lane Segment Two-lane Undivided 0 0 0 ro-Lane Segment Two-lane Undivided 0 0 0 ro-Lane Segment Two-lane Undivided 0 0 0 ro-Lane Segment Two-lane Undivided 0 0 0 ro-Lane Segment Two-lane Undivided 0 0 0 0 ro-Lane Segment Two-lane Undivided 0 0 0 0 0 ro-Lane Segment Two-lane Undivided 0 0 0 0 0 ro-Lane Segment Two-lane Undivided 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Undivided 0 0 405.00  ro-Lane Segment Two-lane Undivided 0 0 135.00  ro-Lane Segment Two-lane Undivided 0 0 0 36.25  ro-Lane Segment Two-lane Undivided 0 0 0 36.25  ro-Lane Segment Two-lane Undivided 0 0 0 0 36.25  ro-Lane Segment Two-lane Undivided 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Undivided 0 0 405.00 0.0767  To-Lane Segment Two-lane Undivided 199+00.00 0 135.00 0.0256  To-Lane Segment Two-lane Undivided 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Undivided 0 0 405.00 0.0767 2019-2022: 2,085; 2023: 2,134 0-Lane Segment Two-lane Undivided 190-Lane Segment Two-lane 221+00.00 226+00.00 0 0 0 0 0 0 0 0 0 0 0 0.0947 2019-2022: 2,085; 2023: 2,134 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Undivided 0 0 405.00 0.0767 2019-2022: 2,085; 2023: 2,134 12.00   ro-Lane Segment Two-lane Undivided 199+00.00 0 135.00 0.0256 2019-2022: 2,085; 2023: 2,134 12.00   ro-Lane Segment Two-lane Undivided 201+63.75 0 202+00.00 0 0 36.25 0.0500 2019-2022: 2,085; 2023: 2,134 12.00   ro-Lane Segment Two-lane Undivided 202+00.00 0 0 36.25 0.0069 2019-2022: 2,085; 2023: 2,134 12.00   ro-Lane Segment Two-lane Undivided 207+00.00 0 500.00 0.0947 2019-2022: 2,085; 2023: 2,134 12.00   ro-Lane Segment Two-lane Undivided 207+49.76 0 0 49.76 0.0094 2019-2022: 2,085; 2023: 2,134 12.00   ro-Lane Segment Two-lane Undivided 207+49.76 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Undivided 0 0 405.00 0.0767 2019-2022: 2,085; 2023: 2,134 12.00 12	Undivided 0 0 405.00 0.0767 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00   70-Lane Segment Two-lane Undivided 199+00.00 0 135.00 0.0256 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00   70-Lane Segment Two-lane Undivided 201+63.75 0 202+00.00 0 0 36.25 0.0500 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00   70-Lane Segment Two-lane Undivided 202+00.00 0 0 36.25 0.0069 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00   70-Lane Segment Two-lane Undivided 202+00.00 0 207+00.00 0 500.00 0.0947 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00   70-Lane Segment Two-lane Undivided 207+00.00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Undivided 0 0 405.00 0.0767 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 00-Lane Segment Two-lane Undivided 1 199+00.00 0 135.00 0.0256 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 00-Lane Segment Two-lane Undivided 2 0 0 0 36.25 0.0069 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 00-Lane Segment Two-lane Undivided 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Undivided 0 0 405.00 0.0767 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -0.94   Pro-Lane Segment Two-lane Undivided 197+65.00 0 199+00.00 0 135.00 0.0256 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -1.94   Pro-Lane Segment Two-lane Undivided 201+63.75 0 202+00.00 0 0 36.25 0.0500 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -1.94   Pro-Lane Segment Two-lane Undivided 201+63.75 0 202+00.00 0 0 36.25 0.0069 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 0.13   Pro-Lane Segment Two-lane Undivided 202+00.00 0 207+49.76 0 0 0 0 0 0 0 0 0 0 0 0 0.0947 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 0.13   Pro-Lane Segment Two-lane Undivided 207+49.76 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Undivided 0 0 405.00 0.0767 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 -0.94 6.2   70-Lane Segment Two-lane Undivided 197+65.00 0 135.00 0.0256 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 -1.94 6.2   70-Lane Segment Two-lane Undivided 201+63.75 0 0.0500 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 -1.94 6.2   70-Lane Segment Two-lane Undivided 202+00.00 0 0 36.25 0.0069 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 0.13 6.2   70-Lane Segment Two-lane Undivided 202+00.00 0 0 0.0947 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 0.13 6.2   70-Lane Segment Two-lane Undivided 207+00.00 0 0 0.0947 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 0.13 6.2   70-Lane Segment Two-lane Undivided 207+00.00 0 0 0.0947 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 0.13 6.2   70-Lane Segment Two-lane Undivided 207+49.76 0 0.094 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 0.13 6.2   70-Lane Segment Two-lane Undivided 217+74.25 0 0.094 0.1940 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 0.13 6.2   70-Lane Segment Two-lane Undivided 217+74.25 0 0.094 0.1940 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 0.13 6.2   70-Lane Segment Two-lane Undivided 217+74.25 0.004 0 0.004 0.004 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 0.77 6.2   70-Lane Segment Two-lane Undivided 217+74.25 0.004 0 0.004 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 0.77 6.2   70-Lane Segment Two-lane Undivided 217+74.25 0.004 0 0.004 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 0.77 6.2   70-Lane Segment Two-lane Undivided 217+74.25 0.004 0 0.004 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 0.77 6.2   70-Lane Segment Two-lane Undivided 0 0 0.004 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 0.77 6.2   70-Lane Segment Two-lane 217+74.25 0.004 0 0.004 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 0.77 6.2   70-Lane Segment Two-lane 217+0.000 0 0.004 2019-2022: 2.085; 2023: 2,134 12.00 12.00 8.00 8.00 0.77 6.2   70-Lane Segment Two-lane 217+0.000 0 0.004 2019	Undivided 0 0 0 405.00 0.0767 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -0.94 6.2 3 ro-Lane Segment Two-lane Undivided 199+00.00 0 135.00 0.0256 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -1.94 6.2 3 ro-Lane Segment Two-lane Undivided 2014-63.75 0 36.25 0.069 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -1.94 6.2 3 ro-Lane Segment Two-lane Undivided 2024-00.00 0 36.25 0.069 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 0.13 6.2 3 ro-Lane Segment Two-lane Undivided 2024-00.00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Undivided 0 0 405.00 0.0767 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -0.94 6.2 3 false ro-Lane Segment Two-lane Undivided 1 197+65.00 0 135.00 0.0256 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -1.94 6.2 3 false ro-Lane Segment Two-lane Undivided 2 199+00.00 0 36.25 0.0500 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -1.94 6.2 3 false ro-Lane Segment Two-lane Undivided 3 202+00.00 0 36.25 0.069 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 0.13 6.2 3 false ro-Lane Segment Two-lane Undivided 2 202+00.00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Undivided 0 0 405.00 0.0767 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -0.94 6.2 3 false 0 0 100-Lane Segment Two-lane Undivided 1 199+00.00 0 135.00 0.0256 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -1.94 6.2 3 false 0 0 100-Lane Segment Two-lane Undivided 2 101+63.75 0 0 36.25 0.069 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 0.13 6.2 3 false 0 0 100-Lane Segment Two-lane Undivided 2 101+63.75 0 0 36.25 0.069 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 0.13 6.2 3 false 0 0 100-Lane Segment Two-lane Undivided 2 117-74.25 1.024.4 9 0.1940 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 8.00 0.13 6.2 3 false 0 100-Lane Segment Two-lane Undivided 2 117-74.25 1.024.4 9 0.1940 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 8.00 0.77 6.2 3 false 0 100-Lane Segment Two-lane Undivided 2 117-74.25 1.024.4 9 0.1940 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 0.77 6.2 3 false 0 100-Lane Segment Two-lane Undivided 1 100-Lane Segment Two-lane 100-Lane Segment Two-Lane 100-Lane Segment Two-Lane 100-Lane Segment Two-Lane 100-Lane Segment Two-Lane 100-Lane Segment Two-Lane 100-Lane Segment Two-Lane 100-Lane Segment Two-Lane 100-Lane Segment Two-Lane 100-Lane Segment Two-Lane 100-Lane Segment Two-Lane 100-Lane Segment Two-Lane 100-Lane Segment Two-Lane 100-Lane Segment	Undivided 0 0 405.00 0.0767 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -0.94 6.2 3 false 0 false obtained 197-65.00 199+00.00 135.00 0.0256 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -1.94 6.2 3 false 0 false obtained 199+00.00 135.00 0.0256 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -1.94 6.2 3 false 0 false obtained 199+00.00 201+63.75 0 263.75 0.050 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -1.94 6.2 3 false 0 false obtained 199+00.00 201+63.75 202+00.00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Undivided 0 0 405.00 0.0767 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -0.94 6.2 3 false 0 false false ro-Lane Segment Two-lane Undivided 199+00.00 0 135.00 0.0256 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -1.94 6.2 3 false 0 false false ro-Lane Segment Two-lane Undivided 2014-63.75 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Undivided 0 0 0 405.00 0.0767 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -0.94 6.2 3 false 0 false false false obtained false false false obtained false false obtained false false false obtained false false obtained false false false obtained false false false obtained false false false obtained false false false obtained false false false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false false obtained false false obtained false false obtained false false obtained false false obtained false false false obtained false false false obtained false false false obtained false false false obtained false false obtained false false false obtained false false false obtained false false false obtained false false false obtained fal	Undivided 0 0 0 405.00 0.0767 2019-2022: 2,085; 2023: 2,134 12.00 12.00 8.00 8.00 -0.94 6.2 3 false 0 false false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false obtained false false false obtained false false obtained false false false false obtained false false false obtained false false false obtained false false false false false obtained false false false false obtained false false false false obtained false false false false false obtained false false false false obtained false false false false false obtained false false false false false obtained false obtained false false false false false false false obtained false false false false false obtained false false false false obtained false false false obtained false false false obtained false false false obtained false false false obtained false false false false obtained false false false false obtained false false false false obtained false false false false obtained false false false false false obtained false false false false obtained false false false false false false false obtained false false false false false false false obtained false	Undivided 0 0 405.00 0.0767 2019-2022: 2.085; 2023: 2.134 12.00 12.00 8.00 8.00 -0.94 6.2 3 false 0 false false false 0 Undivided 0 197-65.00 199+00.00 135.00 0.0256 2019-2022: 2.085; 2023: 2.134 12.00 12.00 8.00 8.00 -1.94 6.2 3 false 0 false false false 0 Undivided 0 199+00.00 135.00 0.0256 2019-2022: 2.085; 2023: 2.134 12.00 12.00 8.00 8.00 -1.94 6.2 3 false 0 false false false 0 199+00.00 10 140 140 140 140 140 140 140 140 14	Undivided

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazar d Rating	Centerline Rumble Strip	Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
26	Rural Two-Lane Segment Two-lane Undivided	230+66.25 0	231+39.70 0	73.45	0.0139	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.77	6.2	3	false	0	false	false	false	5,644.64	2.0	true	70
27	Rural Two-Lane Segment Two-lane Undivided	231+39.70 0	235+00.00 0	360.30	0.0682	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-2.00	6.2	3	false	0	false	false	false	5,644.64	2.0	true	70
28	Rural Two-Lane Segment Two-lane Undivided	235+00.00 0	241+61.39 0	661.39	0.1253	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-2.00	6.2	3	false	0	false	false	false	5,644.64	2.0	true	70
29	Rural Two-Lane Segment Two-lane Undivided	241+61.39 0	242+00.00 0	38.61	0.0073	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	1.16	6.2	3	false	0	false	false	false	5,644.64	2.0	true	70
30	Rural Two-Lane Segment Two-lane Undivided	242+00.00 0	245+14.28 0	314.28	0.0595	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	1.16	6.2	3	false	0	false	false	false	5,644.64	2.0	true	70
31	Rural Two-Lane Segment Two-lane Undivided	245+14.28 0	246+55.10 0	140.82	0.0267	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	1.16	6.2	3	false	0	false	false	false				
32	Rural Two-Lane Segment Two-lane Undivided	246+55.10 0	248+00.00 0	144.90	0.0274	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.42	6.2	3	false	0	false	false	false				
33	Rural Two-Lane Segment Two-lane Undivided	248+00.00 0	249+00.00 0	100.00	0.0189	2019-2022: 2,085; 2023: 2,134	12.00	12.00	0.00	0.00	-0.42	6.2	3	false	0	false	false	false				
34	Rural Two-Lane Segment Two-lane Undivided	249+00.00 0	251+21.98 0	221.98	0.0420	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.42	6.2	3	false	0	false	false	false				
35	Rural Two-Lane Segment Two-lane Undivided	251+21.98 0	252+40.24 0	118.26	0.0224	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	3.43	6.2	3	false	0	false	false	false				
36	Rural Two-Lane Segment Two-lane Undivided	252+40.24 0	263+22.60 0	1,082.3 6	0.2050	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
37	Rural Two-Lane Segment Two-lane Undivided	263+22.60 0	272+66.74 0	944.14	0.1788	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.02	6.2	3	false	0	false	false	false	12,237.0 0	2.0	true	70
38	Rural Two-Lane Segment Two-lane Undivided	272+66.74 0	280+00.00 0	733.26	0.1389	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
39	Rural Two-Lane Segment Two-lane Undivided	280+00.00 0	283+15.05 0	315.05	0.0597	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
40	Rural Two-Lane Segment Two-lane Undivided	283+15.05 0	284+08.54 0	93.49	0.0177	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	4.47	6.2	3	false	0	false	false	false				
41	Rural Two-Lane Segment Two-lane Undivided	284+08.54 0	288+50.00 0	441.46	0.0836	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.47	6.2	3	false	0	false	false	false				
42	Rural Two-Lane Segment Two-lane Undivided	288+50.00 0	289+00.00 0	50.00	0.0095	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.47	6.2	3	false	0	false	false	false				
43	Rural Two-Lane Segment Two-lane Undivided	289+00.00 0	295+90.00 0	690.00	0.1307	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.47	6.2	3	false	0	false	false	false				
44	Rural Two-Lane Segment Two-lane Undivided	295+90.00 0	296+00.00 0	10.00	0.0019	2019-2022: 2,085; 2023: 2,134	12.00	12.00	0.00	0.00	-1.47	6.2	3	false	0	false	false	false				
45	Rural Two-Lane Segment Two-lane Undivided	296+00.00 0	296+10.00 0	10.00	0.0019	2019-2022: 2,085; 2023: 2,134	12.00	12.00	0.00	0.00	-1.47	6.2	3	false	0	false	false	false				
46	Rural Two-Lane Segment Two-lane Undivided	296+10.00 0	296+96.52 0	86.52	0.0164	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.47	6.2	3	false	0	false	false	false				
47	Rural Two-Lane Segment Two-lane Undivided	296+96.52 0	298+33.66 0	137.14	0.0260	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.47	6.2	3	false	0	false	false	false				
48	Rural Two-Lane Segment Two-lane Undivided	298+33.66 0	303+50.00 0	516.34	0.0978	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.61	6.2	3	false	0	false	false	false				
49	Rural Two-Lane Segment Two-lane Undivided	303+50.00 0	304+50.00 0	100.00	0.0189	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.61	6.2	3	false	0	false	false	false				
50	Rural Two-Lane Segment Two-lane Undivided	304+50.00 0	305+02.03 9	52.04	0.0099	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.61	6.2	3	false	0	false	false	false				
51	Rural Two-Lane Segment Two-lane Undivided	305+02.03 9	309+35.49 0	433.45	0.0821	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.15	6.2	3	false	0	false	false	false				
52	Rural Two-Lane Segment Two-lane Undivided	309+35.49 0	311+70.00 0	234.51	0.0444	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	1.24	6.2	3	false	0	false	false	false				

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazar d Rating	Centerline Rumble Strip	Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
53	Rural Two-Lane Segment Two-lane Undivided	311+70.00 0	313+25.00 0	155.00	0.0294	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	1.24	6.2	3	false	0	false	false	false				
54	Rural Two-Lane Segment Two-lane Undivided	313+25.00 0	323+00.00 0	975.00	0.1847	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.33	6.2	3	false	0	false	false	false				
55	Rural Two-Lane Segment Two-lane Undivided	323+00.00 0	323+26.98 0	26.98	0.0051	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.33	6.2	3	false	0	false	false	false				
56	Rural Two-Lane Segment Two-lane Undivided	323+26.98 0	328+89.23 0	562.25	0.1065	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.26	6.2	3	false	0	false	false	false				
57	Rural Two-Lane Segment Two-lane Undivided	328+89.23 0	329+81.74 0	92.51	0.0175	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.52	6.2	3	false	0	false	false	false				
58	Rural Two-Lane Segment Two-lane Undivided	329+81.74 0	333+24.92 0	343.18	0.0650	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.52	6.2	3	false	0	false	false	false	4,010.13	2.0	true	70
59	Rural Two-Lane Segment Two-lane Undivided	333+24.92 0	334+00.00 0	75.08	0.0142	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-2.17	6.2	3	false	0	false	false	false	4,010.13	2.0	true	70
60	Rural Two-Lane Segment Two-lane Undivided	334+00.00 0	335+39.96 0	139.96	0.0265	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-2.17	6.2	3	false	0	false	false	false	4,010.13	2.0	true	70
61	Rural Two-Lane Segment Two-lane Undivided	335+39.96 0	342+39.00 0	699.04	0.1324	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-2.17	6.2	3	false	0	false	false	false				
62	Rural Two-Lane Segment Two-lane Undivided	342+39.00 0	343+00.00 0	61.00	0.0116	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false				
63	Rural Two-Lane Segment Two-lane Undivided	343+00.00 0	351+20.00 0	820.00	0.1553	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false				
64	Rural Two-Lane Segment Two-lane Undivided	351+20.00 0	352+00.00 0	80.00	0.0152	2019-2022: 2,085; 2023: 2,134	12.00	12.00	0.00	0.00	-0.24	6.2	3	false	0	false	false	false				
65	Rural Two-Lane Segment Two-lane Undivided	352+00.00 0	352+20.00 0	20.00	0.0038	2019-2022: 2,085; 2023: 2,134	12.00	12.00	0.00	0.00	-0.24	6.2	3	false	0	false	false	false				
66	Rural Two-Lane Segment Two-lane Undivided	352+20.00 0	362+50.00 0	1,030.0 0	0.1951	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false				
67	Rural Two-Lane Segment Two-lane Undivided	362+50.00 0	369+14.99 0	664.99	0.1259	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false				
68	Rural Two-Lane Segment Two-lane Undivided	369+14.99 0	370+30.00 0	115.01	0.0218	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
69	Rural Two-Lane Segment Two-lane Undivided	370+30.00 0	370+60.00 0	30.00	0.0057	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
70	Rural Two-Lane Segment Two-lane Undivided	370+60.00 0	376+83.61 0	623.61	0.1181	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
71	Rural Two-Lane Segment Two-lane Undivided	376+83.61 0	378+00.00 0	116.39	0.0220	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
72	Rural Two-Lane Segment Two-lane Undivided	378+00.00 0	378+40.00 0	40.00	0.0076	2019-2022: 2,085; 2023: 2,134	12.00	12.00	0.00	8.00	-1.04	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
73	Rural Two-Lane Segment Two-lane Undivided	378+40.00 0	378+60.00 0	20.00	0.0038	2019-2022: 2,085; 2023: 2,134	12.00	12.00	0.00	8.00	-1.04	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
74	Rural Two-Lane Segment Two-lane Undivided	378+60.00 0	379+00.00 0	40.00	0.0076	2019-2022: 2,085; 2023: 2,134	12.00	12.00	0.00	8.00	-1.04	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
75	Rural Two-Lane Segment Two-lane Undivided	379+00.00 0	379+62.69 0	62.69	0.0119	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
76	Rural Two-Lane Segment Two-lane Undivided	379+62.69 0	385+22.97 0	560.28	0.1061	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false				
77	Rural Two-Lane Segment Two-lane Undivided	385+22.97 0	386+60.00 0	137.03	0.0260	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
78	Rural Two-Lane Segment Two-lane Undivided	386+60.00 0	389+50.00 0	290.00	0.0549	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
79	Rural Two-Lane Segment Two-lane Undivided	389+50.00 0	394+00.00 0	450.00	0.0852	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazar d Rating	Centerline Rumble Strip	Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
80	Rural Two-Lane Segment Two-lane Undivided	394+00.00 0	396+46.15 0	246.15	0.0466	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
81	Rural Two-Lane Segment Two-lane Undivided	396+46.15 0	397+00.00 0	53.85	0.0102	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
82	Rural Two-Lane Segment Two-lane Undivided	397+00.00 0	399+00.00 0	200.00	0.0379	2019-2022: 2,085; 2023: 2,134	12.00	12.00	0.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
83	Rural Two-Lane Segment Two-lane Undivided	399+00.00 0	405+75.41 0	675.41	0.1279	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
84	Rural Two-Lane Segment Two-lane Undivided	405+75.41 0	406+00.00 0	24.59	0.0047	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.72	6.2	3	false	0	false	false	false				
85	Rural Two-Lane Segment Two-lane Undivided	406+00.00 0	407+00.00 0	100.00	0.0189	2019-2022: 2,085; 2023: 2,134	12.00	12.00	0.00	8.00	0.72	6.2	3	false	0	false	false	false				
86	Rural Two-Lane Segment Two-lane Undivided	407+00.00 0	443+25.00 0	3,625.0 0	0.6866	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.72	6.2	3	false	0	false	false	false				
87	Rural Two-Lane Segment Two-lane Undivided	443+25.00 0	445+50.00 0	225.00	0.0426	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
88	Rural Two-Lane Segment Two-lane Undivided	445+50.00 0	452+50.00 0	700.00	0.1326	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
89	Rural Two-Lane Segment Two-lane Undivided	452+50.00 0	459+00.00 0	650.00	0.1231	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
90	Rural Two-Lane Segment Two-lane Undivided	459+00.00 0	460+00.00 0	100.00	0.0189	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	0.00	-0.96	6.2	3	false	0	false	false	false				
91	Rural Two-Lane Segment Two-lane Undivided	460+00.00 0	460+58.58 0	58.58	0.0111	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
92	Rural Two-Lane Segment Two-lane Undivided	460+58.58 0	485+61.23 0	2,502.6 5	0.4740	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-0.01	6.2	3	false	0	false	false	false				
93	Rural Two-Lane Segment Two-lane Undivided	485+61.23 0	503+00.00 0	1,738.7 7	0.3293	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.07	6.2	3	false	0	false	false	false				
94	Rural Two-Lane Segment Two-lane Undivided	503+00.00 0	507+00.00 0	400.00	0.0758	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.07	6.2	3	false	0	false	false	false				
95	Rural Two-Lane Segment Two-lane Undivided	507+00.00 0	508+00.00 0	100.00	0.0189	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.07	6.2	3	false	0	false	true	false				
96	Rural Two-Lane Segment Two-lane Undivided	508+00.00 0	508+08.24 0	8.24	0.0016	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	-1.07	6.2	3	false	0	false	true	false				
97	Rural Two-Lane Segment Two-lane Undivided	508+08.24 0	510+30.00 0	221.76	0.0420	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.21	6.2	3	false	0	false	true	false				
98	Rural Two-Lane Segment Two-lane Undivided	510+30.00 0	512+00.00 0	170.00	0.0322	2019-2022: 2,085; 2023: 2,134	12.00	12.00	8.00	8.00	0.21	6.2	3	false	0	false	false	false				
99	Rural Two-Lane Segment Two-lane Undivided	512+00.00 0	513+00.00 0	100.00	0.0189	2019-2022: 4,325; 2023: 5,081	12.00	12.00	0.00	0.00	0.21	6.2	3	false	0	false	true	false				
100	Rural Two-Lane Segment Two-lane Undivided	513+00.00 0	515+00.00 0	200.00	0.0379	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	0.21	6.2	3	false	0	false	true	false				
101	Rural Two-Lane Segment Two-lane Undivided	515+00.00 0	520+00.00 0	500.00	0.0947	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	0.21	6.2	3	false	0	true	true	false				
102	Rural Two-Lane Segment Two-lane Undivided	520+00.00 0	520+49.15 0	49.15	0.0093	2019-2022: 4,325; 2023: 5,081	12.00	12.00	0.00	0.00	0.21	6.2	3	false	0	false	true	false				
103	Rural Two-Lane Segment Two-lane Undivided	520+49.15 0	521+00.00 0	50.85	0.0096	2019-2022: 4,325; 2023: 5,081	12.00	12.00	0.00	0.00	0.21	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
104	Rural Two-Lane Segment Two-lane Undivided	521+00.00 0	523+38.60 0	238.60	0.0452	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	0.21	6.2	3	false	0	true	true	false	2,458.49	2.0	true	45
105	Rural Two-Lane Segment Two-lane Undivided	523+38.60 0	524+00.00 0	61.40	0.0116	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	1.90	6.2	3	false	0	true	true	false	2,458.49	2.0	true	45
106	Rural Two-Lane Segment Two-lane Undivided	524+00.00 0	525+00.00 0	100.00	0.0189	2019-2022: 4,325; 2023: 5,081	12.00	12.00	0.00	0.00	1.90	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)		Left Shoulder Width (ft)	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazar d Rating	Centerline Rumble Strip	Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
107	Rural Two-Lane Segment Two-lane Undivided	525+00.00 0	525+18.58 0	18.58	0.0035	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	1.90	6.2	3	false	0	true	true	false	2,458.49	2.0	true	45
108	Rural Two-Lane Segment Two-lane Undivided	525+18.58 0	528+00.00 0	281.42	0.0533	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	-0.02	6.2	3	false	0	true	true	false	2,458.49	2.0	true	45
109	Rural Two-Lane Segment Two-lane Undivided	528+00.00 0	529+00.00 0	100.00	0.0189	2019-2022: 4,325; 2023: 5,081	12.00	12.00	0.00	0.00	-0.02	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
110	Rural Two-Lane Segment Two-lane Undivided	529+00.00 0	539+00.00 0	1,000.0 0	0.1894	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	-0.02	6.2	3	false	0	true	true	false	2,458.49	2.0	true	45
111	Rural Two-Lane Segment Two-lane Undivided	539+00.00 0	539+50.00 0	50.00	0.0095	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	-0.02	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
112	Rural Two-Lane Segment Two-lane Undivided	539+50.00 0	540+00.00 0	50.00	0.0095	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
113	Rural Two-Lane Segment Two-lane Undivided	540+00.00 0	540+50.00 0	50.00	0.0095	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
114	Rural Two-Lane Segment Two-lane Undivided	540+50.00 0	540+74.37 0	24.37	0.0046	2019-2022: 4,325; 2023: 5,081	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
115	Rural Two-Lane Segment Two-lane Undivided	540+74.37 0	541+00.00 0	25.63	0.0049	2019-2022: 4,325; 2023: 5,081	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	true	false				
116	Rural Two-Lane Segment Two-lane Undivided	541+00.00 0	541+50.00 0	50.00	0.0095	2019-2022: 4,325; 2023: 5,081	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	true	false				
117	Rural Two-Lane Segment Two-lane Undivided	541+50.00 0	541+70.00 0	20.00	0.0038	2019-2022: 4,325; 2023: 5,081	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	true	false				
118	Rural Two-Lane Segment Two-lane Undivided	541+70.00 0	542+30.00 0	60.00	0.0114	2019-2022: 4,325; 2023: 5,081	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	true	false				
119	Rural Two-Lane Segment Two-lane Undivided	542+30.00 0	542+64.00 0	34.00	0.0064	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	true	false				
120	Rural Two-Lane Segment Two-lane Undivided	542+64.00 0	543+34.00 0	70.00	0.0133	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	true	false				
121	Rural Two-Lane Segment Two-lane Undivided	543+34.00 0	544+00.00 0	66.00	0.0125	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	true	false				
122	Rural Two-Lane Segment Two-lane Undivided	544+00.00 0	545+00.00 0	100.00	0.0189	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
123	Rural Two-Lane Segment Two-lane Undivided	545+00.00 0	548+23.00 0	323.00	0.0612	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
124	Rural Two-Lane Segment Two-lane Undivided	548+23.00 0	553+70.00 0	547.00	0.1036	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
125	Rural Two-Lane Segment Two-lane Undivided	553+70.00 0	554+00.00 0	30.00	0.0057	2019-2022: 4,325; 2023: 5,081	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	false	false				
126	Rural Two-Lane Segment Two-lane Undivided	554+00.00 0	554+20.00 0	20.00	0.0038	2019-2022: 4,325; 2023: 5,081	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	false	false				
127	Rural Two-Lane Segment Two-lane Undivided	554+20.00 0	560+00.00 0	580.00	0.1098	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
128	Rural Two-Lane Segment Two-lane Undivided	560+00.00 0	562+58.56 0	258.56	0.0490	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
129	Rural Two-Lane Segment Two-lane Undivided	562+58.56 0	564+00.00 0	141.44	0.0268	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false				
130	Rural Two-Lane Segment Two-lane Undivided	564+00.00 0	565+00.00 0	100.00	0.0189	2019-2022: 4,325; 2023: 5,081	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false				
131	Rural Two-Lane Segment Two-lane Undivided	565+00.00 0	565+77.00 0	77.00	0.0146	2019-2022: 4,325; 2023: 5,245	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false				
132	Rural Two-Lane Segment Two-lane Undivided	565+77.00 0	566+10.00 0	33.00	0.0063	2019-2022: 4,325; 2023: 5,245	12.00	12.00	0.00	0.00	-0.20	6.2	3	false	0	false	false	false				
133	Rural Two-Lane Segment Two-lane Undivided	566+10.00 0	566+50.00 0	40.00	0.0076	2019-2022: 4,325; 2023: 5,245	12.00	12.00	0.00	0.00	-0.20	6.2	3	false	0	false	false	false				

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADI	Lane Widt	Right Lane Widt h (ft)	Shoulder Width (ft)		Grad e (%)	Driveway Density (driveways/ mi)	Hazar	Centerline Rumble Strip	Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
134	Rural Two-Lane Segment Two-lane Undivided	566+50.00 0	569+37.00 0	287.00	0.0544	2019-2022: 4,325; 2023: 5,245	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false				
135	Rural Two-Lane Segment Two-lane Undivided	569+37.00 0	569+70.00 0	33.00	0.0063	2019-2022: 4,325; 2023: 5,245	12.00	12.00	8.00	0.00	-0.20	6.2	3	false	0	false	false	false				
136	Rural Two-Lane Segment Two-lane Undivided	569+70.00 0	570+00.00 0	30.00	0.0057	2019-2022: 4,325; 2023: 5,245	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false				
137	Rural Two-Lane Segment Two-lane Undivided	570+00.00 0	575+00.00 0	500.00	0.0947	2019-2022: 4,325; 2023: 5,245	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	true	false	false				
138	Rural Two-Lane Segment Two-lane Undivided	575+00.00 0	579+50.00 0	450.00	0.0852	2019-2022: 4,325; 2023: 5,245	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false				
139	Rural Two-Lane Segment Two-lane Undivided	579+50.00 0	579+70.00 0	20.00	0.0038	2019-2022: 4,325; 2023: 5,245	12.00	12.00	0.00	0.00	-0.20	6.2	3	false	0	false	false	false				
140	Rural Two-Lane Segment Two-lane Undivided	579+70.00 0	580+10.00 0	40.00	0.0076	2019-2022: 4,325; 2023: 5,245	12.00	12.00	0.00	0.00	-0.20	6.2	3	false	0	false	false	false				

**Table 4. Evaluation Intersection - Section 1** 

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
1	West Central School/SD38 (v2)	Rural Two-Lane Intersection Three-Legged w/STOP control	569+50.000	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,735; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	2025: 912; 2026: 932; 2027: 951; 2028: 970; 2029: 990; 2030: 1,013; 2031: 1,036; 2032: 1,059; 2033: 1,082; 2034: 1,105; 2035: 1,129; 2036: 1,152; 2036: 1,175; 2038: 1,175; 2038: 1,219; 2030: 1,21; 2040: 1,245; 2041: 1,273; 2042: 1,302; 2043: 1,330; 2044: 1,359; 2045: 1,387; 2046: 1,416; 2047: 1,444; 2048: 1,473; 2049: 1,501; 2050: 1,530	3	Stop-Controlled	0	0	1.37		false
2	2nd/SD38 (v3)	Rural Two-Lane Intersection Four-Legged w/STOP control	566+00.000	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,735; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	2025: 1,338; 2026: 1,366; 2027: 1,394; 2028: 1,422; 2029: 1,450; 2030: 1,484; 2031: 1,518; 2032: 1,552; 2033: 1,586; 2034: 1,620; 2035: 1,654; 2036: 1,688; 2037: 1,722; 2038: 1,756; 2039: 1,790; 2040: 1,825; 2041: 1,867; 2042: 1,909; 2043: 1,951; 2044: 1,993; 2045: 2,035; 2046: 2,077; 2047: 2,119; 2048: 2,161; 2049: 2,203; 2050: 2,245	4	Stop-Controlled	1	0	41.37	41.37	false
4	459/SD38 (v2)	Rural Two-Lane Intersection Four-Legged w/STOP control	296+00.000	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	2025: 320; 2026: 329; 2027: 337; 2028: 346; 2029: 355; 2030: 363; 2031: 371; 2032: 379; 2033: 387; 2034: 395; 2035: 404; 2036: 412; 2037: 420; 2038: 428: 2039: 436; 2040: 445; 2041: 455; 2042: 465; 2043: 475; 2044: 485; 2045: 495; 2046: 505; 2047: 515; 2048: 525; 2049: 535; 2050: 545	4	Stop-Controlled	2	0	0.04	0.04	false
5	SD38/SD19_Build (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	187+50.000	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378: 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	2025: 2,094; 2026: 2,140; 2027: 2,187; 2028: 2,233; 2029: 2,280; 2030: 2,336; 2031: 2,392; 2032: 2,499; 2033: 2,505; 2034: 2,561; 2035: 2,618; 2036: 2,674; 2037: 2,730; 2038: 2,787; 2039: 2,843; 2040: 2,900; 2041: 2,967; 2042: 3,034; 2043: 3,101; 2044: 3,168; 2045: 3,235; 2046: 3,302; 2047: 3,369; 2048: 3,436; 2049: 3,503; 2050: 3,570	4	Stop-Controlled	2	0	5.84	5.84	false
6	I90 SPEEDWAY/SD38 (v1)	Rural Two-Lane Intersection Three-Legged w/STOP control	378+50.000	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	2025: 260; 2026: 264; 2027: 267; 2028: 271; 2029: 275; 2030: 281; 2031: 288; 2032: 295; 2033: 302; 2034: 309; 2035: 315; 2036: 322; 2037: 329; 2038: 336; 2039: 333; 2040: 350; 2041: 372; 2042: 395; 2043: 417; 2044: 440; 2045: 462; 2046: 485; 2047: 507; 2048: 530; 2049: 552; 2050: 575	3	Stop-Controlled	1	1	5.46		false
7	463/SD38 (v3)	Rural Two-Lane Intersection Four-Legged w/STOP control	512+00.000	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070: 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	2025: 3,802; 2026: 3,882; 2027: 3,963; 2028: 4,044; 2029: 4,125; 2030: 4,221; 2031: 4,318; 2032: 4,415; 2033: 4,512; 2034: 4,609; 2035: 4,705; 2036: 4,802; 2037: 4,899; 2038: 4,996; 2039: 5,093; 2040: 5,190; 2041: 5,308; 2042: 5,427; 2043: 5,545: 2044: 5,664; 2045: 5,782; 2046: 5,901; 2047: 6,019; 2048: 6,138; 2049: 6,256; 2050: 6,375	4	Stop-Controlled	1	0	1.43	1.43	true
8	Main Ave/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	524+50.000	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	2025: 1,367; 2026: 1,397; 2027: 1,426; 2028: 1,455; 2029: 1,485; 2030: 1,520; 2031: 1,555; 2032: 1,590; 2033: 1,625; 2034: 1,660; 2035: 1,695; 2036: 1,730; 2037: 1,765; 2038: 1,800; 2039: 1,835; 2040: 1,870; 2041: 1,912; 2042: 1,955; 2043: 1,997; 2044: 2,040; 2045: 2,082; 2046: 2,125; 2047: 2,167; 2048: 2,210; 2049: 2,252; 2050: 2,295	4	Stop-Controlled	0	0	11.00	10.54	false
9	Vandemark/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	541+50.000	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	2025: 657; 2026: 672; 2027: 686; 2028: 700; 2029: 715; 2030: 731; 2031: 747; 2032: 764; 2033: 780; 2034: 796; 2035: 813; 2036: 829; 2037: 845; 2038: 862; 2039: 878; 2040: 895; 2041: 915; 2042: 936; 2043: 956; 2044: 977; 2045: 997; 2046: 1,018; 2047: 1,038; 2048: 1,059; 2049: 1,079; 2050: 1,100	4	Stop-Controlled	1	0	48.63	48.63	false

 $\begin{tabular}{ll} \textbf{Table 5.} & \textbf{Crash History Intersection - Section 1} \\ \end{tabular}$ 

Inter. No.	Title	Туре	Location (Sta.	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
1	West Central School/SD38 (v2)	Rural Two-Lane Intersection Three-Legged w/STOP control	569+50.000	2019-2022: 4,325; 2023: 5,245	2019-2022: 855; 2023: 874	3	Stop-Controlled	0	0	1.37		false
2	2nd/SD38 (v3)	Rural Two-Lane Intersection Four-Legged w/STOP control	566+00.000	2019-2022: 4,325; 2023: 5,245	2019-2022: 1,255; 2023: 1,282	4	Stop-Controlled	1	0	41.37	41.37	false
4	459/SD38 (v2)	Rural Two-Lane Intersection Four-Legged w/STOP control	296+00.000	2019-2022: 2,085; 2023: 2,134	2019-2022: 295; 2023: 303	4	Stop-Controlled	2	0	0.04	0.04	false
5	SD38/SD19_Build (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	187+50.000	2019-2022: 2,085; 2023: 2,134	2019-2022: 1,955; 2023: 2,001	4	Stop-Controlled	2	0	5.84	5.84	false
6	I90 SPEEDWAY/SD38 (v1)	Rural Two-Lane Intersection Three-Legged w/STOP control	378+50.000	2019-2022: 2,085; 2023: 2,134	2019-2022: 250; 2023: 253	3	Stop-Controlled	1	1	5.46		false
7	463/SD38 (v3)	Rural Two-Lane Intersection Four-Legged w/STOP control	512+00.000	2019-2022: 4,325; 2023: 5,081	2019-2022: 3,560; 2023: 3,640	4	Stop-Controlled	1	0	1.43	1.43	true
8	Main Ave/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	524+50.000	2019-2022: 4,325; 2023: 5,081	2019-2022: 1,280; 2023: 1,309	4	Stop-Controlled	0	0	11.00	10.54	false
9	Vandemark/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	541+50.000	2019-2022: 4,325; 2023: 5,081	2019-2022: 615; 2023: 629	4	Stop-Controlled	1	0	48.63	48.63	false

Table 6. Expected Highway Crash Rates and Frequencies Summary (Section 1)

First Year of Analysis	2025
Last Year of Analysis	2050
Evaluated Length (mi)	7.7398
Average Future Road AADT (vpd)	4,284
Expected Crashes	
Total Crashes	422.63
Fatal and Injury Crashes	177.58
Property-Damage-Only Crashes	245.06
Percent of Total Expected Crashes	
Percent Fatal and Injury Crashes (%)	42
Percent Property-Damage-Only Crashes (%)	58
Expected Crash Rate	
Crash Rate (crashes/mi/yr)	2.1002
FI Crash Rate (crashes/mi/yr)	0.8824
PDO Crash Rate (crashes/mi/yr)	1.2178
Expected Travel Crash Rate	
Total Travel (million veh-mi)	314.63
Travel Crash Rate (crashes/million veh-mi)	1.34
Travel FI Crash Rate (crashes/million veh-mi)	0.56
Travel PDO Crash Rate (crashes/million veh-mi)	0.78

Table 7. Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 1)

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi llion veh- mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
1	171+44.000	172+42.000	0.0186	0.237	0.402	0.0091	0.0033	0.0058	0.0155	0.0050	0.0105	-0.0064	-0.0017	-0.0047	0.4911	0.45	
2	172+42.000	174+52.690	0.0399	0.481	0.786	0.0185	0.0067	0.0118	0.0302	0.0097	0.0205	-0.0117	-0.0030	-0.0087	0.4638	0.43	
3	174+52.690	176+25.000	0.0326	0.498	0.980	0.0192	0.0072	0.0120	0.0377	0.0121	0.0256	-0.0185	-0.0049	-0.0136	0.5873	0.54	
4	176+25.000	178+85.250	0.0493	0.753	1.479	0.0289	0.0108	0.0181	0.0569	0.0183	0.0386	-0.0280	-0.0074	-0.0205	0.5873	0.54	
5	178+85.250	183+75.370	0.0928	1.119	1.829	0.0431	0.0155	0.0276	0.0703	0.0226	0.0478	-0.0273	-0.0071	-0.0202	0.4638	0.43	
6	183+75.370	184+00.000	0.0047	0.056	0.092	0.0022	0.0008	0.0014	0.0035	0.0011	0.0024	-0.0014	-0.0004	-0.0010	0.4638	0.43	
7	184+00.000	184+45.000	0.0085	0.103	0.168	0.0040	0.0014	0.0025	0.0065	0.0021	0.0044	-0.0025	-0.0007	-0.0019	0.4638	0.43	
8	184+45.000	185+20.000	0.0142	0.171	0.280	0.0066	0.0024	0.0042	0.0108	0.0035	0.0073	-0.0042	-0.0011	-0.0031	0.4638	0.43	
9	185+20.000	186+60.000	0.0265	0.320	0.522	0.0123	0.0044	0.0079	0.0201	0.0064	0.0136	-0.0078	-0.0020	-0.0058	0.4638	0.43	
10	186+60.000	187+20.000	0.0114	0.137	0.224	0.0053	0.0019	0.0034	0.0086	0.0028	0.0058	-0.0033	-0.0009	-0.0025	0.4638	0.43	
11	187+20.000	187+60.000	0.0076	0.110	0.208	0.0042	0.0016	0.0027	0.0080	0.0026	0.0054	-0.0037	-0.0010	-0.0028	0.5601	0.52	
SD38/SD19_Build (v1)	187+50.000			18.347	41.067	0.7056	0.3225	0.3831	1.5795	0.6808	0.8987	-0.8739	-0.3582	-0.5156			0.3
12	187+60.000	190+00.000	0.0455	0.548	0.895	0.0211	0.0076	0.0135	0.0344	0.0111	0.0234	-0.0134	-0.0035	-0.0099	0.4638	0.43	
13	190+00.000	192+00.000	0.0379	0.457	0.746	0.0176	0.0063	0.0112	0.0287	0.0092	0.0195	-0.0111	-0.0029	-0.0082	0.4638	0.43	
14	192+00.000	192+39.270	0.0074	0.090	0.146	0.0034	0.0012	0.0022	0.0056	0.0018	0.0038	-0.0022	-0.0006	-0.0016	0.4638	0.43	
15	192+39.270	193+60.000	0.0229	0.276	0.451	0.0106	0.0038	0.0068	0.0173	0.0056	0.0118	-0.0067	-0.0017	-0.0050	0.4638	0.43	
16	193+60.000	197+65.000	0.0767	0.925	1.511	0.0356	0.0128	0.0228	0.0581	0.0187	0.0395	-0.0225	-0.0059	-0.0167	0.4638	0.43	
17	197+65.000	199+00.000	0.0256	0.308	0.504	0.0119	0.0043	0.0076	0.0194	0.0062	0.0132	-0.0075	-0.0020	-0.0056	0.4638	0.43	
18	199+00.000	201+63.750	0.0500	0.602	0.984	0.0232	0.0083	0.0148	0.0379	0.0121	0.0257	-0.0147	-0.0038	-0.0109	0.4638	0.43	
19	201+63.750	202+00.000	0.0069	0.083	0.135	0.0032	0.0011	0.0020	0.0052	0.0017	0.0035	-0.0020	-0.0005	-0.0015	0.4638	0.43	
20	202+00.000	207+00.000	0.0947	1.142	1.866	0.0439	0.0158	0.0281	0.0718	0.0230	0.0487	-0.0278	-0.0072	-0.0206	0.4638	0.43	
21	207+00.000	207+49.760	0.0094	0.114	0.186	0.0044	0.0016	0.0028	0.0071	0.0023	0.0048	-0.0028	-0.0007	-0.0021	0.4638	0.43	
22	207+49.760	217+74.250	0.1940	2.340	3.823	0.0900	0.0324	0.0576	0.1470	0.0472	0.0998	-0.0570	-0.0148	-0.0422	0.4638	0.43	
23	217+74.250	221+00.000	0.0617	3.590	1.215	0.1381	0.0144	0.1237	0.0467	0.0150	0.0317	0.0913	-0.0006	0.0919	2.2379	2.06	
24	221+00.000	226+00.000	0.0947	1.142	1.866	0.0439	0.0158	0.0281	0.0718	0.0230	0.0487	-0.0278	-0.0072	-0.0206	0.4638	0.43	
25	226+00.000	230+66.250	0.0883	1.065	1.740	0.0410	0.0147	0.0262	0.0669	0.0215	0.0454	-0.0260	-0.0067	-0.0192	0.4638	0.43	
26	230+66.250	231+39.700	0.0139	0.188	0.333	0.0072	0.0026	0.0046	0.0128	0.0041	0.0087	-0.0056	-0.0015	-0.0041	0.5197	0.48	
27	231+39.700	235+00.000	0.0682	0.922	1.631	0.0355	0.0130	0.0225	0.0627	0.0201	0.0426	-0.0273	-0.0072	-0.0201	0.5197	0.48	
28	235+00.000	241+61.390	0.1253	4.881	2.994	0.1877	0.1173	0.0705	0.1152	0.0370	0.0782	0.0726	0.0803	-0.0077	1.4988	1.38	
29	241+61.390	242+00.000	0.0073	0.099	0.175	0.0038	0.0014	0.0024	0.0067	0.0022	0.0046	-0.0029	-0.0008	-0.0022	0.5197	0.48	
30	242+00.000	245+14.280	0.0595	0.804	1.423	0.0309	0.0113	0.0196	0.0547	0.0176	0.0372	-0.0238	-0.0062	-0.0175	0.5197	0.48	
31	245+14.280	246+55.100	0.0267	0.322	0.525	0.0124	0.0044	0.0079	0.0202	0.0065	0.0137	-0.0078	-0.0020	-0.0058	0.4638	0.43	
32	246+55.100	248+00.000	0.0274	0.331	0.541	0.0127	0.0046	0.0081	0.0208	0.0067	0.0141	-0.0081	-0.0021	-0.0060	0.4638	0.43	
33	248+00.000	249+00.000	0.0189	3.713	0.519	0.1428	0.0060	0.1368	0.0200	0.0064	0.0136	0.1228	-0.0004	0.1233	7.5395	6.96	
34	249+00.000	251+21.980	0.0420	0.507	0.828	0.0195	0.0070	0.0125	0.0319	0.0102	0.0216	-0.0124	-0.0032	-0.0091	0.4638	0.43	
35	251+21.980	252+40.240	0.0224	0.286	0.485	0.0110	0.0040	0.0070	0.0187	0.0060	0.0127	-0.0077	-0.0020	-0.0057	0.4911	0.45	
36	252+40.240	263+22.600	0.2050	2.472	4.038	0.0951	0.0342	0.0609	0.1553	0.0499	0.1055	-0.0603	-0.0157	-0.0446	0.4638	0.43	
37	263+22.600	272+66.740	0.1788	2.346	4.061	0.0902	0.0329	0.0574	0.1562	0.0501	0.1060	-0.0659	-0.0173	-0.0487	0.5047	0.47	
38	272+66.740	280+00.000	0.1389	1.675	2.736	0.0644	0.0232	0.0412	0.1052	0.0338	0.0715	-0.0408	-0.0106	-0.0302	0.4638	0.43	

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi llion veh- mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
39	280+00.000	283+15.050	0.0597	0.720	1.175	0.0277	0.0100	0.0177	0.0452	0.0145	0.0307	-0.0175	-0.0046	-0.0130	0.4638	0.43	
40	283+15.050	284+08.540	0.0177	0.226	0.384	0.0087	0.0032	0.0055	0.0148	0.0047	0.0100	-0.0061	-0.0016	-0.0045	0.4911	0.45	
41	284+08.540	288+50.000	0.0836	3.854	1.647	0.1482	0.0190	0.1292	0.0634	0.0203	0.0430	0.0849	-0.0013	0.0862	1.7729	1.64	
42	288+50.000	289+00.000	0.0095	0.114	0.187	0.0044	0.0016	0.0028	0.0072	0.0023	0.0049	-0.0028	-0.0007	-0.0021	0.4638	0.43	
43	289+00.000	295+90.000	0.1307	1.576	2.575	0.0606	0.0218	0.0388	0.0990	0.0318	0.0672	-0.0384	-0.0100	-0.0284	0.4638	0.43	
44	295+90.000	296+00.000	0.0019	0.028	0.052	0.0011	0.0004	0.0007	0.0020	0.0006	0.0014	-0.0009	-0.0002	-0.0007	0.5601	0.52	
459/SD38 (v2)	296+00.000			11.417	12.695	0.4391	0.1711	0.2680	0.4883	0.2104	0.2778	-0.0492	-0.0394	-0.0098			0.37
45	296+00.000	296+10.000	0.0019	0.028	0.052	0.0011	0.0004	0.0007	0.0020	0.0006	0.0014	-0.0009	-0.0002	-0.0007	0.5601	0.52	
46	296+10.000	296+96.520	0.0164	0.198	0.323	0.0076	0.0027	0.0049	0.0124	0.0040	0.0084	-0.0048	-0.0013	-0.0036	0.4638	0.43	
47	296+96.520	298+33.660	0.0260	0.313	0.512	0.0120	0.0043	0.0077	0.0197	0.0063	0.0134	-0.0076	-0.0020	-0.0057	0.4638	0.43	
48	298+33.660	303+50.000	0.0978	1.179	1.927	0.0454	0.0163	0.0290	0.0741	0.0238	0.0503	-0.0287	-0.0075	-0.0213	0.4638	0.43	
49	303+50.000	304+50.000	0.0189	0.228	0.373	0.0088	0.0032	0.0056	0.0144	0.0046	0.0097	-0.0056	-0.0014	-0.0041	0.4638	0.43	
50	304+50.000	305+02.039	0.0099	0.119	0.194	0.0046	0.0016	0.0029	0.0075	0.0024	0.0051	-0.0029	-0.0008	-0.0021	0.4638	0.43	
51	305+02.039	309+35.490	0.0821	0.990	1.617	0.0381	0.0137	0.0244	0.0622	0.0200	0.0422	-0.0241	-0.0063	-0.0179	0.4638	0.43	
52	309+35.490	311+70.000	0.0444	0.536	0.875	0.0206	0.0074	0.0132	0.0337	0.0108	0.0229	-0.0131	-0.0034	-0.0097	0.4638	0.43	
53	311+70.000	313+25.000	0.0294	0.354	0.578	0.0136	0.0049	0.0087	0.0222	0.0071	0.0151	-0.0086	-0.0022	-0.0064	0.4638	0.43	
54	313+25.000	323+00.000	0.1847	2.227	3.638	0.0856	0.0308	0.0548	0.1399	0.0449	0.0950	-0.0543	-0.0141	-0.0402	0.4638	0.43	
55	323+00.000	323+26.980	0.0051	0.062	0.101	0.0024	0.0009	0.0015	0.0039	0.0012	0.0026	-0.0015	-0.0004	-0.0011	0.4638	0.43	
56	323+26.980	328+89.230	0.1065	1.284	2.098	0.0494	0.0178	0.0316	0.0807	0.0259	0.0548	-0.0313	-0.0081	-0.0232	0.4638	0.43	
57	328+89.230	329+81.740	0.0175	0.211	0.345	0.0081	0.0029	0.0052	0.0133	0.0043	0.0090	-0.0052	-0.0013	-0.0038	0.4638	0.43	
58	329+81.740	333+24.920	0.0650	0.934	1.738	0.0359	0.0133	0.0226	0.0669	0.0215	0.0454	-0.0309	-0.0082	-0.0228	0.5529	0.51	
59	333+24.920	334+00.000	0.0142	0.204	0.380	0.0079	0.0029	0.0050	0.0146	0.0047	0.0099	-0.0068	-0.0018	-0.0050	0.5529	0.51	
60	334+00.000	335+39.960	0.0265	7.167	0.709	0.2756	0.0083	0.2673	0.0273	0.0088	0.0185	0.2484	-0.0004	0.2488	10.3988	9.59	
61	335+39.960	342+39.000	0.1324	1.597	2.608	0.0614	0.0221	0.0393	0.1003	0.0322	0.0681	-0.0389	-0.0101	-0.0288	0.4638	0.43	
62	342+39.000	343+00.000	0.0116	0.139	0.228	0.0054	0.0019	0.0034	0.0088	0.0028	0.0059	-0.0034	-0.0009	-0.0025	0.4638	0.43	
63	343+00.000	351+20.000	0.1553	4.718	3.060	0.1815	0.1064	0.0751	0.1177	0.0378	0.0799	0.0638	0.0686	-0.0048	1.1686	1.08	
64	351+20.000	352+00.000	0.0152	0.221	0.415	0.0085	0.0031	0.0053	0.0160	0.0051	0.0108	-0.0075	-0.0020	-0.0055	0.5601	0.52	
65	352+00.000	352+20.000	0.0038	0.055	0.104	0.0021	0.0008	0.0013	0.0040	0.0013	0.0027	-0.0019	-0.0005	-0.0014	0.5601	0.52	
66	352+20.000	362+50.000	0.1951	2.352	3.843	0.0905	0.0325	0.0579	0.1478	0.0474	0.1004	-0.0573	-0.0149	-0.0424	0.4638	0.43	
67	362+50.000	369+14.990	0.1259	1.519	2.481	0.0584	0.0210	0.0374	0.0954	0.0306	0.0648	-0.0370	-0.0096	-0.0274	0.4638	0.43	
68	369+14.990	370+30.000	0.0218	0.304	0.553	0.0117	0.0043	0.0074	0.0213	0.0068	0.0144	-0.0096	-0.0025	-0.0070	0.5374	0.50	
69	370+30.000	370+60.000	0.0057	0.079	0.144	0.0031	0.0011	0.0019	0.0055	0.0018	0.0038	-0.0025	-0.0007	-0.0018	0.5374	0.50	
70	370+60.000	376+83.610	0.1181	4.948	2.998	0.1903	0.1210	0.0693	0.1153	0.0370	0.0783	0.0750	0.0840	-0.0090	1.6113	1.49	
71	376+83.610	378+00.000	0.0220	0.308	0.559	0.0118	0.0044	0.0075	0.0215	0.0069	0.0146	-0.0097	-0.0025	-0.0071	0.5374	0.50	
72	378+00.000	378+40.000	0.0076	0.116	0.230	0.0045	0.0017	0.0028	0.0088	0.0028	0.0060	-0.0044	-0.0012	-0.0032	0.5906	0.55	
73	378+40.000	378+60.000	0.0038	0.058	0.115	0.0022	0.0008	0.0014	0.0044	0.0014	0.0030	-0.0022	-0.0006	-0.0016	0.5906	0.55	
I90 SPEEDWAY/SD38 (v1)	378+50.000			4.676	6.719	0.1798	0.0773	0.1025	0.2584	0.1073	0.1512	-0.0786	-0.0299	-0.0487			0.16
74	378+60.000	379+00.000	0.0076	0.116	0.230	0.0045	0.0017	0.0028	0.0088	0.0028	0.0060	-0.0044	-0.0012	-0.0032	0.5906	0.55	
75	379+00.000	379+62.690	0.0119	0.166	0.301	0.0064	0.0023	0.0040	0.0116	0.0037	0.0079	-0.0052	-0.0014	-0.0038	0.5374	0.50	
76	379+62.690	385+22.970	0.1061	4.125	2.091	0.1587	0.1022	0.0564	0.0804	0.0258	0.0546	0.0783	0.0764	0.0018	1.4952	1.38	
77	385+22.970	386+60.000	0.0260	0.358	0.643	0.0138	0.0051	0.0087	0.0247	0.0079	0.0168	-0.0110	-0.0029	-0.0081	0.5303	0.49	
78	386+60.000	389+50.000	0.0549	0.757	1.361	0.0291	0.0107	0.0184	0.0524	0.0168	0.0355	-0.0232	-0.0061	-0.0171	0.5303	0.49	

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi Ilion veh- mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
79	389+50.000	394+00.000	0.0852	1.175	2.112	0.0452	0.0166	0.0286	0.0812	0.0261	0.0552	-0.0360	-0.0095	-0.0266	0.5303	0.49	
80	394+00.000	396+46.150	0.0466	0.643	1.155	0.0247	0.0091	0.0156	0.0444	0.0143	0.0302	-0.0197	-0.0052	-0.0145	0.5303	0.49	
81	396+46.150	397+00.000	0.0102	0.141	0.253	0.0054	0.0020	0.0034	0.0097	0.0031	0.0066	-0.0043	-0.0011	-0.0032	0.5303	0.49	
82	397+00.000	399+00.000	0.0379	0.575	1.122	0.0221	0.0083	0.0138	0.0432	0.0139	0.0293	-0.0211	-0.0056	-0.0155	0.5834	0.54	
83	399+00.000	405+75.410	0.1279	1.764	3.170	0.0678	0.0249	0.0429	0.1219	0.0391	0.0828	-0.0541	-0.0142	-0.0399	0.5303	0.49	
84	405+75.410	406+00.000	0.0047	0.056	0.092	0.0022	0.0008	0.0014	0.0035	0.0011	0.0024	-0.0014	-0.0004	-0.0010	0.4638	0.43	
85	406+00.000	407+00.000	0.0189	0.254	0.446	0.0098	0.0036	0.0062	0.0172	0.0055	0.0116	-0.0074	-0.0019	-0.0055	0.5153	0.47	
86	407+00.000	443+25.000	0.6866	22.508	13.526	0.8657	0.3493	0.5164	0.5202	0.1670	0.3532	0.3455	0.1823	0.1632	1.2609	1.16	
87	443+25.000	445+50.000	0.0426	0.514	0.840	0.0198	0.0071	0.0127	0.0323	0.0104	0.0219	-0.0125	-0.0033	-0.0093	0.4638	0.43	
88	445+50.000	452+50.000	0.1326	4.444	2.612	0.1709	0.1042	0.0667	0.1005	0.0322	0.0682	0.0705	0.0720	-0.0015	1.2894	1.19	
89	452+50.000	459+00.000	0.1231	1.484	2.425	0.0571	0.0205	0.0366	0.0933	0.0299	0.0633	-0.0362	-0.0094	-0.0268	0.4638	0.43	
90	459+00.000	460+00.000	0.0189	0.254	0.446	0.0098	0.0036	0.0062	0.0172	0.0055	0.0116	-0.0074	-0.0019	-0.0055	0.5153	0.47	
91	460+00.000	460+58.580	0.0111	0.134	0.219	0.0051	0.0019	0.0033	0.0084	0.0027	0.0057	-0.0033	-0.0008	-0.0024	0.4638	0.43	
92	460+58.580	485+61.230	0.4740	8.561	9.338	0.3293	0.0898	0.2395	0.3592	0.1153	0.2439	-0.0299	-0.0255	-0.0044	0.6947	0.64	
93	485+61.230	503+00.000	0.3293	3.971	6.488	0.1527	0.0549	0.0978	0.2495	0.0801	0.1694	-0.0968	-0.0252	-0.0716	0.4638	0.43	
94	503+00.000	507+00.000	0.0758	0.913	1.492	0.0351	0.0126	0.0225	0.0574	0.0184	0.0390	-0.0223	-0.0058	-0.0165	0.4638	0.43	
95	507+00.000	508+00.000	0.0189	0.217	0.344	0.0083	0.0030	0.0054	0.0132	0.0042	0.0090	-0.0049	-0.0013	-0.0036	0.4408	0.41	
96	508+00.000	508+08.240	0.0016	0.018	0.028	0.0007	0.0002	0.0004	0.0011	0.0003	0.0007	-0.0004	-0.0001	-0.0003	0.4408	0.41	
97	508+08.240	510+30.000	0.0420	0.481	0.762	0.0185	0.0066	0.0119	0.0293	0.0094	0.0199	-0.0108	-0.0028	-0.0080	0.4408	0.41	
98	510+30.000	512+00.000	0.0322	0.388	0.634	0.0149	0.0054	0.0096	0.0244	0.0078	0.0166	-0.0095	-0.0025	-0.0070	0.4638	0.43	
463/SD38 (v3)	512+00.000			59.412	153.769	2.2851	1.1850	1.1001	5.9142	2.5490	3.3652	-3.6291	-1.3640	-2.2651			0.60
99	512+00.000	513+00.000	0.0189	0.613	1.666	0.0236	0.0094	0.0142	0.0641	0.0206	0.0435	-0.0405	-0.0112	-0.0293	1.2446	0.32	
100	513+00.000	515+00.000	0.0379	1.072	2.395	0.0412	0.0158	0.0254	0.0921	0.0296	0.0626	-0.0509	-0.0138	-0.0372	1.0882	0.28	
101	515+00.000	520+00.000	0.0947	2.640	5.796	0.1015	0.0388	0.0627	0.2229	0.0716	0.1514	-0.1214	-0.0327	-0.0886	1.0722	0.28	
102	520+00.000	520+49.150	0.0093	0.301	0.819	0.0116	0.0046	0.0070	0.0315	0.0101	0.0214	-0.0199	-0.0055	-0.0144	1.2446	0.32	
103	520+49.150 521+00.000	521+00.000 523+38.600	0.0096	0.335	1.049 3.425	0.0129	0.0053	0.0076 0.0323	0.0403	0.0130	0.0274	-0.0274 -0.0786	-0.0077	-0.0197 -0.0571	1.3395	0.35	
105	521+00.000 523+38.600	523+38.600	0.0452	0.355	0.881	0.0531	0.0208	0.0323	0.0339	0.0423	0.0895	-0.0786	-0.0215 -0.0055	-0.05/1	1.1753	0.30	
105	524+00.000	525+00.000	0.0116	0.660	2.063	0.0157	0.0033	0.0083	0.0339	0.0109	0.0230	-0.0202	-0.0053	-0.0147	1.3395	0.35	
Main Ave/SD38 (v1)	524+50.000	323+00.000	0.0169	41.391	132.778	1.5920	0.6668	0.9251	5.1069	2.2011	2,9058	-3.5149	-1.5342	-1.9807	1.5595	0.33	0.37
107	525+00.000	525+18.580	0.0035	0.107	0.267	0.0041	0.0016	0.0025	0.0103	0.0033	0.0070	-0.0061	-0.0017	-0.0044	1.1753	0.30	0.57
108	525+18.580	528+00.000	0.0533	1.629	4.040	0.0626	0.0245	0.0381	0.1554	0.0499	0.1055	-0.0927	-0.0254	-0.0674	1.1753	0.30	
109	528+00.000	529+00.000	0.0333	0.660	2.063	0.0020	0.0103	0.0150	0.0793	0.0255	0.0539	-0.0540	-0.0254	-0.0388	1.3395	0.35	
110	529+00.000	539+00.000	0.1894	5,787	14.355	0.2226	0.0871	0.1355	0.5521	0.1772	0.3749	-0.3295	-0.0902	-0.2394	1.1753	0.30	
111	539+00.000	539+50.000	0.0095	0.293	0.742	0.0113	0.0044	0.0068	0.0285	0.0092	0.0194	-0.0172	-0.0047	-0.0125	1.1907	0.31	
112	539+50.000	540+00.000	0.0095	0.293	0.742	0.0113	0.0044	0.0068	0.0285	0.0092	0.0194	-0.0172	-0.0047	-0.0125	1.1907	0.31	
113	540+00.000	540+50.000	0.0095	0.293	0.742	0.0113	0.0044	0.0068	0.0285	0.0092	0.0194	-0.0172	-0.0047	-0.0125	1.1907	0.31	
114	540+50.000	540+74.370	0.0046	0.161	0.503	0.0062	0.0025	0.0037	0.0193	0.0062	0.0131	-0.0132	-0.0037	-0.0095	1.3395	0.35	
115	540+74.370	541+00.000	0.0049	0.157	0.427	0.0060	0.0024	0.0037	0.0164	0.0053	0.0111	-0.0104	-0.0029	-0.0075	1.2446	0.32	
116	541+00.000	541+50.000	0.0095	0.306	0.833	0.0118	0.0047	0.0071	0.0320	0.0103	0.0218	-0.0202	-0.0056	-0.0147	1.2446	0.32	
Vandemark/SD38 (v1)	541+50.000			27.929	74.904	1.0742	0.4965	0.5777	2.8809	1.2417	1.6393	-1.8068	-0.7452	-1.0616			0.27
117	541+50.000	541+70.000	0.0038	0.123	0.333	0.0047	0.0019	0.0028	0.0128	0.0041	0.0087	-0.0081	-0.0022	-0.0059	1.2446	0.32	

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi llion veh- mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
118	541+70.000	542+30.000	0.0114	0.368	1.000	0.0141	0.0056	0.0085	0.0384	0.0123	0.0261	-0.0243	-0.0067	-0.0176	1.2446	0.32	
119	542+30.000	542+64.000	0.0064	0.182	0.407	0.0070	0.0027	0.0043	0.0157	0.0050	0.0106	-0.0087	-0.0023	-0.0063	1.0882	0.28	
120	542+64.000	543+34.000	0.0133	0.375	0.838	0.0144	0.0055	0.0089	0.0322	0.0104	0.0219	-0.0178	-0.0048	-0.0130	1.0882	0.28	
121	543+34.000	544+00.000	0.0125	0.354	0.790	0.0136	0.0052	0.0084	0.0304	0.0098	0.0206	-0.0168	-0.0045	-0.0123	1.0882	0.28	
122	544+00.000	545+00.000	0.0189	0.555	1.300	0.0214	0.0083	0.0131	0.0500	0.0160	0.0339	-0.0286	-0.0078	-0.0208	1.1278	0.29	
123	545+00.000	548+23.000	0.0612	1.794	4.198	0.0690	0.0267	0.0423	0.1615	0.0518	0.1096	-0.0925	-0.0251	-0.0673	1.1278	0.29	
124	548+23.000	553+70.000	0.1036	9.958	7.109	0.3830	0.2582	0.1248	0.2734	0.0878	0.1857	0.1096	0.1704	-0.0609	3.6968	0.96	
125	553+70.000	554+00.000	0.0057	0.189	0.542	0.0073	0.0029	0.0044	0.0209	0.0067	0.0142	-0.0136	-0.0038	-0.0098	1.2816	0.33	
126	554+00.000	554+20.000	0.0038	0.126	0.361	0.0049	0.0019	0.0029	0.0139	0.0045	0.0094	-0.0090	-0.0025	-0.0065	1.2816	0.33	
127	554+20.000	560+00.000	0.1098	3.221	7.538	0.1239	0.0479	0.0759	0.2899	0.0931	0.1969	-0.1660	-0.0451	-0.1209	1.1278	0.29	
128	560+00.000	562+58.560	0.0490	1.436	3.360	0.0552	0.0214	0.0339	0.1292	0.0415	0.0878	-0.0740	-0.0201	-0.0539	1.1278	0.29	
129	562+58.560	564+00.000	0.0268	0.785	1.838	0.0302	0.0117	0.0185	0.0707	0.0227	0.0480	-0.0405	-0.0110	-0.0295	1.1278	0.29	
130	564+00.000	565+00.000	0.0189	0.555	1.300	0.0214	0.0083	0.0131	0.0500	0.0160	0.0339	-0.0286	-0.0078	-0.0208	1.1278	0.29	
131	565+00.000	565+77.000	0.0146	0.482	1.133	0.0185	0.0072	0.0114	0.0436	0.0140	0.0296	-0.0250	-0.0068	-0.0182	1.2719	0.29	
132	565+77.000	566+10.000	0.0063	0.235	0.675	0.0090	0.0036	0.0054	0.0260	0.0083	0.0176	-0.0170	-0.0047	-0.0122	1.4446	0.33	
2nd/SD38 (v3)	566+00.000			50.602	119.976	1.9462	0.7275	1.2187	4.6145	1.9888	2.6256	-2.6682	-1.2613	-1.4069			0.40
133	566+10.000	566+50.000	0.0076	0.284	0.819	0.0109	0.0044	0.0065	0.0315	0.0101	0.0214	-0.0205	-0.0057	-0.0148	1.4446	0.33	
134	566+50.000	569+37.000	0.0544	17.406	4.224	0.6695	0.2593	0.4102	0.1625	0.0522	0.1103	0.5070	0.2071	0.2999	12.3164	2.82	
135	569+37.000	569+70.000	0.0063	0.222	0.581	0.0085	0.0034	0.0052	0.0223	0.0072	0.0152	-0.0138	-0.0038	-0.0100	1.3670	0.31	
West Central School/SD38 (v2)	569+50.000			18.448	73.624	0.7095	0.3302	0.3794	2.8317	1.1752	1.6565	-2.1222	-0.8450	-1.2772			0.15
136	569+70.000	570+00.000	0.0057	0.188	0.442	0.0072	0.0028	0.0044	0.0170	0.0055	0.0115	-0.0098	-0.0027	-0.0071	1.2719	0.29	
137	570+00.000	575+00.000	0.0947	3.088	7.122	0.1188	0.0458	0.0729	0.2739	0.0879	0.1860	-0.1552	-0.0421	-0.1131	1.2542	0.29	
138	575+00.000	579+50.000	0.0852	2.818	6.623	0.1084	0.0420	0.0664	0.2547	0.0818	0.1730	-0.1463	-0.0398	-0.1066	1.2719	0.29	
139	579+50.000	579+70.000	0.0038	0.142	0.409	0.0055	0.0022	0.0033	0.0157	0.0051	0.0107	-0.0103	-0.0029	-0.0074	1.4446	0.33	
140	579+70.000	580+10.000	0.0076	0.284	0.819	0.0109	0.0044	0.0065	0.0315	0.0101	0.0214	-0.0205	-0.0057	-0.0148	1.4446	0.33	
All Segments			7.7398	190.411	230.845	7.3235	2.8529	4.4706	8.8786	2.8500	6.0286	-1.5551	0.0029	-1.5580	0.9462	0.60	
All Intersections				232.221	615.534	8.9316	3.9769	4.9547	23.6744	10.1542	13.5202	-14.7428	-6.1773	-8.5655			0.34
Total			7.7398	422.632	846.378	16.2551	6.8299	9.4252	32.5530	13.0043	19.5488	-16.2979	-6.1744	-10.1235	2.1002		

**Table 8. Expected Crash Frequencies and Rates by Horizontal Design Element (Section 1)** 

Title	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi Ilion veh- mi)
Tangent	171+44.000	174+52.690	0.0585	0.718	1.188	0.0276	0.0100	0.0177	0.0457	0.0147	0.0310	-0.0181	-0.0047	-0.0134	0.4725	0.44
Simple Curve 1	174+52.690	178+85.250	0.0819	1.251	2.459	0.0481	0.0180	0.0301	0.0946	0.0304	0.0642	-0.0465	-0.0124	-0.0341	0.5873	0.54
Tangent	178+85.250	230+66.250	0.9812	14.697	19.390	0.5653	0.1681	0.3971	0.7458	0.2394	0.5064	-0.1805	-0.0713	-0.1092	0.5761	0.53
Simple Curve 2	230+66.250	245+14.280	0.2742	6.894	6.555	0.2652	0.1456	0.1196	0.2521	0.0809	0.1712	0.0131	0.0647	-0.0516	0.9669	0.89
Tangent	245+14.280	263+22.600	0.3425	7.630	6.937	0.2935	0.0602	0.2332	0.2668	0.0856	0.1812	0.0266	-0.0254	0.0521	0.8569	0.79
Simple Curve 3	263+22.600	272+66.740	0.1788	2.346	4.061	0.0902	0.0329	0.0574	0.1562	0.0501	0.1060	-0.0659	-0.0173	-0.0487	0.5047	0.47
Tangent	272+66.740	296+96.470	0.4602	8.417	9.130	0.3237	0.0822	0.2415	0.3511	0.1127	0.2384	-0.0274	-0.0305	0.0031	0.7035	0.65
Simple Curve 4	296+96.470	296+96.520	0.0000	0.000	0.000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	-0.0000	-0.0000	-0.0000	0.4638	0.43
Tangent	296+96.520	329+81.740	0.6222	7.503	12.258	0.2886	0.1038	0.1848	0.4715	0.1513	0.3201	-0.1829	-0.0475	-0.1354	0.4638	0.43
Simple Curve 5	329+81.740	335+39.960	0.1057	8.306	2.828	0.3194	0.0245	0.2949	0.1088	0.0349	0.0738	0.2107	-0.0104	0.2211	3.0215	2.79
Tangent	335+39.960	369+14.990	0.6392	10.601	12.739	0.4077	0.1879	0.2199	0.4900	0.1573	0.3327	-0.0822	0.0306	-0.1128	0.6379	0.59
Simple Curve 6	369+14.990	379+62.690	0.1984	6.096	5.131	0.2345	0.1373	0.0972	0.1973	0.0633	0.1340	0.0371	0.0740	-0.0368	1.1817	1.09
Tangent	379+62.690	385+22.970	0.1061	4.125	2.091	0.1587	0.1022	0.0564	0.0804	0.0258	0.0546	0.0783	0.0764	0.0018	1.4952	1.38
Simple Curve 7	385+22.970	405+75.410	0.3887	5.412	9.817	0.2082	0.0766	0.1316	0.3776	0.1212	0.2564	-0.1694	-0.0446	-0.1248	0.5355	0.49
Tangent	405+75.410	520+49.150	2.1731	48.824	50.368	1.8778	0.7321	1.1457	1.9372	0.6218	1.3154	-0.0594	0.1103	-0.1697	0.8641	0.74
Simple Curve 8	520+49.150	540+74.370	0.3836	11.955	30.871	0.4598	0.1810	0.2788	1.1874	0.3811	0.8062	-0.7276	-0.2001	-0.5275	1.1988	0.31
Tangent	540+74.370	580+10.000	0.7454	45.636	55.023	1.7552	0.7904	0.9648	2.1163	0.6793	1.4370	-0.3611	0.1111	-0.4722	2.3548	0.57

Table 9. Predicted Crash Frequencies by Year (Section 1)

Year	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
2025	21.17	8.41	39.738	12.76	60.262
2026	22.73	9.04	39.783	13.69	60.217
2027	24.27	9.67	39.821	14.61	60.179
2028	25.80	10.28	39.854	15.52	60.146
2029	27.32	10.90	39.881	16.43	60.119
2030	27.93	11.14	39.889	16.79	60.111
2031	28.55	11.39	39.897	17.16	60.103
2032	29.17	11.64	39.905	17.53	60.095
2033	29.79	11.89	39.912	17.90	60.088
2034	30.41	12.14	39.920	18.27	60.080
2035	31.03	12.39	39.927	18.64	60.073
2036	31.65	12.64	39.934	19.01	60.066
2037	32.28	12.89	39.941	19.39	60.059
2038	32.91	13.14	39.948	19.76	60.052
2039	33.53	13.40	39.955	20.13	60.045
2040	34.17	13.65	39.962	20.51	60.038
2041	34.92	13.96	39.970	20.96	60.030
2042	35.69	14.27	39.978	21.42	60.022
2043	36.44	14.57	39.986	21.87	60.014
2044	37.21	14.88	39.994	22.33	60.006
2045	37.97	15.19	40.002	22.78	59.998
2046	38.74	15.50	40.010	23.24	59.990
2047	39.51	15.81	40.017	23.70	59.983
2048	40.29	16.12	40.024	24.16	59.976
2049	41.06	16.43	40.031	24.62	59.969
2050	41.83	16.75	40.038	25.08	59.962
Total	846.38	338.11	39.948	508.27	60.052
Average	32.55	13.00	39.948	19.55	60.052

Table 10. Expected Crash Frequencies by Year (Section 1)

Year	Total Crashes	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
2025	10.57	4.42	41.795	6.15	58.186
2026	11.35	4.75	41.844	6.60	58.142
2027	12.12	5.08	41.884	7.04	58.105
2028	12.88	5.40	41.917	7.48	58.074
2029	13.64	5.72	41.947	7.92	58.048
2030	13.95	5.85	41.955	8.10	58.040
2031	14.26	5.98	41.963	8.27	58.032
2032	14.56	6.11	41.971	8.45	58.025
2033	14.87	6.24	41.979	8.63	58.018
2034	15.18	6.38	41.987	8.81	58.011
2035	15.49	6.51	41.995	8.99	58.003
2036	15.80	6.64	42.002	9.17	57.997
2037	16.12	6.77	42.009	9.35	57.990
2038	16.43	6.90	42.017	9.53	57.983
2039	16.74	7.04	42.024	9.71	57.977
2040	17.06	7.17	42.031	9.89	57.970
2041	17.44	7.33	42.040	10.11	57.962
2042	17.82	7.49	42.049	10.33	57.954
2043	18.20	7.65	42.057	10.54	57.946
2044	18.58	7.82	42.066	10.77	57.938
2045	18.96	7.98	42.073	10.98	57.931
2046	19.35	8.14	42.081	11.21	57.924
2047	19.73	8.30	42.089	11.43	57.917
2048	20.12	8.47	42.097	11.65	57.910
2049	20.50	8.63	42.104	11.87	57.903
2050	20.89	8.80	42.112	12.09	57.896
Total	422.63	177.58	42.017	245.06	57.983
Average	16.25	6.83	42.017	9.43	57.983

Table 11. Comparing Predicted and Expected Crashes for the Evaluation Period (Section 1)

Scope	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
Predicted	846.38	338.11	39.948	508.27	60.052
Expected	422.63	177.58	42.017	245.06	57.983
Expected - Predicted	-423.75	-160.53		-263.21	
Percent Difference	-100.26	-90.40		-107.41	

**Table 12. Expected Crash Type Distribution (Section 1)** 

Element Type	Crash Type	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)	Total Crashes	Percent Total (%)
Highway Segment	Collision with Animal	2.82	0.7	21.39	5.1	23.04	5.5
Highway Segment	Collision with Bicycle	0.30	0.1	0.12	0.0	0.38	0.1
Highway Segment	Other Single-vehicle Collision	0.52	0.1	3.37	0.8	4.00	0.9
Highway Segment	Overturned	2.75	0.7	1.74	0.4	4.76	1.1
Highway Segment	Collision with Pedestrian	0.52	0.1	0.12	0.0	0.57	0.1
Highway Segment	Run Off Road	40.43	9.6	58.70	13.9	99.20	23.5
Highway Segment	Total Single Vehicle Crashes	47.33	11.2	85.43	20.2	131.96	31.2
Highway Segment	Angle Collision	7.49	1.8	8.37	2.0	16.18	3.8
Highway Segment	Head-on Collision	2.52	0.6	0.35	0.1	3.05	0.7
Highway Segment	Other Multiple-vehicle Collision	1.93	0.5	3.49	0.8	5.14	1.2
Highway Segment	Rear-end Collision	12.24	2.9	14.18	3.4	27.04	6.4
Highway Segment	Sideswipe	2.82	0.7	4.42	1.0	7.04	1.7
Highway Segment	Total Multiple Vehicle Crashes	27.00	6.4	30.80	7.3	58.46	13.8
Highway Segment	Total Highway Segment Crashes	74.33	17.6	116.23	27.5	190.41	45.1
Intersection	Collision with Animal	0.64	0.2	1.95	0.5	2.53	0.6
Intersection	Collision with Bicycle	0.10	0.0	0.13	0.0	0.23	0.1
Intersection	Other Single-vehicle Collision	0.49	0.1	1.41	0.3	1.86	0.4
Intersection	Overturned	0.79	0.2	0.55	0.1	1.35	0.3
Intersection	Collision with Pedestrian	0.10	0.0	0.13	0.0	0.23	0.1
Intersection	Run Off Road	11.27	2.7	19.84	4.7	31.15	7.4
Intersection	Total Single Vehicle Crashes	13.39	3.2	24.02	5.7	37.35	8.8
Intersection	Angle Collision	52.29	12.4	43.80	10.4	95.60	22.6
Intersection	Head-on Collision	6.43	1.5	3.31	0.8	9.57	2.3
Intersection	Other Multiple-vehicle Collision	4.43	1.0	4.72	1.1	9.13	2.2
Intersection	Rear-end Collision	22.24	5.3	34.59	8.2	57.03	13.5
Intersection	Sideswipe	4.62	1.1	18.39	4.4	23.36	5.5
Intersection	Total Multiple Vehicle Crashes	90.01	21.3	104.80	24.8	194.69	46.1
Intersection	Total Intersection Crashes	103.40	24.5	128.82	30.5	232.04	54.9
	Total Crashes	177.72	42.1	245.06	58.0	422.45	100.0

**Table 13. Evaluation Message** 

Start Location (Sta. ft)	End Location (Sta. ft)	Message
580+00.000	580+00.000	Warning: for intersection #3 (580+00.000 to 580+00.000 ), SE SD-38 at 580+00.000 has more than one lane exiting. No intersection crash prediction computed.
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,560 vpd) for 2019 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,560 vpd) for 2020 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,560 vpd) for 2021 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,560 vpd) for 2022 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,640 vpd) for 2023 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
580+00.000	580+00.000	Warning: for intersection #3 (580+00.000 to 580+00.000 ), SE SD-38 at 580+00.000 has more than one lane exiting. No intersection crash prediction computed.
187+50.000	187+50.000	Warning: for intersection #5 (187+50.000 to 187+50.000), minor road traffic volume (3,503 vpd) for 2049 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
187+50.000	187+50.000	Warning: for intersection #5 (187+50.000 to 187+50.000), minor road traffic volume (3,570 vpd) for 2050 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,802 vpd) for 2025 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,882 vpd) for 2026 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,963 vpd) for 2027 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,044 vpd) for 2028 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,125 vpd) for 2029 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000), minor road traffic volume (4,221 vpd) for 2030 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,318 vpd) for 2031 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,415 vpd) for 2032 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,512 vpd) for 2033 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST

Start Location (Sta. ft)	End Location (Sta. ft)	Message
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,609 vpd) for 2034 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,705 vpd) for 2035 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,802 vpd) for 2036 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,899 vpd) for 2037 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,996 vpd) for 2038 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,093 vpd) for 2039 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,190 vpd) for 2040 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,308 vpd) for 2041 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,427 vpd) for 2042 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,545 vpd) for 2043 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,664 vpd) for 2044 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,782 vpd) for 2045 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,901 vpd) for 2046 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (6,019 vpd) for 2047 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (6,138 vpd) for 2048 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (6,256 vpd) for 2049 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (6,375 vpd) for 2050 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST

# Interactive Highway Safety Design Model

# **Crash Prediction Evaluation Report**

### Disclaimer

The Interactive Highway Design Model (IHSDM) software is disseminated under the sponsorship of the Department of Transportation in the interest of information exchange. The United States Government assumes no liability for its content or use thereof. This document does not constitute a standard, specification, or regulation.

The United States Government does not endorse products or manufacturers. Trade and manufacturers' names may appear in this software and documentation only because they are considered essential to the objective of the software.

### **Limited Warranty and Limitations of Remedies**

This software product is provided "as-is," without warranty of any kind-either expressed or implied (but not limited to the implied warranties of merchantability and fitness for a particular purpose). The FHWA do not warrant that the functions contained in the software will meet the end-user's requirements or that the operation of the software will be uninterrupted and error-free.

Under no circumstances will the FHWA be liable to the end-user for any damages or claimed lost profits, lost savings, or other incidental or consequential damages rising out of the use or inability to use the software (even if these organizations have been advised of the possibility of such damages), or for any claim by any other party.

#### **Notice**

The use of the IHSDM software is being done strictly on a voluntary basis. In exchange for provision of IHSDM, the user agrees that the Federal Highway Administration (FHWA), U.S. Department of Transportation and any other agency of the Federal Government shall not be responsible for any errors, damage or other liability that may result from any and all use of the software, including installation and testing of the software. The user further agrees to hold the FHWA and the Federal Government harmless from any resulting liability. The user agrees that this hold harmless provision shall flow to any person to whom or any entity to which the user provides the IHSDM software. It is the user's full responsibility to inform any person to whom or any entity to which it provides the IHSDM software of this hold harmless provision.

## **Table of Contents**

Report Overview	1
Disclaimer Regarding Crash Prediction Method	2
Section Types	3
Section 4 Evaluation	3
Section 3 Evaluation	31
Section 5 Evaluation	48
Section 6 Evaluation	65
List of Tables	
Table Observed Crashes Used in the Evaluation (Section 4)	5
Table Evaluation Highway - Homogeneous Segments (Section 4)	6
Table User Defined CMF Used in the Eval Segment CPM Evaluation (Section 4)	10
Table Crash History Highway - Homogeneous Segments (Section 4)	11
Table Evaluation Intersection - Section 4	14
Table Evaluation Intersection - Section 4	15
Table Evaluation Intersection (Section 4)	16
Table Evaluation Ramp Terminal - Site (Section 4)	17
Table Crash History Intersection - Section 4	18
Table Crash History Intersection - Section 4	19
Table Crash History Intersection (Section 4)	20
Table Crash Highway Ramp Terminal - Site (Highway with Crash History)	21
Table Expected Highway Crash Rates and Frequencies Summary (Section 4)	22
Table Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 4)	23
Table Expected Crash Frequencies and Rates by Horizontal Design Element (Section 4)	25
Table Predicted Crash Frequencies by Year (Section 4)	26
Table Expected Crash Frequencies by Year (Section 4)	27
Table Comparing Predicted and Expected Crashes for the Evaluation Period (Section 4)	28
Table Expected Crash Severity by Ramp Terminal or Roundabout (Section 4)	28
Table Expected Crash Type Distribution (Section 4)	29
Table Evaluation Message	30
Table Observed Crashes Used in the Evaluation (Section 3)	33
Table Evaluation Highway - Homogeneous Segments (Section 3)	34
Table User Defined CMF Used in the Eval Segment CPM Evaluation (Section 3)	35
Table Crash History Highway - Homogeneous Segments (Section 3)	36
Table Evaluation Intersection (Section 3)	37
Table Crash History Intersection (Section 3)	38

Table Expected Highway Crash Rates and Frequencies Summary (Section 3)	39
Table Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 3)	40
Table Expected Crash Frequencies and Rates by Horizontal Design Element (Section 3)	41
Table Predicted Crash Frequencies by Year (Section 3)	42
Table Expected Crash Frequencies by Year (Section 3)	43
Table Comparing Predicted and Expected Crashes for the Evaluation Period (Section 3)	44
Table Expected Crash Type Distribution (Section 3)	45
Table Evaluation Message	46
Table Observed Crashes Used in the Evaluation (Section 5)	50
Table Evaluation Highway - Homogeneous Segments (Section 5)	51
Table User Defined CMF Used in the Eval Segment CPM Evaluation (Section 5)	52
Table Crash History Highway - Homogeneous Segments (Section 5)	53
Table Evaluation Intersection (Section 5)	54
Table Crash History Intersection (Section 5)	55
Table Expected Highway Crash Rates and Frequencies Summary (Section 5)	56
Table Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 5)	57
Table Expected Crash Frequencies and Rates by Horizontal Design Element (Section 5)	58
Table Predicted Crash Frequencies by Year (Section 5)	<b>5</b> 9
Table Expected Crash Frequencies by Year (Section 5)	60
Table Comparing Predicted and Expected Crashes for the Evaluation Period (Section 5)	61
Table Expected Crash Type Distribution (Section 5)	62
Table Evaluation Message	63
Table Observed Crashes Used in the Evaluation (Section 6)	67
Table Evaluation Highway - Homogeneous Segments (Section 6)	68
Table Crash Highway Highway - Homogeneous Segments (Section 6)	70
Table Evaluation Intersection (Section 6)	71
Table Crash History Intersection (Section 6)	72
Table Expected Highway Crash Rates and Frequencies Summary (Section 6)	73
Table Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 6)	74
Table Expected Crash Frequencies and Rates by Horizontal Design Element (Section 6)	75
Table Predicted Crash Frequencies by Year (Section 6)	76
Table Expected Crash Frequencies by Year (Section 6)	77
Table Comparing Predicted and Expected Crashes for the Evaluation Period (Section 6)	78
Table Expected Five Lane or Fewer Crash Type Distribution (Section 6)	<b>7</b> 9
List of Figures	
Figure Crash Prediction Summary (Section 4)	4
Figure Crash Prediction Summary (Section 3)	32
Figure Crash Prediction Summary (Section 5)	49

66

### **Report Overview**

Report Generated: Jun 1, 2024 2:26 PM

Report Template: System: Single Page, 508 Compliant [System] (mlcpm5, Dec 5, 2019 2:16 PM)

**Evaluation Date:** Sat Jun 01 14:21:35 CDT 2024 **IHSDM Version:** v17.0.0 (Sep 22, 2021)

Crash Prediction Module: v12.0.0 (Sep 22, 2021)

User Name: naveen.mallipaddi

**Organization Name:** 

Phone: E-Mail:

Project Title: SD-38\_Build\_Option1\_I90EBRamp\_I

Project Comment: Created Mon Mar 27 16:47:43 CDT 2023

Project Unit System: U.S. Customary

**Highway Title: SD-38** 

Highway Comment: Created Mon Mar 27 16:49:47 CDT 2023

**Highway Version: 22** 

**Evaluation Title:** Evaluation 55

Evaluation Comment: Created Sat Jun 01 14:16:24 CDT 2024

**Minimum Location:** 585+00.000 **Maximum Location:** 974+11.000

Policy for Superelevation: AASHTO 2011 U.S. Customary

Calibration: HSM Configuration

Crash Distribution: HSM Configuration Model/CMF: HSM Configuration

First Year of Analysis: 2025 Last Year of Analysis: 2050

**Empirical-Bayes Analysis:** Site-Specific **Highway with Crash History:** SD-38

Highway with Crash History Comment: Created Mon Mar 27 16:49:47 CDT 2023

Highway with Crash History Version: 22 First Year of Observed Crashes: 2018 Last Year of Observed Crashes: 2022

### **Disclaimer Regarding Crash Prediction Method**

IMPORTANT NOTICE ABOUT COMPARING RESULTS FROM HIGHWAY SAFETY MANUAL FIRST EDITION (2010) MODELS TO RESULTS FROM NEW MODELS DEVELOPED UNDER NCHRP PROJECTS 17-70, 17-58, AND 17-68

Since the publication of the Highway Safety Manual - First Edition (HSM-1), in 2010 by the American Association of State Highway and Transportation Officials (AASHTO), multiple research efforts have been undertaken through the National Cooperative Highway Research Program (NCHRP) to develop safety performance models for road segment and intersection facility types that were not initially reflected in the HSM-1, in order to expand the breadth and depth of the HSM in the future.

The IHSDM Crash Prediction Module (CPM) is intended as a faithful implementation of HSM Part C predictive methods. As NCHRP projects to develop new predictive methods for the HSM are completed, FHWA works to incorporate the new methods into IHSDM, sometimes in advance of publication in the HSM. The following new crash predictive methods have been accepted by NCHRP project panels and incorporated into IHSDM, while pending AASHTO's approval for incorporation into a future edition of the HSM:

- Roundabouts: completed in 2018 under NCHRP Project 17-70, the new methods will provide improved outcomes for the safety analysis of roundabouts.
- 6+ lane and one-way urban/suburban arterials (including models for segments and intersections): completed under NCHRP Project 17-58.
- Intersection crash prediction methods for some intersection configurations and traffic control types not currently addressed in the HSM (e.g., all-way stop; rural 3-leg signalized; 3-leg stop-controlled where the major leg turns; urban 5-leg signalized; urban high-speed intersections): completed in 2021 under NCHRP Project 17-68.

However, in the absence of local calibration factors (see HSM-1 Part C, Appendix A for guidance on calibration of the predictive models), it is neither appropriate nor advisable to directly compare the results from new models (from NCHRP Projects 17-58, 17-68, and 17-70) to results from HSM-1 models, as the models were not calibrated to the same base state data sets, and consequently can produce unexpected results. If local calibration factors are available and applied to both new models and HSM-1 models, then it may be appropriate to directly compare the results. [Note: Work being performed under NCHRP Project 17-72 (Update of Crash Modification Factors for the Highway Safety Manual) is expected to re-calibrate many of the old (HSM-1) and new (e.g., NCHRP 17-70) models to data from a single (or small number of) states, that would allow results from all models to be directly compared.]

The models produced for NCHRP Project 17-70 have independent value in terms of informing the design of a roundabout and assessing the effects of different design characteristics on the expected safety performance of a roundabout.

The HSM-1 interim method previously included in IHSDM for evaluating roundabouts on urban/suburban arterials (i.e., evaluating an existing intersection and then applying a Crash Modification Factor for replacing the existing intersection with a roundabout) has been deactivated in IHSDM, to minimize any confusion with the new roundabout methodology.

# **Section Types**

### **Section 4 Evaluation**

Section: Section 4

**Evaluation Start Location:** 676+00.000 **Evaluation End Location:** 862+60.000

Area Type: Rural

Functional Class: Arterial

Type of Alignment: Undivided, Two Lane

Model Category: Rural, Two Lane

**Calibration Factor:** 2U=1.0; 3ST=1.0; 4ST=1.0; RT\_ST\_FI=1.0; RT\_ST\_PDO=1.0;

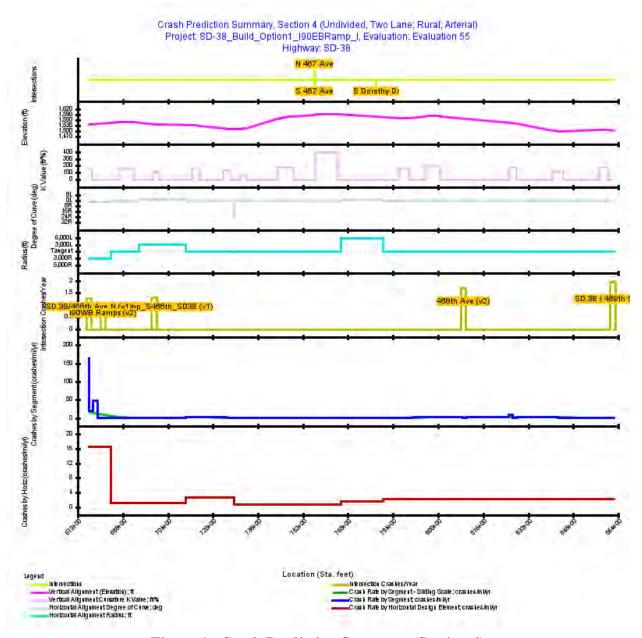


Figure 1. Crash Prediction Summary (Section 4)

Table 1. Observed Crashes Used in the Evaluation (Section 4)

Year	Observed Crashes	Total Crashes Used	FI Crashes	FI no/C Crashes	PDO Crashes
2018	6	6	5	5	1
2019	7	7	3	0	4
2020	3	3	1	0	2
2021	4	4	2	1	2
2022	7	7	4	0	3
All Years	27 <sup>[1]</sup>	27	15	6	12

#### **Footnotes**

[1] Note: Observed crash data that does not comply with the associated CPM model requirements may not be used in EB processing.

 Table 2. Evaluation Highway - Homogeneous Segments (Section 4)

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	y Density	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Design Speed d (mpl
22	Rural Two-Lane Segment Two-lane Undivided	676+00. 000	676+15. 090	15.09	0.002 9	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,529; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.0 0	12.0	0.00	8.00	2.61	4.0	3	false	0	false	false	false	3,101. 89	2.0	true	e 7
23	Rural Two-Lane Segment Two-lane Undivided	676+15. 090	677+50. 000	134.9 1	0.025	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,888; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.0 0	12.0	0.00	8.00	1.34	4.0	3	false	0	false	false	false	3,101. 89	2.0	true	e 7
24	Rural Two-Lane Segment Two-lane Undivided	677+50. 000	679+00. 000	150.0 0	0.028 4	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.0 0	12.0 0	8.00	8.00	1.34	4.0	3	false	0	false	false	false	3,101. 89	2.0	true	e 7
25	Rural Two-Lane Segment Two-lane Undivided	679+00. 000	680+80. 000	180.0 0	0.034	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.0	12.0	8.00	8.00	1.34	4.0	3	false	0	false	false	false	3,101. 89	2.0	true	e 7
26	Rural Two-Lane Segment Two-lane Undivided	680+80. 000	680+90. 000	10.00	0.001	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.0 0	12.0	0.00	8.00	1.34	4.0	3	false	0	false	false	false	3,101. 89	2.0	true	e 7
27	Rural Two-Lane Segment Two-lane Undivided	680+90. 000	682+20. 000	130.0	0.024	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.0 0	12.0	0.00	8.00	1.34	4.0	3	false	0	false	false	false	3,101. 89	2.0	true	e 7
28	Rural Two-Lane Segment Two-lane Undivided	682+20. 000	682+30. 000	10.00	0.001	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.0 0	12.0	8.00	8.00	1.34	4.0	3	false	0	false	false	false	3,101. 89	2.0	true	e 7
29	Rural Two-Lane Segment Two-lane Undivided	682+30. 000	683+82. 710	152.7 1	0.028	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.0 0	12.0	8.00	8.00	1.34	4.0	3	false	0	false	false	false	3,101. 89	2.0	true	e 7
30	Rural Two-Lane Segment Two-lane Undivided	683+82. 710	689+36. 990	554.2 8	0.105	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.0	12.0	8.00	8.00	1.34	4.0	3	false	0	false	false	false				
31	Rural Two-Lane Segment Two-lane Undivided	689+36. 990	691+50. 000	213.0	0.040	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.0	12.0	8.00	8.00	1.67	4.0	3	false	0	false	false	false				
32	Rural Two-Lane Segment Two-lane Undivided	691+50. 000	692+70. 000	120.0	0.022 7	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.0	12.0	8.00	8.00	1.67	4.0	3	false	0	false	false	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
33	Rural Two-Lane Segment Two-lane Undivided	692+70. 000	693+85. 010		0.021	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.0	12.0	8.00	8.00	1.67	4.0	3	false	0	false	false	false				
34	Rural Two-Lane Segment Two-lane Undivided	693+85. 010	698+70. 000	484.9 9	0.091	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.0	12.0	8.00	8.00	1.67	4.0	3	false	0	false	false	false	3,038. 64	2.0	tru	e 70
35	Rural Two-Lane Segment Two-lane Undivided	698+70. 000	699+00. 000	30.00	0.005	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.0	12.0	0.00	8.00	1.67	4.0	3	false	0	false	false	false	3,038. 64	2.0	tru	e 70
36	Rural Two-Lane Segment Two-lane Undivided	699+00. 000	699+20. 000	20.00	0.003	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.0 0	12.0	0.00	0.00	1.67	4.0	3	false	0	false	false	false	3,038. 64	2.0	tru	e 70
37	Rural Two-Lane Segment Two-lane Undivided	699+20. 000	699+73. 960	53.96	0.010	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.0	12.0	0.00	0.00	1.67	4.0	3	false	0	false	false	false	3,038. 64	2.0	tru	e 70
38	Rural Two-Lane Segment Two-lane Undivided	699+73. 960	700+50. 000	76.04	0.014 4	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351: 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.0 0	12.0 0	0.00	0.00	0.29	4.0	3	false	0	false	false	false	3,038. 64	2.0	tru	e 70
39	Rural Two-Lane Segment Two-lane Undivided	700+50. 000	702+00. 000	150.0 0	0.028 4	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.0 0	12.0 0	8.00	0.00	0.29	4.0	3	false	0	false	false	false	3,038. 64	2.0	tru	e 70
40	Rural Two-Lane Segment Two-lane Undivided	702+00. 000	702+50. 000	50.00	0.009	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0	8.00	0.00	0.29	4.0	3	false	0	false	false	false	3,038. 64	2.0	tru	e 70
41	Rural Two-Lane Segment Two-lane Undivided	702+50. 000	710+47. 850	797.8 5	0.151	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0	8.00	8.00	0.29	4.0	3	false	0	false	false	false	3,038. 64	2.0	tru	e 70
42	Rural Two-Lane Segment Two-lane Undivided	710+47. 850	713+88. 360	340.5 1	0.064	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0 0	8.00	8.00	0.29	4.0	3	false	0	false	false	false				
43	Rural Two-Lane Segment Two-lane Undivided	713+88. 360	725+01. 220	1,112. 86		2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,544; 2049: 9,712; 2050: 9,860	12.0 0	12.0 0	8.00	8.00	1.80	4.0	3	false	0	false	false	false				
44	Rural Two-Lane Segment Two-lane Undivided	725+01. 220	727+00. 000	198.7 8	0.037	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2048: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0 0	8.00	8.00	0.06	4.0	3	false	0	false	false	false				
45	Rural Two-Lane Segment Two-lane Undivided	727+00. 000	727+52. 350	52.35		2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2048: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0	8.00	8.00	0.06	4.0	3	false	0	false	false	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
46	Rural Two-Lane Segment Two-lane Undivided	727+52. 350	730+90. 510	338.1 6	0.064 0	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2049: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,842; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0	12.0 0	8.00	8.00	0.06	4.0	3	false	0	false	false	false				
47	Rural Two-Lane Segment Two-lane Undivided	730+90. 510	735+00. 000	409.4 9		2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0 0	8.00	8.00	4.47	4.0	3	false	0	false	false	false				
48	Rural Two-Lane Segment Two-lane Undivided	735+00. 000	739+00. 000	400.0 0		2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2034: 8,842; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0 0	8.00	8.00	4.47	4.0	3	false	0	false	false	false				
49	Rural Two-Lane Segment Two-lane Undivided	739+00. 000	744+50. 000	550.0 0		2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0 0	8.00	8.00	4.47	4.0	3	false	0	false	false	false				
50	Rural Two-Lane Segment Two-lane Undivided	744+50. 000	745+69. 220	119.2 2		2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0 0	8.00	8.00	4.47	4.0	3	false	0	false	false	false				
51	Rural Two-Lane Segment Two-lane Undivided	745+69. 220	751+00. 000	530.7 8	0.100 5	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0 0	8.00	8.00	1.12	4.0	3	false	0	false	false	false				
52	Rural Two-Lane Segment Two-lane Undivided	751+00. 000	760+50. 000	950.0 0		2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0 0	8.00	8.00	1.12	4.0	3	false	0	false	false	false				
53	Rural Two-Lane Segment Two-lane Undivided	760+50. 000	765+52. 550	502.5 5		2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0 0	8.00	8.00	0.88	4.0	3	false	0	false	false	false				
54	Rural Two-Lane Segment Two-lane Undivided	765+52. 550	767+00. 000	147.4 5		2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0 0	8.00	8.00	0.88	4.0	3	false	0	false	false	false	5,888. 09	2.0	true	70
55	Rural Two-Lane Segment Two-lane Undivided	767+00. 000	780+45. 930	1,345. 93		2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0 0	8.00	8.00	0.88	4.0	3	false	0	false	false	false	5,888. 09	2.0	true	70
56	Rural Two-Lane Segment Two-lane Undivided	780+45. 930	787+93. 440	747.5 1	0.141	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0 0	8.00	8.00	0.88	4.0	3	false	0	false	false	false				
57	Rural Two-Lane Segment Two-lane Undivided	787+93. 440	791+00. 000	306.5 6	0.058	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0 0	12.0 0	8.00	8.00	1.46	4.0	3	false	0	false	false	false				
58	Rural Two-Lane Segment Two-lane Undivided	791+00. 000	791+10. 000	10.00		2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0	12.0 0	8.00	8.00	1.46	4.0	3	false	0	false	false	false				
59	Rural Two-Lane Segment Two-lane Undivided	791+10. 000	798+12. 000	702.0 0		2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0	12.0 0	8.00	8.00	1.46	4.0	3	false	0	false	false	false				
60	Rural Two-Lane Segment Two-lane Undivided	798+12. 000	808+80. 000	1,068. 00	0.202	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.0	12.0 0	8.00	8.00	1.46	4.0	3	false	0	false	false	false				

Seg No.	Type	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
61	Rural Two-Lane Segment Two-lane Undivided	808+80. 000	809+00. 000	20.00	0.003	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,554; 2049: 9,712; 2050: 9,860	12.0 0	12.0 0	8.00	8.00	- 1.46	4.0	3	false	0	false	false	false				
62	Rural Two-Lane Segment Two-lane Undivided	809+00. 000	810+00. 000	100.0 0	0.018 9	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,690; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.0 0	12.0 0	0.00	0.00	1.46	4.0	3	false	0	false	false	false				
63	Rural Two-Lane Segment Two-lane Undivided	810+00. 000	825+00. 000	1,500. 00	0.284	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,690; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.0	12.0	8.00	8.00	1.46	4.0	3	false	0	false	false	false				
64	Rural Two-Lane Segment Two-lane Undivided	825+00. 000	826+54. 070	154.0 7	0.029	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,690; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.0	12.0	8.00	8.00	1.46	4.0	3	false	0	false	false	false				
65	Rural Two-Lane Segment Two-lane Undivided	826+54. 070	828+00. 000	145.9 3	0.027 6	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,690; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.0	12.0 0	8.00	8.00	2.84	4.0	3	false	0	false	false	false				
66	Rural Two-Lane Segment Two-lane Undivided	828+00. 000	842+53. 930	1,453. 93	0.275 4	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,690; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.0	12.0	8.00	8.00	2.84	4.0	3	false	0	false	false	false				
67	Rural Two-Lane Segment Two-lane Undivided	842+53. 930	854+00. 000	1,146. 07	0.217 1	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,690; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.0 0	12.0 0	8.00	8.00	0.59	4.0	3	false	0	false	false	false				
68	Rural Two-Lane Segment Two-lane Undivided	854+00. 000	854+70. 000	70.00	0.013 3	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,690; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.0 0	12.0 0	8.00	8.00	0.59	4.0	3	false	0	false	false	false				
69	Rural Two-Lane Segment Two-lane Undivided	854+70. 000	855+80. 000	110.0 0	0.020	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,690; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.0	12.0	8.00	8.00	0.59	4.0	3	false	0	false	false	false				
70	Rural Two-Lane Segment Two-lane Undivided	855+80. 000	858+75. 120	295.1 2	0.055	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,690; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.0	12.0	8.00	8.00	0.59	4.0	3	false	0	false	false	false				
71	Rural Two-Lane Segment Two-lane Undivided	858+75. 120	861+85. 000	309.8 8	0.058 7	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,690; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.0 0	12.0 0	8.00	8.00	1.07	4.0	3	false	0	false	false	false				
72	Rural Two-Lane Segment Two-lane Undivided	861+85. 000	862+00. 000	15.00	0.002	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,690; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.0	12.0	8.00	0.00	1.07	4.0	3	false	0	false	false	false				

Seg No	Туре		End Locatio n (Sta. ft)	Lengt		AADT	(ft)		Left Shoulde r Width (ft)	Shoulde		Drivewa y Density (drivewa ys/mi)		Centerline Rumble Strip	σ	Lon	Lightin g	Automated Speed Enforceme nt	Radiu	Superelevation (%)	Desig n Spee d (mph
73	Rural Two-Lane Segment Two-lane Undivided	862+00. 000	862+50. 000		0.009	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.0	12.0	8.00	0.00	1.07	4.0	3	false	0	false	false	false			
74	Rural Two-Lane Segment Two-lane Undivided	862+50. 000	862+60. 000	10.00	0.001	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.0	12.0	8.00	8.00	1.07	4.0	3	false	0	false	false	false			

**Table 3. User Defined CMF Used in the Eval Segment CPM Evaluation (Section 4)** 

Nan	ne Description	Start Loc. (Sta. ft)	End Loc. (Sta. ft)	Start CMF Year	End CMF Year	Severity	CMF Value
1	TWLTL	676+00.000	680+90.000	2025	2025	Total	0.6900

**Table 4. Crash History Highway - Homogeneous Segments (Section 4)** 

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazard Rating	Centerline Rumble Strip	Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
22	Rural Two-Lane Segment Two-lane Undivided	676+00.00 0	676+15.09 0	15.09	0.0029	2018-2022: 4,325	12.00	12.00	0.00	8.00	2.61	4.0	3	false	0	false	false	false	3,101.89	2.0	true	70
23	Rural Two-Lane Segment Two-lane Undivided	676+15.09 0	677+50.00 0	134.91	0.0256	2018-2022: 4,325	12.00	12.00	0.00	8.00	1.34	4.0	3	false	0	false	false	false	3,101.89	2.0	true	70
24	Rural Two-Lane Segment Two-lane Undivided	677+50.00 0	679+00.00 0	150.00	0.0284	2018-2022: 4,325	12.00	12.00	8.00	8.00	1.34	4.0	3	false	0	false	false	false	3,101.89	2.0	true	70
25	Rural Two-Lane Segment Two-lane Undivided	679+00.00 0	680+80.00 0	180.00	0.0341	2018-2022: 4,150	12.00	12.00	8.00	8.00	1.34	4.0	3	false	0	false	false	false	3,101.89	2.0	true	70
26	Rural Two-Lane Segment Two-lane Undivided	680+80.00 0	680+90.00 0	10.00	0.0019	2018-2022: 4,150	12.00	12.00	0.00	8.00	1.34	4.0	3	false	0	false	false	false	3,101.89	2.0	true	70
27	Rural Two-Lane Segment Two-lane Undivided	680+90.00 0	682+20.00 0	130.00	0.0246	2018-2022: 4,150	12.00	12.00	0.00	8.00	1.34	4.0	3	false	0	false	false	false	3,101.89	2.0	true	70
28	Rural Two-Lane Segment Two-lane Undivided	682+20.00 0	682+30.00 0	10.00	0.0019	2018-2022: 4,150	12.00	12.00	8.00	8.00	1.34	4.0	3	false	0	false	false	false	3,101.89	2.0	true	70
29	Rural Two-Lane Segment Two-lane Undivided	682+30.00 0	683+82.71 0	152.71	0.0289	2018-2022: 4,150	12.00	12.00	8.00	8.00	1.34	4.0	3	false	0	false	false	false	3,101.89	2.0	true	70
30	Rural Two-Lane Segment Two-lane Undivided	683+82.71 0	689+36.99 0	554.28	0.1050	2018-2022: 4,150	12.00	12.00	8.00	8.00	1.34	4.0	3	false	0	false	false	false				
31	Rural Two-Lane Segment Two-lane Undivided	689+36.99 0	691+50.00 0	213.01	0.0403	2018-2022: 4,150	12.00	12.00	8.00	8.00	-1.67	4.0	3	false	0	false	false	false				
32	Rural Two-Lane Segment Two-lane Undivided	691+50.00 0	692+70.00 0	120.00	0.0227	2018-2022: 4,150	12.00	12.00	8.00	8.00	-1.67	4.0	3	false	0	false	false	false				
33	Rural Two-Lane Segment Two-lane Undivided	692+70.00 0	693+85.01 0	115.01	0.0218	2018-2022: 4,150	12.00	12.00	8.00	8.00	-1.67	4.0	3	false	0	false	false	false				
34	Rural Two-Lane Segment Two-lane Undivided	693+85.01 0	698+70.00 0	484.99	0.0919	2018-2022: 4,150	12.00	12.00	8.00	8.00	-1.67	4.0	3	false	0	false	false	false	3,038.64	2.0	true	70
35	Rural Two-Lane Segment Two-lane Undivided	698+70.00 0	699+00.00 0	30.00	0.0057	2018-2022: 4,150	12.00	12.00	0.00	8.00	-1.67	4.0	3	false	0	false	false	false	3,038.64	2.0	true	70
36	Rural Two-Lane Segment Two-lane Undivided	699+00.00 0	699+20.00 0	20.00	0.0038	2018-2022: 4,150	12.00	12.00	0.00	0.00	-1.67	4.0	3	false	0	false	false	false	3,038.64	2.0	true	70
37	Rural Two-Lane Segment Two-lane Undivided	699+20.00 0	699+73.96 0	53.96	0.0102	2018-2022: 4,150	12.00	12.00	0.00	0.00	-1.67	4.0	3	false	0	false	false	false	3,038.64	2.0	true	70
38	Rural Two-Lane Segment Two-lane Undivided	699+73.96 0	700+50.00 0	76.04	0.0144	2018-2022: 4,150	12.00	12.00	0.00	0.00	-0.29	4.0	3	false	0	false	false	false	3,038.64	2.0	true	70
39	Rural Two-Lane Segment Two-lane Undivided	700+50.00 0	702+00.00 0	150.00	0.0284	2018-2022: 4,150	12.00	12.00	8.00	0.00	-0.29	4.0	3	false	0	false	false	false	3,038.64	2.0	true	70
40	Rural Two-Lane Segment Two-lane Undivided	702+00.00 0	702+50.00 0	50.00	0.0095	2018-2022: 4,900	12.00	12.00	8.00	0.00	-0.29	4.0	3	false	0	false	false	false	3,038.64	2.0	true	70
41	Rural Two-Lane Segment Two-lane Undivided	702+50.00 0	710+47.85 0	797.85	0.1511	2018-2022: 4,900	12.00	12.00	8.00	8.00	-0.29	4.0	3	false	0	false	false	false	3,038.64	2.0	true	70
42	Rural Two-Lane Segment Two-lane Undivided	710+47.85 0	713+88.36 0	340.51	0.0645	2018-2022: 4,900	12.00	12.00	8.00	8.00	-0.29	4.0	3	false	0	false	false	false				
43	Rural Two-Lane Segment Two-lane Undivided	713+88.36 0	725+01.22 0	1,112.86	0.2108	2018-2022: 4,900	12.00	12.00	8.00	8.00	-1.80	4.0	3	false	0	false	false	false				
44	Rural Two-Lane Segment Two-lane Undivided	725+01.22 0	727+00.00 0	198.78	0.0376	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.06	4.0	3	false	0	false	false	false				
45	Rural Two-Lane Segment Two-lane Undivided	727+00.00 0	727+52.35 0	52.35	0.0099	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.06	4.0	3	false	0	false	false	false				
46	Rural Two-Lane Segment Two-lane Undivided	727+52.35 0	730+90.51 0	338.16	0.0640	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.06	4.0	3	false	0	false	false	false				

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazard Rating	Centerline Rumble Strip	Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
47	Rural Two-Lane Segment Two-lane Undivided	730+90.51 0	735+00.00 0	409.49	0.0776	2018-2022: 4,900	12.00	12.00	8.00	8.00	4.47	4.0	3	false	0	false	false	false				
48	Rural Two-Lane Segment Two-lane Undivided	735+00.00 0	739+00.00 0	400.00	0.0758	2018-2022: 4,900	12.00	12.00	8.00	8.00	4.47	4.0	3	false	0	false	false	false				
49	Rural Two-Lane Segment Two-lane Undivided	739+00.00 0	744+50.00 0	550.00	0.1042	2018-2022: 4,900	12.00	12.00	8.00	8.00	4.47	4.0	3	false	0	false	false	false				
50	Rural Two-Lane Segment Two-lane Undivided	744+50.00 0	745+69.22 0	119.22	0.0226	2018-2022: 4,900	12.00	12.00	8.00	8.00	4.47	4.0	3	false	0	false	false	false				
51	Rural Two-Lane Segment Two-lane Undivided	745+69.22 0	751+00.00 0	530.78	0.1005	2018-2022: 4,900	12.00	12.00	8.00	8.00	1.12	4.0	3	false	0	false	false	false				
52	Rural Two-Lane Segment Two-lane Undivided	751+00.00 0	760+50.00 0	950.00	0.1799	2018-2022: 4,900	12.00	12.00	8.00	8.00	1.12	4.0	3	false	0	false	false	false				
53	Rural Two-Lane Segment Two-lane Undivided	760+50.00 0	765+52.55 0	502.55	0.0952	2018-2022: 4,900	12.00	12.00	8.00	8.00	-0.88	4.0	3	false	0	false	false	false				
54	Rural Two-Lane Segment Two-lane Undivided	765+52.55 0	767+00.00 0	147.45	0.0279	2018-2022: 4,900	12.00	12.00	8.00	8.00	-0.88	4.0	3	false	0	false	false	false	5,888.09	2.0	true	70
55	Rural Two-Lane Segment Two-lane Undivided	767+00.00 0	780+45.93 0	1,345.93	0.2549	2018-2022: 4,900	12.00	12.00	8.00	8.00	-0.88	4.0	3	false	0	false	false	false	5,888.09	2.0	true	70
56	Rural Two-Lane Segment Two-lane Undivided	780+45.93 0	787+93.44 0	747.51	0.1416	2018-2022: 4,900	12.00	12.00	8.00	8.00	-0.88	4.0	3	false	0	false	false	false				
57	Rural Two-Lane Segment Two-lane Undivided	787+93.44 0	791+00.00 0	306.56	0.0581	2018-2022: 4,900	12.00	12.00	8.00	8.00	1.46	4.0	3	false	0	false	false	false				
58	Rural Two-Lane Segment Two-lane Undivided	791+00.00 0	791+10.00 0	10.00	0.0019	2018-2022: 4,900	12.00	12.00	8.00	8.00	1.46	4.0	3	false	0	false	false	false				
59	Rural Two-Lane Segment Two-lane Undivided	791+10.00 0	798+12.00 0	702.00	0.1330	2018-2022: 4,900	12.00	12.00	8.00	8.00	1.46	4.0	3	false	0	false	false	false				
60	Rural Two-Lane Segment Two-lane Undivided	798+12.00 0	808+80.00 0	1,068.00	0.2023	2018-2022: 4,900	12.00	12.00	8.00	8.00	-1.46	4.0	3	false	0	false	false	false				
61	Rural Two-Lane Segment Two-lane Undivided	808+80.00 0	809+00.00 0	20.00	0.0038	2018-2022: 4,900	12.00	12.00	8.00	8.00	-1.46	4.0	3	false	0	false	false	false				
62	Rural Two-Lane Segment Two-lane Undivided	809+00.00 0	810+00.00 0	100.00	0.0189	2018-2022: 4,900	12.00	12.00	0.00	0.00	-1.46	4.0	3	false	0	false	false	false				
63	Rural Two-Lane Segment Two-lane Undivided	810+00.00 0	825+00.00 0	1,500.00	0.2841	2018-2022: 4,900	12.00	12.00	8.00	8.00	-1.46	4.0	3	false	0	false	false	false				
64	Rural Two-Lane Segment Two-lane Undivided	825+00.00 0	826+54.07 0	154.07	0.0292	2018-2022: 4,900	12.00	12.00	8.00	8.00	-1.46	4.0	3	false	0	false	false	false				
65	Rural Two-Lane Segment Two-lane Undivided	826+54.07 0	828+00.00 0	145.93	0.0276	2018-2022: 4,900	12.00	12.00	8.00	8.00	-2.84	4.0	3	false	0	false	false	false				
66	Rural Two-Lane Segment Two-lane Undivided	828+00.00 0	842+53.93 0	1,453.93	0.2754	2018-2022: 4,900	12.00	12.00	8.00	8.00	-2.84	4.0	3	false	0	false	false	false				
67	Rural Two-Lane Segment Two-lane Undivided	842+53.93 0	854+00.00 0	1,146.07	0.2171	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.59	4.0	3	false	0	false	false	false				
68	Rural Two-Lane Segment Two-lane Undivided	854+00.00 0	854+70.00 0	70.00	0.0133	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.59	4.0	3	false	0	false	false	false				
69	Rural Two-Lane Segment Two-lane Undivided	854+70.00 0	855+80.00 0	110.00	0.0208	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.59	4.0	3	false	0	false	false	false				
70	Rural Two-Lane Segment Two-lane Undivided	855+80.00 0	858+75.12 0	295.12	0.0559	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.59	4.0	3	false	0	false	false	false				
71	Rural Two-Lane Segment Two-lane Undivided	858+75.12 0	861+85.00 0	309.88	0.0587	2018-2022: 4,900	12.00	12.00	8.00	8.00	-1.07	4.0	3	false	0	false	false	false				
72	Rural Two-Lane Segment Two-lane Undivided	861+85.00 0	862+00.00 0	15.00	0.0028	2018-2022: 4,900	12.00	12.00	8.00	0.00	-1.07	4.0	3	false	0	false	false	false				
73	Rural Two-Lane Segment Two-lane Undivided	862+00.00 0	862+50.00 0	50.00	0.0095	2018-2022: 4,900	12.00	12.00	8.00	0.00	-1.07	4.0	3	false	0	false	false	false				

S	eg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Lane	wiat	Shoulder	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazard Rating		Passing	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
	74	Rural Two-Lane Segment Two-lane Undivided	862+50.00 0	862+60.00 0	10.00	0.0019	2018-2022: 4,900	12.00	12.00	8.00	8.00	-1.07	4.0	3	false	0	false	false	false				

### **Table 5. Evaluation Intersection - Section 4**

1	nter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
	2	I90EBRamp_S466th_SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	699+20.000	2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,148: 2045: 11,740: 2046: 12,002: 2047: 12,204: 2048: 12,586:	2025: 630; 2026: 644; 2027: 657; 2028: 671; 2029: 685; 2030: 700; 2031: 716; 2032: 732; 2033: 748; 2034: 764; 2035: 780; 2036: 796; 2037: 812; 2038: 828; 2039: 844; 2040: 860; 2041: 1,166; 2042: 1,473; 2043: 1,779; 2044: 2,086; 2045: 2,392; 2046: 2,699; 2047: 3,005; 2048: 3,312; 2049: 3,618; 2050: 3,925	4	Stop-Controlled	1	0	4.64	4.27	false

### **Table 6. Evaluation Intersection - Section 4**

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
5	468th Ave (v2)	Rural Two-Lane Intersection Four-Legged w/STOP control	809+00.000	2035: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,690; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700: 2041: 9,005: 2042: 10,110: 2043: 10, 315: 2044:	2025: 667; 2026: 682; 2027: 696; 2028: 710; 2029: 725; 2030: 741; 2031: 758; 2032: 775; 2033: 792; 2034: 809; 2035: 825; 2036: 842; 2037: 859; 2038: 876; 2039: 893; 2040: 910; 2041: 1,052; 2042: 1,195; 2043: 1,337; 2044: 1,480; 2045: 1,622; 2046: 1,765; 2047: 1,907; 2048: 2,050; 2049: 2,192; 2050: 2,335	4	Stop-Controlled	0	0	0.00	0.00	false
6	SD 38 / 469th St (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	862+00.000	2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048:	2025: 2,990; 2026: 3,054; 2027: 3,117; 2028: 3,181; 2029: 3,245; 2030: 3,321; 2031: 3,397; 2032: 3,474; 2033: 3,550; 2034: 3,626; 2035: 3,703: 2,036: 3,779; 2037: 3,855; 2038: 3,932; 2039: 4,008; 2040: 4,085; 2041: 4,178; 2042: 4,271; 2043: 4,364; 2044: 4,457; 2045: 4,550; 2046: 4,643; 2047: 4,736; 2048: 4,829; 2049: 4,922; 2050: 5,015	4	Stop-Controlled	0	1	0.00	0.00	false

## **Table 7. Evaluation Intersection (Section 4)**

I	nter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
			Rural Multi-Lane Intersection Three-Legged w/STOP control	676+00.000	12,6 (0), 2030; 12,905; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,01; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047;	2025: 118; 2026: 121; 2027: 124; 2028: 127; 2029: 130; 2030: 133; 2031: 136; 2032: 139; 2033: 142; 2034: 145; 2035: 149; 2036: 152; 2037: 155; 2038: 158; 2039: 161; 2040: 165; 2041: 168; 2042: 172; 2043: 175; 2044: 179; 2045: 182; 2046: 186; 2047: 189; 2048: 193; 2049: 196; 2050: 200	3	Stop-Controlled	0	0	8.65		false

# **Table 8. Evaluation Ramp Terminal - Site (Section 4)**

Inter. No.	Title	Туре	Area Type	Legs	Location (Sta. ft)	Traffic Control	AADT
	4 190WB Ramps (v2)	Freeway Ramp Terminal A2 - Three-Leg at Two-Quadrant Parcio A	Rural	4	681+00.000	Stop-Controlled	Inside: 2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,251; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,216; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170; Outside: 2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170: Entrance: 2025: 856; 2026: 875; 2027: 893; 2028: 911; 2029: 930; 2030: 951; 2031: 973; 2032: 995; 2033: 1,017; 2034: 1,039; 2035: 1,060; 2036: 1,082; 2037: 1,104; 2038: 1,126; 2039: 1,148; 2040: 1,170; 2041: 1,339; 2042: 1,508; 2043: 1,677; 2044: 1,846; 2045: 2,015; 2046: 2,184; 2047: 2,333; 2048: 2,522; 2049: 2,691; 2050: 2,860

# $\ \, \textbf{Table 9. Crash History Intersection - Section 4} \\$

Inter. No.	Title	Туре	Location (Sta.	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
2	I90EBRamp_S466th_SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	699+20.000	2018-2022: 4,150	2018-2022: 590	4	Stop-Controlled	1	0	4.64	4.27	false

## $\ \, \textbf{Table 10. Crash History Intersection - Section 4} \\$

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
5	468th Ave (v2)	Rural Two-Lane Intersection Four-Legged w/STOP control	809+00.000	2018-2022: 4,900	2018-2022: 625	4	Stop-Controlled	0	0	0.00	0.00	false
6	SD 38 / 469th St (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	862+00.000	2018-2022: 4,900	2018-2022: 2,800	4	Stop-Controlled	0	1	0.00	0.00	false

## **Table 11. Crash History Intersection (Section 4)**

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
3	SD 38/466th Ave N (v1)	Rural Multi-Lane Intersection Three-Legged w/STOP control	676+00.000	2018-2022: 4,325	2018-2022: 110	3	Stop-Controlled	0	0	8.65		false

### Table 12. Crash Highway Ramp Terminal - Site (Highway with Crash History)

Inter. No.	Title	Туре	Area Type	Legs	Location (Sta. ft)	Traffic Control	AADT
4	I90WB Ramps (v2)	Freeway Ramp Terminal A2 - Three-Leg at Two-Quadrant Parclo A	Rural	4	681+00.000	Stop-Controlled	Inside: 2018-2022: 4,150; Outside: 2018-2022: 4,150 :: Entrance: 2018-2022: 802

Table 13. Expected Highway Crash Rates and Frequencies Summary (Section 4)

First Year of Analysis   2025		
Evaluated Length (mi)   3.5341     Average Future Road AADT (vpd)   8,719     Expected Crashes	202	First Year of Analysis
Average Future Road AADT (vpd)   8,719	205	Last Year of Analysis
Total Crashes   395.38   395.38   Fatal and Injury Crashes   180.11	3.534	Evaluated Length (mi)
Total Crashes   395.38   180.11   Fatal and Injury Crashes   180.11   Fatal and Serious Injury Crashes   9.32   Property-Damage-Only Crashes   215.27   Percent of Total Expected Crashes   Percent Fatal and Injury Crashes (%)   46   Percent Fatal and Serious Injury Crashes (%)   2   Percent Property-Damage-Only Crashes (%)   54   Expected Crash Rate   Crash Rate (crashes/mi/yr)   4.3029   FI Crash Rate (crashes/mi/yr)   1.9602   FI no/C Crash Rate (crashes/mi/yr)   0.1015   PDO Crash Rate (crashes/mi/yr)   2.3428   Expected Travel Crash Rate   Total Travel (million veh-mi)   292.42   Travel FI Crash Rate (crashes/million veh-mi)   1.35   Travel FI Crash Rate (crashes/million veh-mi)   0.62   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.63   Travel FI Crash Rate (crashes/million veh-mi)   0.64   Travel FI Crash Rate (crashes/million veh-mi)   0.65   Travel FI Crash Rate (crashes/million veh-mi)   0.65   Travel FI Crash Rate (crashes/million veh-mi)   0.65   Travel FI Crash Rate (crashes/million veh-mi)   0.65   Travel FI Crash Rate (crashes/million veh-mi)   0.65   Travel FI Crash Rate (cr	8,71	Average Future Road AADT (vpd)
Fatal and Injury Crashes   180.11		Expected Crashes
Fatal and Serious Injury Crashes   9.32     Property-Damage-Only Crashes   215.27     Percent of Total Expected Crashes     Percent Fatal and Injury Crashes (%)   46     Percent Fatal and Serious Injury Crashes (%)   2     Percent Property-Damage-Only Crashes (%)   54     Expected Crash Rate     Crash Rate (crashes/mi/yr)   4.3029     FI Crash Rate (crashes/mi/yr)   1.9602     FI no/C Crash Rate (crashes/mi/yr)   0.1015     PDO Crash Rate (crashes/mi/yr)   2.3428     Expected Travel Crash Rate     Total Travel (million veh-mi)   1.35     Travel FI Crash Rate (crashes/million veh-mi)   0.62     Travel FI Crash Rate (crashes/million veh-mi)   0.03	395.3	Total Crashes
Property-Damage-Only Crashes  Percent of Total Expected Crashes  Percent Fatal and Injury Crashes (%) 46  Percent Fatal and Serious Injury Crashes (%) 2  Percent Property-Damage-Only Crashes (%) 54  Expected Crash Rate  Crash Rate (crashes/mi/yr) 4.3029  FI Crash Rate (crashes/mi/yr) 1.9602  FI no/C Crash Rate (crashes/mi/yr) 0.1015  PDO Crash Rate (crashes/mi/yr) 2.3428  Expected Travel Crash Rate  Total Travel (million veh-mi) 292.42  Travel Crash Rate (crashes/million veh-mi) 1.35  Travel FI Crash Rate (crashes/million veh-mi) 0.62  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.63	180.1	Fatal and Injury Crashes
Percent of Total Expected Crashes  Percent Fatal and Injury Crashes (%)  Percent Fatal and Serious Injury Crashes (%)  Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  FI no/C Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  O.62	9.3	Fatal and Serious Injury Crashes
Percent Fatal and Injury Crashes (%)  Percent Fatal and Serious Injury Crashes (%)  Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  FI no/C Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  1.35  Travel FI Crash Rate (crashes/million veh-mi)  O.62  Travel FI no/C Crash Rate (crashes/million veh-mi)	215.2	Property-Damage-Only Crashes
Percent Fatal and Serious Injury Crashes (%)  Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  FI no/C Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  O.62  Travel FI no/C Crash Rate (crashes/million veh-mi)  O.63		Percent of Total Expected Crashes
Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  FI no/C Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  O.62  Travel FI no/C Crash Rate (crashes/million veh-mi)  O.63	4	Percent Fatal and Injury Crashes (%)
Expected Crash Rate  Crash Rate (crashes/mi/yr) 4.3029  FI Crash Rate (crashes/mi/yr) 1.9602  FI no/C Crash Rate (crashes/mi/yr) 0.1015  PDO Crash Rate (crashes/mi/yr) 2.3428  Expected Travel Crash Rate  Total Travel (million veh-mi) 292.42  Travel Crash Rate (crashes/million veh-mi) 1.35  Travel FI Crash Rate (crashes/million veh-mi) 0.62  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.03		Percent Fatal and Serious Injury Crashes (%)
Crash Rate (crashes/mi/yr) 4.3029  FI Crash Rate (crashes/mi/yr) 1.9602  FI no/C Crash Rate (crashes/mi/yr) 0.1015  PDO Crash Rate (crashes/mi/yr) 2.3428  Expected Travel Crash Rate  Total Travel (million veh-mi) 292.42  Travel Crash Rate (crashes/million veh-mi) 1.35  Travel FI Crash Rate (crashes/million veh-mi) 0.62  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.03	5	Percent Property-Damage-Only Crashes (%)
FI Crash Rate (crashes/mi/yr)  1.9602  FI no/C Crash Rate (crashes/mi/yr)  DO Crash Rate (crashes/mi/yr)  2.3428  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  0.62  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.03		Expected Crash Rate
FI no/C Crash Rate (crashes/mi/yr) 0.1015  PDO Crash Rate (crashes/mi/yr) 2.3428  Expected Travel Crash Rate  Total Travel (million veh-mi) 292.42  Travel Crash Rate (crashes/million veh-mi) 1.35  Travel FI Crash Rate (crashes/million veh-mi) 0.62  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.03	4.302	Crash Rate (crashes/mi/yr)
PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  292.42  Travel Crash Rate (crashes/million veh-mi)  1.35  Travel FI Crash Rate (crashes/million veh-mi)  0.62  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.03	1.960	FI Crash Rate (crashes/mi/yr)
Expected Travel Crash Rate  Total Travel (million veh-mi) 292.42  Travel Crash Rate (crashes/million veh-mi) 1.35  Travel FI Crash Rate (crashes/million veh-mi) 0.62  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.03	0.101	FI no/C Crash Rate (crashes/mi/yr)
Total Travel (million veh-mi)  292.42  Travel Crash Rate (crashes/million veh-mi)  1.35  Travel FI Crash Rate (crashes/million veh-mi)  0.62  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.03	2.342	PDO Crash Rate (crashes/mi/yr)
Travel Crash Rate (crashes/million veh-mi)  1.35  Travel FI Crash Rate (crashes/million veh-mi)  0.62  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.03		Expected Travel Crash Rate
Travel FI Crash Rate (crashes/million veh-mi) 0.62  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.03	292.4	Total Travel (million veh-mi)
Travel FI no/C Crash Rate (crashes/million veh-mi) 0.03	1.3	Travel Crash Rate (crashes/million veh-mi)
	0.6	Travel FI Crash Rate (crashes/million veh-mi)
Travel PDO Crash Rate (crashes/million veh-mi) 0.74	0.0	Travel FI no/C Crash Rate (crashes/million veh-mi)
	0.7	Travel PDO Crash Rate (crashes/million veh-mi)

Table 14. Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 4)

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr	Expected FI Crash Frequency (crashes/yr	Expected FI no/C Crash Frequency (crashes/yr	Expected PDO Crash Frequency (crashes/yr	Predicted Total Crash Frequency (crashes/yr	Predicted FI Crash Frequency (crashes/yr	Predicted FI no/C Crash Frequency (crashes/yr	Predicted PDO Crash Frequency (crashes/yr	(Expected - Predicted) Total Crash Frequency (crashes/yr	(Expected - Predicted) FI Crash Frequency (crashes/yr	(Expected - Predicted) FI no/C Crash Frequency (crashes/yr	(Expected - Predicted) PDO Crash Frequency (crashes/yr	Expected Crash Rate (crashes/ mi/yr)	Expected Travel Crash Rate (crashes/ million veh-mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
SD 38/466th Ave N (v1)	676+00.000			32.818	37.742	1.2622	0.4932	0.3585	0.7690	1.4516	0.6469	0.4102	0.8047	-0.1894	-0.1537	-0.0517	-0.0357			0.23
22	676+00.000	676+15.090	0.0029	12.143	0.440	0.4670	0.4590		0.0081	0.0169	0.0054		0.0115	0.4501	0.4535		-0.0034	163.4115	29.87	
23	676+15.090	677+50.000	0.0256	13.296	3.936	0.5114	0.4472		0.0642	0.1514	0.0486		0.1028	0.3600	0.3986		-0.0386	20.0145	3.66	
24	677+50.000	679+00.000	0.0284	35.169	3.661	1.3526	0.3493		1.0034	0.1408	0.0452		0.0956	1.2118	0.3041		0.9078	47.6127	8.70	
25	679+00.000	680+80.000	0.0341	1.083	2.846	0.0416	0.0165		0.0252	0.1095	0.0351		0.0743	-0.0678	-0.0187		-0.0492	1.2212	0.34	
26	680+80.000	680+90.000	0.0019	0.064	0.189	0.0025	0.0010		0.0015	0.0073	0.0023		0.0049	-0.0048	-0.0013		-0.0035	1.3022	0.37	
27	680+90.000	682+20.000	0.0246	0.840	2.475	0.0323	0.0130		0.0193	0.0952	0.0306		0.0646	-0.0629	-0.0175		-0.0454	1.3113	0.37	
I90WB Ramps (v2)	681+00.000			20.354	15.321	0.7828	0.4306		0.3522	0.5893	0.2002		0.3891	0.1936	0.2305		-0.0369			0.21
28	682+20.000	682+30.000	0.0019	0.061	0.159	0.0023	0.0009		0.0014	0.0061	0.0020		0.0042	-0.0038	-0.0010		-0.0028	1.2298	0.35	
29	682+30.000	683+82.710	0.0289	0.925	2.432	0.0356	0.0141		0.0215	0.0935	0.0300		0.0635	-0.0580	-0.0160		-0.0420	1.2298	0.35	
30	683+82.710	689+36.990	0.1050	2.966	6.558	0.1141	0.0437		0.0704	0.2522	0.0810		0.1713	-0.1381	-0.0373		-0.1008	1.0868	0.31	
31	689+36.990	691+50.000	0.0403	1.140	2.520	0.0438	0.0168		0.0271	0.0969	0.0311		0.0658	-0.0531	-0.0143		-0.0388	1.0868	0.31	
32	691+50.000	692+70.000	0.0227	0.642	1.420	0.0247	0.0095		0.0152	0.0546	0.0175		0.0371	-0.0299	-0.0081		-0.0218	1.0868	0.31	
33	692+70.000	693+85.010	0.0218	0.616	1.361	0.0237	0.0091		0.0146	0.0523	0.0168		0.0355	-0.0287	-0.0077		-0.0209	1.0868	0.31	
34	693+85.010	698+70.000	0.0919	2.910	7.536	0.1119	0.0441		0.0678	0.2899	0.0930		0.1968	-0.1779	-0.0489		-0.1290	1.2183	0.34	
35	698+70.000	699+00.000	0.0057	0.192	0.557	0.0074	0,0030		0.0044	0.0214	0.0069		0.0146	-0.0140	-0.0039		-0.0101	1.3004	0.37	
36	699+00.000	699+20.000	0.0038	0.135	0.432	0.0052	0.0021		0.0031	0.0166	0.0053		0.0113	-0.0114	-0.0032		-0.0082	1.3665	0.39	
I90EBRamp S466th SD38 (v1)	699+20.000			33.839	75.514	1.3015	0.5948		0.7067	2.9044	1.2518		1.6526	-1.6029	-0.6570		-0.9459			0.32
37	699+20.000	699+73.960	0.0102	0.363	1.166	0.0140	0.0057		0.0083	0.0449	0.0144		0.0305	-0.0309	-0.0087		-0.0222	1.3665	0.39	
38	699+73,960	700+50.000	0.0144	0.512	1.643	0.0197	0.0080		0.0116	0.0632	0.0203		0.0429	-0.0435	-0.0122		-0.0313	1.3665	0.39	
39	700+50.000	702+00.000	0.0284	0.961	2.786	0.0369	0.0149		0.0221	0.1072	0.0344		0.0728	-0.0702	-0.0195		-0.0507	1.3004	0.37	
40	702+00.000	702+50.000	0.0095	0.237	0.769	0.0091	0.0037		0.0054	0.0296	0.0095		0.0201	-0.0205	-0.0058		-0.0147	0.9631	0.33	
41	702+50.000	710+47.850	0.1511	3.569	10.270	0.1373	0.0551		0.0821	0.3950	0.1268		0.2682	-0.2577	-0.0717		-0.1860	0.9084	0.31	
42	710+47.850	713+88.360	0.0645	6,400	3.337	0.2462	0.0297		0.2165	0.1284	0.0412		0.0872	0.1178	-0.0115		0.1293	3.8170	1.30	
43	713+88.360	725+01.220	0.2108	14.542	10.906	0.5593	0.2180		0.3413	0.4195	0.1347		0.2848	0.1398	0.0833		0.0565	2.6537	0.90	
44	725+01.220	727+00.000	0.0376	0.802	1.948	0.0308	0.0120		0.0188	0.0749	0.0241		0.0509	-0.0441	-0.0120		-0.0321	0.8192	0.28	
45	727+00.000	727+52.350	0.0099	0.211	0.513	0.0081	0.0032		0.0050	0.0197	0.0063		0.0134	-0.0116	-0.0032		-0.0084	0.8192	0.28	
46	727+52.350	730+90.510	0.0640	1.364	3.314	0.0525	0.0204		0.0320	0.1275	0.0409		0.0865	-0.0750	-0.0205		-0.0545	0.8192	0.28	
47	730+90.510	735+00.000	0.0776	1.716	4.415	0.0660	0.0260		0.0400	0.1698	0.0545		0.1153	-0.1038	-0.0285		-0.0753	0.8510	0.29	
48	735+00.000	739+00.000	0.0758	1.676	4.413	0.0645	0.0254		0.0391	0.1659	0.0532		0.1135	-0.1038	-0.0283		-0.0735	0.8510	0.29	
49	739+00.000	744+50.000	0.1042	2.305	5.929	0.0886	0.0234		0.0537	0.1039	0.0732		0.1120	-0.1394	-0.0278		-0.1011	0.8510	0.29	
50	744+50.000	745+69.220	0.0226	0.500	1.285	0.0192	0.0076		0.0116	0.0494	0.0159		0.0336	-0.0302	-0.0083		-0.0219	0.8510	0.29	
51	745+69.220	751+00.000	0.1005	2.141	5.202	0.0192	0.0076		0.0503	0.2001	0.0139		0.0336	-0.0302	-0.0083		-0.0219	0.8310	0.29	
52	751+00.000	760+50.000	0.1003	3.832	9.310	0.0824	0.0521		0.0900	0.3581	0.0042		0.1338	-0.1177	-0.0521		-0.0836	0.8192	0.28	
53	760+50.000	765+52.550	0.1799	2.027	4.925	0.0780	0.0374		0.0900	0.3381	0.0608		0.1286	-0.2107	-0.0373		-0.1332	0.8192	0.28	
53	765+52.550	767+00.000	0.0952	0.639	1.740	0.0780	0.0304		0.0476	0.1894	0.0008		0.1286	-0.1115	-0.0304		-0.0306	0.8192	0.28	
55	767+00.000	780+45.930	0.0279	11.241	15.885	0.0246	0.0098		0.0148	0.0009	0.0215		0.0454	-0.0423	-0.0117		-0.0306	1.6960	0.58	
56		787+93.440	0.2549				0.1104		0.3219		0.1961									
	780+45.930		0.1.110	3.015	7.326	0.1160				0.2818			0.1913	-0.1658	-0.0452		-0.1205	0.8192	0.28	
57	787+93.440	791+00.000	0.0581	1.237	3.004	0.0476	0.0185	<u> </u>	0.0290	0.1156	0.0371		0.0785	-0.0680	-0.0186		-0.0494	0.8192	0.28	

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr	Expected FI Crash Frequency (crashes/yr	Expected FI no/C Crash Frequency (crashes/yr	Expected PDO Crash Frequency (crashes/yr	Predicted Total Crash Frequency (crashes/yr	Predicted FI Crash Frequency (crashes/yr	Predicted FI no/C Crash Frequency (crashes/yr	Predicted PDO Crash Frequency (crashes/yr	(Expected - Predicted) Total Crash Frequency (crashes/yr	(Expected - Predicted) FI Crash Frequency (crashes/yr	(Expected - Predicted) FI no/C Crash Frequency (crashes/yr	(Expected - Predicted) PDO Crash Frequency (crashes/yr	Expected Crash Rate (crashes/ mi/yr)	Expected Travel Crash Rate (crashes/ million veh-mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
58	791+00.000	791+10.000	0.0019	0.040	0.098	0.0016	0.0006		0.0009	0.0038	0.0012		0.0026	-0.0022	-0.0006		-0.0016	0.8192	0.28	
59	791+10.000	798+12.000	0.1330	7.858	6.880	0.3022	0.0565		0.2457	0.2646	0.0849		0.1797	0.0376	-0.0284		0.0660	2.2733	0.77	
60	798+12.000	808+80.000	0.2023	14.361	10.467	0.5524	0.3758		0.1765	0.4026	0.1292		0.2733	0.1498	0.2466		-0.0968	2.7308	0.93	
61	808+80.000	809+00.000	0.0038	0.081	0.196	0.0031	0.0012		0.0019	0.0075	0.0024		0.0051	-0.0044	-0.0012		-0.0032	0.8192	0.28	
468th Ave (v2)	809+00.000			44.287	87.103	1.7033	0.9573		0.7460	3.3501	1.4439		1.9062	-1.6468	-0.4866		-1.1602			0.50
62	809+00.000	810+00.000	0.0189	0.525	1.568	0.0202	0.0082		0.0120	0.0603	0.0194		0.0409	-0.0401	-0.0112		-0.0289	1.0655	0.32	
63	810+00.000	825+00.000	0.2841	18.524	16.910	0.7125	0.2777		0.4348	0.6504	0.2088		0.4416	0.0621	0.0689		-0.0068	2.5079	0.74	
64	825+00.000	826+54.070	0.0292	6.497	1.737	0.2499	0.0164		0.2335	0.0668	0.0214		0.0454	0.1831	-0.0050		0.1881	8.5634	2.53	
65	826+54.070	828+00.000	0.0276	0.677	1.645	0.0260	0.0101		0.0159	0.0633	0.0203		0.0430	-0.0372	-0.0102		-0.0271	0.9423	0.28	
66	828+00.000	842+53.930	0.2754	18.310	16.391	0.7042	0.2745		0.4298	0.6304	0.2024		0.4280	0.0738	0.0721		0.0017	2.5575	0.76	
67	842+53.930	854+00.000	0.2171	11.100	12.920	0.4269	0.2439		0.1830	0.4969	0.1595		0.3374	-0.0700	0.0844		-0.1544	1.9668	0.58	
68	854+00.000	854+70.000	0.0133	0.325	0.789	0.0125	0.0049		0.0076	0.0304	0.0097		0.0206	-0.0179	-0.0049		-0.0130	0.9423	0.28	
69	854+70.000	855+80.000	0.0208	0.510	1.240	0.0196	0.0077		0.0120	0.0477	0.0153		0.0324	-0.0281	-0.0077		-0.0204	0.9423	0.28	
70	855+80.000	858+75.120	0.0559	1.369	3.327	0.0527	0.0205		0.0321	0.1280	0.0411		0.0869	-0.0753	-0.0205		-0.0547	0.9423	0.28	
71	858+75.120	861+85.000	0.0587	1.438	3.493	0.0553	0.0216		0.0338	0.1344	0.0431		0.0912	-0.0791	-0.0216		-0.0575	0.9423	0.28	
72	861+85.000	862+00.000	0.0028	0.075	0.202	0.0029	0.0011		0.0017	0.0078	0.0025		0.0053	-0.0049	-0.0014		-0.0035	1.0103	0.30	
SD 38 / 469th St (v1)	862+00.000			50.584	174.380	1.9455	0.9282		1.0173	6.7069	2.8907		3.8162	-4.7614	-1.9625		-2.7989			0.38
73	862+00.000	862+50.000	0.0095	0.285	0.772	0.0110	0.0044		0.0066	0.0297	0.0095		0.0202	-0.0187	-0.0052		-0.0136	1.1576	0.30	
74	862+50.000	862+60.000	0.0019	0.053	0.129	0.0020	0.0008		0.0012	0.0050	0.0016		0.0034	-0.0029	-0.0008		-0.0021	1.0797	0.28	
All Segments			3.5341	213.499	215.276	8.2115	3.5234		4.6881	8.2798	2.6578		5.6220	-0.0683	0.8655		-0.9339	2.3235	0.73	
All Intersections				181.881	390.059	6.9954	3.4041		3.5914	15.0023	6.4334		8.5689	-8.0069	-3.0294		-4.9775			0.32
Total			3.5341	395.380	605.335	15.2069	6.9274		8.2795	23.2821	9.0912		14.1909	-8.0752	-2.1638		-5.9114	4.3029		

Table 15. Expected Crash Frequencies and Rates by Horizontal Design Element (Section 4)

Title	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)		Expected Travel Crash Rate (crashes/mi llion veh- mi)
Simple Curve 1	676+00.000	683+82.710	0.1482	63.579	16.139	2.4453	1.3009	1.1444	0.6207	0.1993	0.4215	1.8246	1.1016	0.7230	16.4957	3.09
Tangent	683+82.710	693+85.010	0.1898	5.364	11.858	0.2063	0.0790	0.1273	0.4561	0.1464	0.3097	-0.2498	-0.0674	-0.1824	1.0868	0.31
Simple Curve 2	693+85.010	710+47.850	0.3149	8.878	25.160	0.3414	0.1367	0.2048	0.9677	0.3106	0.6571	-0.6263	-0.1740	-0.4523	1.0842	0.33
Tangent	710+47.850	727+51.450	0.3227	21.952	16.696	0.8443	0.2628	0.5815	0.6422	0.2061	0.4360	0.2021	0.0566	0.1455	2.6168	0.89
Simple Curve 3	727+51.450	727+52.350	0.0002	0.004	0.009	0.0001	0.0001	0.0001	0.0003	0.0001	0.0002	-0.0002	-0.0001	-0.0001	0.8192	0.28
Tangent	727+52.350	765+52.550	0.7197	15.562	38.693	0.5985	0.2342	0.3643	1.4882	0.4777	1.0105	-0.8897	-0.2435	-0.6462	0.8316	0.28
Simple Curve 4	765+52.550	780+45.930	0.2828	11.880	17.626	0.4569	0.1202	0.3367	0.6779	0.2176	0.4603	-0.2210	-0.0974	-0.1236	1.6155	0.55
Tangent	780+45.930	862+60.000	1.5557	86.281	89.095	3.3185	1.3896	1.9290	3.4267	1.1000	2.3267	-0.1082	0.2896	-0.3978	2.1331	0.66

Table 16. Predicted Crash Frequencies by Year (Section 4)

Year	Total Crashes	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
2025	13.90	5.40	38.844	8.50	61.156
2026	14.96	5.80	38.801	9.15	61.199
2027	15.93	6.18	38.789	9.75	61.211
2028	16.91	6.56	38.776	10.35	61.224
2029	17.90	6.94	38.762	10.96	61.238
2030	18.35	7.11	38.771	11.23	61.229
2031	18.81	7.29	38.780	11.51	61.220
2032	19.27	7.47	38.790	11.79	61.210
2033	19.73	7.65	38.798	12.07	61.202
2034	20.19	7.83	38.806	12.35	61.194
2035	20.65	8.02	38.815	12.64	61.185
2036	21.12	8.20	38.823	12.92	61.177
2037	21.59	8.38	38.829	13.21	61.171
2038	22.06	8.57	38.836	13.49	61.164
2039	22.53	8.75	38.843	13.78	61.157
2040	23.00	8.94	38.850	14.07	61.150
2041	24.31	9.47	38.965	14.84	61.035
2042	25.58	9.99	39.062	15.59	60.938
2043	26.82	10.50	39.143	16.32	60.857
2044	28.06	11.00	39.215	17.05	60.785
2045	29.27	11.50	39.276	17.77	60.724
2046	30.48	11.99	39.331	18.49	60.669
2047	31.68	12.48	39.379	19.21	60.621
2048	32.88	12.96	39.422	19.92	60.578
2049	34.08	13.45	39.460	20.63	60.540
2050	35.28	13.93	39.495	21.34	60.505
Total	605.34	236.37	39.048	368.96	60.952
Average	23.28	9.09	39.048	14.19	60.952

**Table 17. Expected Crash Frequencies by Year (Section 4)** 

Year	Total Crashes	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
2025	9.08	4.12	45.316	4.96	54.628
2026	9.77	4.42	45.266	5.34	54.666
2027	10.41	4.71	45.252	5.69	54.677
2028	11.05	5.00	45.237	6.04	54.689
2029	11.69	5.29	45.220	6.39	54.701
2030	11.98	5.42	45.231	6.55	54.693
2031	12.28	5.56	45.242	6.72	54.685
2032	12.58	5.69	45.253	6.88	54.676
2033	12.88	5.83	45.263	7.04	54.669
2034	13.19	5.97	45.272	7.21	54.662
2035	13.49	6.11	45.282	7.37	54.654
2036	13.79	6.25	45.291	7.54	54.647
2037	14.10	6.39	45.299	7.70	54.641
2038	14.41	6.53	45.307	7.87	54.635
2039	14.71	6.67	45.315	8.04	54.629
2040	15.03	6.81	45.323	8.21	54.623
2041	15.88	7.22	45.457	8.66	54.520
2042	16.71	7.61	45.570	9.10	54.433
2043	17.52	8.00	45.665	9.52	54.361
2044	18.32	8.38	45.749	9.95	54.297
2045	19.12	8.76	45.820	10.37	54.242
2046	19.91	9.14	45.885	10.79	54.193
2047	20.70	9.51	45.940	11.21	54.150
2048	21.48	9.88	45.991	11.62	54.111
2049	22.26	10.25	46.035	12.04	54.078
2050	23.04	10.62	46.076	12.45	54.047
Total	395.38	180.11	45.554	215.27	54.446
Average	15.21	6.93	45.554	8.28	54.446

Table 18. Comparing Predicted and Expected Crashes for the Evaluation Period (Section 4)

Scope	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
Predicted	605.34	236.37	39.048	368.96	60.952
Expected	395.38	180.11	45.554	215.27	54.446
Expected - Predicted	-209.96	-56.26		-153.70	
Percent Difference	-53.10	-31.24		-71.40	

Table 19. Expected Crash Severity by Ramp Terminal or Roundabout (Section 4)

Seg. No.	Туре	Fatal (K) Crashes (crashes)	Incapacitating Injury (A) Crashes (crashes)	Non-Incapacitating Injury (B) Crashes (crashes)	Possible Injury (C) Crashes (crashes)	No Injury (O) Crashes (crashes)
4	FRERampTerminal	0.1324	0.6953	2.3001	8.0684	9.1576

Table 20. Expected Crash Type Distribution (Section 4)

					ı		
Element Type	Crash Type	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)	Total Crashes	Percent Total (%)
Highway Segment	Collision with Animal	3.48	0.9	22.43	5.7	25.83	6.5
Highway Segment	Collision with Bicycle	0.37	0.1	0.12	0.0	0.43	0.1
Highway Segment	Other Single-vehicle Collision	0.64	0.2	3.54	0.9	4.48	1.1
Highway Segment	Overturned	3.39	0.9	1.83	0.5	5.34	1.4
Highway Segment	Collision with Pedestrian	0.64	0.2	0.12	0.0	0.64	0.2
Highway Segment	Run Off Road	49.93	12.6	61.55	15.6	111.23	28.1
Highway Segment	Total Single Vehicle Crashes	58.45	14.8	89.59	22.7	147.96	37.4
Highway Segment	Angle Collision	9.25	2.3	8.78	2.2	18.15	4.6
Highway Segment	Head-on Collision	3.12	0.8	0.37	0.1	3.42	0.9
Highway Segment	Other Multiple-vehicle Collision	2.38	0.6	3.66	0.9	5.76	1.5
Highway Segment	Rear-end Collision	15.12	3.8	14.87	3.8	30.32	7.7
Highway Segment	Sideswipe	3.48	0.9	4.63	1.2	7.90	2.0
Highway Segment	Total Multiple Vehicle Crashes	33.34	8.4	32.30	8.2	65.54	16.6
Highway Segment	Total Highway Segment Crashes	91.79	23.2	121.89	30.8	213.50	54.0
Intersection	Collision with Animal	0.39	0.1	0.90	0.2	1.29	0.3
Intersection	Collision with Bicycle	0.06	0.0	0.06	0.0	0.13	0.0
Intersection	Other Single-vehicle Collision	0.26	0.1	0.64	0.2	1.03	0.3
Intersection	Overturned	0.39	0.1	0.26	0.1	0.64	0.2
Intersection	Collision with Pedestrian	0.06	0.0	0.06	0.0	0.13	0.0
Intersection	Run Off Road	6.06	1.5	9.25	2.3	15.70	4.0
Intersection	Single	2.81	0.7	4.88	1.2	7.68	1.9
Intersection	Total Single Vehicle Crashes	10.03	2.5	16.05	4.1	26.60	6.7
Intersection	Angle Collision	39.04	9.9	26.69	6.8	64.11	16.2
Intersection	Head-on Collision	4.42	1.1	2.00	0.5	6.10	1.5
Intersection	Other Multiple-vehicle Collision	2.71	0.7	2.38	0.6	5.02	1.3
Intersection	Rear-end Collision	16.71	4.2	23.38	5.9	40.63	10.3
Intersection	Sideswipe	3.58	0.9	12.63	3.2	17.36	4.4
Intersection	Total Multiple Vehicle Crashes	66.46	16.8	67.08	17.0	133.22	33.7
Intersection	Total Intersection Crashes	77.31	19.6	84.02	21.2	161.53	40.9
Intersection	Other Collision	0.82	0.2	0.88	0.2	1.71	0.4
Ramp Terminal	Collision with Animal	0.00	0.0	0.00	0.0	0.00	0.0
Ramp Terminal	Collision with Fixed Object	0.87	0.2	1.45	0.4	2.32	0.6
Ramp Terminal	Collision with Other Object	0.00	0.0	0.05	0.0	0.05	0.0
Ramp Terminal	Other Single-vehicle Collision	0.73	0.2	0.24	0.1	0.97	0.2
Ramp Terminal	Collision with Parked Vehicle	0.08	0.0	0.14	0.0	0.22	0.1
Ramp Terminal	Total Single Vehicle Crashes	1.68	0.4	1.87	0.5	3.55	0.9
Ramp Terminal	Angle Collision	5.84	1.5	3.41	0.9	9.25	2.3
Ramp Terminal	Head-on Collision	0.22	0.1	0.14	0.0	0.36	0.1
Ramp Terminal	Other Multiple-vehicle Collision	0.15	0.0	0.24	0.1	0.38	0.1
Ramp Terminal	Rear-end Collision	3.08	0.8	2.53	0.6	5.61	1.4
Ramp Terminal	Sideswipe, Same Direction Collision	0.22	0.1	0.98	0.2	1.20	0.3
Ramp Terminal	Total Multiple Vehicle Crashes	9.52	2.4	7.29	1.8	16.81	4.3
Ramp Terminal	Total Ramp Terminal Crashes	11.20	2.8	9.16	2.3	20.35	5.1
	Total Crashes	180.30	45.6	215.07	54.4	395.38	100.0

**Table 21. Evaluation Message** 

Start Location (Sta. ft)	End Location (Sta. ft)	Message
676+00.000	676+15.090	Warning: for segment #22 (676+00.000 to 676+15.090 ), segment traffic volume (18,122 vpd) for 2046 is not within the model limit (17,800 vpd) for reliable results for segment type 2U
676+00.000	676+15.090	Warning: for segment #22 (676+00.000 to 676+15.090 ), segment traffic volume (18,489 vpd) for 2047 is not within the model limit (17,800 vpd) for reliable results for segment type 2U
676+00.000	676+15.090	Warning: for segment #22 (676+00.000 to 676+15.090 ), segment traffic volume (18,856 vpd) for 2048 is not within the model limit (17,800 vpd) for reliable results for segment type 2U
676+00.000	676+15.090	Warning: for segment #22 (676+00.000 to 676+15.090 ), segment traffic volume (19,223 vpd) for 2049 is not within the model limit (17,800 vpd) for reliable results for segment type 2U
676+00.000	676+15.090	Warning: for segment #22 (676+00.000 to 676+15.090 ), segment traffic volume (19,590 vpd) for 2050 is not within the model limit (17,800 vpd) for reliable results for segment type 2U
676+15.090	677+50.000	Warning: for segment #23 (676+15.090 to 677+50.000 ), segment traffic volume (18,122 vpd) for 2046 is not within the model limit (17,800 vpd) for reliable results for segment type 2U
676+15.090	677+50.000	Warning: for segment #23 (676+15.090 to 677+50.000 ), segment traffic volume (18,489 vpd) for 2047 is not within the model limit (17,800 vpd) for reliable results for segment type 2U
676+15.090	677+50.000	Warning: for segment #23 (676+15.090 to 677+50.000 ), segment traffic volume (18,856 vpd) for 2048 is not within the model limit (17,800 vpd) for reliable results for segment type 2U
676+15.090	677+50.000	Warning: for segment #23 (676+15.090 to 677+50.000 ), segment traffic volume (19,223 vpd) for 2049 is not within the model limit (17,800 vpd) for reliable results for segment type 2U
676+15.090	677+50.000	Warning: for segment #23 (676+15.090 to 677+50.000 ), segment traffic volume (19,590 vpd) for 2050 is not within the model limit (17,800 vpd) for reliable results for segment type 2U
677+50.000	679+00.000	Warning: for segment #24 (677+50.000 to 679+00.000 ), segment traffic volume (18,122 vpd) for 2046 is not within the model limit (17,800 vpd) for reliable results for segment type 2U
677+50.000	679+00.000	Warning: for segment #24 (677+50.000 to 679+00.000 ), segment traffic volume (18,489 vpd) for 2047 is not within the model limit (17,800 vpd) for reliable results for segment type 2U
677+50.000	679+00.000	Warning: for segment #24 (677+50.000 to 679+00.000 ), segment traffic volume (18,856 vpd) for 2048 is not within the model limit (17,800 vpd) for reliable results for segment type 2U
677+50.000	679+00.000	Warning: for segment #24 (677+50.000 to 679+00.000 ), segment traffic volume (19,223 vpd) for 2049 is not within the model limit (17,800 vpd) for reliable results for segment type 2U
677+50.000	679+00.000	Warning: for segment #24 (677+50.000 to 679+00.000 ), segment traffic volume (19,590 vpd) for 2050 is not within the model limit (17,800 vpd) for reliable results for segment type 2U
699+20.000	699+20.000	Warning: for intersection #2 (699+20.000 to 699+20.000), minor road traffic volume (3,618 vpd) for 2049 is no within the model limit (3,500 vpd) for reliable results for intersection type 4ST
699+20.000	699+20.000	Warning: for intersection #2 (699+20.000 to 699+20.000), minor road traffic volume (3,925 vpd) for 2050 is no within the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000), minor road traffic volume (3,550 vpd) for 2033 is no within the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000), minor road traffic volume (3,626 vpd) for 2034 is no within the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000), minor road traffic volume (3,703 vpd) for 2035 is now ithin the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000), minor road traffic volume (3,779 vpd) for 2036 is no within the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000), minor road traffic volume (3,855 vpd) for 2037 is now it in the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000), minor road traffic volume (3,932 vpd) for 2038 is now ithin the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000), minor road traffic volume (4,008 vpd) for 2039 is now it in the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000), minor road traffic volume (4,085 vpd) for 2040 is now within the model limit (3,500 vpd) for reliable results for intersection type 4ST

Start Location (Sta. ft)	End Location (Sta. ft)	Message
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000 ), minor road traffic volume (4,178 vpd) for 2041 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000 ), minor road traffic volume (4,271 vpd) for 2042 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000 ), minor road traffic volume (4,364 vpd) for 2043 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000), minor road traffic volume (4,457 vpd) for 2044 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000), minor road traffic volume (4,550 vpd) for 2045 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000), minor road traffic volume (4,643 vpd) for 2046 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000), minor road traffic volume (4,736 vpd) for 2047 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000), minor road traffic volume (4,829 vpd) for 2048 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000), minor road traffic volume (4,922 vpd) for 2049 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
862+00.000	862+00.000	Warning: for intersection #6 (862+00.000 to 862+00.000), minor road traffic volume (5,015 vpd) for 2050 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST

### **Section 3 Evaluation**

**Section:** Section 3

**Evaluation Start Location:** 585+00.000 **Evaluation End Location:** 676+00.000

Area Type: Rural

Functional Class: Arterial

Type of Alignment: Undivided, Multilane

**Model Category:** Rural, Multilane **Calibration Factor:** 4ST=1.0; 4U=1.0;

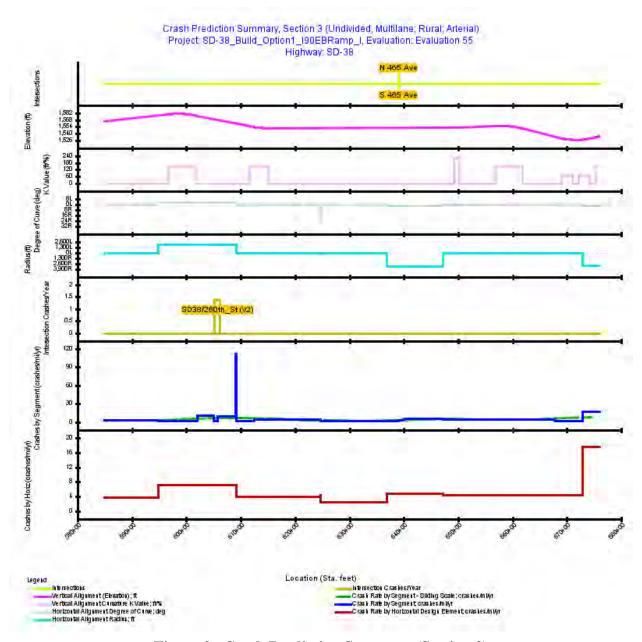


Figure 2. Crash Prediction Summary (Section 3)

**Table 22. Observed Crashes Used in the Evaluation (Section 3)** 

Year	Observed Crashes	Total Crashes Used	FI Crashes	FI no/C Crashes	PDO Crashes
2018	1	1	1	0	0
2019	3	3	1	0	2
2020	3	2	0	0	2
2021	3	3	2	0	1
2022	1	1	0	0	1
All Years	11 <sup>[1]</sup>	10	4	0	6

#### **Footnotes**

<sup>[1]</sup> Note: Observed crash data that does not comply with the associated CPM model requirements may not be used in EB processing.

 Table 23. Evaluation Highway - Homogeneous Segments (Section 3)

Seg. No.	Type	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Width (ft)	Right Lane Width (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Righ Side Slop
1	Rural Multi-Lane Segment Four- lane Undivided	585+00.000	594+84.940	984.94	0.1865	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,398; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 (
2	Rural Multi-Lane Segment Four- lane Undivided	594+84.940	600+00.000	515.06	0.0975	2025; 7,087; 2026; 8,007; 2027; 8,928; 2028; 9,849; 2029; 10,770; 2030; 10,937; 2031; 11,104; 2032; 11,271; 2033; 11,439; 2034; 11,606; 2035; 11,773; 2036; 11,940; 2037; 12,108; 2038; 12,275; 2039; 12,442; 2040; 12,610; 2041; 12,806; 2042; 13,002; 2043; 13,198; 2044; 13,394; 2045; 13,599; 2046; 13,786; 2047; 13,598; 2048; 14,178; 2049; 14,374; 2050; 14,570	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 (
3	Rural Multi-Lane Segment Four- lane Undivided	600+00.000	601+00.000	100.00	0.0189	2025; 7,087; 2026; 8,007; 2027; 8,928; 2028; 9,849; 2029; 10,770; 2030; 10,937; 2031; 11,104; 2032; 11,271; 2033; 11,439; 2034; 11,606; 2035; 11,773; 2036; 11,940; 2037; 12,108; 2038; 12,275; 2039; 12,442; 2040; 12,610; 2041; 12,806; 2042; 13,002; 2043; 13,198; 2044; 13,394; 2045; 13,599; 2046; 13,786; 2047; 13,982; 2048; 14,178; 2049; 14,374; 2050; 14,570	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 (
4	Rural Multi-Lane Segment Four- lane Undivided	601+00.000	602+00.000	100.00	0.0189	2025; 7,087; 2026; 8,007; 2027; 8,928; 2028; 9,849; 2029; 10,770; 2030; 10,937; 2031; 11,104; 2032; 11,271; 2033; 11,439; 2034; 11,606; 2035; 11,773; 2036; 11,940; 2037; 12,108; 2038; 12,275; 2039; 12,442; 2040; 12,610; 2041; 12,806; 2042; 13,002; 2043; 13,198; 2044; 13,394; 2045; 13,590; 2046; 13,786; 2047; 13,592; 2048; 14,178; 2049; 14,374; 2050; 14,570	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 (
5	Rural Multi-Lane Segment Four- lane Undivided	602+00.000	605+00.000	300.00	0.0568	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 0
6	Rural Multi-Lane Segment Four- lane Undivided	605+00.000	605+60.000	60.00	0.0114	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	0.00	0.00	None	0.00	false	false	0:1	1 (
7	Rural Multi-Lane Segment Four- lane Undivided	605+60.000	605+70.000	10.00	0.0019	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2056; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 0
8	Rural Multi-Lane Segment Four- lane Undivided	605+70.000	605+75.000	5.00	0.0009	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 (
9	Rural Multi-Lane Segment Four- lane Undivided	605+75.000	609+00.000	325.00	0.0616	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2056; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 (
10	Rural Multi-Lane Segment Four- lane Undivided	609+00.000	609+21.930	21.93	0.0042	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 0
11	Rural Multi-Lane Segment Four- lane Undivided	609+21.930	611+40.000	218.07	0.0413	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 0
12	Rural Multi-Lane Segment Four- lane Undivided	611+40.000	612+50.000	110.00	0.0208	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 0
13	Rural Multi-Lane Segment Four- lane Undivided	612+50.000	624+64.530	1,214.53	0.2300	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 0
14	Rural Multi-Lane Segment Four- lane Undivided	624+64.530	636+92.820	1,228.29	0.2326	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 0

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT		Right Lane Width (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
15	Rural Multi-Lane Segment Four- lane Undivided	636+92.820	639+00.000	207.18	0.0392	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,529; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,500	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
16	Rural Multi-Lane Segment Four- lane Undivided	639+00.000	640+00.000	100.00	0.0189	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1
17	Rural Multi-Lane Segment Four- lane Undivided	640+00.000	647+26.050	726.05	0.1375	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
18	Rural Multi-Lane Segment Four- lane Undivided	647+26.050	667+80.000	2,053.95	0.3890	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
19	Rural Multi-Lane Segment Four- lane Undivided	667+80.000	668+80.000	100.00	0.0189	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	0.00	0.00	None	0.00	false	false	0:1	0:1
20	Rural Multi-Lane Segment Four- lane Undivided	668+80.000	672+86.110	406.11	0.0769	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
21	Rural Multi-Lane Segment Four- lane Undivided	672+86.110	676+00.000	313.89	0.0594	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,529; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,500	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1

Table 24. User Defined CMF Used in the Eval Segment CPM Evaluation (Section 3)

Name	Description	Start Loc. (Sta. ft)	End Loc. (Sta. ft)	Start CMF Year	End CMF Year	Severity	CMF Value
1	TWLTL	585+00.000	600+00.000	2025	2025	Total	0.6900
1	TWLTL	612+50.000	639+00.000	2025	2025	Total	0.6900
1	TWLTL	640+00.000	676+00.000	2025	2025	Total	0.6900

Table 25. Crash History Highway - Homogeneous Segments (Section 3)

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Lengt h (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Media n Width (ft)	Media n Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
1	Rural Multi-Lane Segment Four-lane Undivided	585+00.00 0	594+84.94 0	984.94	0.1865	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
2	Rural Multi-Lane Segment Four-lane Undivided	594+84.94 0	600+00.00 0	515.06	0.0975	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
3	Rural Multi-Lane Segment Four-lane Undivided	600+00.00 0	601+00.00 0	100.00	0.0189	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
4	Rural Multi-Lane Segment Four-lane Undivided	601+00.00 0	602+00.00 0	100.00	0.0189	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
5	Rural Multi-Lane Segment Four-lane Undivided	602+00.00 0	605+00.00 0	300.00	0.0568	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
6	Rural Multi-Lane Segment Four-lane Undivided	605+00.00 0	605+60.00 0	60.00	0.0114	2018-2022: 4,325	12.00	12.00	8.00	0.00	0.00	None	0.00	false	false	0:1	0:1
7	Rural Multi-Lane Segment Four-lane Undivided	605+60.00 0	605+70.00 0	10.00	0.0019	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
8	Rural Multi-Lane Segment Four-lane Undivided	605+70.00 0	605+75.00 0	5.00	0.0009	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
9	Rural Multi-Lane Segment Four-lane Undivided	605+75.00 0	609+00.00 0	325.00	0.0616	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
10	Rural Multi-Lane Segment Four-lane Undivided	609+00.00 0	609+21.93 0	21.93	0.0042	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
11	Rural Multi-Lane Segment Four-lane Undivided	609+21.93 0	611+40.00 0	218.07	0.0413	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
12	Rural Multi-Lane Segment Four-lane Undivided	611+40.00 0	612+50.00 0	110.00	0.0208	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
13	Rural Multi-Lane Segment Four-lane Undivided	612+50.00 0	624+64.53 0	1,214.5 3	0.2300	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
14	Rural Multi-Lane Segment Four-lane Undivided	624+64.53 0	636+92.82 0	1,228.2 9	0.2326	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
15	Rural Multi-Lane Segment Four-lane Undivided	636+92.82 0	639+00.00 0	207.18	0.0392	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
16	Rural Multi-Lane Segment Four-lane Undivided	639+00.00 0	640+00.00 0	100.00	0.0189	2018-2022: 4,325	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1
17	Rural Multi-Lane Segment Four-lane Undivided	640+00.00 0	647+26.05 0	726.05	0.1375	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
18	Rural Multi-Lane Segment Four-lane Undivided	647+26.05 0	667+80.00 0	2,053.9 5	0.3890	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
19	Rural Multi-Lane Segment Four-lane Undivided	667+80.00 0	668+80.00 0	100.00	0.0189	2018-2022: 4,325	12.00	12.00	8.00	0.00	0.00	None	0.00	false	false	0:1	0:1
20	Rural Multi-Lane Segment Four-lane Undivided	668+80.00 0	672+86.11 0	406.11	0.0769	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
21	Rural Multi-Lane Segment Four-lane Undivided	672+86.11 0	676+00.00 0	313.89	0.0594	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1

# **Table 26. Evaluation Intersection (Section 3)**

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
1		Rural Multi-Lane Intersection Four-Legged w/STOP control	605+70.000	12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654;	2025: 1,508; 2026: 1,706; 2027: 1,904; 2028: 2,102; 2029: 2,300; 2030: 2,472; 2031: 2,645; 2032: 2,818; 2033: 2,909; 2034: 3,163; 2035: 3,356; 2036: 3,509; 2037: 3,681: 2038: 3,854; 2039: 4,027; 2040: 4,200; 2041: 4,260; 2042: 4,320; 2043: 4,380; 2044: 4,440; 2045: 4,500; 2046: 4,560; 2047: 4,620; 2048: 4,680; 2049: 4,740; 2050: 4,800	4	Stop-Controlled	2	1	16.83	13.71	false

# **Table 27.** Crash History Intersection (Section 3)

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
1	SD38/260th_St (v2)	Rural Multi-Lane Intersection Four-Legged w/STOP control	605+70.000	2018-2022: 4,325	2018-2022: 915	4	Stop-Controlled	2	1	16.83	13.71	false

Table 28. Expected Highway Crash Rates and Frequencies Summary (Section 3)

Last Year of Analysis   2050		
Evaluated Length (mi)   1.7235     Average Future Road AADT (vpd)   14,422     Expected Crashes   253.20     Total Crashes   253.20     Fatal and Injury Crashes   132.44     Fatal and Serious Injury Crashes   82.70     Property-Damage-Only Crashes   120.76     Percent of Total Expected Crashes     Percent Fatal and Injury Crashes (%)   52     Percent Fatal and Serious Injury Crashes (%)   33     Percent Property-Damage-Only Crashes (%)   48     Expected Crash Rate       Crash Rate (crashes/mi/yr)   5.6505     FI Crash Rate (crashes/mi/yr)   2.9555     FI no/C Crash Rate (crashes/mi/yr)   1.8456     PDO Crash Rate (crashes/mi/yr)   2.6950     Expected Travel Crash Rate       Total Travel (million veh-mi)   235.89     Travel FI Crash Rate (crashes/million veh-mi)   0.56     Travel FI Crash Rate (crashes/million veh-mi)   0.35     rst Year of Analysis	2025	
Average Future Road AADT (vpd)   14,422	Last Year of Analysis	2050
Expected Crashes   253.20     Fatal and Injury Crashes   132.44     Fatal and Serious Injury Crashes   82.70     Property-Damage-Only Crashes   120.76     Percent of Total Expected Crashes     Percent Fatal and Injury Crashes (%)   52     Percent Fatal and Serious Injury Crashes (%)   33     Percent Fatal and Serious Injury Crashes (%)   48     Expected Crash Rate   Crashes/mi/yr   5.6505     FI Crash Rate (crashes/mi/yr)   2.9555     FI no/C Crash Rate (crashes/mi/yr)   1.8456     PDO Crash Rate (crashes/mi/yr)   2.6950     Expected Travel Crash Rate     Total Travel (million veh-mi)   235.89     Travel FI Crash Rate (crashes/million veh-mi)   1.07     Travel FI Crash Rate (crashes/million veh-mi)   0.56     Travel FI no/C Crash Rate (crashes/million veh-mi)   0.56	Evaluated Length (mi)	1.7235
Total Crashes   253.20     Fatal and Injury Crashes   132.44     Fatal and Serious Injury Crashes   82.70     Property-Damage-Only Crashes   120.76     Percent of Total Expected Crashes     Percent Fatal and Injury Crashes (%)   52     Percent Fatal and Serious Injury Crashes (%)   33     Percent Property-Damage-Only Crashes (%)   48     Expected Crash Rate     Crash Rate (crashes/mi/yr)   5.6505     FI Crash Rate (crashes/mi/yr)   2.9555     FI no/C Crash Rate (crashes/mi/yr)   1.8456     PDO Crash Rate (crashes/mi/yr)   2.6950     Expected Travel Crash Rate     Total Travel (million veh-mi)   235.89     Travel FI Crash Rate (crashes/million veh-mi)   1.07     Travel FI Crash Rate (crashes/million veh-mi)   0.56     Travel FI Crash Rate (crashes/million veh-mi)   0.56	Average Future Road AADT (vpd)	14,422
Fatal and Injury Crashes  Fatal and Serious Injury Crashes  Property-Damage-Only Crashes  Percent of Total Expected Crashes  Percent Fatal and Injury Crashes (%)  Percent Fatal and Serious Injury Crashes (%)  Percent Fatal and Serious Injury Crashes (%)  Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  O.56  Travel FI no/C Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)	Expected Crashes	
Fatal and Serious Injury Crashes   82.70	Total Crashes	253.20
Property-Damage-Only Crashes  Percent of Total Expected Crashes  Percent Fatal and Injury Crashes (%) 52  Percent Fatal and Serious Injury Crashes (%) 33  Percent Property-Damage-Only Crashes (%) 48  Expected Crash Rate  Crash Rate (crashes/mi/yr) 5.6505  FI Crash Rate (crashes/mi/yr) 2.9555  FI no/C Crash Rate (crashes/mi/yr) 1.8456  PDO Crash Rate (crashes/mi/yr) 2.6950  Expected Travel Crash Rate  Total Travel (million veh-mi) 235.89  Travel Crash Rate (crashes/million veh-mi) 1.07  Travel FI Crash Rate (crashes/million veh-mi) 0.56  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.56	Fatal and Injury Crashes	132.44
Percent of Total Expected Crashes  Percent Fatal and Injury Crashes (%)  Percent Fatal and Serious Injury Crashes (%)  Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  5.6505  FI Crash Rate (crashes/mi/yr)  2.9555  FI no/C Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  0.56  Travel FI Crash Rate (crashes/million veh-mi)  0.55	Fatal and Serious Injury Crashes	82.70
Percent Fatal and Injury Crashes (%)  Percent Fatal and Serious Injury Crashes (%)  Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  O.56  Travel FI no/C Crash Rate (crashes/million veh-mi)  O.56	Property-Damage-Only Crashes	120.76
Percent Fatal and Serious Injury Crashes (%)  Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  5.6505  FI Crash Rate (crashes/mi/yr)  2.9555  FI no/C Crash Rate (crashes/mi/yr)  1.8456  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  0.56  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.55	Percent of Total Expected Crashes	
Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  5.6505  FI Crash Rate (crashes/mi/yr)  2.9555  FI no/C Crash Rate (crashes/mi/yr)  1.8456  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  235.89  Travel Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  0.56  Travel FI no/C Crash Rate (crashes/million veh-mi)	Percent Fatal and Injury Crashes (%)	52
Expected Crash Rate  Crash Rate (crashes/mi/yr) 5.6505  FI Crash Rate (crashes/mi/yr) 2.9555  FI no/C Crash Rate (crashes/mi/yr) 1.8456  PDO Crash Rate (crashes/mi/yr) 2.6950  Expected Travel Crash Rate  Total Travel (million veh-mi) 235.89  Travel Crash Rate (crashes/million veh-mi) 1.07  Travel FI Crash Rate (crashes/million veh-mi) 0.56  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.35	Percent Fatal and Serious Injury Crashes (%)	33
Crash Rate (crashes/mi/yr) 5.6505  FI Crash Rate (crashes/mi/yr) 2.9555  FI no/C Crash Rate (crashes/mi/yr) 1.8456  PDO Crash Rate (crashes/mi/yr) 2.6950  Expected Travel Crash Rate  Total Travel (million veh-mi) 235.89  Travel Crash Rate (crashes/million veh-mi) 1.07  Travel FI Crash Rate (crashes/million veh-mi) 0.56  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.35	Percent Property-Damage-Only Crashes (%)	48
FI Crash Rate (crashes/mi/yr)  2.9555 FI no/C Crash Rate (crashes/mi/yr) 1.8456 PDO Crash Rate (crashes/mi/yr) 2.6950  Expected Travel Crash Rate  Total Travel (million veh-mi) 235.89  Travel Crash Rate (crashes/million veh-mi) 1.07  Travel FI Crash Rate (crashes/million veh-mi) 0.56  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.35	Expected Crash Rate	
FI no/C Crash Rate (crashes/mi/yr)  1.8456  PDO Crash Rate (crashes/mi/yr)  2.6950  Expected Travel Crash Rate  Total Travel (million veh-mi)  235.89  Travel Crash Rate (crashes/million veh-mi)  1.07  Travel FI Crash Rate (crashes/million veh-mi)  0.56  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.35	Crash Rate (crashes/mi/yr)	5.6505
PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  235.89  Travel Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  0.56  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.35	FI Crash Rate (crashes/mi/yr)	2.9555
Expected Travel Crash Rate  Total Travel (million veh-mi)  235.89  Travel Crash Rate (crashes/million veh-mi)  1.07  Travel FI Crash Rate (crashes/million veh-mi)  0.56  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.35	FI no/C Crash Rate (crashes/mi/yr)	1.8456
Total Travel (million veh-mi)  235.89  Travel Crash Rate (crashes/million veh-mi)  1.07  Travel FI Crash Rate (crashes/million veh-mi)  0.56  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.35	PDO Crash Rate (crashes/mi/yr)	2.6950
Travel Crash Rate (crashes/million veh-mi)  1.07  Travel FI Crash Rate (crashes/million veh-mi)  0.56  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.35	Expected Travel Crash Rate	
Travel FI Crash Rate (crashes/million veh-mi) 0.56  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.35	Total Travel (million veh-mi)	235.89
Travel FI no/C Crash Rate (crashes/million veh-mi) 0.35	Travel Crash Rate (crashes/million veh-mi)	1.07
	Travel FI Crash Rate (crashes/million veh-mi)	0.56
Travel PDO Crash Rate (crashes/million veh-mi) 0.51	Travel FI no/C Crash Rate (crashes/million veh-mi)	0.35
	Travel PDO Crash Rate (crashes/million veh-mi)	0.51

Table 29. Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 3)

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Crash Frequency		PDO Crash Frequency	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted FI no/C Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) FI no/C Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/ mi/yr)	Expected Travel Crash Rate (crashes/ million veh-mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
1	585+00.000	594+84.940	0.1865	17.884	18.701	0.6878	0.5117	0.3188	0.1762	0.7193	0.4240	0.2248	0.2953	-0.0314	0.0877	0.0940	-0.1191	3.6873	0.84	
2	594+84.940	600+00.000	0.0975	4.666	9.779	0.1795	0.1087	0.0678	0.0707	0.3761	0.2217	0.1176	0.1544	-0.1967	-0.1130	-0.0498	-0.0837	1.8399	0.42	
3	600+00.000	601+00.000	0.0189	0.912	1.911	0.0351	0.0212	0.0132	0.0138	0.0735	0.0433	0.0230	0.0302	-0.0384	-0.0221	-0.0098	-0.0163	1.8518	0.42	
4	601+00.000	602+00.000	0.0189	0.912	1.911	0.0351	0.0212	0.0132	0.0138	0.0735	0.0433	0.0230	0.0302	-0.0384	-0.0221	-0.0098	-0.0163	1.8518	0.42	
5	602+00.000	605+00.000	0.0568	15.348	7.486	0.5903	0.1555	0.0969	0.4348	0.2879	0.1665	0.0852	0.1214	0.3024	-0.0110	0.0117	0.3134	10.3895	1.90	
6	605+00.000	605+60.000	0.0114	0.743	1.629	0.0286	0.0173	0.0108	0.0113	0.0627	0.0362	0.0185	0.0264	-0.0341	-0.0190	-0.0078	-0.0151	2.5153	0.46	
7	605+60.000	605+70.000	0.0019	0.119	0.249	0.0046	0.0028	0.0017	0.0018	0.0096	0.0056	0.0028	0.0040	-0.0050	-0.0028	-0.0011	-0.0022	2.4181	0.44	
SD38/260th_St (v2)	605+70.000			35.375	96.924	1.3606	0.5456	0.3469	0.8150	3.7279	1.7583	0.8841	1.9696	-2.3673	-1.2127	-0.5372	-1.1546			0.20
8	605+70.000	605+75.000	0.0009	0.059	0.125	0.0023	0.0014	0.0009	0.0009	0.0048	0.0028	0.0014	0.0020	-0.0025	-0.0014	-0.0006	-0.0011	2.4181	0.44	
9	605+75.000	609+00.000	0.0616	15.646	8.110	0.6018	0.5117	0.3188	0.0901	0.3119	0.1804	0.0923	0.1315	0.2898	0.3313	0.2266	-0.0415	9.7763	1.79	
10	609+00.000	609+21.930	0.0042	12.037	0.547	0.4630	0.0149	0.0093	0.4480	0.0210	0.0122	0.0062	0.0089	0.4419	0.0028	0.0031	0.4392	111.4663	20.38	
11	609+21.930	611+40.000	0.0413	2.597	5.442	0.0999	0.0605	0.0377	0.0394	0.2093	0.1210	0.0619	0.0883	-0.1094	-0.0605	-0.0242	-0.0489	2.4181	0.44	
12	611+40.000	612+50.000	0.0208	1.310	2.745	0.0504	0.0305	0.0190	0.0199	0.1056	0.0611	0.0312	0.0445	-0.0552	-0.0305	-0.0122	-0.0247	2.4181	0.44	
13	612+50.000	624+64.530	0.2300	26.091	30.137	1.0035	0.4602	0.2868	0.5433	1.1591	0.6701	0.3426	0.4890	-0.1556	-0.2099	-0.0558	0.0543	4.3625	0.80	
14	624+64.530	636+92.820	0.2326	14.544	30.479	0.5594	0.3389	0.2112	0.2205	1.1723	0.6777	0.3465	0.4945	-0.6129	-0.3388	-0.1353	-0.2740	2.4045	0.44	
15	636+92.820	639+00.000	0.0392	2.453	5.141	0.0944	0.0572	0.0356	0.0372	0.1977	0.1143	0.0584	0.0834	-0.1034	-0.0572	-0.0228	-0.0462	2.4045	0.44	
16	639+00.000	640+00.000	0.0189	1.282	2.935	0.0493	0.0297	0.0185	0.0196	0.1129	0.0653	0.0334	0.0476	-0.0636	-0.0356	-0.0149	-0.0280	2.6043	0.48	
17	640+00.000	647+26.050	0.1375	20.307	18.016	0.7810	0.3079	0.1918	0.4732	0.6929	0.4006	0.2048	0.2923	0.0881	-0.0927	-0.0130	0.1808	5.6799	1.04	
18	647+26.050	667+80.000	0.3890	47.740	50.967	1.8362	0.8064	0.5024	1.0298	1.9603	1.1333	0.5793	0.8269	-0.1241	-0.3270	-0.0769	0.2029	4.7201	0.86	
19	667+80.000	668+80.000	0.0189	1.232	2.700	0.0474	0.0286	0.0178	0.0188	0.1039	0.0600	0.0307	0.0438	-0.0565	-0.0314	-0.0129	-0.0250	2.5012	0.46	
20	668+80.000	672+86.110	0.0769	4.809	10.077	0.1849	0.1120	0.0698	0.0729	0.3876	0.2241	0.1145	0.1635	-0.2026	-0.1120	-0.0447	-0.0906	2.4045	0.44	
21	672+86.110	676+00.000	0.0594	27.137	7.789	1.0437	0.9499	0.5919	0.0939	0.2996	0.1732	0.0885	0.1264	0.7442	0.7767	0.5033	-0.0325	17.5567	3.21	
All Segments			1.7235	217.827	216.876	8.3780	4.5482	2.8340	3.8297	8.3414	4.8368	2.4867	3.5046	0.0366	-0.2886	0.3472	0.3252	4.8611	0.92	
All Intersections				35.375	96.924	1.3606	0.5456	0.3469	0.8150	3.7279	1.7583	0.8841	1.9696	-2.3673	-1.2127	-0.5372	-1.1546			0.20
Total			1.7235	253.202	313.801	9.7385	5.0938	3.1808	4.6448	12.0693	6.5951	3.3708	5.4742	-2.3307	-1.5013	-0.1900	-0.8294	5.6505		

Table 30. Expected Crash Frequencies and Rates by Horizontal Design Element (Section 3)

Title	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected FI no/C Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)		Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) FI no/C Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	/vr)	Expected Travel Crash Rate (crashes/mi Ilion veh- mi)
Tangent	585+00.000	594+84.940	0.1865	17.884	18.701	0.6878	0.5117	0.3188	0.1762	0.7193	0.4240	0.2248	0.2953	-0.0314	0.0877	0.0940	-0.1191	3.6873	0.84
Simple Curve 1	594+84.940	609+21.930	0.2722	50.443	31.748	1.9401	0.8548	0.5326	1.0853	1.2211	0.7120	0.3700	0.5090	0.7190	0.1428	0.1626	0.5762	7.1286	1.35
Tangent	609+21.930	624+64.300	0.2921	29.992	38.318	1.1535	0.5512	0.3434	0.6024	1.4738	0.8521	0.4357	0.6217	-0.3202	-0.3009	-0.0922	-0.0193	3.9489	0.72
Simple Curve 2	624+64.300	624+64.530	0.0000	0.005	0.006	0.0002	0.0001	0.0001	0.0001	0.0002	0.0001	0.0001	0.0001	-0.0000	-0.0000	-0.0000	0.0000	4.3625	0.80
Tangent	624+64.530	636+92.820	0.2326	14.544	30.479	0.5594	0.3389	0.2112	0.2205	1.1723	0.6777	0.3465	0.4945	-0.6129	-0.3388	-0.1353	-0.2740	2.4045	0.44
Simple Curve 3	636+92.820	647+26.050	0.1957	24.043	26.092	0.9247	0.3947	0.2460	0.5300	1.0036	0.5802	0.2966	0.4233	-0.0788	-0.1855	-0.0507	0.1067	4.7254	0.86
Tangent	647+26.050	672+86.110	0.4849	53.780	63.744	2.0685	0.9470	0.5901	1.1215	2.4517	1.4174	0.7246	1.0343	-0.3832	-0.4704	-0.1345	0.0872	4.2661	0.78
Simple Curve 4	672+86.110	676+00.000	0.0594	27.137	7.789	1.0437	0.9499	0.5919	0.0939	0.2996	0.1732	0.0885	0.1264	0.7442	0.7767	0.5033	-0.0325	17.5567	3.21

Table 31. Predicted Crash Frequencies by Year (Section 3)

Year	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)
2025	4.43	2.44	55.091	1.40	31.489	1.99	44.909
2026	6.47	3.62	55.893	2.02	31.235	2.85	44.107
2027	7.48	4.16	55.579	2.28	30.476	3.32	44.421
2028	8.52	4.71	55.309	2.54	29.817	3.81	44.691
2029	9.58	5.28	55.073	2.80	29.238	4.30	44.927
2030	9.90	5.45	55.007	2.88	29.064	4.45	44.993
2031	10.22	5.62	54.946	2.95	28.898	4.61	45.054
2032	10.54	5.79	54.891	3.03	28.738	4.76	45.109
2033	10.87	5.96	54.840	3.11	28.585	4.91	45.160
2034	11.19	6.13	54.794	3.18	28.438	5.06	45.206
2035	11.52	6.31	54.752	3.26	28.296	5.21	45.248
2036	11.84	6.48	54.712	3.33	28.160	5.36	45.288
2037	12.17	6.65	54.676	3.41	28.029	5.52	45.324
2038	12.50	6.83	54.643	3.49	27.901	5.67	45.357
2039	12.83	7.01	54.612	3.56	27.779	5.82	45.388
2040	13.16	7.18	54.583	3.64	27.659	5.98	45.417
2041	13.50	7.36	54.542	3.72	27.547	6.14	45.458
2042	13.84	7.54	54.503	3.80	27.438	6.30	45.497
2043	14.19	7.73	54.465	3.88	27.331	6.46	45.535
2044	14.53	7.91	54.427	3.96	27.227	6.62	45.573
2045	14.88	8.09	54.391	4.04	27.126	6.79	45.609
2046	15.23	8.28	54.356	4.12	27.027	6.95	45.644
2047	15.57	8.46	54.321	4.19	26.930	7.12	45.679
2048	15.93	8.65	54.287	4.27	26.836	7.28	45.713
2049	16.28	8.83	54.255	4.35	26.743	7.45	45.745
2050	16.63	9.02	54.222	4.43	26.653	7.61	45.778
Total	313.80	171.47	54.644	87.64	27.929	142.33	45.356
Average	12.07	6.59	54.644	3.37	27.929	5.47	45.356

Table 32. Expected Crash Frequencies by Year (Section 3)

Year	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)
2025	3.57	1.89	52.734	1.32	36.826	1.69	47.224
2026	5.22	2.79	53.501	1.91	36.529	2.42	46.381
2027	6.04	3.21	53.201	2.15	35.641	2.82	46.711
2028	6.87	3.64	52.942	2.40	34.871	3.23	46.995
2029	7.73	4.07	52.716	2.64	34.194	3.65	47.243
2030	7.99	4.21	52.653	2.71	33.990	3.78	47.313
2031	8.25	4.34	52.595	2.79	33.795	3.91	47.377
2032	8.51	4.47	52.542	2.86	33.609	4.04	47.435
2033	8.77	4.60	52.494	2.93	33.430	4.16	47.488
2034	9.03	4.74	52.449	3.00	33.258	4.29	47.536
2035	9.29	4.87	52.409	3.08	33.092	4.42	47.581
2036	9.56	5.00	52.371	3.15	32.933	4.55	47.623
2037	9.82	5.14	52.336	3.22	32.779	4.68	47.660
2038	10.09	5.28	52.304	3.29	32.630	4.81	47.696
2039	10.35	5.41	52.275	3.36	32.487	4.94	47.728
2040	10.62	5.55	52.247	3.44	32.347	5.07	47.758
2041	10.89	5.69	52.209	3.51	32.216	5.21	47.801
2042	11.17	5.83	52.171	3.58	32.089	5.34	47.843
2043	11.45	5.97	52.134	3.66	31.964	5.48	47.883
2044	11.72	6.11	52.098	3.73	31.842	5.62	47.922
2045	12.01	6.25	52.064	3.81	31.724	5.76	47.960
2046	12.29	6.39	52.030	3.88	31.608	5.90	47.998
2047	12.57	6.54	51.997	3.96	31.495	6.04	48.034
2048	12.85	6.68	51.964	4.03	31.384	6.18	48.069
2049	13.13	6.82	51.933	4.11	31.276	6.32	48.104
2050	13.42	6.96	51.902	4.18	31.170	6.46	48.138
Total	253.20	132.44	52.305	82.70	32.662	120.76	47.695
Average	9.74	5.09	52.305	3.18	32.662	4.64	47.695

Table 33. Comparing Predicted and Expected Crashes for the Evaluation Period (Section 3)

Scope	Total Crashes	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)
Predicted	313.80	171.47	54.644	87.64	27.929	142.33	45.356
Expected	253.20	132.44	52.305	82.70	32.662	120.76	47.695
Expected - Predicted	-60.60	-39.03		-4.94		-21.57	
Percent Difference	-23.93	-29.47		-5.97		-17.86	

 Table 34. Expected Crash Type Distribution (Section 3)

Element Type	Crash Type	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)	Total Crashes	Percent Total (%)
Highway Segment	Single	28.14	11.1	22.40	8.8	23.60	9.3	51.84	20.5
Highway Segment	Total Single Vehicle Crashes	28.14	11.1	22.40	8.8	23.60	9.3	51.84	20.5
Highway Segment	Angle Collision	41.62	16.4	25.64	10.1	35.65	14.1	77.55	30.6
Highway Segment	Head-on Collision	3.43	1.4	3.17	1.3	0.10	0.0	1.96	0.8
Highway Segment	Rear-end Collision	36.07	14.2	15.99	6.3	21.91	8.7	53.59	21.2
Highway Segment	Sideswipe	5.68	2.2	3.24	1.3	11.95	4.7	21.35	8.4
Highway Segment	Total Multiple Vehicle Crashes	86.80	34.3	48.04	19.0	69.60	27.5	154.44	61.0
Highway Segment	Total Highway Segment Crashes	118.25	46.7	73.68	29.1	99.57	39.3	217.83	86.0
Highway Segment	Other Collision	3.31	1.3	3.24	1.3	6.37	2.5	11.54	4.6
Intersection	Single	2.10	0.8	1.79	0.7	5.15	2.0	7.15	2.8
Intersection	Total Single Vehicle Crashes	2.10	0.8	1.79	0.7	5.15	2.0	7.15	2.8
Intersection	Angle Collision	7.57	3.0	5.15	2.0	6.19	2.4	13.97	5.5
Intersection	Head-on Collision	0.26	0.1	0.21	0.1	0.32	0.1	0.57	0.2
Intersection	Rear-end Collision	3.02	1.2	0.97	0.4	5.09	2.0	8.07	3.2
Intersection	Sideswipe	0.60	0.2	0.36	0.1	3.31	1.3	3.79	1.5
Intersection	Total Multiple Vehicle Crashes	11.45	4.5	6.69	2.6	14.90	5.9	26.39	10.4
Intersection	Total Intersection Crashes	14.20	5.6	9.02	3.6	21.21	8.4	35.34	14.0
Intersection	Other Collision	0.65	0.3	0.53	0.2	1.17	0.5	1.80	0.7
	Total Crashes	132.45	52.3	82.70	32.7	120.78	47.7	253.17	100.0

**Table 35. Evaluation Message** 

Start Location (Sta. ft)	End Location (Sta. ft)	Message
585+00.000	594+84.940	Warning: for segment #1 (585+00.000 to 594+84.940 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
585+00.000	594+84.940	Warning: for segment #1 (585+00.000 to 594+84.940 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
594+84.940	600+00.000	Warning: for segment #2 (594+84.940 to 600+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
594+84.940	600+00.000	Warning: for segment #2 (594+84.940 to 600+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
600+00.000	601+00.000	Warning: for segment #3 (600+00.000 to 601+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
600+00.000	601+00.000	Warning: for segment #3 (600+00.000 to 601+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
601+00.000	602+00.000	Warning: for segment #4 (601+00.000 to 602+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
601+00.000	602+00.000	Warning: for segment #4 (601+00.000 to 602+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
602+00.000	605+00.000	Warning: for segment #5 (602+00.000 to 605+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
602+00.000	605+00.000	Warning: for segment #5 (602+00.000 to 605+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+00.000	605+60.000	Warning: for segment #6 (605+00.000 to 605+60.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+00.000	605+60.000	Warning: for segment #6 (605+00.000 to 605+60.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+60.000	605+70.000	Warning: for segment #7 (605+60.000 to 605+70.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+60.000	605+70.000	Warning: for segment #7 (605+60.000 to 605+70.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+70.000	605+75.000	Warning: for segment #8 (605+70.000 to 605+75.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+70.000	605+75.000	Warning: for segment #8 (605+70.000 to 605+75.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+75.000	609+00.000	Warning: for segment #9 (605+75.000 to 609+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+75.000	609+00.000	Warning: for segment #9 (605+75.000 to 609+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
609+00.000	609+21.930	Warning: for segment #10 (609+00.000 to 609+21.930 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
609+00.000	609+21.930	Warning: for segment #10 (609+00.000 to 609+21.930 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
609+21.930	611+40.000	Warning: for segment #11 (609+21.930 to 611+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
609+21.930	611+40.000	Warning: for segment #11 (609+21.930 to 611+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
611+40.000	612+50.000	Warning: for segment #12 (611+40.000 to 612+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
611+40.000	612+50.000	Warning: for segment #12 (611+40.000 to 612+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
612+50.000	624+64.530	Warning: for segment #13 (612+50.000 to 624+64.530 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
612+50.000	624+64.530	Warning: for segment #13 (612+50.000 to 624+64.530 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
624+64.530	636+92.820	Warning: for segment #14 (624+64.530 to 636+92.820 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
624+64.530	636+92.820	Warning: for segment #14 (624+64.530 to 636+92.820 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
636+92.820	639+00.000	Warning: for segment #15 (636+92.820 to 639+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
636+92.820	639+00.000	Warning: for segment #15 (636+92.820 to 639+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0

Start Location (Sta. ft)	End Location (Sta. ft)	Message
639+00.000	640+00.000	Warning: for segment #16 (639+00.000 to 640+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
639+00.000	640+00.000	Warning: for segment #16 (639+00.000 to 640+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
640+00.000	647+26.050	Warning: for segment #17 (640+00.000 to 647+26.050 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
640+00.000	647+26.050	Warning: for segment #17 (640+00.000 to 647+26.050 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
647+26.050	667+80.000	Warning: for segment #18 (647+26.050 to 667+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
647+26.050	667+80.000	Warning: for segment #18 (647+26.050 to 667+80.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
667+80.000	668+80.000	Warning: for segment #19 (667+80.000 to 668+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
667+80.000	668+80.000	Warning: for segment #19 (667+80.000 to 668+80.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
668+80.000	672+86.110	Warning: for segment #20 (668+80.000 to 672+86.110 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
668+80.000	672+86.110	Warning: for segment #20 (668+80.000 to 672+86.110 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
672+86.110	676+00.000	Warning: for segment #21 (672+86.110 to 676+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
672+86.110	676+00.000	Warning: for segment #21 (672+86.110 to 676+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
585+00.000	594+84.940	Warning: for segment #1 (585+00.000 to 594+84.940), no foreslope data available for left side of road for use by AFM3ru, using 1.0
585+00.000	594+84.940	Warning: for segment #1 (585+00.000 to 594+84.940 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
594+84.940	600+00.000	Warning: for segment #2 (594+84.940 to 600+00.000), no foreslope data available for left side of road for use by AFM3ru, using 1.0
594+84.940	600+00.000	Warning: for segment #2 (594+84.940 to 600+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
600+00.000	601+00.000	Warning: for segment #3 (600+00.000 to 601+00.000), no foreslope data available for left side of road for use by AFM3ru, using 1.0
600+00.000	601+00.000	Warning: for segment #3 (600+00.000 to 601+00.000), no foreslope data available for right side of road for use by AFM3ru, using 1.0
601+00.000	602+00.000	Warning: for segment #4 (601+00.000 to 602+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
601+00.000	602+00.000	Warning: for segment #4 (601+00.000 to 602+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
602+00.000	605+00.000	Warning: for segment #5 (602+00.000 to 605+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
602+00.000	605+00.000	Warning: for segment #5 (602+00.000 to 605+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+00.000	605+60.000	Warning: for segment #6 (605+00.000 to 605+60.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+00.000	605+60.000	Warning: for segment #6 (605+00.000 to 605+60.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+60.000	605+70.000	Warning: for segment #7 (605+60.000 to 605+70.000), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+60.000	605+70.000	Warning: for segment #7 (605+60.000 to 605+70.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+70.000	605+75.000	Warning: for segment #8 (605+70.000 to 605+75.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+70.000	605+75.000	Warning: for segment #8 (605+70.000 to 605+75.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+75.000	609+00.000	Warning: for segment #9 (605+75.000 to 609+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+75.000	609+00.000	Warning: for segment #9 (605+75.000 to 609+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
609+00.000	609+21.930	Warning: for segment #10 (609+00.000 to 609+21.930 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
609+00.000	609+21.930	Warning: for segment #10 (609+00.000 to 609+21.930 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0

Start Location (Sta. ft)	End Location (Sta. ft)	Message
609+21.930	611+40.000	Warning: for segment #11 (609+21.930 to 611+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
609+21.930	611+40.000	Warning: for segment #11 (609+21.930 to 611+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
611+40.000	612+50.000	Warning: for segment #12 (611+40.000 to 612+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
611+40.000	612+50.000	Warning: for segment #12 (611+40.000 to 612+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
612+50.000	624+64.530	Warning: for segment #13 (612+50.000 to 624+64.530 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
612+50.000	624+64.530	Warning: for segment #13 (612+50.000 to 624+64.530 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
624+64.530	636+92.820	Warning: for segment #14 (624+64.530 to 636+92.820 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
624+64.530	636+92.820	Warning: for segment #14 (624+64.530 to 636+92.820 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
636+92.820	639+00.000	Warning: for segment #15 (636+92.820 to 639+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
636+92.820	639+00.000	Warning: for segment #15 (636+92.820 to 639+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
639+00.000	640+00.000	Warning: for segment #16 (639+00.000 to 640+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
639+00.000	640+00.000	Warning: for segment #16 (639+00.000 to 640+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
640+00.000	647+26.050	Warning: for segment #17 (640+00.000 to 647+26.050 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
640+00.000	647+26.050	Warning: for segment #17 (640+00.000 to 647+26.050 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
647+26.050	667+80.000	Warning: for segment #18 (647+26.050 to 667+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
647+26.050	667+80.000	Warning: for segment #18 (647+26.050 to 667+80.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
667+80.000	668+80.000	Warning: for segment #19 (667+80.000 to 668+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
667+80.000	668+80.000	Warning: for segment #19 (667+80.000 to 668+80.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
668+80.000	672+86.110	Warning: for segment #20 (668+80.000 to 672+86.110 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
668+80.000	672+86.110	Warning: for segment #20 (668+80.000 to 672+86.110 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
672+86.110	676+00.000	Warning: for segment #21 (672+86.110 to 676+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
672+86.110	676+00.000	Warning: for segment #21 (672+86.110 to 676+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0

# **Section 5 Evaluation**

**Section:** Section 5

**Evaluation Start Location:** 862+60.000 **Evaluation End Location:** 948+50.000

Area Type: Rural

Functional Class: Arterial

**Type of Alignment:** Undivided, Multilane **Model Category:** Rural, Multilane **Calibration Factor:** 4ST=1.0; 4U=1.0;

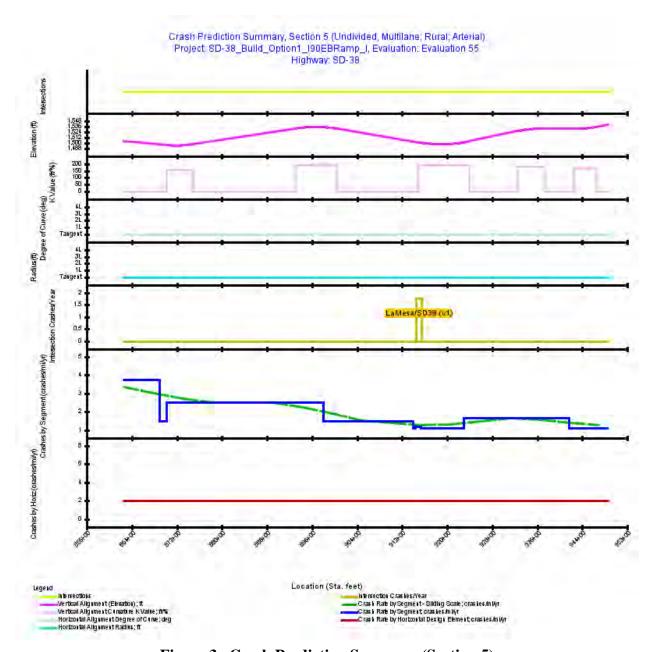


Figure 3. Crash Prediction Summary (Section 5)

**Table 36. Observed Crashes Used in the Evaluation (Section 5)** 

Year	Observed Crashes	Total Crashes Used	FI Crashes	FI no/C Crashes	PDO Crashes
2018	1	1	0	0	1
2019	0	0	0	0	0
2020	3	3	2	2	1
2021	2	2	1	1	1
2022	1	1	1	1	0
All Years	7 <sup>[1]</sup>	7	4	4	3

#### **Footnotes**

<sup>[1]</sup> Note: Observed crash data that does not comply with the associated CPM model requirements may not be used in EB processing.

**Table 37. Evaluation Highway - Homogeneous Segments (Section 5)** 

eg. Vo.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Width (ft)	Right Lane Width (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Rig Sid Slop
75	Rural Multi-Lane Segment Four- lane Undivided	862+60.000	869+00.000	640.00	0.1212	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 (
76	Rural Multi-Lane Segment Four- lane Undivided	869+00.000	870+20.000	120.00	0.0227	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1
77	Rural Multi-Lane Segment Four- lane Undivided	870+20.000	898+00.000	2,780.00	0.5265	2025; 6,704; 2026; 7,305; 2027; 7,907; 2028; 8,508; 2029; 9,110; 2030; 9,295; 2031; 9,480; 2032; 9,666; 2033; 9,851; 2034; 10,037; 2035; 10,222; 2036; 10,408; 2037; 10,593; 2038; 10,779; 2039; 10,964; 2040; 11,150; 2041; 11,375; 2042; 11,600; 2043; 11,825; 2044; 12,050; 2045; 12,275; 2046; 12,500; 2047; 12,725; 2048; 12,950; 2049; 13,175; 2050; 13,400	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1
78	Rural Multi-Lane Segment Four- lane Undivided	898+00.000	906+70.000	870.00	0.1648	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1
79	Rural Multi-Lane Segment Four- lane Undivided	906+70.000	907+80.000	110.00	0.0208	2025; 6,704; 2026; 7,305; 2027; 7,907; 2028; 8,508; 2029; 9,110; 2030; 9,295; 2031; 9,480; 2032; 9,666; 2033; 9,851; 2034; 10,037; 2035; 10,222; 2036; 10,408; 2037; 10,593; 2038; 10,779; 2039; 10,964; 2040; 11,150; 2041; 11,375; 2042; 11,600; 2043; 11,825; 2044; 12,050; 2045; 12,275; 2046; 12,500; 2047; 12,725; 2048; 12,950; 2049; 13,175; 2050; 13,400	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1
80	Rural Multi-Lane Segment Four- lane Undivided	907+80.000	914+00.000	620.00	0.1174	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	l
31	Rural Multi-Lane Segment Four- lane Undivided	914+00.000	914+30.000	30.00	0.0057	2025; 5,988; 2026; 6,351; 2027; 6,714; 2028; 7,077; 2029; 7,440; 2030; 7,545; 2031; 7,650; 2032; 7,756; 2033; 7,861; 2034; 7,967; 2035; 8,072; 2036; 8,178; 2037; 8,283; 2038; 8,389; 2039; 8,494; 2040; 8,600; 2041; 8,722; 2042; 8,844; 2043; 8,966; 2044; 9,088; 2045; 9,210; 2046; 9,332; 2047; 9,454; 2048; 9,576; 2049; 9,698; 2050; 9,820	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	ı
12	Rural Multi-Lane Segment Four- lane Undivided	914+30.000	914+40.000	10.00	0.0019	2025; 5,988; 2026; 6,351; 2027; 6,714; 2028; 7,077; 2029; 7,440; 2030; 7,545; 2031; 7,650; 2032; 7,756; 2033; 7,861; 2034; 7,967; 2035; 8,072; 2036; 8,178; 2037; 8,283; 2038; 8,389; 2039; 8,494; 2040; 8,600; 2041; 8,722; 2042; 8,844; 2043; 8,966; 2044; 9,088; 2045; 9,210; 2046; 9,332; 2047; 9,454; 2048; 9,576; 2049; 9,698; 2050; 9,820	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	ı
33	Rural Multi-Lane Segment Four- lane Undivided	914+40.000	915+40.000	100.00	0.0189	2025; 5,988; 2026; 6,351; 2027; 6,714; 2028; 7,077; 2029; 7,440; 2030; 7,545; 2031; 7,650; 2032; 7,756; 2033; 7,861; 2034; 7,967; 2035; 8,072; 2036; 8,178; 2037; 8,283; 2038; 8,389; 2039; 8,494; 2040; 8,600; 2041; 8,722; 2042; 8,844; 2043; 8,966; 2044; 9,088; 2045; 9,210; 2046; 9,332; 2047; 9,454; 2048; 9,576; 2049; 9,698; 2050; 9,820	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	l
4	Rural Multi-Lane Segment Four- lane Undivided	915+40.000	921+00.000	560.00	0.1061	2025; 5,988; 2026; 6,351; 2027; 6,714; 2028; 7,077; 2029; 7,440; 2030; 7,545; 2031; 7,650; 2032; 7,756; 2033; 7,861; 2034; 7,967; 2035; 8,072; 2036; 8,178; 2037; 8,283; 2038; 8,389; 2039; 8,494; 2040; 8,600; 2041; 8,722; 2042; 8,844; 2043; 8,966; 2044; 9,088; 2045; 9,210; 2046; 9,332; 2047; 9,454; 2048; 9,576; 2049; 9,698; 2050; 9,820	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	l
5	Rural Multi-Lane Segment Four- lane Undivided	921+00.000	921+90.000	90.00	0.0170	2025; 5,988; 2026; 6,351; 2027; 6,714; 2028; 7,077; 2029; 7,440; 2030; 7,545; 2031; 7,650; 2032; 7,756; 2033; 7,861; 2034; 7,967; 2035; 8,072; 2036; 8,178; 2037; 8,283; 2038; 8,389; 2039; 8,494; 2040; 8,600; 2041; 8,722; 2042; 8,844; 2043; 8,966; 2044; 9,088; 2045; 9,210; 2046; 9,332; 2047; 9,454; 2048; 9,576; 2049; 9,698; 2050; 9,820	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	l
6	Rural Multi-Lane Segment Four- lane Undivided	921+90.000	923+00.000	110.00	0.0208	2025; 5,988; 2026; 6,351; 2027; 6,714; 2028; 7,077; 2029; 7,440; 2030; 7,545; 2031; 7,650; 2032; 7,756; 2033; 7,861; 2034; 7,967; 2035; 8,072; 2036; 8,178; 2037; 8,283; 2038; 8,389; 2039; 8,494; 2040; 8,600; 2041; 8,722; 2042; 8,844; 2043; 8,966; 2044; 9,088; 2045; 9,210; 2046; 9,332; 2047; 9,454; 2048; 9,576; 2049; 9,698; 2050; 9,820	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	l
17	Rural Multi-Lane Segment Four- lane Undivided	923+00.000	941+70.000	1,870.00	0.3542	2025; 5,988; 2026; 6,351; 2027; 6,714; 2028; 7,077; 2029; 7,440; 2030; 7,545; 2031; 7,650; 2032; 7,756; 2033; 7,861; 2034; 7,967; 2035; 8,072; 2036; 8,178; 2037; 8,283; 2038; 8,389; 2039; 8,494; 2040; 8,600; 2041; 8,722; 2042; 8,844; 2043; 8,966; 2044; 9,088; 2045; 9,210; 2046; 9,332; 2047; 9,454; 2048; 9,576; 2049; 9,698; 2050; 9,820	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	ı
8	Rural Multi-Lane Segment Four- lane Undivided	941+70.000	948+00.000	630.00	0.1193	2025; 5,988; 2026; 6,351; 2027; 6,714; 2028; 7,077; 2029; 7,440; 2030; 7,545; 2031; 7,650; 2032; 7,756; 2033; 7,861; 2034; 7,967; 2035; 8,072; 2036; 8,178; 2037; 8,283; 2038; 8,389; 2039; 8,494; 2040; 8,600; 2041; 8,722; 2042; 8,844; 2043; 8,966; 2044; 9,088; 2045; 9,210; 2046; 9,332; 2047; 9,454; 2048; 9,576; 2049; 9,698; 2050; 9,820	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	l

eg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT		Right Lane Width (ft)	Shoulder Shoulder	Right Shoulder Width (ft)	wiam	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Side	Right Side Slope
89	Rural Multi-Lane Segment Four- lane Undivided	948+00.000	948+50.000	50.00		2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1

# Table 38. User Defined CMF Used in the Eval Segment CPM Evaluation (Section 5)

Name	Description	Start Loc. (Sta. ft)	End Loc. (Sta. ft)	Start CMF Year	End CMF Year	Severity	CMF Value
1	TWLTL	862+60.000	948+50.000	2025	2025	Total	0.6900

**Table 39.** Crash History Highway - Homogeneous Segments (Section 5)

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Width (ft)	Right Lane Width (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
75	Rural Multi-Lane Segment Four-lane Undivided	862+60.000	869+00.000	640.00	0.1212	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
76	Rural Multi-Lane Segment Four-lane Undivided	869+00.000	870+20.000	120.00	0.0227	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
77	Rural Multi-Lane Segment Four-lane Undivided	870+20.000	898+00.000	2,780.00	0.5265	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
78	Rural Multi-Lane Segment Four-lane Undivided	898+00.000	906+70.000	870.00	0.1648	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
79	Rural Multi-Lane Segment Four-lane Undivided	906+70.000	907+80.000	110.00	0.0208	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
80	Rural Multi-Lane Segment Four-lane Undivided	907+80.000	914+00.000	620.00	0.1174	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
81	Rural Multi-Lane Segment Four-lane Undivided	914+00.000	914+30.000	30.00	0.0057	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
82	Rural Multi-Lane Segment Four-lane Undivided	914+30.000	914+40.000	10.00	0.0019	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
83	Rural Multi-Lane Segment Four-lane Undivided	914+40.000	915+40.000	100.00	0.0189	2018-2022: 4,900	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1
84	Rural Multi-Lane Segment Four-lane Undivided	915+40.000	921+00.000	560.00	0.1061	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
85	Rural Multi-Lane Segment Four-lane Undivided	921+00.000	921+90.000	90.00	0.0170	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
86	Rural Multi-Lane Segment Four-lane Undivided	921+90.000	923+00.000	110.00	0.0208	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
87	Rural Multi-Lane Segment Four-lane Undivided	923+00.000	941+70.000	1,870.00	0.3542	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
88	Rural Multi-Lane Segment Four-lane Undivided	941+70.000	948+00.000	630.00	0.1193	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
89	Rural Multi-Lane Segment Four-lane Undivided	948+00.000	948+50.000	50.00	0.0095	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1

# **Table 40. Evaluation Intersection (Section 5)**

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
7		Rural Multi-Lane Intersection Four-Legged w/STOP control	915+00.000	2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088;	2025: 1,266; 2026: 1,293; 2027: 1,320; 2028: 1,347; 2029: 1,375; 2030: 1,407; 2031: 1,439; 2032: 1,471; 2033: 1,504; 2034: 1,536; 2035: 1,568; 2036: 1,725; 2037: 1,949; 2038: 2,172; 2039; 2,396; 2040: 2,620; 2041: 2,940; 2042: 3,261; 2043: 3,581; 2044: 3,902; 2045: 4,222; 2046: 4,543; 2047: 4,863; 2048: 5,184; 2049: 5,504; 2050: 5,825	4	Stop-Controlled	0	0	0.00	0.00	false

# **Table 41. Crash History Intersection (Section 5)**

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
7	LaMesa/SD38 (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	915+00.000	2018-2022: 4,900	2018-2022: 1,185	4	Stop-Controlled	0	0	0.00	0.00	false

Table 42. Expected Highway Crash Rates and Frequencies Summary (Section 5)

First Year of Analysis  Last Year of Analysis  Evaluated Length (mi)  Average Future Road AADT (vpd)  Expected Crashes  Total Crashes  12  Fatal and Injury Crashes  7  Fatal and Serious Injury Crashes
Evaluated Length (mi) 1.6  Average Future Road AADT (vpd) 9  Expected Crashes  Total Crashes 12  Fatal and Injury Crashes 7
Average Future Road AADT (vpd)  Expected Crashes  Total Crashes  12  Fatal and Injury Crashes  7
Expected Crashes  Total Crashes  12  Fatal and Injury Crashes  7
Total Crashes 12 Fatal and Injury Crashes 7
Fatal and Injury Crashes 7
Fatal and Serious Injury Crashes 4
Property-Damage-Only Crashes 5
Percent of Total Expected Crashes
Percent Fatal and Injury Crashes (%)
Percent Fatal and Serious Injury Crashes (%)
Percent Property-Damage-Only Crashes (%)
Expected Crash Rate
Crash Rate (crashes/mi/yr) 3.0
FI Crash Rate (crashes/mi/yr) 1.0
FI no/C Crash Rate (crashes/mi/yr) 1.0
PDO Crash Rate (crashes/mi/yr) 1.3
Expected Travel Crash Rate
Total Travel (million veh-mi) 14
Travel Crash Rate (crashes/million veh-mi)
Travel FI Crash Rate (crashes/million veh-mi)
Travel FI no/C Crash Rate (crashes/million veh-mi)
Travel PDO Crash Rate (crashes/million veh-mi)

Table 43. Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 5)

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Total Crash Frequency	Crash Frequency		PDO Crash Frequency	Frequency	Predicted FI Crash Frequency (crashes/yr)	Predicted FI no/C Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)		(Expected - Predicted) FI no/C Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/ mi/yr)	Expected Travel Crash Rate (crashes/ million veh-mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
75	862+60.000	869+00.000	0.1212	11.835	10.550	0.4552	0.1678	0.1025	0.2874	0.4058	0.2415	0.1305	0.1643	0.0494	-0.0737	-0.0279	0.1231	3.7553	0.97	
76	869+00.000	870+20.000	0.0227	0.872	1.978	0.0335	0.0200	0.0122	0.0135	0.0761	0.0453	0.0245	0.0308	-0.0425	-0.0252	-0.0122	-0.0173	1.4754	0.38	
77	870+20.000	898+00.000	0.5265	34.568	45.826	1.3295	0.9411	0.5751	0.3884	1.7625	1.0491	0.5667	0.7135	-0.4330	-0.1079	0.0085	-0.3251	2.5252	0.65	
78	898+00.000	906+70.000	0.1648	6.321	14.341	0.2431	0.1453	0.0888	0.0978	0.5516	0.3283	0.1773	0.2233	-0.3085	-0.1830	-0.0885	-0.1255	1.4754	0.38	
79	906+70.000	907+80.000	0.0208	0.799	1.813	0.0307	0.0184	0.0112	0.0124	0.0697	0.0415	0.0224	0.0282	-0.0390	-0.0231	-0.0112	-0.0159	1.4754	0.38	
80	907+80.000	914+00.000	0.1174	4.504	10.220	0.1732	0.1036	0.0633	0.0697	0.3931	0.2340	0.1264	0.1591	-0.2198	-0.1304	-0.0631	-0.0894	1.4754	0.38	
81	914+00.000	914+30.000	0.0057	0.162	0.368	0.0062	0.0037	0.0023	0.0025	0.0142	0.0086	0.0048	0.0056	-0.0079	-0.0049	-0.0026	-0.0030	1.0982	0.36	
82	914+30.000	914+40.000	0.0019	0.054	0.123	0.0021	0.0012	0.0008	0.0008	0.0047	0.0029	0.0016	0.0019	-0.0026	-0.0016	-0.0009	-0.0010	1.0982	0.36	
83	914+40.000	915+40.000	0.0189	0.579	1.443	0.0223	0.0132	0.0081	0.0090	0.0555	0.0337	0.0190	0.0218	-0.0332	-0.0205	-0.0109	-0.0127	1.1758	0.39	
LaMesa/SD38 (v1)	915+00.000			45.389	83.561	1.7457	0.7046	0.4318	1.0411	3.2139	1.8143	0.9753	1.3996	-1.4682	-1.1097	-0.5434	-0.3585			0.47
84	915+40.000	921+00.000	0.1061	3.028	6.871	0.1165	0.0696	0.0425	0.0468	0.2643	0.1607	0.0903	0.1036	-0.1478	-0.0910	-0.0478	-0.0568	1.0982	0.36	
85	921+00.000	921+90.000	0.0170	0.487	1.104	0.0187	0.0112	0.0068	0.0075	0.0425	0.0258	0.0145	0.0167	-0.0238	-0.0146	-0.0077	-0.0091	1.0982	0.36	
86	921+90.000	923+00.000	0.0208	0.595	1.350	0.0229	0.0137	0.0084	0.0092	0.0519	0.0316	0.0177	0.0204	-0.0290	-0.0179	-0.0094	-0.0111	1.0982	0.36	
87	923+00.000	941+70.000	0.3542	15.460	22.944	0.5946	0.4078	0.2492	0.1868	0.8825	0.5365	0.3016	0.3460	-0.2878	-0.1287	-0.0524	-0.1592	1.6789	0.56	
88	941+70.000	948+00.000	0.1193	3.407	7.730	0.1310	0.0783	0.0479	0.0527	0.2973	0.1807	0.1016	0.1166	-0.1663	-0.1024	-0.0537	-0.0639	1.0982	0.36	
89	948+00.000	948+50.000	0.0095	0.270	0.614	0.0104	0.0062	0.0038	0.0042	0.0236	0.0143	0.0081	0.0093	-0.0132	-0.0081	-0.0043	-0.0051	1.0982	0.36	
All Segments			1.6269	82.941	127.275	3.1901	2.0012	1.2229	1.1888	4.8952	2.9344	1.6070	1.9607	-1.7051	-0.9332	-0.3840	-0.7719	1.9608	0.56	
All Intersections		·		45.389	83.561	1.7457	0.7046	0.4318	1.0411	3.2139	1.8143	0.9753	1.3996	-1.4682	-1.1097	-0.5434	-0.3585			0.47
Total			1.6269	128.331	210.836	4.9358	2.7058	1.6548	2.2300	8.1091	4.7487	2.5822	3.3604	-3.1733	-2.0429	-0.9275	-1.1304	3.0339		

# Table 44. Expected Crash Frequencies and Rates by Horizontal Design Element (Section 5)

Title	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Total Crash Frequency	Frequency	no/C Crash Frequency	Expected PDO Crash Frequency (crashes/yr)	Total Crash Frequency	Crash Frequency	Predicted FI no/C Crash Frequency (crashes/yr)	PDO Crash Frequency	Total Crash Frequency	Crash Frequency	(Expected - Predicted) FI no/C Crash Frequency (crashes/yr)	PDO Crash Frequency	(crashes/mi/	Expected Travel Crash Rate (crashes/mil lion veh-mi)
Tangent	862+60.000	948+50.000	1.6269	82.941	127.275	3.1901	2.0012	1.2229	1.1888	4.8952	2.9344	1.6070	1.9607	-1.7051	-0.9332	-0.3840	-0.7719	1.9608	0.55

**Table 45. Predicted Crash Frequencies by Year (Section 5)** 

Year	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)
2025	3.86	2.22	57.635	1.32	34.127	1.63	42.365
2026	5.19	3.03	58.409	1.77	34.162	2.16	41.591
2027	5.58	3.26	58.303	1.89	33.779	2.33	41.697
2028	5.99	3.48	58.200	2.00	33.421	2.50	41.800
2029	6.39	3.71	58.099	2.11	33.082	2.68	41.901
2030	6.53	3.79	58.080	2.15	32.973	2.74	41.920
2031	6.67	3.87	58.062	2.19	32.866	2.80	41.938
2032	6.81	3.95	58.044	2.23	32.760	2.86	41.956
2033	6.95	4.03	58.027	2.27	32.657	2.92	41.973
2034	7.09	4.11	58.010	2.31	32.557	2.98	41.990
2035	7.23	4.20	57.993	2.35	32.459	3.04	42.007
2036	7.46	4.33	58.041	2.41	32.316	3.13	41.959
2037	7.73	4.50	58.123	2.49	32.159	3.24	41.877
2038	8.00	4.66	58.206	2.56	32.012	3.34	41.794
2039	8.26	4.81	58.289	2.63	31.873	3.44	41.711
2040	8.52	4.97	58.371	2.70	31.741	3.54	41.629
2041	8.85	5.17	58.493	2.79	31.579	3.67	41.507
2042	9.17	5.37	58.611	2.88	31.428	3.79	41.389
2043	9.49	5.57	58.727	2.97	31.286	3.92	41.273
2044	9.80	5.77	58.839	3.05	31.151	4.03	41.161
2045	10.11	5.96	58.947	3.14	31.023	4.15	41.053
2046	10.42	6.16	59.053	3.22	30.901	4.27	40.947
2047	10.73	6.35	59.156	3.30	30.784	4.38	40.844
2048	11.03	6.54	59.255	3.38	30.672	4.50	40.745
2049	11.34	6.73	59.352	3.46	30.565	4.61	40.648
2050	11.64	6.92	59.446	3.54	30.462	4.72	40.554
Total	210.84	123.47	58.560	67.14	31.844	87.37	41.440
Average	8.11	4.75	58.560	2.58	31.844	3.36	41.440

Table 46. Expected Crash Frequencies by Year (Section 5)

Year	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)
2025	2.35	1.27	53.954	0.84	35.929	1.08	46.189
2026	3.16	1.73	54.679	1.14	35.966	1.43	45.344
2027	3.40	1.85	54.580	1.21	35.563	1.54	45.459
2028	3.64	1.99	54.483	1.28	35.186	1.66	45.572
2029	3.89	2.12	54.389	1.35	34.829	1.78	45.683
2030	3.97	2.16	54.371	1.38	34.714	1.82	45.703
2031	4.06	2.21	54.354	1.40	34.601	1.86	45.723
2032	4.14	2.25	54.337	1.43	34.490	1.90	45.742
2033	4.23	2.30	54.321	1.46	34.382	1.94	45.761
2034	4.32	2.34	54.305	1.48	34.276	1.98	45.779
2035	4.40	2.39	54.290	1.50	34.173	2.02	45.797
2036	4.54	2.47	54.334	1.55	34.023	2.08	45.746
2037	4.71	2.56	54.411	1.59	33.857	2.15	45.656
2038	4.87	2.65	54.489	1.64	33.702	2.22	45.566
2039	5.03	2.74	54.566	1.69	33.556	2.29	45.475
2040	5.18	2.83	54.643	1.73	33.417	2.35	45.386
2041	5.38	2.95	54.757	1.79	33.247	2.44	45.253
2042	5.58	3.06	54.869	1.85	33.088	2.52	45.123
2043	5.77	3.17	54.976	1.90	32.938	2.60	44.998
2044	5.97	3.29	55.081	1.96	32.796	2.68	44.876
2045	6.16	3.40	55.183	2.01	32.661	2.75	44.757
2046	6.34	3.51	55.282	2.06	32.533	2.83	44.642
2047	6.53	3.62	55.378	2.12	32.410	2.91	44.531
2048	6.72	3.73	55.471	2.17	32.292	2.98	44.422
2049	6.90	3.83	55.562	2.22	32.179	3.06	44.316
2050	7.08	3.94	55.650	2.27	32.071	3.13	44.214
Total	128.33	70.35	54.821	43.02	33.526	57.98	45.179
Average	4.94	2.71	54.821	1.66	33.526	2.23	45.179

Table 47. Comparing Predicted and Expected Crashes for the Evaluation Period (Section 5)

Scope	Total Crashes	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)
Predicted	210.84	123.47	58.560	67.14	31.844	87.37	41.440
Expected	128.33	70.35	54.821	43.02	33.526	57.98	45.179
Expected - Predicted	-82.50	-53.12		-24.11		-29.39	
Percent Difference	-64.29	-75.50		-56.05		-50.69	

 Table 48. Expected Crash Type Distribution (Section 5)

Element Type	Crash Type	FI Crashes	Percent FI	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)	Total Crashes	Percent Total (%)
Highway Segment	Single	12.38	9.7	9.67	7.5	7.33	5.7	19.74	15.4
Highway Segment	Total Single Vehicle Crashes	12.38	9.7	9.67	7.5	7.33	5.7	19.74	15.4
Highway Segment	Angle Collision	18.32	14.3	11.06	8.6	11.06	8.6	29.53	23.0
Highway Segment	Head-on Collision	1.51	1.2	1.37	1.1	0.03	0.0	0.75	0.6
Highway Segment	Rear-end Collision	15.87	12.4	6.90	5.4	6.80	5.3	20.40	15.9
Highway Segment	Sideswipe	2.50	1.9	1.40	1.1	3.71	2.9	8.13	6.3
Highway Segment	Total Multiple Vehicle Crashes	38.19	29.8	20.73	16.2	21.61	16.8	58.80	45.8
Highway Segment	Total Highway Segment Crashes	52.03	40.6	31.80	24.8	30.91	24.1	82.94	64.7
Highway Segment	Other Collision	1.46	1.1	1.40	1.1	1.98	1.5	4.40	3.4
Intersection	Single	2.71	2.1	2.23	1.7	6.58	5.1	9.17	7.1
Intersection	Total Single Vehicle Crashes	2.71	2.1	2.23	1.7	6.58	5.1	9.17	7.1
Intersection	Angle Collision	9.78	7.6	6.41	5.0	7.90	6.2	17.93	14.0
Intersection	Head-on Collision	0.33	0.3	0.26	0.2	0.41	0.3	0.73	0.6
Intersection	Rear-end Collision	3.90	3.0	1.21	0.9	6.50	5.1	10.35	8.1
Intersection	Sideswipe	0.77	0.6	0.45	0.3	4.22	3.3	4.86	3.8
Intersection	Total Multiple Vehicle Crashes	14.78	11.5	8.33	6.5	19.03	14.8	33.86	26.4
Intersection	Total Intersection Crashes	18.34	14.3	11.23	8.8	27.10	21.1	45.34	35.3
Intersection	Other Collision	0.84	0.7	0.66	0.5	1.49	1.2	2.31	1.8
_	Total Crashes	70.37	54.9	43.02	33.5	58.01	45.2	128.28	100.0

**Table 49. Evaluation Message** 

Start Location (Sta. ft)	End Location (Sta. ft)	Message
862+60.000	869+00.000	Warning: for segment #75 (862+60.000 to 869+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
862+60.000	869+00.000	Warning: for segment #75 (862+60.000 to 869+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
869+00.000	870+20.000	Warning: for segment #76 (869+00.000 to 870+20.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
869+00.000	870+20.000	Warning: for segment #76 (869+00.000 to 870+20.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
870+20.000	898+00.000	Warning: for segment #77 (870+20.000 to 898+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
870+20.000	898+00.000	Warning: for segment #77 (870+20.000 to 898+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
898+00.000	906+70.000	Warning: for segment #78 (898+00.000 to 906+70.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
898+00.000	906+70.000	Warning: for segment #78 (898+00.000 to 906+70.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
906+70.000	907+80.000	Warning: for segment #79 (906+70.000 to 907+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
906+70.000	907+80.000	Warning: for segment #79 (906+70.000 to 907+80.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
907+80.000	914+00.000	Warning: for segment #80 (907+80.000 to 914+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
907+80.000	914+00.000	Warning: for segment #80 (907+80.000 to 914+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
914+00.000	914+30.000	Warning: for segment #81 (914+00.000 to 914+30.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
914+00.000	914+30.000	Warning: for segment #81 (914+00.000 to 914+30.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
914+30.000	914+40.000	Warning: for segment #82 (914+30.000 to 914+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
914+30.000	914+40.000	Warning: for segment #82 (914+30.000 to 914+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
914+40.000	915+40.000	Warning: for segment #83 (914+40.000 to 915+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
914+40.000	915+40.000	Warning: for segment #83 (914+40.000 to 915+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
915+40.000	921+00.000	Warning: for segment #84 (915+40.000 to 921+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
915+40.000	921+00.000	Warning: for segment #84 (915+40.000 to 921+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
921+00.000	921+90.000	Warning: for segment #85 (921+00.000 to 921+90.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
921+00.000	921+90.000	Warning: for segment #85 (921+00.000 to 921+90.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
921+90.000	923+00.000	Warning: for segment #86 (921+90.000 to 923+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
921+90.000	923+00.000	Warning: for segment #86 (921+90.000 to 923+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
923+00.000	941+70.000	Warning: for segment #87 (923+00.000 to 941+70.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
923+00.000	941+70.000	Warning: for segment #87 (923+00.000 to 941+70.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
941+70.000	948+00.000	Warning: for segment #88 (941+70.000 to 948+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
941+70.000	948+00.000	Warning: for segment #88 (941+70.000 to 948+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
948+00.000	948+50.000	Warning: for segment #89 (948+00.000 to 948+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
948+00.000	948+50.000	Warning: for segment #89 (948+00.000 to 948+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0

Start Location (Sta. ft)	End Location (Sta. ft)	Message
862+60.000	869+00.000	Warning: for segment #75 (862+60.000 to 869+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
862+60.000	869+00.000	Warning: for segment #75 (862+60.000 to 869+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
869+00.000	870+20.000	Warning: for segment #76 (869+00.000 to 870+20.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
869+00.000	870+20.000	Warning: for segment #76 (869+00.000 to 870+20.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
870+20.000	898+00.000	Warning: for segment #77 (870+20.000 to 898+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
870+20.000	898+00.000	Warning: for segment #77 (870+20.000 to 898+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
898+00.000	906+70.000	Warning: for segment #78 (898+00.000 to 906+70.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
898+00.000	906+70.000	Warning: for segment #78 (898+00.000 to 906+70.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
906+70.000	907+80.000	Warning: for segment #79 (906+70.000 to 907+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
906+70.000	907+80.000	Warning: for segment #79 (906+70.000 to 907+80.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
907+80.000	914+00.000	Warning: for segment #80 (907+80.000 to 914+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
907+80.000	914+00.000	Warning: for segment #80 (907+80.000 to 914+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
914+00.000	914+30.000	Warning: for segment #81 (914+00.000 to 914+30.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
914+00.000	914+30.000	Warning: for segment #81 (914+00.000 to 914+30.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
914+30.000	914+40.000	Warning: for segment #82 (914+30.000 to 914+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
914+30.000	914+40.000	Warning: for segment #82 (914+30.000 to 914+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
914+40.000	915+40.000	Warning: for segment #83 (914+40.000 to 915+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
914+40.000	915+40.000	Warning: for segment #83 (914+40.000 to 915+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
915+40.000	921+00.000	Warning: for segment #84 (915+40.000 to 921+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
915+40.000	921+00.000	Warning: for segment #84 (915+40.000 to 921+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
921+00.000	921+90.000	Warning: for segment #85 (921+00.000 to 921+90.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
921+00.000	921+90.000	Warning: for segment #85 (921+00.000 to 921+90.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
921+90.000	923+00.000	Warning: for segment #86 (921+90.000 to 923+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
921+90.000	923+00.000	Warning: for segment #86 (921+90.000 to 923+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
923+00.000	941+70.000	Warning: for segment #87 (923+00.000 to 941+70.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
923+00.000	941+70.000	Warning: for segment #87 (923+00.000 to 941+70.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
941+70.000	948+00.000	Warning: for segment #88 (941+70.000 to 948+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
941+70.000	948+00.000	Warning: for segment #88 (941+70.000 to 948+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
948+00.000	948+50.000	Warning: for segment #89 (948+00.000 to 948+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
948+00.000	948+50.000	Warning: for segment #89 (948+00.000 to 948+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0

### **Section 6 Evaluation**

**Section:** Section 6

**Evaluation Start Location:** 948+50.000 **Evaluation End Location:** 974+11.000

Area Type: Urban

Functional Class: Arterial

**Type of Alignment:** Undivided, Multilane **Model Category:** Urban/Suburban Arterial **Calibration Factor:** 4D=1.0; 4SG=1.0; 4U=1.0;

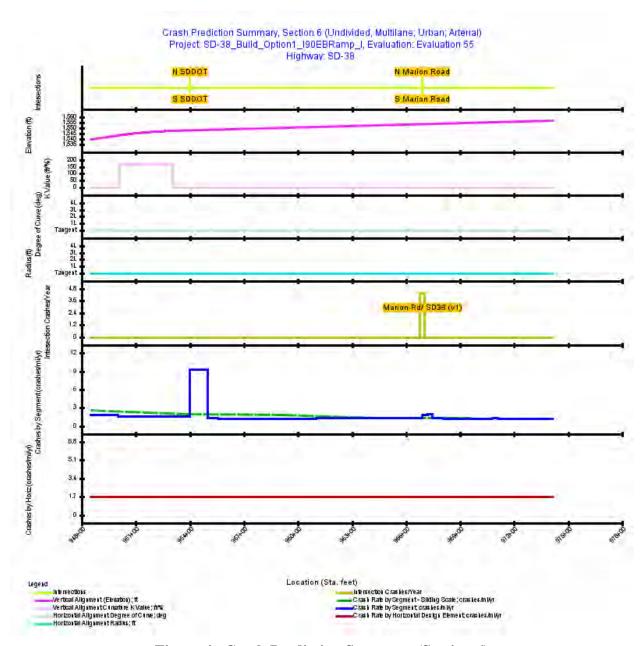


Figure 4. Crash Prediction Summary (Section 6)

**Table 50. Observed Crashes Used in the Evaluation (Section 6)** 

Year	Observed Crashes	Total Crashes Used	FI Crashes	FI no/C Crashes	PDO Crashes
2018	5	5	4	0	1
2019	1	1	0	0	1
2020	2	2	0	0	2
2021	2	2	2	0	0
2022	2	2	0	0	2
All Years	12 <sup>[1]</sup>	12	6	0	6

#### **Footnotes**

<sup>[1]</sup> Note: Observed crash data that does not comply with the associated CPM model requirements may not be used in EB processing.

**Table 51. Evaluation Highway - Homogeneous Segments (Section 6)** 

Se g. N o.	Туре	Start Locati on (Sta. ft)	End Locati on (Sta. ft)	Len gth (ft)		AADT	Number Major Commer icial Drivewa ys	Number Minor Commer icial Drivewa ys	Number Major Industial/I nstitutiona I	Number Minor Industial/I nstitutiona I	Number Major Resident ial Drivewa ys	Number Minor Resident ial Drivewa ys	Numbe r Other Drivew ays	Lighti ng	Automat ed Speed Enforce ment	Dens ity (fixe d objec ts/mi	Me dian Wid th (ft)	Туре	Effecti ve Media n Width (ft)	Speed Level	Numbe r Rail Highw ay Crossi ngs	Avera ge Shoul der Width (ft)	Aver age Lane Widt h (ft)
90	Urban/Suburban Arterial Segment Four-lane Undivided	948+5 0.000	950+0 0.000	150. 00			0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediat e/High	0	8.00	11.50
91	Urban/Suburban Arterial Segment Four-lane Undivided	950+0 0.000	954+0 0.000	400. 00	0.07 58	025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 5,45; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 036: 8,178; 2037: 8,237: 8,232: 0328: 8,289; 2039: 8,494; 2040: 8,600; 2041: 7,222; 2042: 8,844; 2043: 8,966: 2044: 9,088; 2045: 9,210; 2046: 9,332; 047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820		0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediat e/High	0	8.00	11.50
92	Urban/Suburban Arterial Segment Four-lane Undivided	954+0 0.000	955+0 0.000	100. 00	0.01 89	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,066; 2044: 9,982; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	2	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediat e/High	0	4.00	11.50
93	Urban/Suburban Arterial Segment Four-lane Divided	955+0 0.000	955+5 5.000	55.0 0	0.01 04	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,444; 2043: 8,666; 2044: 9,985; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	4.01	Non- Traversable Median	4.01	Intermediat e/High	0	8.00	11.50
94	Urban/Suburban Arterial Segment Four-lane Divided	955+5 5.000	958+2 8.000			2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,066; 2044: 9,085; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	10.0 2	Non- Traversable Median	10.02	Intermediat e/High	0	8.00	11.50
95	Urban/Suburban Arterial Segment Four-lane Divided	958+2 8.000	961+0 1.000	273. 00	0.05 17	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,066; 2044: 9,085; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	20.0	Non- Traversable Median	19.99	Intermediat e/High	0	8.00	11.50
96	Urban/Suburban Arterial Segment Four-lane Divided	961+0 1.000	962+0 0.000	99.0 0	0.01 87	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,066; 2044: 9,085; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	23.1	Traversable Median	23.12	Intermediat e/High	0	8.00	11.50
97	Urban/Suburban Arterial Segment Four-lane Divided	962+0 0.000	963+6 9.000		0.03 20		0	0	0	0	0	0	0	false	false	0.0		Traversable Median	30.13	Intermediat e/High	0	8.00	11.50
98	Urban/Suburban Arterial Segment Four-lane Divided	963+6 9.000	965+0 0.000	131. 00		2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,484; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0		Traversable Median	24.55	Intermediat e/High	0	8.00	11.50
99	Urban/Suburban Arterial Segment Four-lane Divided	965+0 0.000	966+3 8.000	138. 00	0.02 61	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,066; 2044: 9,082; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	7.54	Traversable Median	19.54	Intermediat e/High	0	4.00	11.50
10	Urban/Suburban Arterial Segment Four-lane Divided	966+3 8.000	966+7 0.000	32.0	0.00 61	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,282; 2038: 8,289; 2038: 8,404; 2040: 8,200; 2041:		0	0	0	0	0	0	false	false	0.0	4.38	Traversable Median	16.38	Intermediat e/High	0	4.00	11.50

Se g. N o.	Туре	Start Locati on (Sta. ft)	End Locat on (Sta. ft)	i Ler gth (ft)	th	AADT	Number Major Commer icial Drivewa ys	Number Minor Commer icial Drivewa ys	Number Major Industial/I nstitutiona I	Number Minor Industial/I nstitutiona I	Number Major Resident ial Drivewa ys	Number Minor Resident ial Drivewa ys	Numbe r Other Drivew ays	Lighti ng	Automat ed Speed Enforce ment	Dens ity (fixe d objec ts/mi	Me dian Wid th (ft)	Туре	Effecti ve Media n Width (ft)	Speed Level	Numbe r Rail Highw ay Crossi ngs	Avera ge Shoul der Width (ft)	Aver age Lane Widt h (ft)
10	Urban/Suburban Arterial Segment Four-lane Divided	966+7 0.000	966+9 1.000	9 21.0	.00 40	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,441; 2043: 8,066; 2044: 9,085; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	3.39	Traversable Median	15.39	Intermediat e/High	0	0.00	11.50
10 2	Urban/Suburban Arterial Segment Four-lane Undivided	966+9 1.000	967+1 4.000		.00 44	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,882; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	true	false	0.0	0.00	None	0.00	Intermediat e/High	0	0.00	11.50
10	Urban/Suburban Arterial Segment Four-lane Undivided	967+1 4.000	967+2 0.000		.00	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,882; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	true	false	0.0	0.00	None	0.00	Intermediat e/High	0	8.00	11.50
10 4	Urban/Suburban Arterial Segment Four-lane Undivided	967+2 0.000	967+4 5.000	4 25.0	.00 47	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,882; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediat e/High	0	8.00	11.50
10	Urban/Suburban Arterial Segment Four-lane Divided	967+4 5.000	968+0 6.000	61.0	.01 16	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,441; 2043: 8,066; 2044: 9,085; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	4.01	Non- Traversable Median	18.01	Intermediat e/High	0	8.00	11.50
10	Urban/Suburban Arterial Segment Four-lane Divided	968+0 6.000			.05 17	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,882; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	9.52	Non- Traversable Median	23.52	Intermediat e/High	0	8.00	11.50
10 7	Urban/Suburban Arterial Segment Four-lane Divided	970+7 9.000	971+0 9.000		.00 57	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2024: 8,441; 2043: 8,066; 2044: 9,085; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	14.5	Non- Traversable Median	14.53	Intermediat e/High	0	8.00	11.50
10 8	Urban/Suburban Arterial Segment Four-lane Divided	971+0 9.000			.05 72	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	20.0	Non- Traversable Median	20.01	Intermediat e/High	0	8.00	11.50

 Table 52. Crash Highway Highway - Homogeneous Segments (Section 6)

Se g. No	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Leng th (ft)	Leng th (mi)	AADT	Number Major Commerici al Driveways	Number Minor Commerici al Driveways	Number Major Industial/Inst itutional	Number Minor Industial/Inst itutional	Number Major Residential Driveways	Number Minor Residential Driveways	Number Other Drivewa ys	Lightin g	Automated Speed Enforceme nt	Densit y (fixed object s/mi)	Medi an Widt h (ft)	Type	Effective Median Width (ft)	Speed Level	Number Rail Highwa y Crossing s	Averag e Should er Width (ft)	Avera ge Lane Width (ft)
90	Urban/Suburban Arterial Segment Four-lane Undivided	948+50. 000	950+00. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediate/ High	0	8.00	11.50
91	Urban/Suburban Arterial Segment Four-lane Undivided	950+00. 000	954+00. 000	400.0 0		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediate/ High	0	8.00	11.50
92	Urban/Suburban Arterial Segment Four-lane Undivided	954+00. 000	955+00. 000			2018-2022: 4,900	0	0	2	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediate/ High	0	4.00	11.50
93	Urban/Suburban Arterial Segment Four-lane Divided	955+00. 000	955+55. 000	55.00		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	4.01	Non-Traversable Median	4.01	Intermediate/ High	0	8.00	11.50
94	Urban/Suburban Arterial Segment Four-lane Divided	955+55. 000	958+28. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	10.02	Non-Traversable Median	10.02	Intermediate/ High	0	8.00	11.50
95	Urban/Suburban Arterial Segment Four-lane Divided	958+28. 000	961+01. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	20.03	Non-Traversable Median	19.99	Intermediate/ High	0	8.00	11.50
96	Urban/Suburban Arterial Segment Four-lane Divided	961+01. 000	962+00. 000	99.00		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	23.12	Traversable Median	23.12	Intermediate/ High	0	8.00	11.50
97	Urban/Suburban Arterial Segment Four-lane Divided	962+00. 000	963+69. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	18.13	Traversable Median	30.13	Intermediate/ High	0	8.00	11.50
98	Urban/Suburban Arterial Segment Four-lane Divided	963+69. 000	965+00. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	12.55	Traversable Median	24.55	Intermediate/ High	0	8.00	11.50
99	Urban/Suburban Arterial Segment Four-lane Divided	965+00. 000	966+38. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	7.54	Traversable Median	19.54	Intermediate/ High	0	4.00	11.50
10 0	Urban/Suburban Arterial Segment Four-lane Divided	966+38. 000	966+70. 000	32.00		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	4.38	Traversable Median	16.38	Intermediate/ High	0	4.00	11.50
10 1	Urban/Suburban Arterial Segment Four-lane Divided	966+70. 000	966+91. 000	21.00		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	3.39	Traversable Median	15.39	Intermediate/ High	0	0.00	11.50
10 2	Urban/Suburban Arterial Segment Four-lane Undivided	966+91. 000	967+14. 000	23.00		2018-2022: 4,900	0	0	0	0	0	0	0	true	false	0.0	0.00	None	0.00	Intermediate/ High	0	0.00	11.50
10 3	Urban/Suburban Arterial Segment Four-lane Undivided	967+14. 000	967+20. 000			2018-2022: 4,900	0	0	0	0	0	0	0	true	false	0.0	0.00	None	0.00	Intermediate/ High	0	8.00	11.50
10 4	Urban/Suburban Arterial Segment Four-lane Undivided	967+20. 000	967+45. 000	25.00		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediate/ High	0	8.00	11.50
10 5	Urban/Suburban Arterial Segment Four-lane Divided	967+45. 000	968+06. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	4.01	Non-Traversable Median	18.01	Intermediate/ High	0	8.00	11.50
10 6	Urban/Suburban Arterial Segment Four-lane Divided	968+06. 000	970+79. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	9.52	Non-Traversable Median	23.52	Intermediate/ High	0	8.00	11.50
10 7	Urban/Suburban Arterial Segment Four-lane Divided	970+79. 000	971+09. 000	30.00		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	14.53	Non-Traversable Median	14.53	Intermediate/ High	0	8.00	11.50
10 8	Urban/Suburban Arterial Segment Four-lane Divided	971+09. 000	974+11. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	20.01	Non-Traversable Median	20.01	Intermediate/ High	0	8.00	11.50

# **Table 53. Evaluation Intersection (Section 6)**

nter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Le gs	Traffic Control	Approache s w/Left Turn Lanes	Approache s w/Right Turn Lanes	Right	Pedestrian Volume (crossings/ day)	d at	Light	l Near	Num ber of Bus Stops	Number of	Max Lanes Crosse d	
8	Marion Rd/ SD38 (v1)	Urban/Suburban Arterial Intersection Four-Legged Signalized	966+91. 000	6,255; 2030: 6,660; 2031: 7,065; 2032: 7,470; 2035: 7,875; 2034: 8,280; 2035: 8,685; 2036: 9,900; 2037: 9,495; 2038: 9,900; 2039: 10,305; 2040: 10,710; 2041: 11,861; 2042: 13,012; 2043: 14,163; 2044: 15,314; 2045: 16,465; 2046: 17,516; 2047: 18,767; 2038: 10,918; 2049: 10,918; 2059: 2059	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	4	Signalized	4	3	0	20	false	false	false	0	0	6	j

## **Table 54.** Crash History Intersection (Section 6)

Int N		Title	Type	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Approaches w/Left Turn Lanes	Approaches w/Right Turn Lanes	Approaches w/o Right Turn on Red	Pedestrian Volume (crossings/day	Lighted at Night	Red Light Camer a			Number of Alcohol Sales Establishments	Max Lanes Crossed
	8	Marion Rd/ SD38 (v1)	Urban/Suburban Arterial Intersection Four-Legged Signalized	966+91.000	2018-2022: 5.400	2018-2022: 4,900	4	Signalized	4	3	0	20	false	false	false	0	0	6

Table 55. Expected Highway Crash Rates and Frequencies Summary (Section 6)

First Year of Analysis	2025
Last Year of Analysis	2050
Evaluated Length (mi)	0.4850
Average Future Road AADT (vpd)	8,272
Expected Crashes	
Total Crashes	135.85
Fatal and Injury Crashes	46.51
Property-Damage-Only Crashes	89.34
Percent of Total Expected Crashes	
Percent Fatal and Injury Crashes (%)	34
Percent Property-Damage-Only Crashes (%)	66
Expected Crash Rate	
Crash Rate (crashes/mi/yr)	10.7725
FI Crash Rate (crashes/mi/yr)	3.6878
PDO Crash Rate (crashes/mi/yr)	7.0847
Expected Travel Crash Rate	
Total Travel (million veh-mi)	38.08
Travel Crash Rate (crashes/million veh-mi)	3.57
Travel FI Crash Rate (crashes/million veh-mi)	1.22
Travel PDO Crash Rate (crashes/million veh-mi)	2.35

Table 56. Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 6)

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi/ yr)	Expected Travel Crash Rate (crashes/mil lion veh-mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
90	948+50.000	950+00.000	0.0284	1.343	1.459	0.0517	0.0179	0.0337	0.0561	0.0181	0.0380	-0.0044	-0.0001	-0.0043	1.8184	0.60	
91	950+00.000	954+00.000	0.0758	3.172	3.889	0.1220	0.0437	0.0783	0.1496	0.0481	0.1014	-0.0276	-0.0044	-0.0232	1.6103	0.53	
92	954+00.000	955+00.000	0.0189	4.560	6.162	0.1754	0.0651	0.1103	0.2370	0.0817	0.1553	-0.0616	-0.0167	-0.0449	9.2608	3.07	
93	955+00.000	955+55.000	0.0104	0.373	0.382	0.0143	0.0039	0.0104	0.0147	0.0039	0.0108	-0.0004	-0.0000	-0.0003	1.3757	0.46	
94	955+55.000	958+28.000	0.0517	1.685	1.897	0.0648	0.0182	0.0466	0.0730	0.0195	0.0535	-0.0082	-0.0013	-0.0069	1.2532	0.42	
95	958+28.000	961+01.000	0.0517	1.685	1.897	0.0648	0.0182	0.0466	0.0730	0.0195	0.0535	-0.0082	-0.0013	-0.0069	1.2532	0.42	
96	961+01.000	962+00.000	0.0187	0.651	0.681	0.0251	0.0069	0.0182	0.0262	0.0070	0.0192	-0.0011	-0.0001	-0.0010	1.3360	0.44	
97	962+00.000	963+69.000	0.0320	1.069	1.151	0.0411	0.0114	0.0297	0.0443	0.0118	0.0325	-0.0032	-0.0004	-0.0027	1.2842	0.42	
98	963+69.000	965+00.000	0.0248	0.850	0.901	0.0327	0.0090	0.0237	0.0347	0.0092	0.0254	-0.0020	-0.0002	-0.0017	1.3176	0.44	
99	965+00.000	966+38.000	0.0261	0.893	0.949	0.0343	0.0095	0.0249	0.0365	0.0097	0.0268	-0.0022	-0.0003	-0.0019	1.3136	0.43	
100	966+38.000	966+70.000	0.0061	0.217	0.220	0.0083	0.0023	0.0061	0.0085	0.0023	0.0062	-0.0001	0.0000	-0.0001	1.3765	0.46	
101	966+70.000	966+91.000	0.0040	0.143	0.144	0.0055	0.0015	0.0040	0.0056	0.0015	0.0041	-0.0001	0.0000	-0.0001	1.3834	0.46	
Marion Rd/ SD38 (v1)	966+91.000			114.572	49.969	4.4066	1.5303	2.8763	1.9219	0.6310	1.2909	2.4847	0.8993	1.5854			0.65
102	966+91.000	967+14.000	0.0044	0.203	0.205	0.0078	0.0027	0.0051	0.0079	0.0025	0.0054	-0.0001	0.0001	-0.0002	1.7894	0.59	
103	967+14.000	967+20.000	0.0011	0.053	0.053	0.0021	0.0007	0.0014	0.0021	0.0007	0.0014	-0.0000	0.0000	-0.0000	1.8054	0.60	
104	967+20.000	967+45.000	0.0047	0.240	0.243	0.0092	0.0031	0.0061	0.0093	0.0030	0.0063	-0.0001	0.0001	-0.0003	1.9466	0.65	
105	967+45.000	968+06.000	0.0116	0.412	0.424	0.0159	0.0043	0.0115	0.0163	0.0043	0.0120	-0.0005	-0.0000	-0.0004	1.3720	0.45	
106	968+06.000	970+79.000	0.0517	1.685	1.897	0.0648	0.0182	0.0466	0.0730	0.0195	0.0535	-0.0082	-0.0013	-0.0069	1.2532	0.42	
107	970+79.000	971+09.000	0.0057	0.206	0.208	0.0079	0.0021	0.0058	0.0080	0.0021	0.0059	-0.0001	0.0000	-0.0001	1.3915	0.46	
108	971+09.000	974+11.000	0.0572	1.842	2.098	0.0708	0.0199	0.0509	0.0807	0.0215	0.0592	-0.0099	-0.0016	-0.0083	1.2387	0.41	
All Segments			0.4850	21.280	24.862	0.8185	0.2585	0.5600	0.9562	0.2860	0.6702	-0.1378	-0.0275	-0.1102	1.6874	0.56	
All Intersections				114.572	49.969	4.4066	1.5303	2.8763	1.9219	0.6310	1.2909	2.4847	0.8993	1.5854			0.65
Total			0.4850	135.852	74.831	5.2251	1.7887	3.4363	2.8781	0.9170	1.9611	2.3470	0.8718	1.4752	10.7725		

Table 57. Expected Crash Frequencies and Rates by Horizontal Design Element (Section 6)

	Title	Start Location (Sta. ft)	End Location (Sta. ft)		Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Total Crash Frequency	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	Total Crash Frequency	Frequency	PDO Crach		Expected Travel Crash Rate (crashes/mi llion veh- mi)
Т	angent	948+50.000	974+11.000	0.4850	21.280	24.862	0.8185	0.2585	0.5600	0.9562	0.2860	0.6702	-0.1378	-0.0275	-0.1102	1.6874	0.56

Table 58. Predicted Crash Frequencies by Year (Section 6)

Year	Total Crashes	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
2025	1.54	0.47	30.768	1.07	69.232
2026	1.62	0.50	30.791	1.12	69.209
2027	1.69	0.52	30.812	1.17	69.188
2028	1.77	0.55	30.830	1.22	69.170
2029	1.85	0.57	30.847	1.28	69.153
2030	1.93	0.60	30.902	1.33	69.098
2031	2.01	0.62	30.960	1.39	69.040
2032	2.10	0.65	31.018	1.45	68.982
2033	2.18	0.68	31.078	1.50	68.922
2034	2.27	0.71	31.139	1.56	68.862
2035	2.35	0.73	31.199	1.62	68.801
2036	2.44	0.76	31.260	1.68	68.740
2037	2.53	0.79	31.321	1.74	68.679
2038	2.61	0.82	31.381	1.79	68.618
2039	2.70	0.85	31.442	1.85	68.558
2040	2.79	0.88	31.502	1.91	68.498
2041	3.01	0.95	31.680	2.06	68.320
2042	3.24	1.03	31.854	2.21	68.146
2043	3.46	1.11	32.026	2.35	67.975
2044	3.69	1.19	32.192	2.50	67.808
2045	3.92	1.27	32.354	2.65	67.646
2046	4.15	1.35	32.511	2.80	67.489
2047	4.39	1.43	32.664	2.95	67.336
2048	4.62	1.52	32.811	3.11	67.189
2049	4.86	1.60	32.954	3.26	67.046
2050	5.10	1.69	33.093	3.41	66.907
Total	74.83	23.84	31.860	50.99	68.140
Average	2.88	0.92	31.860	1.96	68.140

Table 59. Expected Crash Frequencies by Year (Section 6)

Year	Total Crashes	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
2025	2.80	0.93	33.060	1.87	66.820
2026	2.94	0.97	33.085	1.96	66.798
2027	3.07	1.02	33.107	2.05	66.778
2028	3.21	1.06	33.127	2.15	66.760
2029	3.35	1.11	33.145	2.24	66.744
2030	3.50	1.16	33.205	2.34	66.691
2031	3.65	1.22	33.266	2.44	66.635
2032	3.81	1.27	33.329	2.54	66.579
2033	3.96	1.32	33.394	2.63	66.521
2034	4.12	1.38	33.458	2.74	66.463
2035	4.27	1.43	33.524	2.84	66.404
2036	4.43	1.49	33.589	2.94	66.346
2037	4.59	1.54	33.654	3.04	66.287
2038	4.75	1.60	33.719	3.14	66.228
2039	4.91	1.66	33.784	3.25	66.170
2040	5.07	1.72	33.849	3.35	66.112
2041	5.47	1.86	34.040	3.61	65.941
2042	5.88	2.01	34.228	3.86	65.772
2043	6.29	2.16	34.411	4.12	65.607
2044	6.70	2.32	34.590	4.38	65.446
2045	7.12	2.47	34.765	4.65	65.289
2046	7.54	2.63	34.934	4.91	65.138
2047	7.96	2.79	35.097	5.18	64.991
2048	8.39	2.96	35.256	5.44	64.848
2049	8.82	3.12	35.409	5.71	64.710
2050	9.26	3.29	35.558	5.98	64.577
Total	135.85	46.51	34.234	89.34	65.766
Average	5.22	1.79	34.234	3.44	65.766

Table 60. Comparing Predicted and Expected Crashes for the Evaluation Period (Section 6)

Scope	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
Predicted	74.83	23.84	31.860	50.99	68.140
Expected	135.85	46.51	34.234	89.34	65.766
Expected - Predicted	61.02	22.67		38.35	
Percent Difference	44.92	48.74		42.93	

Table 61. Expected Five Lane or Fewer Crash Type Distribution (Section 6)

Element Type	Crash Type	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)	Total Crashes	Percent Total (%)
Highway Segment	Collision with Animal	0.00	0.0	0.21	0.2	0.21	0.2
Highway Segment	Collision with Bicycle	0.09	0.1	0.00	0.0	0.09	0.1
Highway Segment	Collision with Fixed Object	0.57	0.4	3.57	2.6	4.14	3.0
Highway Segment	Collision with Other Object	0.03	0.0	0.09	0.1	0.11	0.1
Highway Segment	Other Single-vehicle Collision	0.43	0.3	0.53	0.4	0.97	0.7
Highway Segment	Collision with Pedestrian	0.34	0.3	0.00	0.0	0.34	0.3
Highway Segment	Total Single Vehicle Crashes	1.47	1.1	4.39	3.2	5.86	4.3
Highway Segment	Angle Collision	0.37	0.3	0.54	0.4	0.91	0.7
Highway Segment	Driveway-related Collision	1.32	1.0	2.27	1.7	3.58	2.6
Highway Segment	Head-on Collision	0.17	0.1	0.05	0.0	0.21	0.2
Highway Segment	Other Multi-vehicle Collision	0.20	0.1	0.58	0.4	0.79	0.6
Highway Segment	Rear-end Collision	2.79	2.1	4.80	3.5	7.59	5.6
Highway Segment	Sideswipe, Opposite Direction Collision	0.15	0.1	0.09	0.1	0.24	0.2
Highway Segment	Sideswipe, Same Direction Collision	0.26	0.2	1.83	1.3	2.09	1.5
Highway Segment	Total Multiple Vehicle Crashes	5.25	3.9	10.16	7.5	15.42	11.3
Highway Segment	Total Highway Segment Crashes	6.72	4.9	14.56	10.7	21.28	15.7
Intersection	Collision with Animal	0.00	0.0	0.00	0.0	0.01	0.0
Intersection	Collision with Bicycle	0.73	0.5	0.00	0.0	0.73	0.5
Intersection	Collision with Fixed Object	0.90	0.7	1.71	1.3	2.61	1.9
Intersection	Non-Collision	0.17	0.1	0.07	0.0	0.24	0.2
Intersection	Collision with Other Object	0.09	0.1	0.14	0.1	0.23	0.2
Intersection	Other Single-vehicle Collision	0.05	0.0	0.04	0.0	0.09	0.1
Intersection	Collision with Parked Vehicle	0.00	0.0	0.00	0.0	0.00	0.0
Intersection	Collision with Pedestrian	0.44	0.3	0.00	0.0	0.44	0.3
Intersection	Total Intersection Single Vehicle Crashes	2.38	1.7	1.97	1.4	4.34	3.2
Intersection	Angle Collision	12.98	9.6	17.77	13.1	30.75	22.6
Intersection	Head-on Collision	1.83	1.3	2.18	1.6	4.02	3.0
Intersection	Other Multi-vehicle Collision	2.06	1.5	15.36	11.3	17.42	12.8
Intersection	Rear-end Collision	16.84	12.4	35.17	25.9	52.01	38.3
Intersection	Sideswipe	3.70	2.7	2.33	1.7	6.03	4.4
Intersection	Total Intersection Multiple Vehicle Crashes	37.41	27.5	72.82	53.6	110.23	81.1
Intersection	Total Intersection Crashes	39.79	29.3	74.78	55.0	114.57	84.3
	Total Crashes	46.51	34.2	89.34	65.8	135.85	100.0

# Interactive Highway Safety Design Model

# **Crash Prediction Evaluation Report**

#### Disclaimer

The Interactive Highway Design Model (IHSDM) software is disseminated under the sponsorship of the Department of Transportation in the interest of information exchange. The United States Government assumes no liability for its content or use thereof. This document does not constitute a standard, specification, or regulation.

The United States Government does not endorse products or manufacturers. Trade and manufacturers' names may appear in this software and documentation only because they are considered essential to the objective of the software.

#### **Limited Warranty and Limitations of Remedies**

This software product is provided "as-is," without warranty of any kind-either expressed or implied (but not limited to the implied warranties of merchantability and fitness for a particular purpose). The FHWA do not warrant that the functions contained in the software will meet the end-user's requirements or that the operation of the software will be uninterrupted and error-free.

Under no circumstances will the FHWA be liable to the end-user for any damages or claimed lost profits, lost savings, or other incidental or consequential damages rising out of the use or inability to use the software (even if these organizations have been advised of the possibility of such damages), or for any claim by any other party.

#### **Notice**

The use of the IHSDM software is being done strictly on a voluntary basis. In exchange for provision of IHSDM, the user agrees that the Federal Highway Administration (FHWA), U.S. Department of Transportation and any other agency of the Federal Government shall not be responsible for any errors, damage or other liability that may result from any and all use of the software, including installation and testing of the software. The user further agrees to hold the FHWA and the Federal Government harmless from any resulting liability. The user agrees that this hold harmless provision shall flow to any person to whom or any entity to which the user provides the IHSDM software. It is the user's full responsibility to inform any person to whom or any entity to which it provides the IHSDM software of this hold harmless provision.

## **Table of Contents**

Report Overview	1
Disclaimer Regarding Crash Prediction Method	2
Section Types	3
Section 1 Evaluation	3
List of Tables	
Table Observed Crashes Used in the Evaluation (Section 1)	5
Table Evaluation Highway - Homogeneous Segments (Section 1)	6
Table Crash History Highway - Homogeneous Segments (Section 1)	17
Table Evaluation Intersection - Section 1	23
Table Crash History Intersection - Section 1	24
Table Expected Highway Crash Rates and Frequencies Summary (Section 1)	25
Table Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 1)	26
Table Expected Crash Frequencies and Rates by Horizontal Design Element (Section 1)	30
Table Predicted Crash Frequencies by Year (Section 1)	31
Table Expected Crash Frequencies by Year (Section 1)	32
Table Comparing Predicted and Expected Crashes for the Evaluation Period (Section 1)	33
Table Expected Crash Type Distribution (Section 1)	34
Table Evaluation Message	35
List of Figures	
Figure Crash Prediction Summary (Section 1)	4

## **Report Overview**

Report Generated: Jun 10, 2024 9:10 AM

Report Template: System: Single Page, 508 Compliant [System] (mlcpm5, Dec 5, 2019 2:16 PM)

Evaluation Date: Mon Jun 10 09:08:16 CDT 2024

**IHSDM Version:** v17.0.0 (Sep 22, 2021)

Crash Prediction Module: v12.0.0 (Sep 22, 2021)

User Name: naveen.mallipaddi

**Organization Name:** 

Phone: E-Mail:

Project Title: SD-38\_Build\_Option2\_I90EBRamp\_I

Project Comment: Created Mon Mar 27 16:47:43 CDT 2023

Project Unit System: U.S. Customary

**Highway Title: SD-38** 

Highway Comment: Created Mon Mar 27 16:49:47 CDT 2023

**Highway Version: 20** 

**Evaluation Title:** Evaluation 53

Evaluation Comment: Created Mon Jun 10 09:07:26 CDT 2024

**Minimum Location:** 171+44.000 **Maximum Location:** 580+10.000

Policy for Superelevation: AASHTO 2011 U.S. Customary

Calibration: HSM Configuration

Crash Distribution: HSM Configuration Model/CMF: HSM Configuration

First Year of Analysis: 2025 Last Year of Analysis: 2050

**Empirical-Bayes Analysis:** Site-Specific **Highway with Crash History:** SD-38

Highway with Crash History Comment: Created Mon Mar 27 16:49:47 CDT 2023

Highway with Crash History Version: 20 First Year of Observed Crashes: 2018 Last Year of Observed Crashes: 2022

## **Disclaimer Regarding Crash Prediction Method**

IMPORTANT NOTICE ABOUT COMPARING RESULTS FROM HIGHWAY SAFETY MANUAL FIRST EDITION (2010) MODELS TO RESULTS FROM NEW MODELS DEVELOPED UNDER NCHRP PROJECTS 17-70, 17-58, AND 17-68

Since the publication of the Highway Safety Manual - First Edition (HSM-1), in 2010 by the American Association of State Highway and Transportation Officials (AASHTO), multiple research efforts have been undertaken through the National Cooperative Highway Research Program (NCHRP) to develop safety performance models for road segment and intersection facility types that were not initially reflected in the HSM-1, in order to expand the breadth and depth of the HSM in the future.

The IHSDM Crash Prediction Module (CPM) is intended as a faithful implementation of HSM Part C predictive methods. As NCHRP projects to develop new predictive methods for the HSM are completed, FHWA works to incorporate the new methods into IHSDM, sometimes in advance of publication in the HSM. The following new crash predictive methods have been accepted by NCHRP project panels and incorporated into IHSDM, while pending AASHTO's approval for incorporation into a future edition of the HSM:

- Roundabouts: completed in 2018 under NCHRP Project 17-70, the new methods will provide improved outcomes for the safety analysis of roundabouts.
- 6+ lane and one-way urban/suburban arterials (including models for segments and intersections): completed under NCHRP Project 17-58.
- Intersection crash prediction methods for some intersection configurations and traffic control types not currently addressed in the HSM (e.g., all-way stop; rural 3-leg signalized; 3-leg stop-controlled where the major leg turns; urban 5-leg signalized; urban high-speed intersections): completed in 2021 under NCHRP Project 17-68.

However, in the absence of local calibration factors (see HSM-1 Part C, Appendix A for guidance on calibration of the predictive models), it is neither appropriate nor advisable to directly compare the results from new models (from NCHRP Projects 17-58, 17-68, and 17-70) to results from HSM-1 models, as the models were not calibrated to the same base state data sets, and consequently can produce unexpected results. If local calibration factors are available and applied to both new models and HSM-1 models, then it may be appropriate to directly compare the results. [Note: Work being performed under NCHRP Project 17-72 (Update of Crash Modification Factors for the Highway Safety Manual) is expected to re-calibrate many of the old (HSM-1) and new (e.g., NCHRP 17-70) models to data from a single (or small number of) states, that would allow results from all models to be directly compared.]

The models produced for NCHRP Project 17-70 have independent value in terms of informing the design of a roundabout and assessing the effects of different design characteristics on the expected safety performance of a roundabout.

The HSM-1 interim method previously included in IHSDM for evaluating roundabouts on urban/suburban arterials (i.e., evaluating an existing intersection and then applying a Crash Modification Factor for replacing the existing intersection with a roundabout) has been deactivated in IHSDM, to minimize any confusion with the new roundabout methodology.

## **Section Types**

### **Section 1 Evaluation**

Section: Section 1

**Evaluation Start Location:** 171+44.000 **Evaluation End Location:** 580+10.000

Area Type: Rural

Functional Class: Arterial

Type of Alignment: Undivided, Two Lane

Model Category: Rural, Two Lane

Calibration Factor: 2U=1.0; 3ST=1.0; 4ST=1.0;

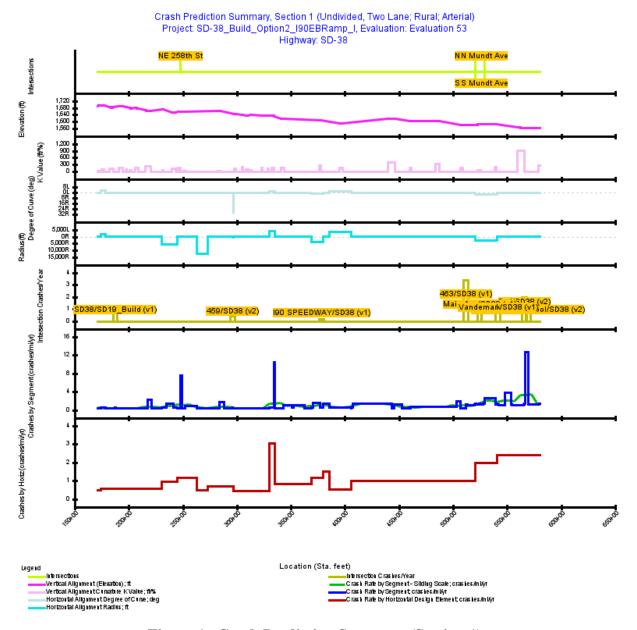


Figure 1. Crash Prediction Summary (Section 1)

**Table 1. Observed Crashes Used in the Evaluation (Section 1)** 

Year	Observed Crashes	Total Crashes Used	FI Crashes	FI no/C Crashes	PDO Crashes
2018	9	9	2	1	7
2019	5	4	1	0	3
2020	9	9	5	1	4
2021	8	7	3	1	4
2022	6	6	3	1	3
All Years	37 <sup>[1]</sup>	35	14	4	21

#### **Footnotes**

<sup>[1]</sup> Note: Observed crash data that does not comply with the associated CPM model requirements may not be used in EB processing.

 Table 2. Evaluation Highway - Homogeneous Segments (Section 1)

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	rd	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
1	Rural Two-Lane Segment Two-lane Undivided	171+44. 000	172+42. 000	98.00	0.018 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,362; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	4.25	6.2	3	false	0	false	false	false				
2	Rural Two-Lane Segment Two-lane Undivided	172+42. 000	174+52. 690	210.6 9	0.039 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.17	6.2	3	false	0	false	false	false				
3	Rural Two-Lane Segment Two-lane Undivided	174+52. 690	176+25. 000	172.3 1	0.032 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.17	6.2	3	false	0	false	false	false	2,074. 80	2.0	true	40
4	Rural Two-Lane Segment Two-lane Undivided	176+25. 000	178+85. 250	260.2 5		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.88	6.2	3	false	0	false	false	false	2,074. 80	2.0	true	40
5	Rural Two-Lane Segment Two-lane Undivided	178+85. 250	183+75. 370	490.1 2	0.092	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,000; 2041: 3,122; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.88	6.2	3	false	0	false	false	false				
6	Rural Two-Lane Segment Two-lane Undivided	183+75. 370	184+00. 000	24.63	0.004 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,000; 2041: 3,122; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
7	Rural Two-Lane Segment Two-lane Undivided	184+00. 000	184+45. 000	45.00	0.008	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
8	Rural Two-Lane Segment Two-lane Undivided	184+45. 000	185+20. 000	75.00	0.014	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
9	Rural Two-Lane Segment Two-lane Undivided	185+20. 000	186+60. 000	140.0 0		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
10	Rural Two-Lane Segment Two-lane Undivided	186+60. 000	187+20. 000	60.00	0.011	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
11	Rural Two-Lane Segment Two-lane Undivided	187+20. 000	187+60. 000	40.00	0.007 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	1.13	6.2	3	false	0	false	false	false				
12	Rural Two-Lane Segment Two-lane Undivided	187+60. 000	190+00. 000	240.0 0	0.045 5	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
13	Rural Two-Lane Segment Two-lane Undivided	190+00. 000	192+00. 000	200.0	0.037 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
14	Rural Two-Lane Segment Two-lane Undivided	192+00. 000	192+39. 270	39.27	0.007 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,362; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
15	Rural Two-Lane Segment Two-lane Undivided	192+39. 270	193+60. 000	120.7 3	0.022 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.94	6.2	3	false	0	false	false	false				
16	Rural Two-Lane Segment Two-lane Undivided	193+60. 000	197+65. 000	405.0 0	0.076 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 0.94	6.2	3	false	0	false	false	false				
17	Rural Two-Lane Segment Two-lane Undivided	197+65. 000	199+00. 000		0.025 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 1.94	6.2	3	false	0	false	false	false				
18	Rural Two-Lane Segment Two-lane Undivided	199+00. 000	201+63. 750		0.050 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 1.94	6.2	3	false	0	false	false	false				
19	Rural Two-Lane Segment Two-lane Undivided	201+63. 750	202+00. 000	36.25	0.006 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
20	Rural Two-Lane Segment Two-lane Undivided	202+00. 000	207+00. 000	500.0 0	0.094 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,362; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
21	Rural Two-Lane Segment Two-lane Undivided	207+00. 000	207+49. 760	49.76	0.009 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
22	Rural Two-Lane Segment Two-lane Undivided	207+49. 760	217+74. 250		0.194 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 1.70	6.2	3	false	0	false	false	false				
23	Rural Two-Lane Segment Two-lane Undivided	217+74. 250	221+00. 000	325.7 5	0.061 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
24	Rural Two-Lane Segment Two-lane Undivided	221+00. 000	226+00. 000	500.0	0.094 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
25	Rural Two-Lane Segment Two-lane Undivided	226+00. 000	230+66. 250	466.2 5	0.088	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
26	Rural Two-Lane Segment Two-lane Undivided	230+66. 250	231+39. 700	73.45	0.013 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.77	6.2	3	false	0	false	false	false	5,644. 64	2.0	true	70
27	Rural Two-Lane Segment Two-lane Undivided	231+39. 700	235+00. 000		0.068 2	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	2.00	6.2	3	false	0	false	false	false	5,644. 64	2.0	true	70
28	Rural Two-Lane Segment Two-lane Undivided	235+00. 000	241+61. 390	661.3 9	0.125	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,096; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	2.00	6.2	3	false	0	false	false	false	5,644. 64	2.0	true	70
29	Rural Two-Lane Segment Two-lane Undivided	241+61. 390	242+00. 000	38.61	0.007	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	1.16	6.2	3	false	0	false	false	false	5,644. 64	2.0	true	70

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
30	Rural Two-Lane Segment Two-lane Undivided	242+00. 000	245+14. 280	314.2 8		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,096; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.16	6.2	3	false	0	false	false	false	5,644. 64	2.0	true	70
31	Rural Two-Lane Segment Two-lane Undivided	245+14. 280	246+55. 100	140.8 2		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.16	6.2	3	false	0	false	false	false				
32	Rural Two-Lane Segment Two-lane Undivided	246+55. 100	248+00. 000	144.9 0		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2044: 3,046; 2044: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.42	6.2	3	false	0	false	false	false				
33	Rural Two-Lane Segment Two-lane Undivided	248+00. 000	249+00. 000	100.0 0		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	0.42	6.2	3	false	0	false	false	false				
34	Rural Two-Lane Segment Two-lane Undivided	249+00. 000	251+21. 980	221.9 8		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.42	6.2	3	false	0	false	false	false				
35	Rural Two-Lane Segment Two-lane Undivided	251+21. 980	252+40. 240	118.2 6	0.022 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	3.43	6.2	3	false	0	false	false	false				
36	Rural Two-Lane Segment Two-lane Undivided	252+40. 240	263+22. 600	1,082. 36		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
37	Rural Two-Lane Segment Two-lane Undivided	263+22. 600	272+66. 740	944.1 4		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	false	false	false	12,237 .00	2.0	true	70
38	Rural Two-Lane Segment Two-lane Undivided	272+66. 740	280+00. 000	733.2 6		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
39	Rural Two-Lane Segment Two-lane Undivided	280+00. 000	283+15. 050	315.0 5		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
40	Rural Two-Lane Segment Two-lane Undivided	283+15. 050	284+08. 540	93.49	0.017 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	4.47	6.2	3	false	0	false	false	false				
41	Rural Two-Lane Segment Two-lane Undivided	284+08. 540	288+50. 000	441.4 6		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.47	6.2	3	false	0	false	false	false				
42	Rural Two-Lane Segment Two-lane Undivided	288+50. 000	289+00. 000	50.00		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.47	6.2	3	false	0	false	false	false				
43	Rural Two-Lane Segment Two-lane Undivided	289+00. 000	295+90. 000	690.0 0		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,096; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.47	6.2	3	false	0	false	false	false				
44	Rural Two-Lane Segment Two-lane Undivided	295+90. 000	296+00. 000	10.00		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	1.47	6.2	3	false	0	false	false	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
45	Rural Two-Lane Segment Two-lane Undivided	296+00. 000	296+10. 000	10.00	0.001 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,334; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	1.47	6.2	3	false	0	false	false	false				
46	Rural Two-Lane Segment Two-lane Undivided	296+10. 000	296+96. 520	86.52	0.016 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.47	6.2	3	false	0	false	false	false				
47	Rural Two-Lane Segment Two-lane Undivided	296+96. 520	298+33. 660	137.1 4	0.026 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.47	6.2	3	false	0	false	false	false				
48	Rural Two-Lane Segment Two-lane Undivided	298+33. 660	303+50. 000	516.3 4	0.097 8	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.61	6.2	3	false	0	false	false	false				
49	Rural Two-Lane Segment Two-lane Undivided	303+50. 000	304+50. 000		0.018 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 2,324; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.61	6.2	3	false	0	false	false	false				
50	Rural Two-Lane Segment Two-lane Undivided	304+50. 000	305+02. 039	52.04	0.009 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.61	6.2	3	false	0	false	false	false				
51	Rural Two-Lane Segment Two-lane Undivided	305+02. 039	309+35. 490	433.4 5	0.082 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.15	6.2	3	false	0	false	false	false				
52	Rural Two-Lane Segment Two-lane Undivided	309+35. 490	311+70. 000	234.5 1	0.044 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.24	6.2	3	false	0	false	false	false				
53	Rural Two-Lane Segment Two-lane Undivided	311+70. 000	313+25. 000	155.0 0	0.029 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.24	6.2	3	false	0	false	false	false				
54	Rural Two-Lane Segment Two-lane Undivided	313+25. 000	323+00. 000		0.184 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.33	6.2	3	false	0	false	false	false				
55	Rural Two-Lane Segment Two-lane Undivided	323+00. 000	323+26. 980	26.98	0.005 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.33	6.2	3	false	0	false	false	false				
56	Rural Two-Lane Segment Two-lane Undivided	323+26. 980	328+89. 230	562.2 5	0.106 5	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.26	6.2	3	false	0	false	false	false				
57	Rural Two-Lane Segment Two-lane Undivided	328+89. 230	329+81. 740	92.51	0.017 5	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,343; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.52	6.2	3	false	0	false	false	false				
58	Rural Two-Lane Segment Two-lane Undivided	329+81. 740	333+24. 920	343.1 8	0.065 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,343; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.52	6.2	3	false	0	false	false	false	4,010. 13	2.0	true	70
59	Rural Two-Lane Segment Two-lane Undivided	333+24. 920	334+00. 000	75.08	0.014 2	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,343; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	2.17	6.2	3	false	0	false	false	false	4,010. 13	2.0	true	70

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
60	Rural Two-Lane Segment Two-lane Undivided	334+00. 000	335+39. 960	139.9 6	0.026 5	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	2.17	6.2	3	false	O	) false	false	false	4,010. 13	2.0	true	e 70
61	Rural Two-Lane Segment Two-lane Undivided	335+39. 960	342+39. 000	699.0 4	0.132 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	2.17	6.2	3	false	C	false	false	false				
62	Rural Two-Lane Segment Two-lane Undivided	342+39. 000	343+00. 000	61.00	0.011 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.24	6.2	3	false	C	false	false	false				
63	Rural Two-Lane Segment Two-lane Undivided	343+00. 000	351+20. 000	820.0 0	0.155	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.24	6.2	3	false	C	false	false	false				
64	Rural Two-Lane Segment Two-lane Undivided	351+20. 000	352+00. 000	80.00	0.015	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	0.24	6.2	3	false	C	) false	false	false				
65	Rural Two-Lane Segment Two-lane Undivided	352+00. 000	352+20. 000	20.00	0.003	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	0.00	0.00	0.24	6.2	3	false	C	false	false	false				
66	Rural Two-Lane Segment Two-lane Undivided	352+20. 000	362+50. 000	1,030. 00	0.195	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	0.24	6.2	3	false	O	false	false	false				
67	Rural Two-Lane Segment Two-lane Undivided	362+50. 000	369+14. 990	664.9 9	0.125	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	0.24	6.2	3	false	O	false	false	false				
68	Rural Two-Lane Segment Two-lane Undivided	369+14. 990	370+30. 000	115.0 1	0.021	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	0.24	6.2	3	false	O	false	false	false	4,023. 18	2.0	true	e 70
69	Rural Two-Lane Segment Two-lane Undivided	370+30. 000	370+60. 000	30.00	0.005 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	0.24	6.2	3	false	O	false	false	false	4,023. 18	2.0	true	e 70
70	Rural Two-Lane Segment Two-lane Undivided	370+60. 000	376+83. 610	623.6 1	0.118	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	0.24	6.2	3	false	O	false	false	false	4,023. 18	2.0	true	e 70
71	Rural Two-Lane Segment Two-lane Undivided	376+83. 610	378+00. 000	116.3 9		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	1.04	6.2	3	false	O	false	false	false	4,023. 18	2.0	true	e 70
72	Rural Two-Lane Segment Two-lane Undivided	378+00. 000	378+40. 000	40.00	0.007	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	8.00	1.04	6.2	3	false	0	) false	false	false	4,023. 18	2.0	true	е 70
73	Rural Two-Lane Segment Two-lane Undivided	378+40. 000	378+60. 000	20.00		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	8.00	1.04	6.2	3	false	0	) false	false	false	4,023. 18	2.0	true	е 70
74	Rural Two-Lane Segment Two-lane Undivided	378+60. 000	379+00. 000	40.00	0.007	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,12; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,252; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	0.00	8.00	1.04	6.2	3	false	C	) false	false	false	4,023. 18	2.0	true	e 70

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	y Density	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
75	Rural Two-Lane Segment Two-lane Undivided	379+00. 000	379+62. 690	62.69	0.011 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	4,023. 18	2.0	true	e 70
76	Rural Two-Lane Segment Two-lane Undivided	379+62. 690	385+22. 970	560.2 8	0.106 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false				
77	Rural Two-Lane Segment Two-lane Undivided	385+22. 970	386+60. 000	137.0 3	0.026 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
78	Rural Two-Lane Segment Two-lane Undivided	386+60. 000	389+50. 000	290.0 0	0.054 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
79	Rural Two-Lane Segment Two-lane Undivided	389+50. 000	394+00. 000	450.0 0	0.085 2	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
80	Rural Two-Lane Segment Two-lane Undivided	394+00. 000	396+46. 150	246.1 5	0.046 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
81	Rural Two-Lane Segment Two-lane Undivided	396+46. 150	397+00. 000	53.85	0.010 2	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
82	Rural Two-Lane Segment Two-lane Undivided	397+00. 000	399+00. 000	200.0 0	0.037 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
83	Rural Two-Lane Segment Two-lane Undivided	399+00. 000	405+75. 410	675.4 1	0.127 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	70
84	Rural Two-Lane Segment Two-lane Undivided	405+75. 410	406+00. 000	24.59	0.004 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.72	6.2	3	false	0	false	false	false				
85	Rural Two-Lane Segment Two-lane Undivided	406+00. 000	407+00. 000	100.0	0.018 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	8.00	0.72	6.2	3	false	0	false	false	false				
86	Rural Two-Lane Segment Two-lane Undivided	407+00. 000	443+25. 000	3,625. 00	0.686 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.72	6.2	3	false	0	false	false	false				
87	Rural Two-Lane Segment Two-lane Undivided	443+25. 000	445+50. 000	225.0 0	0.042 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 0.96	6.2	3	false	0	false	false	false				
88	Rural Two-Lane Segment Two-lane Undivided	445+50. 000	452+50. 000	700.0 0	0.132 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,096; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 0.96	6.2	3	false	0	false	false	false				
89	Rural Two-Lane Segment Two-lane Undivided	452+50. 000	459+00. 000	650.0 0	0.123 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
90	Rural Two-Lane Segment Two-lane Undivided	459+00. 000	460+00. 000	100.0	0.018 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	0.00	0.96	6.2	3	false	0	false	false	false				
91	Rural Two-Lane Segment Two-lane Undivided	460+00. 000	460+58. 580	58.58	0.011 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,362; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
92	Rural Two-Lane Segment Two-lane Undivided	460+58. 580	485+61. 230	2,502. 65		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.01	6.2	3	false	0	false	false	false				
93	Rural Two-Lane Segment Two-lane Undivided	485+61. 230	503+00. 000	1,738. 77	0.329 3	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.07	6.2	3	false	0	false	false	false				
94	Rural Two-Lane Segment Two-lane Undivided	503+00. 000	507+00. 000		0.075 8	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.07	6.2	3	false	0	false	false	false				
95	Rural Two-Lane Segment Two-lane Undivided	507+00. 000	508+00. 000		0.018 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.07	6.2	3	false	0	false	true	false				
96	Rural Two-Lane Segment Two-lane Undivided	508+00. 000	508+08. 240	8.24	0.001 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.07	6.2	3	false	0	false	true	false				
97	Rural Two-Lane Segment Two-lane Undivided	508+08. 240	510+30. 000	221.7 6	0.042 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.21	6.2	3	false	0	false	true	false				
98	Rural Two-Lane Segment Two-lane Undivided	510+30. 000	512+00. 000	170.0 0	0.032	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.21	6.2	3	false	0	false	false	false				
99	Rural Two-Lane Segment Two-lane Undivided	512+00. 000	513+00. 000	100.0 0	0.018 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.21	6.2	3	false	0	false	true	false				
100	Rural Two-Lane Segment Two-lane Undivided	513+00. 000	515+00. 000		0.037 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.21	6.2	3	false	0	false	true	false				
101	Rural Two-Lane Segment Two-lane Undivided	515+00. 000	520+00. 000	500.0 0	0.094 7	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.21	6.2	3	false	0	true	true	false				
102	Rural Two-Lane Segment Two-lane Undivided	520+00. 000	520+49. 150	49.15	0.009	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.21	6.2	3	false	0	false	true	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
103	Rural Two-Lane Segment Two-lane Undivided	520+49. 150	521+00. 000	50.85	0.009 6	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0	0.00	0.00	0.21	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
104	Rural Two-Lane Segment Two-lane Undivided	521+00. 000	523+38. 600	238.6 0	0.045 2	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.21	6.2	3	false	0	true	true	false	2,458. 49	2.0	true	e 45
105	Rural Two-Lane Segment Two-lane Undivided	523+38. 600	524+00. 000	61.40	0.011 6	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	1.90	6.2	3	false	0	true	true	false	2,458. 49	2.0	true	e 45
106	Rural Two-Lane Segment Two-lane Undivided	524+00. 000			0.018 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	1.90	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
107	Rural Two-Lane Segment Two-lane Undivided	525+00. 000	525+18. 580	18.58	0.003 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	1.90	6.2	3	false	0	true	true	false	2,458. 49	2.0	true	e 45
108	Rural Two-Lane Segment Two-lane Undivided	525+18. 580	528+00. 000		0.053 3	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	true	true	false	2,458. 49	2.0	true	е 45
109	Rural Two-Lane Segment Two-lane Undivided	528+00. 000	529+00. 000	100.0 0	0.018 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.02	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
110	Rural Two-Lane Segment Two-lane Undivided	529+00. 000	539+00. 000	1,000. 00	0.189 4	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	true	true	false	2,458. 49	2.0	true	e 45
111	Rural Two-Lane Segment Two-lane Undivided	539+00. 000	539+50. 000	50.00	0.009 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
112	Rural Two-Lane Segment Two-lane Undivided	539+50. 000	540+00. 000	50.00	0.009 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
113	Rural Two-Lane Segment Two-lane Undivided	540+00. 000	540+50. 000	50.00	0.009 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
114	Rural Two-Lane Segment Two-lane Undivided	540+50. 000	540+74. 370	24.37	0.004 6	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0 0	0.00	0.00	0.96	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	rd	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
115	Rural Two-Lane Segment Two-lane Undivided	540+74. 370	541+00. 000	25.63	0.004 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0	0.00	0.00	0.96	6.2	3	false	0	false	true	false				
116	Rural Two-Lane Segment Two-lane Undivided	541+00. 000	541+50. 000	50.00	0.009 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.96	6.2	3	false	0	false	true	false				
117	Rural Two-Lane Segment Two-lane Undivided	541+50. 000	541+70. 000	20.00	0.003 8	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.96	6.2	3	false	0	false	true	false				
118	Rural Two-Lane Segment Two-lane Undivided	541+70. 000	542+30. 000	60.00	0.011 4	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.96	6.2	3	false	0	false	true	false				
119	Rural Two-Lane Segment Two-lane Undivided	542+30. 000	542+64. 000	34.00	0.006 4	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	true	false				
120	Rural Two-Lane Segment Two-lane Undivided	542+64. 000	543+34. 000	70.00	0.013 3	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	true	false				
121	Rural Two-Lane Segment Two-lane Undivided	543+34. 000	544+00. 000	66.00	0.012 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	true	false				
122	Rural Two-Lane Segment Two-lane Undivided	544+00. 000	545+00. 000	100.0	0.018 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
123	Rural Two-Lane Segment Two-lane Undivided	545+00. 000	548+23. 000	323.0 0	0.061 2	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
124	Rural Two-Lane Segment Two-lane Undivided	548+23. 000	553+70. 000	547.0 0	0.103 6	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
125	Rural Two-Lane Segment Two-lane Undivided	553+70. 000	554+00. 000	30.00	0.005 7	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0	0.00	0.00	0.96	6.2	3	false	0	false	false	false				
126	Rural Two-Lane Segment Two-lane Undivided	554+00. 000	554+20. 000	20.00	0.003	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0	0.00	0.00	0.96	6.2	3	false	0	false	false	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
127	Rural Two-Lane Segment Two-lane Undivided	554+20. 000	560+00. 000	580.0 0	0.109	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
128	Rural Two-Lane Segment Two-lane Undivided	560+00. 000	562+58. 560	258.5 6	0.049	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
129	Rural Two-Lane Segment Two-lane Undivided	562+58. 560	564+00. 000	141.4 4	0.026	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				
130	Rural Two-Lane Segment Two-lane Undivided	564+00. 000	565+00. 000	100.0 0	0.018	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,833; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				
131	Rural Two-Lane Segment Two-lane Undivided	565+00. 000	565+77. 000	77.00	0.014 6	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				
132	Rural Two-Lane Segment Two-lane Undivided	565+77. 000	566+10. 000	33.00	0.006	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	0.00	0.00	0.20	6.2	3	false	0	false	false	false				
133	Rural Two-Lane Segment Two-lane Undivided	566+10. 000	566+50. 000	40.00	0.007 6	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,771; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	0.00	0.00	0.20	6.2	3	false	0	false	false	false				
134	Rural Two-Lane Segment Two-lane Undivided	566+50. 000	569+37. 000	287.0 0	0.054 4	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				
135	Rural Two-Lane Segment Two-lane Undivided	569+37. 000	569+70. 000	33.00	0.006	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,3394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	8.00	0.00	0.20	6.2	3	false	0	false	false	false				
136	Rural Two-Lane Segment Two-lane Undivided	569+70. 000	570+00. 000	30.00	0.005	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,3394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				
137	Rural Two-Lane Segment Two-lane Undivided	570+00. 000	575+00. 000	500.0 0	0.094	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,3394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	true	false	false				
138	Rural Two-Lane Segment Two-lane Undivided	575+00. 000	579+50. 000	450.0 0	0.085	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,3394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				

Se N	g o.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)		AADT	Lan e Wid th		Left Shoulde r Width (ft)	r Width	Gra	Drivewa y Density (drivewa ys/mi)	rd	Rumble	Passin	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Superelevation (%)	Desig n Spee d (mph
13		ural Two-Lane Segment Two-lane Undivided	579+50. 000	579+70. 000	20.00	0.003 8	2025; 7,087; 2026; 8,007; 2027; 8,928; 2028; 9,849; 2029; 10,770; 2030; 10,937; 2031; 11,104; 2032; 11,271; 2033; 11,439; 2034; 11,606; 2035; 11,773; 2036; 11,940; 2037; 12,108; 2038; 12,275; 2039; 12,442; 2040; 12,610; 2041; 12,806; 2042; 13,002; 2043; 13,198; 2044; 13,394; 2045; 13,590; 2046; 13,786; 2047; 13,982; 2048; 14,178; 2049; 14,374; 2050; 14,570	12.0	12.0	0.00	0.00	0.20	6.2	3	false	0	false	false	false		
14		ural Two-Lane Segment Two-lane Undivided	579+70. 000	580+10. 000		0.007 6	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0	12.0	0.00	0.00	0.20	6.2	3	false	0	false	false	false		

**Table 3. Crash History Highway - Homogeneous Segments (Section 1)** 

g		Start	End	T4h	T41			Right	Left	Right	C1	Driveway	п	Centerline	D'.	TWL		Automated	D. P			Design
Seg. No.	Туре	Location (Sta. ft)	Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Lane Widt h (ft)	Lane Widt h (ft)	Shoulder Width (ft)	Shoulder Width (ft)	Grad e (%)	Density (driveways/ mi)	Hazard Rating	Rumble Strip	Passing Lanes	T Lane	Lighting	Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Speed (mph)
1	Rural Two-Lane Segment Two-lane Undivided	171+44.00 0	172+42.00 0	98.00	0.0186	2018-2022: 2,085	12.00	12.00	8.00	8.00	4.25	6.2	3	false	0	false	false	false				
2	Rural Two-Lane Segment Two-lane Undivided	172+42.00 0	174+52.69 0	210.69	0.0399	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.17	6.2	3	false	0	false	false	false				
3	Rural Two-Lane Segment Two-lane Undivided	174+52.69 0	176+25.00 0	172.31	0.0326	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.17	6.2	3	false	0	false	false	false	2,074.80	2.0	true	40
4	Rural Two-Lane Segment Two-lane Undivided	176+25.00 0	178+85.25 0	260.25	0.0493	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.88	6.2	3	false	0	false	false	false	2,074.80	2.0	true	40
5	Rural Two-Lane Segment Two-lane Undivided	178+85.25 0	183+75.37 0	490.12	0.0928	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.88	6.2	3	false	0	false	false	false				
6	Rural Two-Lane Segment Two-lane Undivided	183+75.37 0	184+00.00 0	24.63	0.0047	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
7	Rural Two-Lane Segment Two-lane Undivided	184+00.00 0	184+45.00 0	45.00	0.0085	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
8	Rural Two-Lane Segment Two-lane Undivided	184+45.00 0	185+20.00 0	75.00	0.0142	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
9	Rural Two-Lane Segment Two-lane Undivided	185+20.00 0	186+60.00 0	140.00	0.0265	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
10	Rural Two-Lane Segment Two-lane Undivided	186+60.00 0	187+20.00 0	60.00	0.0114	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
11	Rural Two-Lane Segment Two-lane Undivided	187+20.00 0	187+60.00 0	40.00	0.0076	2018-2022: 2,085	12.00	12.00	0.00	0.00	1.13	6.2	3	false	0	false	false	false				
12	Rural Two-Lane Segment Two-lane Undivided	187+60.00 0	190+00.00 0	240.00	0.0455	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
13	Rural Two-Lane Segment Two-lane Undivided	190+00.00 0	192+00.00 0	200.00	0.0379	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
14	Rural Two-Lane Segment Two-lane Undivided	192+00.00 0	192+39.27 0	39.27	0.0074	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
15	Rural Two-Lane Segment Two-lane Undivided	192+39.27 0	193+60.00 0	120.73	0.0229	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.94	6.2	3	false	0	false	false	false				
16	Rural Two-Lane Segment Two-lane Undivided	193+60.00 0	197+65.00 0	405.00	0.0767	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.94	6.2	3	false	0	false	false	false				
17	Rural Two-Lane Segment Two-lane Undivided	197+65.00 0	199+00.00	135.00	0.0256	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.94	6.2	3	false	0	false	false	false				
18	Rural Two-Lane Segment Two-lane Undivided	199+00.00 0	201+63.75	263.75	0.0500	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.94	6.2	3	false	0	false	false	false				
19	Rural Two-Lane Segment Two-lane Undivided	201+63.75 0	202+00.00 0	36.25	0.0069	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
20	Rural Two-Lane Segment Two-lane Undivided	202+00.00 0	207+00.00	500.00	0.0947	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
21	Rural Two-Lane Segment Two-lane Undivided	207+00.00	207+49.76 0	49.76	0.0094	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
22	Rural Two-Lane Segment Two-lane Undivided	207+49.76 0	217+74.25 0	1,024.49	0.1940	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.70	6.2	3	false	0	false	false	false				
23	Rural Two-Lane Segment Two-lane Undivided	217+74.25 0	221+00.00	325.75	0.0617	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
24	Rural Two-Lane Segment Two-lane Undivided	221+00.00 0	226+00.00	500.00	0.0947	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
25	Rural Two-Lane Segment Two-lane Undivided	226+00.00	230+66.25	466.25	0.0883	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
			•	-	-	•					-		-		•							

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazard Rating	Centerline Rumble Strip	Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
26	Rural Two-Lane Segment Two-lane Undivided	230+66.25 0	231+39.70 0	73.45	0.0139	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.77	6.2	3	false	0	false	false	false	5,644.64	2.0	true	70
27	Rural Two-Lane Segment Two-lane Undivided	231+39.70 0	235+00.00 0	360.30	0.0682	2018-2022: 2,085	12.00	12.00	8.00	8.00	-2.00	6.2	3	false	0	false	false	false	5,644.64	2.0	true	70
28	Rural Two-Lane Segment Two-lane Undivided	235+00.00 0	241+61.39 0	661.39	0.1253	2018-2022: 2,085	12.00	12.00	8.00	8.00	-2.00	6.2	3	false	0	false	false	false	5,644.64	2.0	true	70
29	Rural Two-Lane Segment Two-lane Undivided	241+61.39 0	242+00.00 0	38.61	0.0073	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.16	6.2	3	false	0	false	false	false	5,644.64	2.0	true	70
30	Rural Two-Lane Segment Two-lane Undivided	242+00.00 0	245+14.28 0	314.28	0.0595	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.16	6.2	3	false	0	false	false	false	5,644.64	2.0	true	70
31	Rural Two-Lane Segment Two-lane Undivided	245+14.28 0	246+55.10 0	140.82	0.0267	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.16	6.2	3	false	0	false	false	false				
32	Rural Two-Lane Segment Two-lane Undivided	246+55.10 0	248+00.00 0	144.90	0.0274	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.42	6.2	3	false	0	false	false	false				
33	Rural Two-Lane Segment Two-lane Undivided	248+00.00 0	249+00.00 0	100.00	0.0189	2018-2022: 2,085	12.00	12.00	0.00	0.00	-0.42	6.2	3	false	0	false	false	false				
34	Rural Two-Lane Segment Two-lane Undivided	249+00.00 0	251+21.98 0	221.98	0.0420	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.42	6.2	3	false	0	false	false	false				
35	Rural Two-Lane Segment Two-lane Undivided	251+21.98 0	252+40.24 0	118.26	0.0224	2018-2022: 2,085	12.00	12.00	8.00	8.00	3.43	6.2	3	false	0	false	false	false				
36	Rural Two-Lane Segment Two-lane Undivided	252+40.24 0	263+22.60 0	1,082.36	0.2050	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
37	Rural Two-Lane Segment Two-lane Undivided	263+22.60 0	272+66.74 0	944.14	0.1788	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.02	6.2	3	false	0	false	false	false	12,237.00	2.0	true	70
38	Rural Two-Lane Segment Two-lane Undivided	272+66.74 0	280+00.00 0	733.26	0.1389	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
39	Rural Two-Lane Segment Two-lane Undivided	280+00.00 0	283+15.05 0	315.05	0.0597	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
40	Rural Two-Lane Segment Two-lane Undivided	283+15.05 0	284+08.54 0	93.49	0.0177	2018-2022: 2,085	12.00	12.00	8.00	8.00	4.47	6.2	3	false	0	false	false	false				
41	Rural Two-Lane Segment Two-lane Undivided	284+08.54 0	288+50.00 0	441.46	0.0836	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.47	6.2	3	false	0	false	false	false				
42	Rural Two-Lane Segment Two-lane Undivided	288+50.00 0	289+00.00 0	50.00	0.0095	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.47	6.2	3	false	0	false	false	false				
43	Rural Two-Lane Segment Two-lane Undivided	289+00.00 0	295+90.00 0	690.00	0.1307	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.47	6.2	3	false	0	false	false	false				
44	Rural Two-Lane Segment Two-lane Undivided	295+90.00 0	296+00.00 0	10.00	0.0019	2018-2022: 2,085	12.00	12.00	0.00	0.00	-1.47	6.2	3	false	0	false	false	false				
45	Rural Two-Lane Segment Two-lane Undivided	296+00.00 0	296+10.00 0	10.00	0.0019	2018-2022: 2,085	12.00	12.00	0.00	0.00	-1.47	6.2	3	false	0	false	false	false				
46	Rural Two-Lane Segment Two-lane Undivided	296+10.00 0	296+96.52 0	86.52	0.0164	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.47	6.2	3	false	0	false	false	false				
47	Rural Two-Lane Segment Two-lane Undivided	296+96.52 0	298+33.66 0	137.14	0.0260	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.47	6.2	3	false	0	false	false	false				
48	Rural Two-Lane Segment Two-lane Undivided	298+33.66 0	303+50.00 0	516.34	0.0978	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.61	6.2	3	false	0	false	false	false				
49	Rural Two-Lane Segment Two-lane Undivided	303+50.00 0	304+50.00 0	100.00	0.0189	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.61	6.2	3	false	0	false	false	false				
50	Rural Two-Lane Segment Two-lane Undivided	304+50.00 0	305+02.03 9	52.04	0.0099	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.61	6.2	3	false	0	false	false	false				
51	Rural Two-Lane Segment Two-lane Undivided	305+02.03 9	309+35.49 0	433.45	0.0821	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.15	6.2	3	false	0	false	false	false				
52	Rural Two-Lane Segment Two-lane Undivided	309+35.49 0	311+70.00 0	234.51	0.0444	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.24	6.2	3	false	0	false	false	false				

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Lane Widt	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazard Rating	Centerline Rumble Strip	Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
53	Rural Two-Lane Segment Two-lane Undivided	311+70.00 0	313+25.00 0	155.00	0.0294	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.24	6.2	3	false	0	false	false	false				
54	Rural Two-Lane Segment Two-lane Undivided	313+25.00 0	323+00.00 0	975.00	0.1847	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.33	6.2	3	false	0	false	false	false				
55	Rural Two-Lane Segment Two-lane Undivided	323+00.00 0	323+26.98 0	26.98	0.0051	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.33	6.2	3	false	0	false	false	false				
56	Rural Two-Lane Segment Two-lane Undivided	323+26.98 0	328+89.23 0	562.25	0.1065	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.26	6.2	3	false	0	false	false	false				
57	Rural Two-Lane Segment Two-lane Undivided	328+89.23 0	329+81.74 0	92.51	0.0175	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.52	6.2	3	false	0	false	false	false				
58	Rural Two-Lane Segment Two-lane Undivided	329+81.74 0	333+24.92 0	343.18	0.0650	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.52	6.2	3	false	0	false	false	false	4,010.13	2.0	true	70
59	Rural Two-Lane Segment Two-lane Undivided	333+24.92 0	334+00.00 0	75.08	0.0142	2018-2022: 2,085	12.00	12.00	8.00	8.00	-2.17	6.2	3	false	0	false	false	false	4,010.13	2.0	true	70
60	Rural Two-Lane Segment Two-lane Undivided	334+00.00 0	335+39.96 0	139.96	0.0265	2018-2022: 2,085	12.00	12.00	8.00	8.00	-2.17	6.2	3	false	0	false	false	false	4,010.13	2.0	true	70
61	Rural Two-Lane Segment Two-lane Undivided	335+39.96 0	342+39.00 0	699.04	0.1324	2018-2022: 2,085	12.00	12.00	8.00	8.00	-2.17	6.2	3	false	0	false	false	false				
62	Rural Two-Lane Segment Two-lane Undivided	342+39.00 0	343+00.00 0	61.00	0.0116	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false				
63	Rural Two-Lane Segment Two-lane Undivided	343+00.00 0	351+20.00 0	820.00	0.1553	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false				
64	Rural Two-Lane Segment Two-lane Undivided	351+20.00 0	352+00.00 0	80.00	0.0152	2018-2022: 2,085	12.00	12.00	0.00	0.00	-0.24	6.2	3	false	0	false	false	false				
65	Rural Two-Lane Segment Two-lane Undivided	352+00.00 0	352+20.00 0	20.00	0.0038	2018-2022: 2,085	12.00	12.00	0.00	0.00	-0.24	6.2	3	false	0	false	false	false				
66	Rural Two-Lane Segment Two-lane Undivided	352+20.00 0	362+50.00 0	1,030.00	0.1951	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false				
67	Rural Two-Lane Segment Two-lane Undivided	362+50.00 0	369+14.99 0	664.99	0.1259	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false				
68	Rural Two-Lane Segment Two-lane Undivided	369+14.99 0	370+30.00 0	115.01	0.0218	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
69	Rural Two-Lane Segment Two-lane Undivided	370+30.00 0	370+60.00 0	30.00	0.0057	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
70	Rural Two-Lane Segment Two-lane Undivided	370+60.00 0	376+83.61 0	623.61	0.1181	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
71	Rural Two-Lane Segment Two-lane Undivided	376+83.61 0	378+00.00 0	116.39	0.0220	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
72	Rural Two-Lane Segment Two-lane Undivided	378+00.00 0	378+40.00 0	40.00	0.0076	2018-2022: 2,085	12.00	12.00	0.00	8.00	-1.04	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
73	Rural Two-Lane Segment Two-lane Undivided	378+40.00 0	378+60.00 0	20.00	0.0038	2018-2022: 2,085	12.00	12.00	0.00	8.00	-1.04	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
74	Rural Two-Lane Segment Two-lane Undivided	378+60.00 0	379+00.00 0	40.00	0.0076	2018-2022: 2,085	12.00	12.00	0.00	8.00	-1.04	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
75	Rural Two-Lane Segment Two-lane Undivided	379+00.00 0	379+62.69 0	62.69	0.0119	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
76	Rural Two-Lane Segment Two-lane Undivided	379+62.69 0	385+22.97 0	560.28	0.1061	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false				
77	Rural Two-Lane Segment Two-lane Undivided	385+22.97 0	386+60.00 0	137.03	0.0260	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
78	Rural Two-Lane Segment Two-lane Undivided	386+60.00 0	389+50.00 0	290.00	0.0549	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
79	Rural Two-Lane Segment Two-lane Undivided	389+50.00 0	394+00.00 0	450.00	0.0852	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazard Rating	Centerline Rumble Strip	Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
80	Rural Two-Lane Segment Two-lane Undivided	394+00.00 0	396+46.15 0	246.15	0.0466	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
81	Rural Two-Lane Segment Two-lane Undivided	396+46.15 0	397+00.00 0	53.85	0.0102	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
82	Rural Two-Lane Segment Two-lane Undivided	397+00.00 0	399+00.00 0	200.00	0.0379	2018-2022: 2,085	12.00	12.00	0.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
83	Rural Two-Lane Segment Two-lane Undivided	399+00.00 0	405+75.41 0	675.41	0.1279	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
84	Rural Two-Lane Segment Two-lane Undivided	405+75.41 0	406+00.00 0	24.59	0.0047	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.72	6.2	3	false	0	false	false	false				
85	Rural Two-Lane Segment Two-lane Undivided	406+00.00 0	407+00.00 0	100.00	0.0189	2018-2022: 2,085	12.00	12.00	0.00	8.00	0.72	6.2	3	false	0	false	false	false				
86	Rural Two-Lane Segment Two-lane Undivided	407+00.00 0	443+25.00 0	3,625.00	0.6866	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.72	6.2	3	false	0	false	false	false				
87	Rural Two-Lane Segment Two-lane Undivided	443+25.00 0	445+50.00 0	225.00	0.0426	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
88	Rural Two-Lane Segment Two-lane Undivided	445+50.00 0	452+50.00 0	700.00	0.1326	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
89	Rural Two-Lane Segment Two-lane Undivided	452+50.00 0	459+00.00 0	650.00	0.1231	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
90	Rural Two-Lane Segment Two-lane Undivided	459+00.00 0	460+00.00 0	100.00	0.0189	2018-2022: 2,085	12.00	12.00	8.00	0.00	-0.96	6.2	3	false	0	false	false	false				
91	Rural Two-Lane Segment Two-lane Undivided	460+00.00 0	460+58.58 0	58.58	0.0111	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
92	Rural Two-Lane Segment Two-lane Undivided	460+58.58 0	485+61.23 0	2,502.65	0.4740	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.01	6.2	3	false	0	false	false	false				
93	Rural Two-Lane Segment Two-lane Undivided	485+61.23 0	503+00.00 0	1,738.77	0.3293	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.07	6.2	3	false	0	false	false	false				
94	Rural Two-Lane Segment Two-lane Undivided	503+00.00 0	507+00.00 0	400.00	0.0758	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.07	6.2	3	false	0	false	false	false				
95	Rural Two-Lane Segment Two-lane Undivided	507+00.00 0	508+00.00 0	100.00	0.0189	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.07	6.2	3	false	0	false	true	false				
96	Rural Two-Lane Segment Two-lane Undivided	508+00.00 0	508+08.24 0	8.24	0.0016	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.07	6.2	3	false	0	false	true	false				
97	Rural Two-Lane Segment Two-lane Undivided	508+08.24 0	510+30.00 0	221.76	0.0420	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.21	6.2	3	false	0	false	true	false				
98	Rural Two-Lane Segment Two-lane Undivided	510+30.00 0	512+00.00 0	170.00	0.0322	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.21	6.2	3	false	0	false	false	false				
99	Rural Two-Lane Segment Two-lane Undivided	512+00.00 0	513+00.00 0	100.00	0.0189	2018-2022: 4,325	12.00	12.00	0.00	0.00	0.21	6.2	3	false	0	false	true	false				
100	Rural Two-Lane Segment Two-lane Undivided	513+00.00 0	515+00.00 0	200.00	0.0379	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.21	6.2	3	false	0	false	true	false				
101	Rural Two-Lane Segment Two-lane Undivided	515+00.00 0	520+00.00 0	500.00	0.0947	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.21	6.2	3	false	0	true	true	false				
102	Rural Two-Lane Segment Two-lane Undivided	520+00.00 0	520+49.15 0	49.15	0.0093	2018-2022: 4,325	12.00	12.00	0.00	0.00	0.21	6.2	3	false	0	false	true	false				
103	Rural Two-Lane Segment Two-lane Undivided	520+49.15 0	521+00.00 0	50.85	0.0096	2018-2022: 4,325	12.00	12.00	0.00	0.00	0.21	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
104	Rural Two-Lane Segment Two-lane Undivided	521+00.00 0	523+38.60 0	238.60	0.0452	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.21	6.2	3	false	0	true	true	false	2,458.49	2.0	true	45
105	Rural Two-Lane Segment Two-lane Undivided	523+38.60 0	524+00.00 0	61.40	0.0116	2018-2022: 4,325	12.00	12.00	8.00	8.00	1.90	6.2	3	false	0	true	true	false	2,458.49	2.0	true	45
106	Rural Two-Lane Segment Two-lane Undivided	524+00.00 0	525+00.00 0	100.00	0.0189	2018-2022: 4,325	12.00	12.00	0.00	0.00	1.90	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazard Rating	Centerline Rumble Strip	Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
107	Rural Two-Lane Segment Two-lane Undivided	525+00.00 0	525+18.58 0	18.58	0.0035	2018-2022: 4,325	12.00	12.00	8.00	8.00	1.90	6.2	3	false	0	true	true	false	2,458.49	2.0	true	45
108	Rural Two-Lane Segment Two-lane Undivided	525+18.58 0	528+00.00 0	281.42	0.0533	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.02	6.2	3	false	0	true	true	false	2,458.49	2.0	true	45
109	Rural Two-Lane Segment Two-lane Undivided	528+00.00 0	529+00.00 0	100.00	0.0189	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.02	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
110	Rural Two-Lane Segment Two-lane Undivided	529+00.00 0	539+00.00 0	1,000.00	0.1894	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.02	6.2	3	false	0	true	true	false	2,458.49	2.0	true	45
111	Rural Two-Lane Segment Two-lane Undivided	539+00.00 0	539+50.00 0	50.00	0.0095	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.02	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
112	Rural Two-Lane Segment Two-lane Undivided	539+50.00 0	540+00.00 0	50.00	0.0095	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
113	Rural Two-Lane Segment Two-lane Undivided	540+00.00 0	540+50.00 0	50.00	0.0095	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
114	Rural Two-Lane Segment Two-lane Undivided	540+50.00 0	540+74.37 0	24.37	0.0046	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
115	Rural Two-Lane Segment Two-lane Undivided	540+74.37 0	541+00.00 0	25.63	0.0049	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	true	false				
116	Rural Two-Lane Segment Two-lane Undivided	541+00.00 0	541+50.00 0	50.00	0.0095	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	true	false				
117	Rural Two-Lane Segment Two-lane Undivided	541+50.00 0	541+70.00 0	20.00	0.0038	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	true	false				
118	Rural Two-Lane Segment Two-lane Undivided	541+70.00 0	542+30.00 0	60.00	0.0114	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	true	false				
119	Rural Two-Lane Segment Two-lane Undivided	542+30.00 0	542+64.00 0	34.00	0.0064	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	true	false				
120	Rural Two-Lane Segment Two-lane Undivided	542+64.00 0	543+34.00 0	70.00	0.0133	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	true	false				
121	Rural Two-Lane Segment Two-lane Undivided	543+34.00 0	544+00.00 0	66.00	0.0125	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	true	false				
122	Rural Two-Lane Segment Two-lane Undivided	544+00.00 0	545+00.00 0	100.00	0.0189	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
123	Rural Two-Lane Segment Two-lane Undivided	545+00.00 0	548+23.00 0	323.00	0.0612	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
124	Rural Two-Lane Segment Two-lane Undivided	548+23.00 0	553+70.00 0	547.00	0.1036	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
125	Rural Two-Lane Segment Two-lane Undivided	553+70.00 0	554+00.00 0	30.00	0.0057	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	false	false				
126	Rural Two-Lane Segment Two-lane Undivided	554+00.00 0	554+20.00 0	20.00	0.0038	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	false	false				
127	Rural Two-Lane Segment Two-lane Undivided	554+20.00 0	560+00.00 0	580.00	0.1098	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
128	Rural Two-Lane Segment Two-lane Undivided	560+00.00 0	562+58.56 0	258.56	0.0490	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
129	Rural Two-Lane Segment Two-lane Undivided	562+58.56 0	564+00.00 0	141.44	0.0268	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false				
130	Rural Two-Lane Segment Two-lane Undivided	564+00.00 0	565+00.00 0	100.00	0.0189	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false				
131	Rural Two-Lane Segment Two-lane Undivided	565+00.00 0	565+77.00 0	77.00	0.0146	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false				
132	Rural Two-Lane Segment Two-lane Undivided	565+77.00 0	566+10.00 0	33.00	0.0063	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.20	6.2	3	false	0	false	false	false				
133	Rural Two-Lane Segment Two-lane Undivided	566+10.00 0	566+50.00 0	40.00	0.0076	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.20	6.2	3	false	0	false	false	false				

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT			Shoulder	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazard Rating		Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Design Speed (mph)
134	Rural Two-Lane Segment Two-lane Undivided	566+50.00 0	569+37.00 0	287.00	0.0544	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false			
135	Rural Two-Lane Segment Two-lane Undivided	569+37.00 0	569+70.00 0	33.00	0.0063	2018-2022: 4,325	12.00	12.00	8.00	0.00	-0.20	6.2	3	false	0	false	false	false			
136	Rural Two-Lane Segment Two-lane Undivided	569+70.00 0	570+00.00 0	30.00	0.0057	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false			
137	Rural Two-Lane Segment Two-lane Undivided	570+00.00 0	575+00.00 0	500.00	0.0947	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	true	false	false			
138	Rural Two-Lane Segment Two-lane Undivided	575+00.00 0	579+50.00 0	450.00	0.0852	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false			
139	Rural Two-Lane Segment Two-lane Undivided	579+50.00 0	579+70.00 0	20.00	0.0038	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.20	6.2	3	false	0	false	false	false			
140	Rural Two-Lane Segment Two-lane Undivided	579+70.00 0	580+10.00 0	40.00	0.0076	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.20	6.2	3	false	0	false	false	false			

**Table 4. Evaluation Intersection - Section 1** 

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
1	West Central School/SD38 (v2)	Rural Two-Lane Intersection Three-Legged w/STOP control	569+50.000	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,735; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	2025: 912; 2026: 932; 2027: 951; 2028: 970; 2029: 990; 2030: 1,013; 2031: 1,036; 2032: 1,059; 2033: 1,082; 2034: 1,105; 2035: 1,129; 2036: 1,152; 2036: 1,175; 2038: 1,175; 2038: 1,219; 2030: 1,21; 2040: 1,245; 2041: 1,273; 2042: 1,302; 2043: 1,330; 2044: 1,359; 2045: 1,387; 2046: 1,416; 2047: 1,444; 2048: 1,473; 2049: 1,501; 2050: 1,530	3	Stop-Controlled	0	0	1.37		false
2	2nd/SD38 (v2)	Rural Two-Lane Intersection Four-Legged w/STOP control	566+00.000	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,735; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	2025: 1,338; 2026: 1,366; 2027: 1,394; 2028: 1,422; 2029: 1,450; 2030: 1,484; 2031: 1,518; 2032: 1,552; 2033: 1,586; 2034: 1,620; 2035: 1,654; 2036: 1,688; 2037: 1,722; 2038: 1,756; 2039: 1,790; 2040: 1,825; 2041: 1,867; 2042: 1,909; 2043: 1,951; 2044: 1,993; 2045: 2,035; 2046: 2,077; 2047: 2,119; 2048: 2,161; 2049: 2,203; 2050: 2,245	4	Stop-Controlled	1	0	41.37	41.37	false
4	459/SD38 (v2)	Rural Two-Lane Intersection Four-Legged w/STOP control	296+00.000	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	2025: 320; 2026: 329; 2027: 337; 2028: 346; 2029: 355; 2030: 363; 2031: 371; 2032: 379; 2033: 387; 2034: 395; 2035: 404; 2036: 412; 2037: 420; 2038: 428: 2039: 436; 2040: 445; 2041: 455; 2042: 465; 2043: 475; 2044: 485; 2044: 485; 2046: 505; 2047: 515; 2048: 525; 2049: 535; 2050: 545	4	Stop-Controlled	2	0	0.04	0.04	false
5	SD38/SD19_Build (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	187+50.000	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378: 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	2025: 2,094; 2026: 2,140; 2027: 2,187; 2028: 2,233; 2029: 2,280; 2030: 2,336; 2031: 2,392; 2032: 2,499; 2033: 2,505; 2034: 2,561; 2035: 2,618; 2036: 2,674; 2037: 2,730; 2038: 2,787; 2039: 2,843; 2040: 2,900; 2041: 2,967; 2042: 3,034; 2043: 3,101; 2044: 3,168; 2045: 3,235; 2046: 3,302; 2047: 3,369; 2048: 3,436; 2049: 3,503; 2050: 3,570	4	Stop-Controlled	2	0	5.84	5.84	false
6	I90 SPEEDWAY/SD38 (v1)	Rural Two-Lane Intersection Three-Legged w/STOP control	378+50.000	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	2025: 260; 2026: 264; 2027: 267; 2028: 271; 2029: 275; 2030: 281; 2031: 288; 2032: 295; 2033: 302; 2034: 309; 2035: 315; 2036: 322; 2037: 329; 2038: 336; 2039: 333; 2040: 350; 2041: 372; 2042: 395; 2043: 417; 2044: 440; 2045: 462; 2046: 485; 2047: 507; 2048: 530; 2049: 552; 2050: 575	3	Stop-Controlled	1	1	5.46		false
7	463/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	512+00.000	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070: 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	2025: 3,802; 2026: 3,882; 2027: 3,963; 2028: 4,044; 2029: 4,125; 2030: 4,221; 2031: 4,318; 2032: 4,415; 2033: 4,512; 2034: 4,609; 2035: 4,705; 2036: 4,802; 2037: 4,899; 2038: 4,996; 2039: 5,093; 2040: 5,190; 2041: 5,308; 2042: 5,427; 2043: 5,545: 2044: 5,664; 2045: 5,782; 2046: 5,901; 2047: 6,019; 2048: 6,138; 2049: 6,256; 2050: 6,375	4	Stop-Controlled	1	0	1.43	1.43	false
8	Main Ave/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	524+50.000	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	2025: 1,367; 2026: 1,397; 2027: 1,426; 2028: 1,455; 2029: 1,485; 2030: 1,520; 2031: 1,555; 2032: 1,590; 2033: 1,625; 2034: 1,660; 2035: 1,695; 2036: 1,730; 2037: 1,765; 2038: 1,800; 2039: 1,835; 2040: 1,870; 2041: 1,912; 2042: 1,955; 2043: 1,997; 2044: 2,040; 2045: 2,082; 2046: 2,125; 2047: 2,167; 2048: 2,210; 2049: 2,252; 2050: 2,295	4	Stop-Controlled	0	0	11.00	10.54	false
9	Vandemark/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	541+50.000	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	2025: 657; 2026: 672; 2027: 686; 2028: 700; 2029: 715; 2030: 731; 2031: 747; 2032: 764; 2033: 780; 2034: 796; 2035: 813; 2036: 829; 2037: 845; 2038: 862; 2039: 878; 2040: 895; 2041: 915; 2042: 936; 2043: 956; 2044: 977; 2045: 997; 2046: 1,018; 2047: 1,038; 2048: 1,059; 2049: 1,079; 2050: 1,100	4	Stop-Controlled	1	0	48.63	48.63	false

 $\begin{tabular}{ll} \textbf{Table 5.} & \textbf{Crash History Intersection - Section 1} \\ \end{tabular}$ 

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
1	West Central School/SD38 (v2)	Rural Two-Lane Intersection Three-Legged w/STOP control	569+50.000	2018-2022: 4,325	2018-2022: 855	3	Stop-Controlled	0	0	1.37		false
2	2nd/SD38 (v2)	Rural Two-Lane Intersection Four-Legged w/STOP control	566+00.000	2018-2022: 4,325	2018-2022: 1,255	4	Stop-Controlled	1	0	41.37	41.37	false
4	459/SD38 (v2)	Rural Two-Lane Intersection Four-Legged w/STOP control	296+00.000	2018-2022: 2,085	2018-2022: 295	4	Stop-Controlled	2	0	0.04	0.04	false
5	SD38/SD19_Build (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	187+50.000	2018-2022: 2,085	2018-2022: 1,955	4	Stop-Controlled	2	0	5.84	5.84	false
6	I90 SPEEDWAY/SD38 (v1)	Rural Two-Lane Intersection Three-Legged w/STOP control	378+50.000	2018-2022: 2,085	2018-2022: 250	3	Stop-Controlled	1	1	5.46		false
7	463/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	512+00.000	2018-2022: 4,325	2018-2022: 3,560	4	Stop-Controlled	1	0	1.43	1.43	false
8	Main Ave/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	524+50.000	2018-2022: 4,325	2018-2022: 1,280	4	Stop-Controlled	0	0	11.00	10.54	false
9	Vandemark/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	541+50.000	2018-2022: 4,325	2018-2022: 615	4	Stop-Controlled	1	0	48.63	48.63	false

Table 6. Expected Highway Crash Rates and Frequencies Summary (Section 1)

	i
First Year of Analysis	2025
Last Year of Analysis	2050
Evaluated Length (mi)	7.7398
Average Future Road AADT (vpd)	4,284
Expected Crashes	
Total Crashes	476.74
Fatal and Injury Crashes	193.60
Property-Damage-Only Crashes	283.14
Percent of Total Expected Crashes	
Percent Fatal and Injury Crashes (%)	41
Percent Property-Damage-Only Crashes (%)	59
Expected Crash Rate	
Crash Rate (crashes/mi/yr)	2.3691
FI Crash Rate (crashes/mi/yr)	0.9621
PDO Crash Rate (crashes/mi/yr)	1.4070
Expected Travel Crash Rate	
Total Travel (million veh-mi)	314.63
Travel Crash Rate (crashes/million veh-mi)	1.51
Travel FI Crash Rate (crashes/million veh-mi)	0.61
Travel PDO Crash Rate (crashes/million veh-mi)	0.90

Table 7. Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 1)

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi llion veh- mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
1	171+44.000	172+42.000	0.0186	0.237	0.402	0.0091	0.0033	0.0058	0.0155	0.0050	0.0105	-0.0063	-0.0017	-0.0047	0.4920	0.45	
2	172+42.000	174+52.690	0.0399	0.482	0.786	0.0185	0.0067	0.0119	0.0302	0.0097	0.0205	-0.0117	-0.0030	-0.0087	0.4646	0.43	
3	174+52.690	176+25.000	0.0326	0.499	0.980	0.0192	0.0072	0.0120	0.0377	0.0121	0.0256	-0.0185	-0.0049	-0.0136	0.5886	0.54	
4	176+25.000	178+85.250	0.0493	0.754	1.479	0.0290	0.0108	0.0182	0.0569	0.0183	0.0386	-0.0279	-0.0074	-0.0205	0.5886	0.54	
5	178+85.250	183+75.370	0.0928	1.121	1.829	0.0431	0.0155	0.0276	0.0703	0.0226	0.0478	-0.0272	-0.0071	-0.0201	0.4646	0.43	
6	183+75.370	184+00.000	0.0047	0.056	0.092	0.0022	0.0008	0.0014	0.0035	0.0011	0.0024	-0.0014	-0.0004	-0.0010	0.4646	0.43	
7	184+00.000	184+45.000	0.0085	0.103	0.168	0.0040	0.0014	0.0025	0.0065	0.0021	0.0044	-0.0025	-0.0006	-0.0018	0.4646	0.43	
8	184+45.000	185+20.000	0.0142	0.172	0.280	0.0066	0.0024	0.0042	0.0108	0.0035	0.0073	-0.0042	-0.0011	-0.0031	0.4646	0.43	
9	185+20.000	186+60.000	0.0265	0.320	0.522	0.0123	0.0044	0.0079	0.0201	0.0064	0.0136	-0.0078	-0.0020	-0.0058	0.4646	0.43	
10	186+60.000	187+20.000	0.0114	0.137	0.224	0.0053	0.0019	0.0034	0.0086	0.0028	0.0058	-0.0033	-0.0009	-0.0025	0.4646	0.43	
11	187+20.000	187+60.000	0.0076	0.111	0.208	0.0043	0.0016	0.0027	0.0080	0.0026	0.0054	-0.0037	-0.0010	-0.0027	0.5613	0.52	
SD38/SD19_Build (v1)	187+50.000			18.404	41.067	0.7079	0.3235	0.3844	1.5795	0.6808	0.8987	-0.8716	-0.3573	-0.5144			0.3
12	187+60.000	190+00.000	0.0455	0.549	0.895	0.0211	0.0076	0.0135	0.0344	0.0111	0.0234	-0.0133	-0.0035	-0.0099	0.4646	0.43	
13	190+00.000	192+00.000	0.0379	0.458	0.746	0.0176	0.0063	0.0113	0.0287	0.0092	0.0195	-0.0111	-0.0029	-0.0082	0.4646	0.43	
14	192+00.000	192+39.270	0.0074	0.090	0.146	0.0035	0.0012	0.0022	0.0056	0.0018	0.0038	-0.0022	-0.0006	-0.0016	0.4646	0.43	
15	192+39.270	193+60.000	0.0229	0.276	0.451	0.0106	0.0038	0.0068	0.0173	0.0056	0.0118	-0.0067	-0.0017	-0.0050	0.4646	0.43	
16	193+60.000	197+65.000	0.0767	0.927	1.511	0.0356	0.0128	0.0228	0.0581	0.0187	0.0395	-0.0225	-0.0058	-0.0166	0.4646	0.43	
17	197+65.000	199+00.000	0.0256	0.309	0.504	0.0119	0.0043	0.0076	0.0194	0.0062	0.0132	-0.0075	-0.0019	-0.0055	0.4646	0.43	
18	199+00.000	201+63.750	0.0500	0.603	0.984	0.0232	0.0083	0.0149	0.0379	0.0121	0.0257	-0.0146	-0.0038	-0.0108	0.4646	0.43	
19	201+63.750	202+00.000	0.0069	0.083	0.135	0.0032	0.0011	0.0020	0.0052	0.0017	0.0035	-0.0020	-0.0005	-0.0015	0.4646	0.43	
20	202+00.000	207+00.000	0.0947	1.144	1.866	0.0440	0.0158	0.0282	0.0718	0.0230	0.0487	-0.0278	-0.0072	-0.0205	0.4646	0.43	
21	207+00.000	207+49.760	0.0094	0.114	0.186	0.0044	0.0016	0.0028	0.0071	0.0023	0.0048	-0.0028	-0.0007	-0.0020	0.4646	0.43	
22	207+49.760	217+74.250	0.1940	2.344	3.823	0.0902	0.0324	0.0577	0.1470	0.0472	0.0998	-0.0569	-0.0148	-0.0421	0.4646	0.43	
23	217+74.250	221+00.000	0.0617	3.596	1.215	0.1383	0.0144	0.1239	0.0467	0.0150	0.0317	0.0916	-0.0006	0.0922	2.2419	2.07	
24	221+00.000	226+00.000	0.0947	1.144	1.866	0.0440	0.0158	0.0282	0.0718	0.0230	0.0487	-0.0278	-0.0072	-0.0205	0.4646	0.43	
25	226+00.000	230+66.250	0.0883	1.067	1.740	0.0410	0.0148	0.0263	0.0669	0.0215	0.0454	-0.0259	-0.0067	-0.0192	0.4646	0.43	
26	230+66.250	231+39.700	0.0139	0.188	0.333	0.0072	0.0026	0.0046	0.0128	0.0041	0.0087	-0.0055	-0.0015	-0.0041	0.5207	0.48	
27	231+39.700	235+00.000	0.0682	0.924	1.631	0.0355	0.0130	0.0225	0.0627	0.0201	0.0426	-0.0272	-0.0071	-0.0201	0.5207	0.48	
28	235+00.000	241+61.390	0.1253	4.891	2.994	0.1881	0.1175	0.0706	0.1152	0.0370	0.0782	0.0730	0.0805	-0.0076	1.5018	1.39	
29	241+61.390	242+00.000	0.0073	0.099	0.175	0.0038	0.0014	0.0024	0.0067	0.0022	0.0046	-0.0029	-0.0008	-0.0021	0.5207	0.48	
30	242+00.000	245+14.280	0.0595	0.806	1.423	0.0310	0.0113	0.0197	0.0547	0.0176	0.0372	-0.0237	-0.0062	-0.0175	0.5207	0.48	
31	245+14.280	246+55.100	0.0267	0.322	0.525	0.0124	0.0045	0.0079	0.0202	0.0065	0.0137	-0.0078	-0.0020	-0.0058	0.4646	0.43	
32	246+55.100	248+00.000	0.0274	0.332	0.541	0.0128	0.0046	0.0082	0.0208	0.0067	0.0141	-0.0080	-0.0021	-0.0060	0.4646	0.43	
33	248+00.000	249+00.000	0.0189	3.721	0.519	0.1431	0.0060	0.1371	0.0200	0.0064	0.0136	0.1231	-0.0004	0.1236	7.5559	6.97	
34	249+00.000	251+21.980	0.0420	0.508	0.828	0.0195	0.0070	0.0125	0.0319	0.0102	0.0216	-0.0123	-0.0032	-0.0091	0.4646	0.43	
35	251+21.980	252+40.240	0.0224	0.286	0.485	0.0110	0.0040	0.0070	0.0187	0.0060	0.0127	-0.0076	-0.0020	-0.0057	0.4920	0.45	
36	252+40.240	263+22.600	0.2050	5.327	4.038	0.2049	0.0424	0.1625	0.1553	0.0499	0.1055	0.0496	-0.0074	0.0570	0.9995	0.92	
37	263+22.600	272+66.740	0.1788	2.351	4.061	0.0904	0.0329	0.0575	0.1562	0.0501	0.1060	-0.0657	-0.0172	-0.0485	0.5057	0.47	
38	272+66.740	280+00.000	0.1389	1.678	2.736	0.0645	0.0232	0.0413	0.1052	0.0338	0.0715	-0.0407	-0.0106	-0.0301	0.4646	0.43	

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi llion veh- mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
39	280+00.000	283+15.050	0.0597	0.721	1.175	0.0277	0.0100	0.0178	0.0452	0.0145	0.0307	-0.0175	-0.0045	-0.0129	0.4646	0.43	
40	283+15.050	284+08.540	0.0177	0.227	0.384	0.0087	0.0032	0.0056	0.0148	0.0047	0.0100	-0.0060	-0.0016	-0.0045	0.4920	0.45	
41	284+08.540	288+50.000	0.0836	3.861	1.647	0.1485	0.0190	0.1295	0.0634	0.0203	0.0430	0.0851	-0.0013	0.0865	1.7761	1.64	
42	288+50.000	289+00.000	0.0095	0.114	0.187	0.0044	0.0016	0.0028	0.0072	0.0023	0.0049	-0.0028	-0.0007	-0.0021	0.4646	0.43	
43	289+00.000	295+90.000	0.1307	1.579	2.575	0.0607	0.0218	0.0389	0.0990	0.0318	0.0672	-0.0383	-0.0100	-0.0284	0.4646	0.43	
44	295+90.000	296+00.000	0.0019	0.028	0.052	0.0011	0.0004	0.0007	0.0020	0.0006	0.0014	-0.0009	-0.0002	-0.0007	0.5613	0.52	
459/SD38 (v2)	296+00.000			11.436	12.695	0.4399	0.1714	0.2685	0.4883	0.2104	0.2778	-0.0484	-0.0391	-0.0093			0.37
45	296+00.000	296+10.000	0.0019	0.028	0.052	0.0011	0.0004	0.0007	0.0020	0.0006	0.0014	-0.0009	-0.0002	-0.0007	0.5613	0.52	
46	296+10.000	296+96.520	0.0164	0.198	0.323	0.0076	0.0027	0.0049	0.0124	0.0040	0.0084	-0.0048	-0.0012	-0.0036	0.4646	0.43	
47	296+96.520	298+33.660	0.0260	0.314	0.512	0.0121	0.0043	0.0077	0.0197	0.0063	0.0134	-0.0076	-0.0020	-0.0056	0.4646	0.43	
48	298+33.660	303+50.000	0.0978	1.181	1.927	0.0454	0.0163	0.0291	0.0741	0.0238	0.0503	-0.0287	-0.0074	-0.0212	0.4646	0.43	
49	303+50.000	304+50.000	0.0189	0.229	0.373	0.0088	0.0032	0.0056	0.0144	0.0046	0.0097	-0.0056	-0.0014	-0.0041	0.4646	0.43	
50	304+50.000	305+02.039	0.0099	0.119	0.194	0.0046	0.0016	0.0029	0.0075	0.0024	0.0051	-0.0029	-0.0008	-0.0021	0.4646	0.43	
51	305+02.039	309+35.490	0.0821	0.992	1.617	0.0381	0.0137	0.0244	0.0622	0.0200	0.0422	-0.0241	-0.0063	-0.0178	0.4646	0.43	
52	309+35.490	311+70.000	0.0444	0.536	0.875	0.0206	0.0074	0.0132	0.0337	0.0108	0.0229	-0.0130	-0.0034	-0.0096	0.4646	0.43	
53	311+70.000	313+25.000	0.0294	0.355	0.578	0.0136	0.0049	0.0087	0.0222	0.0071	0.0151	-0.0086	-0.0022	-0.0064	0.4646	0.43	
54	313+25.000	323+00.000	0.1847	2.231	3.638	0.0858	0.0309	0.0549	0.1399	0.0449	0.0950	-0.0541	-0.0141	-0.0401	0.4646	0.43	
55	323+00.000	323+26.980	0.0051	0.062	0.101	0.0024	0.0009	0.0015	0.0039	0.0012	0.0026	-0.0015	-0.0004	-0.0011	0.4646	0.43	
56	323+26.980	328+89.230	0.1065	1.286	2.098	0.0495	0.0178	0.0317	0.0807	0.0259	0.0548	-0.0312	-0.0081	-0.0231	0.4646	0.43	
57	328+89.230	329+81.740	0.0175	0.212	0.345	0.0081	0.0029	0.0052	0.0133	0.0043	0.0090	-0.0051	-0.0013	-0.0038	0.4646	0.43	
58	329+81.740	333+24.920	0.0650	0.936	1.738	0.0360	0.0133	0.0227	0.0669	0.0215	0.0454	-0.0308	-0.0081	-0.0227	0.5541	0.51	
59	333+24.920	334+00.000	0.0142	0.205	0.380	0.0079	0.0029	0.0050	0.0146	0.0047	0.0099	-0.0067	-0.0018	-0.0050	0.5541	0.51	
60	334+00.000	335+39.960	0.0265	7.182	0.709	0.2762	0.0084	0.2679	0.0273	0.0088	0.0185	0.2490	-0.0004	0.2494	10.4210	9.62	
61	335+39.960	342+39.000	0.1324	1.599	2.608	0.0615	0.0221	0.0394	0.1003	0.0322	0.0681	-0.0388	-0.0101	-0.0287	0.4646	0.43	
62	342+39.000	343+00.000	0.0116	0.140	0.228	0.0054	0.0019	0.0034	0.0088	0.0028	0.0059	-0.0034	-0.0009	-0.0025	0.4646	0.43	
63	343+00.000	351+20.000	0.1553	4.727	3.060	0.1818	0.1065	0.0753	0.1177	0.0378	0.0799	0.0641	0.0687	-0.0046	1.1706	1.08	
64	351+20.000	352+00.000	0.0152	0.221	0.415	0.0085	0.0032	0.0054	0.0160	0.0051	0.0108	-0.0075	-0.0020	-0.0055	0.5613	0.52	
65	352+00.000	352+20.000	0.0038	0.055	0.104	0.0021	0.0008	0.0013	0.0040	0.0013	0.0027	-0.0019	-0.0005	-0.0014	0.5613	0.52	
66	352+20.000	362+50.000	0.1951	5.207	3.843	0.2003	0.0406	0.1597	0.1478	0.0474	0.1004	0.0525	-0.0069	0.0593	1.0267	0.95	
67	362+50.000	369+14.990	0.1259	1.521	2.481	0.0585	0.0210	0.0375	0.0954	0.0306	0.0648	-0.0369	-0.0096	-0.0273	0.4646	0.43	
68	369+14.990	370+30.000	0.0218	0.305	0.553	0.0117	0.0043	0.0074	0.0213	0.0068	0.0144	-0.0095	-0.0025	-0.0070	0.5385	0.50	
69	370+30.000	370+60.000	0.0057	0.080	0.144	0.0031	0.0011	0.0019	0.0055	0.0018	0.0038	-0.0025	-0.0007	-0.0018	0.5385	0.50	
70	370+60.000	376+83.610	0.1181	4.958	2.998	0.1907	0.1212	0.0695	0.1153	0.0370	0.0783	0.0754	0.0842	-0.0088	1.6146	1.49	
71	376+83.610	378+00.000	0.0220	0.309	0.559	0.0119	0.0044	0.0075	0.0215	0.0069	0.0146	-0.0096	-0.0025	-0.0071	0.5385	0.50	
72	378+00.000	378+40.000	0.0076	0.117	0.230	0.0045	0.0017	0.0028	0.0088	0.0028	0.0060	-0.0044	-0.0012	-0.0032	0.5919	0.55	
73	378+40.000	378+60.000	0.0038	0.058	0.115	0.0022	0.0008	0.0014	0.0044	0.0014	0.0030	-0.0022	-0.0006	-0.0016	0.5919	0.55	
I90 SPEEDWAY/SD38 (v1)	378+50.000			4.683	6.719	0.1801	0.0774	0.1027	0.2584	0.1073	0.1512	-0.0783	-0.0298	-0.0485			0.16
74	378+60.000	379+00.000	0.0076	0.117	0.230	0.0045	0.0017	0.0028	0.0088	0.0028	0.0060	-0.0044	-0.0012	-0.0032	0.5919	0.55	
75	379+00.000	379+62.690	0.0119	0.166	0.301	0.0064	0.0024	0.0040	0.0116	0.0037	0.0079	-0.0052	-0.0014	-0.0038	0.5385	0.50	
76	379+62.690	385+22.970	0.1061	4.133	2.091	0.1590	0.1024	0.0566	0.0804	0.0258	0.0546	0.0785	0.0766	0.0020	1.4979	1.38	
77	385+22.970	386+60.000	0.0260	0.359	0.643	0.0138	0.0051	0.0087	0.0247	0.0079	0.0168	-0.0109	-0.0029	-0.0081	0.5314	0.49	
78	386+60.000	389+50.000	0.0549	0.759	1.361	0.0292	0.0107	0.0185	0.0524	0.0168	0.0355	-0.0232	-0.0061	-0.0171	0.5314	0.49	

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi Ilion veh- mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
79	389+50.000	394+00.000	0.0852	1.178	2.112	0.0453	0.0166	0.0287	0.0812	0.0261	0.0552	-0.0359	-0.0095	-0.0265	0.5314	0.49	
80	394+00.000	396+46.150	0.0466	0.644	1.155	0.0248	0.0091	0.0157	0.0444	0.0143	0.0302	-0.0197	-0.0052	-0.0145	0.5314	0.49	
81	396+46.150	397+00.000	0.0102	0.141	0.253	0.0054	0.0020	0.0034	0.0097	0.0031	0.0066	-0.0043	-0.0011	-0.0032	0.5314	0.49	
82	397+00.000	399+00.000	0.0379	0.576	1.122	0.0221	0.0083	0.0139	0.0432	0.0139	0.0293	-0.0210	-0.0056	-0.0154	0.5847	0.54	
83	399+00.000	405+75.410	0.1279	1.768	3.170	0.0680	0.0249	0.0430	0.1219	0.0391	0.0828	-0.0539	-0.0142	-0.0398	0.5314	0.49	
84	405+75.410	406+00.000	0.0047	0.056	0.092	0.0022	0.0008	0.0014	0.0035	0.0011	0.0024	-0.0014	-0.0004	-0.0010	0.4646	0.43	
85	406+00.000	407+00.000	0.0189	0.254	0.446	0.0098	0.0036	0.0062	0.0172	0.0055	0.0116	-0.0074	-0.0019	-0.0054	0.5164	0.48	
86	407+00.000	443+25.000	0.6866	25.399	13.526	0.9769	0.4312	0.5456	0.5202	0.1670	0.3532	0.4567	0.2642	0.1924	1.4229	1.31	
87	443+25.000	445+50.000	0.0426	0.515	0.840	0.0198	0.0071	0.0127	0.0323	0.0104	0.0219	-0.0125	-0.0032	-0.0092	0.4646	0.43	
88	445+50.000	452+50.000	0.1326	4.452	2.612	0.1712	0.1044	0.0669	0.1005	0.0322	0.0682	0.0708	0.0721	-0.0013	1.2917	1.19	
89	452+50.000	459+00.000	0.1231	1.487	2.425	0.0572	0.0206	0.0366	0.0933	0.0299	0.0633	-0.0361	-0.0094	-0.0267	0.4646	0.43	
90	459+00.000	460+00.000	0.0189	0.254	0.446	0.0098	0.0036	0.0062	0.0172	0.0055	0.0116	-0.0074	-0.0019	-0.0054	0.5164	0.48	
91	460+00.000	460+58.580	0.0111	0.134	0.219	0.0052	0.0019	0.0033	0.0084	0.0027	0.0057	-0.0033	-0.0008	-0.0024	0.4646	0.43	
92	460+58.580	485+61.230	0.4740	8.577	9.338	0.3299	0.0899	0.2399	0.3592	0.1153	0.2439	-0.0293	-0.0253	-0.0039	0.6959	0.64	
93	485+61.230	503+00.000	0.3293	6.829	6.488	0.2627	0.0647	0.1979	0.2495	0.0801	0.1694	0.0131	-0.0154	0.0285	0.7976	0.74	
94	503+00.000	507+00.000	0.0758	3.766	1.492	0.1448	0.0174	0.1275	0.0574	0.0184	0.0390	0.0874	-0.0010	0.0885	1.9120	1.76	
95	507+00.000	508+00.000	0.0189	0.217	0.344	0.0084	0.0030	0.0054	0.0132	0.0042	0.0090	-0.0049	-0.0013	-0.0036	0.4416	0.41	
96	508+00.000	508+08.240	0.0016	0.018	0.028	0.0007	0.0002	0.0004	0.0011	0.0003	0.0007	-0.0004	-0.0001	-0.0003	0.4416	0.41	
97	508+08.240	510+30.000	0.0420	0.482	0.762	0.0185	0.0066	0.0119	0.0293	0.0094	0.0199	-0.0108	-0.0028	-0.0080	0.4416	0.41	
98	510+30.000	512+00.000	0.0322	0.389	0.634	0.0150	0.0054	0.0096	0.0244	0.0078	0.0166	-0.0094	-0.0025	-0.0070	0.4646	0.43	
463/SD38 (v1)	512+00.000			87.655	169.483	3.3714	1.6033	1.7681	6.5186	2.8095	3.7091	-3.1472	-1.2062	-1.9410			0.88
99	512+00.000	513+00.000	0.0189	0.626	1.666	0.0241	0.0095	0.0145	0.0641	0.0206	0.0435	-0.0400	-0.0110	-0.0290	1.2714	0.33	
100	513+00.000	515+00.000	0.0379	1.092	2.395	0.0420	0.0161	0.0259	0.0921	0.0296	0.0626	-0.0501	-0.0135	-0.0366	1.1086	0.29	
101	515+00.000	520+00.000	0.0947	2.689	5.796	0.1034	0.0394	0.0640	0.2229	0.0716	0.1514	-0.1195	-0.0322	-0.0873	1.0920	0.28	
102	520+00.000	520+49.150	0.0093	0.308	0.819	0.0118	0.0047	0.0071	0.0315	0.0101	0.0214	-0.0197	-0.0054	-0.0142	1.2714	0.33	
103	520+49.150 521+00.000	521+00.000 523+38.600	0.0096	0.343	1.049	0.0132	0.0054	0.0078	0.0403	0.0130	0.0274	-0.0271 -0.0776	-0.0076 -0.0212	-0.0195 -0.0564	1.3705	0.36	
104	521+00.000 523+38.600	523+38.600	0.0452	0.362	3.425 0.881	0.0542	0.0211	0.0331	0.1317	0.0423	0.0895	-0.0776	-0.0212	-0.0564	1.1991	0.31	
105	524+00.000	525+00.000	0.0116	0.562	2.063	0.0139	0.0034	0.0083	0.0339	0.0109	0.0230	-0.0200	-0.0034	-0.0143	1.1991	0.31	
Main Ave/SD38 (v1)	524+50.000	323+00.000	0.0109	42.110	132.778	1,6196	0.6778	0.9418	5.1069	2.2011	2,9058	-3.4872	-1.5232	-1.9640	1.3703	0.30	0.37
107	525+00.000	525+18.580	0.0035	0.110	0.267	0.0042	0.0016	0.0026	0.0103	0.0033	0.0070	-0.0060	-0.0016	-0.0044	1.1991	0.31	0.57
108	525+18.580	528+00.000	0.0533	1.662	4.040	0.0639	0.0249	0.0390	0.1554	0.0499	0.1055	-0.0915	-0.0250	-0.0665	1.1991	0.31	
109	528+00.000	529+00.000	0.0189	0.675	2.063	0.0260	0.0105	0.0154	0.0793	0.0255	0.0539	-0.0534	-0.0149	-0.0384	1.3705	0.36	
110	529+00.000	539+00.000	0.1894	13.262	14.355	0.5101	0.1129	0.3971	0.5521	0.1772	0.3749	-0.0420	-0.0643	0.0223	2,6933	0.70	
111	539+00.000	539+50.000	0.0095	0.299	0.742	0.0115	0.0045	0.0070	0.0285	0.0092	0.0194	-0.0170	-0.0047	-0.0124	1.2152	0.32	
112	539+50.000	540+00.000	0.0095	0.299	0.742	0.0115	0.0045	0.0070	0.0285	0.0092	0.0194	-0.0170	-0.0047	-0.0124	1.2152	0.32	
113	540+00.000	540+50.000	0.0095	0.299	0.742	0.0115	0.0045	0.0070	0.0285	0.0092	0.0194	-0.0170	-0.0047	-0.0124	1.2152	0.32	
114	540+50.000	540+74.370	0.0046	0.165	0.503	0.0063	0.0026	0.0038	0.0193	0.0062	0.0131	-0.0130	-0.0036	-0.0094	1.3705	0.36	
115	540+74.370	541+00.000	0.0049	0.161	0.427	0.0062	0.0024	0.0037	0.0164	0.0053	0.0111	-0.0102	-0.0028	-0.0074	1.2714	0.33	
116	541+00.000	541+50.000	0.0095	0.313	0.833	0.0120	0.0048	0.0073	0.0320	0.0103	0.0218	-0.0200	-0.0055	-0.0145	1.2714	0.33	
Vandemark/SD38 (v1)	541+50.000			28.334	74.904	1.0898	0.5033	0.5865	2.8809	1.2417	1.6393	-1.7912	-0.7384	-1.0527			0.27
117	541+50.000	541+70.000	0.0038	0.125	0.333	0.0048	0.0019	0.0029	0.0128	0.0041	0.0087	-0.0080	-0.0022	-0.0058	1.2714	0.33	

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi Ilion veh- mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
118	541+70.000	542+30.000	0.0114	0.376	1.000	0.0144	0.0057	0.0087	0.0384	0.0123	0.0261	-0.0240	-0.0066	-0.0174	1.2714	0.33	
119	542+30.000	542+64.000	0.0064	0.186	0.407	0.0071	0.0027	0.0044	0.0157	0.0050	0.0106	-0.0085	-0.0023	-0.0062	1.1086	0.29	
120	542+64.000	543+34.000	0.0133	0.382	0.838	0.0147	0.0056	0.0091	0.0322	0.0104	0.0219	-0.0175	-0.0047	-0.0128	1.1086	0.29	
121	543+34.000	544+00.000	0.0125	0.360	0.790	0.0139	0.0053	0.0086	0.0304	0.0098	0.0206	-0.0165	-0.0045	-0.0121	1.1086	0.29	
122	544+00.000	545+00.000	0.0189	0.566	1.300	0.0218	0.0084	0.0134	0.0500	0.0160	0.0339	-0.0282	-0.0076	-0.0206	1.1497	0.30	
123	545+00.000	548+23.000	0.0612	1.829	4.198	0.0703	0.0271	0.0432	0.1615	0.0518	0.1096	-0.0911	-0.0247	-0.0664	1.1497	0.30	
124	548+23.000	553+70.000	0.1036	10.151	7.109	0.3904	0.2627	0.1277	0.2734	0.0878	0.1857	0.1170	0.1750	-0.0580	3.7686	0.98	
125	553+70.000	554+00.000	0.0057	0.194	0.542	0.0074	0.0030	0.0045	0.0209	0.0067	0.0142	-0.0134	-0.0037	-0.0097	1.3100	0.34	
126	554+00.000	554+20.000	0.0038	0.129	0.361	0.0050	0.0020	0.0030	0.0139	0.0045	0.0094	-0.0089	-0.0025	-0.0065	1.3100	0.34	
127	554+20.000	560+00.000	0.1098	3.284	7.538	0.1263	0.0487	0.0776	0.2899	0.0931	0.1969	-0.1636	-0.0444	-0.1193	1.1497	0.30	
128	560+00.000	562+58.560	0.0490	1.464	3.360	0.0563	0.0217	0.0346	0.1292	0.0415	0.0878	-0.0729	-0.0198	-0.0532	1.1497	0.30	
129	562+58.560	564+00.000	0.0268	0.801	1.838	0.0308	0.0119	0.0189	0.0707	0.0227	0.0480	-0.0399	-0.0108	-0.0291	1.1497	0.30	
130	564+00.000	565+00.000	0.0189	0.566	1.300	0.0218	0.0084	0.0134	0.0500	0.0160	0.0339	-0.0282	-0.0076	-0.0206	1.1497	0.30	
131	565+00.000	565+77.000	0.0146	0.494	1.133	0.0190	0.0073	0.0117	0.0436	0.0140	0.0296	-0.0246	-0.0067	-0.0179	1.3020	0.30	
132	565+77.000	566+10.000	0.0063	0.241	0.675	0.0093	0.0037	0.0056	0.0260	0.0083	0.0176	-0.0167	-0.0046	-0.0121	1.4835	0.34	
2nd/SD38 (v2)	566+00.000			51.588	119.976	1.9842	0.7408	1.2433	4.6145	1.9888	2.6256	-2.6303	-1.2480	-1.3823			0.41
133	566+10.000	566+50.000	0.0076	0.292	0.819	0.0112	0.0045	0.0067	0.0315	0.0101	0.0214	-0.0203	-0.0056	-0.0146	1.4835	0.34	
134	566+50.000	569+37.000	0.0544	17.818	4.224	0.6853	0.2643	0.4210	0.1625	0.0522	0.1103	0.5228	0.2121	0.3107	12.6074	2.89	
135	569+37.000	569+70.000	0.0063	0.228	0.581	0.0088	0.0034	0.0053	0.0223	0.0072	0.0152	-0.0136	-0.0037	-0.0098	1.4018	0.32	
West Central School/SD38 (v2)	569+50.000			18.933	73.624	0.7282	0.3383	0.3899	2.8317	1.1752	1.6565	-2.1035	-0.8368	-1.2667			0.16
136	569+70.000	570+00.000	0.0057	0.192	0.442	0.0074	0.0029	0.0045	0.0170	0.0055	0.0115	-0.0096	-0.0026	-0.0070	1.3020	0.30	
137	570+00.000	575+00.000	0.0947	3.160	7.122	0.1215	0.0467	0.0748	0.2739	0.0879	0.1860	-0.1524	-0.0412	-0.1112	1.2834	0.29	
138	575+00.000	579+50.000	0.0852	2.885	6.623	0.1110	0.0428	0.0682	0.2547	0.0818	0.1730	-0.1438	-0.0390	-0.1048	1.3020	0.30	
139	579+50.000	579+70.000	0.0038	0.146	0.409	0.0056	0.0022	0.0034	0.0157	0.0051	0.0107	-0.0101	-0.0028	-0.0073	1.4835	0.34	
140	579+70.000	580+10.000	0.0076	0.292	0.819	0.0112	0.0045	0.0067	0.0315	0.0101	0.0214	-0.0203	-0.0056	-0.0146	1.4835	0.34	
All Segments			7.7398	213.597	230.845	8.2153	3.0105	5.2047	8.8786	2.8500	6.0286	-0.6634	0.1605	-0.8239	1.0614	0.68	
All Intersections				263.144	631.248	10.1209	4.4358	5.6852	24.2788	10.4147	13.8641	-14.1579	-5.9790	-8.1789			0.39
Total			7.7398	476.741	862.093	18.3362	7.4463	10.8899	33.1574	13.2648	19.8927	-14.8212	-5.8185	-9.0028	2.3691		

**Table 8. Expected Crash Frequencies and Rates by Horizontal Design Element (Section 1)** 

Title	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi Ilion veh- mi)
Tangent	171+44.000	174+52.690	0.0585	0.720	1.188	0.0277	0.0100	0.0177	0.0457	0.0147	0.0310	-0.0180	-0.0047	-0.0133	0.4733	0.44
Simple Curve 1	174+52.690	178+85.250	0.0819	1.254	2.459	0.0482	0.0180	0.0302	0.0946	0.0304	0.0642	-0.0464	-0.0123	-0.0340	0.5886	0.54
Tangent	178+85.250	230+66.250	0.9812	14.723	19.390	0.5663	0.1684	0.3979	0.7458	0.2394	0.5064	-0.1795	-0.0710	-0.1085	0.5771	0.53
Simple Curve 2	230+66.250	245+14.280	0.2742	6.908	6.555	0.2657	0.1459	0.1198	0.2521	0.0809	0.1712	0.0136	0.0649	-0.0513	0.9688	0.89
Tangent	245+14.280	263+22.600	0.3425	10.496	6.937	0.4037	0.0685	0.3352	0.2668	0.0856	0.1812	0.1369	-0.0172	0.1541	1.1787	1.09
Simple Curve 3	263+22.600	272+66.740	0.1788	2.351	4.061	0.0904	0.0329	0.0575	0.1562	0.0501	0.1060	-0.0657	-0.0172	-0.0485	0.5057	0.47
Tangent	272+66.740	296+96.470	0.4602	8.432	9.130	0.3243	0.0823	0.2420	0.3511	0.1127	0.2384	-0.0268	-0.0304	0.0036	0.7047	0.65
Simple Curve 4	296+96.470	296+96.520	0.0000	0.000	0.000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	-0.0000	-0.0000	-0.0000	0.4646	0.43
Tangent	296+96.520	329+81.740	0.6222	7.516	12.258	0.2891	0.1040	0.1851	0.4715	0.1513	0.3201	-0.1824	-0.0474	-0.1350	0.4646	0.43
Simple Curve 5	329+81.740	335+39.960	0.1057	8.323	2.828	0.3201	0.0246	0.2956	0.1088	0.0349	0.0738	0.2114	-0.0103	0.2217	3.0280	2.79
Tangent	335+39.960	369+14.990	0.6392	13.471	12.739	0.5181	0.1961	0.3220	0.4900	0.1573	0.3327	0.0282	0.0389	-0.0107	0.8106	0.75
Simple Curve 6	369+14.990	379+62.690	0.1984	6.109	5.131	0.2350	0.1376	0.0974	0.1973	0.0633	0.1340	0.0376	0.0742	-0.0366	1.1841	1.09
Tangent	379+62.690	385+22.970	0.1061	4.133	2.091	0.1590	0.1024	0.0566	0.0804	0.0258	0.0546	0.0785	0.0766	0.0020	1.4979	1.38
Simple Curve 7	385+22.970	405+75.410	0.3887	5.424	9.817	0.2086	0.0767	0.1319	0.3776	0.1212	0.2564	-0.1690	-0.0445	-0.1245	0.5366	0.50
Tangent	405+75.410	520+49.150	2.1731	57.544	50.368	2.2132	0.8301	1.3832	1.9372	0.6218	1.3154	0.2760	0.2082	0.0678	1.0185	0.88
Simple Curve 8	520+49.150	540+74.370	0.3836	19.560	30.871	0.7523	0.2085	0.5438	1.1874	0.3811	0.8062	-0.4350	-0.1726	-0.2624	1.9614	0.51
Tangent	540+74.370	580+10.000	0.7454	46.632	55.023	1.7936	0.8047	0.9889	2.1163	0.6793	1.4370	-0.3227	0.1253	-0.4481	2.4062	0.58

Table 9. Predicted Crash Frequencies by Year (Section 1)

Year	Total Crashes	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
2025	21.56	8.58	39.798	12.98	60.202
2026	23.15	9.22	39.843	13.93	60.157
2027	24.72	9.86	39.881	14.86	60.119
2028	26.28	10.49	39.913	15.79	60.087
2029	27.83	11.12	39.940	16.71	60.060
2030	28.45	11.37	39.948	17.09	60.052
2031	29.08	11.62	39.955	17.46	60.045
2032	29.71	11.87	39.963	17.84	60.037
2033	30.34	12.13	39.971	18.21	60.029
2034	30.97	12.38	39.978	18.59	60.022
2035	31.61	12.64	39.985	18.97	60.015
2036	32.24	12.89	39.992	19.35	60.008
2037	32.88	13.15	39.999	19.73	60.001
2038	33.52	13.41	40.006	20.11	59.994
2039	34.16	13.67	40.012	20.49	59.988
2040	34.80	13.93	40.019	20.87	59.981
2041	35.57	14.24	40.027	21.33	59.973
2042	36.35	14.55	40.035	21.80	59.965
2043	37.12	14.87	40.043	22.26	59.957
2044	37.90	15.18	40.051	22.72	59.949
2045	38.68	15.49	40.058	23.19	59.942
2046	39.47	15.81	40.066	23.65	59.934
2047	40.25	16.13	40.073	24.12	59.927
2048	41.03	16.45	40.080	24.59	59.920
2049	41.82	16.76	40.087	25.05	59.913
2050	42.61	17.09	40.094	25.53	59.906
Total	862.09	344.88	40.005	517.21	59.995
Average	33.16	13.27	40.005	19.89	59.995

**Table 10. Expected Crash Frequencies by Year (Section 1)** 

Year	Total Crashes	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
2025	11.92	4.82	40.399	7.11	59.596
2026	12.80	5.18	40.445	7.62	59.551
2027	13.67	5.53	40.483	8.14	59.514
2028	14.53	5.89	40.516	8.64	59.482
2029	15.39	6.24	40.543	9.15	59.455
2030	15.73	6.38	40.551	9.35	59.447
2031	16.08	6.52	40.559	9.56	59.440
2032	16.43	6.67	40.567	9.77	59.432
2033	16.78	6.81	40.574	9.97	59.425
2034	17.13	6.95	40.582	10.18	59.418
2035	17.48	7.09	40.589	10.38	59.410
2036	17.83	7.24	40.596	10.59	59.404
2037	18.18	7.38	40.603	10.80	59.397
2038	18.54	7.53	40.610	11.01	59.390
2039	18.89	7.67	40.617	11.22	59.384
2040	19.25	7.82	40.624	11.43	59.377
2041	19.67	7.99	40.632	11.68	59.369
2042	20.10	8.17	40.640	11.93	59.361
2043	20.53	8.34	40.648	12.18	59.353
2044	20.96	8.52	40.656	12.44	59.345
2045	21.39	8.70	40.664	12.69	59.338
2046	21.82	8.88	40.671	12.95	59.330
2047	22.26	9.05	40.678	13.20	59.323
2048	22.69	9.23	40.686	13.46	59.316
2049	23.13	9.41	40.693	13.72	59.309
2050	23.56	9.59	40.700	13.97	59.302
Total	476.74	193.60	40.610	283.14	59.390
Average	18.34	7.45	40.610	10.89	59.390

Table 11. Comparing Predicted and Expected Crashes for the Evaluation Period (Section 1)

Scope	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
Predicted	862.09	344.88	40.005	517.21	59.995
Expected	476.74	193.60	40.610	283.14	59.390
Expected - Predicted	-385.35	-151.28		-234.07	
Percent Difference	-80.83	-78.14		-82.67	

**Table 12. Expected Crash Type Distribution (Section 1)** 

Element Type	Crash Type	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)	Total Crashes	Percent Total (%)
Highway Segment	Collision with Animal	2.97	0.6	24.90	5.2	25.84	5.4
Highway Segment	Collision with Bicycle	0.31	0.1	0.14	0.0	0.43	0.1
Highway Segment	Other Single-vehicle Collision	0.55	0.1	3.92	0.8	4.49	0.9
Highway Segment	Overturned	2.90	0.6	2.03	0.4	5.34	1.1
Highway Segment	Collision with Pedestrian	0.55	0.1	0.14	0.0	0.64	0.1
Highway Segment	Run Off Road	42.66	9.0	68.34	14.3	111.28	23.4
Highway Segment	Total Single Vehicle Crashes	49.94	10.5	99.46	20.9	148.02	31.1
Highway Segment	Angle Collision	7.91	1.7	9.74	2.0	18.16	3.8
Highway Segment	Head-on Collision	2.66	0.6	0.41	0.1	3.42	0.7
Highway Segment	Other Multiple-vehicle Collision	2.04	0.4	4.06	0.9	5.77	1.2
Highway Segment	Rear-end Collision	12.91	2.7	16.51	3.5	30.33	6.4
Highway Segment	Sideswipe	2.97	0.6	5.14	1.1	7.90	1.7
Highway Segment	Total Multiple Vehicle Crashes	28.49	6.0	35.86	7.5	65.57	13.8
Highway Segment	Total Highway Segment Crashes	78.43	16.5	135.32	28.4	213.60	44.8
Intersection	Collision with Animal	0.71	0.1	2.22	0.5	2.84	0.6
Intersection	Collision with Bicycle	0.12	0.0	0.15	0.0	0.26	0.1
Intersection	Other Single-vehicle Collision	0.54	0.1	1.61	0.3	2.10	0.4
Intersection	Overturned	0.86	0.2	0.63	0.1	1.50	0.3
Intersection	Collision with Pedestrian	0.12	0.0	0.15	0.0	0.26	0.1
Intersection	Run Off Road	12.42	2.6	22.60	4.7	34.98	7.3
Intersection	Total Single Vehicle Crashes	14.77	3.1	27.36	5.7	41.97	8.8
Intersection	Angle Collision	58.58	12.3	50.48	10.6	108.83	22.8
Intersection	Head-on Collision	7.15	1.5	3.79	0.8	10.81	2.3
Intersection	Other Multiple-vehicle Collision	4.93	1.0	5.42	1.1	10.33	2.2
Intersection	Rear-end Collision	24.76	5.2	39.65	8.3	64.53	13.5
Intersection	Sideswipe	5.15	1.1	21.12	4.4	26.48	5.6
Intersection	Total Multiple Vehicle Crashes	100.56	21.1	120.46	25.3	220.99	46.4
Intersection	Total Intersection Crashes	115.33	24.2	147.81	31.0	262.95	55.2
	Total Crashes	193.76	40.7	283.14	59.4	476.55	100.0

**Table 13. Evaluation Message** 

Start Location (Sta. ft)	End Location (Sta. ft)	Message
580+00.000	580+00.000	Warning: for intersection #3 (580+00.000 to 580+00.000 ), SE SD-38 at 580+00.000 has more than one lane exiting. No intersection crash prediction computed.
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,560 vpd) for 2018 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,560 vpd) for 2019 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,560 vpd) for 2020 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,560 vpd) for 2021 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,560 vpd) for 2022 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
580+00.000	580+00.000	Warning: for intersection #3 (580+00.000 to 580+00.000 ), SE SD-38 at 580+00.000 has more than one lane exiting. No intersection crash prediction computed.
187+50.000	187+50.000	Warning: for intersection #5 (187+50.000 to 187+50.000), minor road traffic volume (3,503 vpd) for 2049 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
187+50.000	187+50.000	Warning: for intersection #5 (187+50.000 to 187+50.000), minor road traffic volume (3,570 vpd) for 2050 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,802 vpd) for 2025 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,882 vpd) for 2026 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,963 vpd) for 2027 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,044 vpd) for 2028 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,125 vpd) for 2029 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,221 vpd) for 2030 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,318 vpd) for 2031 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,415 vpd) for 2032 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000), minor road traffic volume (4,512 vpd) for 2033 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST

Start Location (Sta. ft)	End Location (Sta. ft)	Message
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,609 vpd) for 2034 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,705 vpd) for 2035 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,802 vpd) for 2036 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,899 vpd) for 2037 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,996 vpd) for 2038 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,093 vpd) for 2039 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,190 vpd) for 2040 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,308 vpd) for 2041 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,427 vpd) for 2042 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,545 vpd) for 2043 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,664 vpd) for 2044 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,782 vpd) for 2045 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,901 vpd) for 2046 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (6,019 vpd) for 2047 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (6,138 vpd) for 2048 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (6,256 vpd) for 2049 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (6,375 vpd) for 2050 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST

## Interactive Highway Safety Design Model

# **Crash Prediction Evaluation Report**

#### Disclaimer

The Interactive Highway Design Model (IHSDM) software is disseminated under the sponsorship of the Department of Transportation in the interest of information exchange. The United States Government assumes no liability for its content or use thereof. This document does not constitute a standard, specification, or regulation.

The United States Government does not endorse products or manufacturers. Trade and manufacturers' names may appear in this software and documentation only because they are considered essential to the objective of the software.

#### **Limited Warranty and Limitations of Remedies**

This software product is provided "as-is," without warranty of any kind-either expressed or implied (but not limited to the implied warranties of merchantability and fitness for a particular purpose). The FHWA do not warrant that the functions contained in the software will meet the end-user's requirements or that the operation of the software will be uninterrupted and error-free.

Under no circumstances will the FHWA be liable to the end-user for any damages or claimed lost profits, lost savings, or other incidental or consequential damages rising out of the use or inability to use the software (even if these organizations have been advised of the possibility of such damages), or for any claim by any other party.

#### **Notice**

The use of the IHSDM software is being done strictly on a voluntary basis. In exchange for provision of IHSDM, the user agrees that the Federal Highway Administration (FHWA), U.S. Department of Transportation and any other agency of the Federal Government shall not be responsible for any errors, damage or other liability that may result from any and all use of the software, including installation and testing of the software. The user further agrees to hold the FHWA and the Federal Government harmless from any resulting liability. The user agrees that this hold harmless provision shall flow to any person to whom or any entity to which the user provides the IHSDM software. It is the user's full responsibility to inform any person to whom or any entity to which it provides the IHSDM software of this hold harmless provision.

## **Table of Contents**

Report Overview	1
Disclaimer Regarding Crash Prediction Method	2
Section Types	3
Section 3 Evaluation	3
Section 4 Evaluation	40
List of Tables	
Table Observed Crashes Used in the Evaluation (Section 3)	5
Table Evaluation Highway - Homogeneous Segments (Section 3)	6
Table User Defined CMF Used in the Eval Segment CPM Evaluation (Section 3)	14
Table Crash History Highway - Homogeneous Segments (Section 3)	15
Table Evaluation Intersection (Section 3)	18
Table Evaluation Intersection (Section 3)	19
Table Evaluation Ramp Terminal - Site (Section 3)	20
Table Crash History Intersection (Section 3)	21
Table Crash History Intersection (Section 3)	22
Table Crash Highway Ramp Terminal - Site (Highway with Crash History)	23
Table Expected Highway Crash Rates and Frequencies Summary (Section 3)	24
Table Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 3)	25
Table Expected Crash Frequencies and Rates by Horizontal Design Element (Section 3)	29
Table Predicted Crash Frequencies by Year (Section 3)	30
Table Expected Crash Frequencies by Year (Section 3)	31
Table Comparing Predicted and Expected Crashes for the Evaluation Period (Section 3)	32
Table Expected Crash Severity by Ramp Terminal or Roundabout (Section 3)	32
Table Expected Crash Type Distribution (Section 3)	33
Table Evaluation Message	35
Table Observed Crashes Used in the Evaluation (Section 4)	42
Table Evaluation Highway - Homogeneous Segments (Section 4)	43
Table Crash Highway Highway - Homogeneous Segments (Section 4)	45
Table Evaluation Intersection (Section 4)	46
Table Crash History Intersection (Section 4)	47
Table Expected Highway Crash Rates and Frequencies Summary (Section 4)	48
Table Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 4)	49
Table Expected Crash Frequencies and Rates by Horizontal Design Element (Section 4)	50
Table Predicted Crash Frequencies by Year (Section 4)	51
Table Expected Crash Frequencies by Year (Section 4)	52

Table Comparing Predicted and Expected Crashes for the Evaluation Period (Section 4)	53
Table Expected Five Lane or Fewer Crash Type Distribution (Section 4)	54
List of Figures	
Figure Crash Prediction Summary (Section 3)	4
Figure Crash Prediction Summary (Section 4)	41

### **Report Overview**

Report Generated: Jun 10, 2024 9:25 AM

Report Template: System: Single Page, 508 Compliant [System] (mlcpm5, Dec 5, 2019 2:16 PM)

Evaluation Date: Mon Jun 10 09:24:09 CDT 2024

**IHSDM Version:** v17.0.0 (Sep 22, 2021)

Crash Prediction Module: v12.0.0 (Sep 22, 2021)

User Name: naveen.mallipaddi

**Organization Name:** 

Phone: E-Mail:

Project Title: SD-38\_Build\_Option2\_I90EBRamp\_I

Project Comment: Created Mon Mar 27 16:47:43 CDT 2023

Project Unit System: U.S. Customary

**Highway Title: SD-38** 

Highway Comment: Created Mon Mar 27 16:49:47 CDT 2023

**Highway Version: 20** 

**Evaluation Title:** Evaluation 55

Evaluation Comment: Created Mon Jun 10 09:16:50 CDT 2024

**Minimum Location:** 585+00.000 **Maximum Location:** 974+11.000

Policy for Superelevation: AASHTO 2011 U.S. Customary

Calibration: HSM Configuration

Crash Distribution: HSM Configuration Model/CMF: HSM Configuration

First Year of Analysis: 2025 Last Year of Analysis: 2050

**Empirical-Bayes Analysis:** Site-Specific **Highway with Crash History:** SD-38

Highway with Crash History Comment: Created Mon Mar 27 16:49:47 CDT 2023

Highway with Crash History Version: 20 First Year of Observed Crashes: 2018 Last Year of Observed Crashes: 2022

## **Disclaimer Regarding Crash Prediction Method**

IMPORTANT NOTICE ABOUT COMPARING RESULTS FROM HIGHWAY SAFETY MANUAL FIRST EDITION (2010) MODELS TO RESULTS FROM NEW MODELS DEVELOPED UNDER NCHRP PROJECTS 17-70, 17-58, AND 17-68

Since the publication of the Highway Safety Manual - First Edition (HSM-1), in 2010 by the American Association of State Highway and Transportation Officials (AASHTO), multiple research efforts have been undertaken through the National Cooperative Highway Research Program (NCHRP) to develop safety performance models for road segment and intersection facility types that were not initially reflected in the HSM-1, in order to expand the breadth and depth of the HSM in the future.

The IHSDM Crash Prediction Module (CPM) is intended as a faithful implementation of HSM Part C predictive methods. As NCHRP projects to develop new predictive methods for the HSM are completed, FHWA works to incorporate the new methods into IHSDM, sometimes in advance of publication in the HSM. The following new crash predictive methods have been accepted by NCHRP project panels and incorporated into IHSDM, while pending AASHTO's approval for incorporation into a future edition of the HSM:

- Roundabouts: completed in 2018 under NCHRP Project 17-70, the new methods will provide improved outcomes for the safety analysis of roundabouts.
- 6+ lane and one-way urban/suburban arterials (including models for segments and intersections): completed under NCHRP Project 17-58.
- Intersection crash prediction methods for some intersection configurations and traffic control types not currently addressed in the HSM (e.g., all-way stop; rural 3-leg signalized; 3-leg stop-controlled where the major leg turns; urban 5-leg signalized; urban high-speed intersections): completed in 2021 under NCHRP Project 17-68.

However, in the absence of local calibration factors (see HSM-1 Part C, Appendix A for guidance on calibration of the predictive models), it is neither appropriate nor advisable to directly compare the results from new models (from NCHRP Projects 17-58, 17-68, and 17-70) to results from HSM-1 models, as the models were not calibrated to the same base state data sets, and consequently can produce unexpected results. If local calibration factors are available and applied to both new models and HSM-1 models, then it may be appropriate to directly compare the results. [Note: Work being performed under NCHRP Project 17-72 (Update of Crash Modification Factors for the Highway Safety Manual) is expected to re-calibrate many of the old (HSM-1) and new (e.g., NCHRP 17-70) models to data from a single (or small number of) states, that would allow results from all models to be directly compared.]

The models produced for NCHRP Project 17-70 have independent value in terms of informing the design of a roundabout and assessing the effects of different design characteristics on the expected safety performance of a roundabout.

The HSM-1 interim method previously included in IHSDM for evaluating roundabouts on urban/suburban arterials (i.e., evaluating an existing intersection and then applying a Crash Modification Factor for replacing the existing intersection with a roundabout) has been deactivated in IHSDM, to minimize any confusion with the new roundabout methodology.

## **Section Types**

#### **Section 3 Evaluation**

**Section:** Section 3

**Evaluation Start Location:** 585+00.000 **Evaluation End Location:** 948+50.000

Area Type: Rural

Functional Class: Arterial

Type of Alignment: Undivided/Divided Multilane

Model Category: Rural, Multilane

**Calibration Factor:** 3ST=1.0; 4D=1.0; 4ST=1.0; 4U=1.0; RT\_ST\_FI=1.0; RT\_ST\_PDO=1.0;

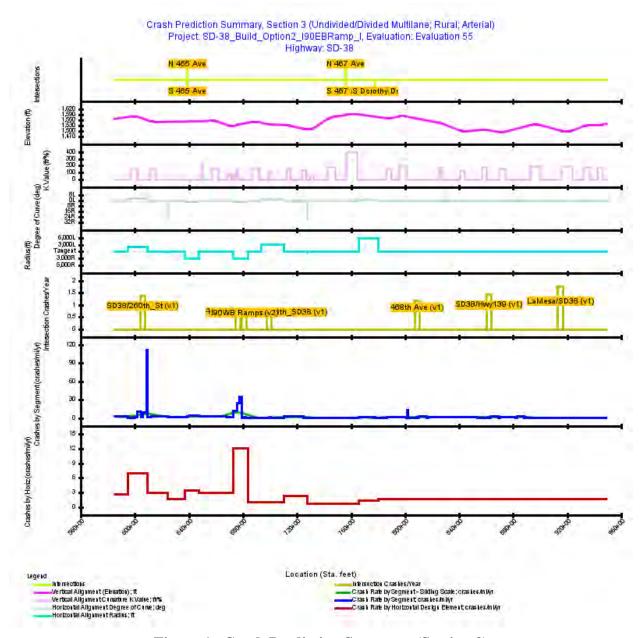


Figure 1. Crash Prediction Summary (Section 3)

**Table 1. Observed Crashes Used in the Evaluation (Section 3)** 

Year	Observed Crashes	Total Crashes Used	FI Crashes	FI no/C Crashes	PDO Crashes
2018	8	8	6	5	2
2019	10	10	4	0	6
2020	7	7	3	2	4
2021	9	9	5	2	4
2022	9	9	5	1	4
All Years	43 <sup>[1]</sup>	43	23	10	20

#### **Footnotes**

<sup>[1]</sup> Note: Observed crash data that does not comply with the associated CPM model requirements may not be used in EB processing.

 Table 2. Evaluation Highway - Homogeneous Segments (Section 3)

leg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Side
1	Rural Multi-Lane Segment Four- lane Undivided	585+00.00 0	594+84.94 0	984.94	0.1865	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,500; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	
2	Rural Multi-Lane Segment Four- lane Undivided	594+84.94 0	600+00.00 0	515.06	0.0975	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,500; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	. (
3	Rural Multi-Lane Segment Four- lane Undivided	600+00.00 0	601+00.00 0	100.00	0.0189	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,500; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1
4	Rural Multi-Lane Segment Four- lane Undivided	601+00.00 0	602+00.00 0	100.00	0.0189	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,500; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	I
5	Rural Multi-Lane Segment Four- lane Undivided	602+00.00 0	605+00.00 0	300.00	0.0568	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,599	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	I
6	Rural Multi-Lane Segment Four- lane Undivided	605+00.00 0	605+60.00 0	60.00	0.0114	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,599	12.00	12.00	8.00	0.00	0.00	None	0.00	false	false	0:1	I
7	Rural Multi-Lane Segment Four- lane Undivided	605+60.00 0	605+70.00 0	10.00	0.0019	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1
8	Rural Multi-Lane Segment Four- lane Undivided	605+70.00 0	605+75.00	5.00	0.0009	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	l
9	Rural Multi-Lane Segment Four- lane Undivided	605+75.00 0	609+00.00	325.00	0.0616	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1
10	Rural Multi-Lane Segment Four- lane Undivided	609+00.00 0	609+21.93 0	21.93	0.0042	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1
11	Rural Multi-Lane Segment Four- lane Undivided	609+21.93 0	611+40.00	218.07	0.0413	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1
12	Rural Multi-Lane Segment Four- lane Undivided	611+40.00 0	612+50.00 0	110.00	0.0208	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,599	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	I
13	Rural Multi-Lane Segment Four- lane Undivided	612+50.00 0	624+64.53 0	1,214.5 3	0.2300	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	ı
14	Rural Multi-Lane Segment Four- lane Undivided	624+64.53 0	636+92.82	1,228.2 9	0.2326	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,024; 2040: 15,920; 2041: 16,287; 2042: 16,054; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	l

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
15	Rural Multi-Lane Segment Four- lane Undivided	636+92.82 0	639+00.00 0	207.18	0.0392	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
16	Rural Multi-Lane Segment Four- lane Undivided	639+00.00 0	640+00.00 0	100.00	0.0189	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1
17	Rural Multi-Lane Segment Four- lane Undivided	640+00.00 0	647+26.05 0	726.05	0.1375	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033: 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
18	Rural Multi-Lane Segment Four- lane Undivided	647+26.05 0	667+80.00 0	2,053.9 5	0.3890	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033: 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
19	Rural Multi-Lane Segment Four- lane Undivided	667+80.00 0	668+80.00 0	100.00	0.0189	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033: 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	0.00	0.00	None	0.00	false	false	0:1	0:1
20	Rural Multi-Lane Segment Four- lane Undivided	668+80.00 0	672+86.11 0	406.11	0.0769	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
21	Rural Multi-Lane Segment Four- lane Undivided	672+86.11 0	676+00.00 0	313.89	0.0594	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
22	Rural Multi-Lane Segment Four- lane Undivided	676+00.00 0	677+50.00 0	150.00	0.0284	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	0.00	8.00	0.00	None	0.00	false	false	0:1	0:1
23	Rural Multi-Lane Segment Four- lane Undivided	677+50.00 0	679+00.00 0	150.00	0.0284	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,529; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
24	Rural Multi-Lane Segment Four- lane Undivided	679+00.00 0	680+80.00 0	180.00	0.0341	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127: 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
25	Rural Multi-Lane Segment Four- lane Undivided	680+80.00 0	680+90.00 0	10.00	0.0019	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127: 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	0.00	8.00	0.00	None	0.00	false	false	0:1	0:1
26	Rural Multi-Lane Segment Four- lane Undivided	680+90.00 0	682+20.00 0	130.00	0.0246	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127: 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	0.00	8.00	0.00	None	0.00	false	false	0:1	0:1
27	Rural Multi-Lane Segment Four- lane Undivided	682+20.00 0	683+82.71 0	162.71	0.0308	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
28	Rural Multi-Lane Segment Four- lane Undivided	683+82.71 0	691+50.00 0	767.29	0.1453	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
29	Rural Multi-Lane Segment Four- lane Undivided	691+50.00 0	692+70.00 0	120.00	0.0227	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Righ Side Slop
30	Rural Multi-Lane Segment Four- lane Undivided	692+70.00 0	693+85.01 0	115.01	0.0218	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 0
31	Rural Multi-Lane Segment Four- lane Undivided	693+85.01 0	698+70.00 0	484.99	0.0919	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1 (
32	Rural Multi-Lane Segment Four- lane Undivided	698+70.00 0	699+00.00 0	30.00	0.0057	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	0.00	8.00	0.00	None	0.00	false	false	0:1	. (
33	Rural Multi-Lane Segment Four- lane Undivided	699+00.00 0	699+20.00 0	20.00	0.0038	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	. (
34	Rural Multi-Lane Segment Four- lane Undivided	699+20.00 0	700+40.00 0	120.00	0.0227	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453: 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	. (
35	Rural Multi-Lane Segment Four- lane Divided	700+40.00 0	700+50.00 0	10.00	0.0019	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	0.00	0.00	8.00	Traversable Median	8.00	false	false		
36	Rural Multi-Lane Segment Four- lane Divided	700+50.00 0	701+10.00 0	60.00	0.0114	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	0.00	8.00	Traversable Median	8.00	false	false		
37	Rural Multi-Lane Segment Four- lane Divided	701+10.00 0	702+00.00 0	90.00	0.0170	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	0.00	8.00	Non-Traversable Median	8.00	false	false		
38	Rural Multi-Lane Segment Four- lane Divided	702+00.00 0	702+50.00 0	50.00	0.0095	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	0.00	8.00	Non-Traversable Median	8.00	false	false		
39	Rural Multi-Lane Segment Four- lane Divided	702+50.00 0	707+00.00 0	450.00	0.0852	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
40	Rural Multi-Lane Segment Four- lane Divided	707+00.00 0	708+00.00 0	100.00	0.0189	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
41	Rural Multi-Lane Segment Four- lane Divided	708+00.00 0	708+80.00 0	80.00	0.0152	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
42	Rural Multi-Lane Segment Four- lane Divided	708+80.00 0	709+00.00 0	20.00	0.0038	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
43	Rural Multi-Lane Segment Four- lane Undivided	709+00.00 0	710+30.00 0	130.00	0.0246	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	
44	Rural Multi-Lane Segment Four- lane Divided	710+30.00 0	710+47.85 0	17.85	0.0034	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
45	Rural Multi-Lane Segment Four- lane Divided	710+47.85 0	725+00.00 0	1,452.1 5	0.2750	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
46	Rural Multi-Lane Segment Four- lane Divided	725+00.00 0	727+52.35 0	252.35	0.0478	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
47	Rural Multi-Lane Segment Four- lane Divided	727+52.35 0	735+00.00 0	747.65	0.1416	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
48	Rural Multi-Lane Segment Four- lane Divided	735+00.00 0	755+50.00 0	2,050.0 0	0.3883	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
49	Rural Multi-Lane Segment Four- lane Undivided	755+50.00 0	756+90.00 0	140.00	0.0265	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,662; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
50	Rural Multi-Lane Segment Four- lane Divided	756+90.00 0	757+00.00 0	10.00	0.0019	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
51	Rural Multi-Lane Segment Four- lane Divided	757+00.00 0	763+30.00 0	630.00	0.1193	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
52	Rural Multi-Lane Segment Four- lane Divided	763+30.00 0	764+00.00 0	70.00	0.0133	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	11.50	Non-Traversable Median	11.50	false	false		
53	Rural Multi-Lane Segment Four- lane Divided	764+00.00 0	764+50.00 0	50.00	0.0095	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	17.50	Non-Traversable Median	17.50	false	false		
54	Rural Multi-Lane Segment Four- lane Divided	764+50.00 0	765+52.55 0	102.55	0.0194	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
55	Rural Multi-Lane Segment Four- lane Divided	765+52.55 0	777+80.00 0	1,227.4 5	0.2325	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
56	Rural Multi-Lane Segment Four- lane Undivided	777+80.00 0	778+80.00 0	100.00	0.0189	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
57	Rural Multi-Lane Segment Four- lane Divided	778+80.00 0	779+00.00 0	20.00	0.0038	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383: 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
58	Rural Multi-Lane Segment Four- lane Divided	779+00.00 0	780+45.93 0	145.93	0.0276	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383: 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
59	Rural Multi-Lane Segment Four- lane Divided	780+45.93 0	785+40.00 0	494.07	0.0936	2025; 5,804; 2026; 6,105; 2027; 6,407; 2028; 6,708; 2029; 7,010; 2030; 7,134; 2031; 7,259; 2032; 7,383; 2033; 7,508; 2034; 7,632; 2035; 7,757; 2036; 7,881; 2037; 8,006; 2038; 8,130; 2039; 8,255; 2040; 8,380; 2041; 8,528; 2042; 8,676; 2043; 8,824; 2044; 8,972; 2045; 9,120; 2046; 9,268; 2047; 9,416; 2048; 9,564; 2049; 9,712; 2050; 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Side
60	Rural Multi-Lane Segment Four- lane Divided	785+40.00 0	785+50.00 0	10.00	0.0019	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
61	Rural Multi-Lane Segment Four- lane Divided	785+50.00 0	786+09.00 0	59.00	0.0112	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	11.54	Non-Traversable Median	11.54	false	false		
62	Rural Multi-Lane Segment Four- lane Divided	786+09.00 0	786+50.00 0	41.00	0.0078	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	17.54	Non-Traversable Median	17.54	false	false		
63	Rural Multi-Lane Segment Four- lane Divided	786+50.00 0	801+10.00 0	1,460.0 0	0.2765	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757: 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
64	Rural Multi-Lane Segment Four- lane Divided	801+10.00 0	801+61.00 0	51.00	0.0097	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757: 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	17.45	Non-Traversable Median	17.45	false	false		
65	Rural Multi-Lane Segment Four- lane Divided	801+61.00 0	802+30.00 0	69.00	0.0131	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	11.45	Non-Traversable Median	11.45	false	false		
66	Rural Multi-Lane Segment Four- lane Divided	802+30.00 0	802+40.00 0	10.00	0.0019	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
67	Rural Multi-Lane Segment Four- lane Divided	802+40.00 0	808+30.00 0	590.00	0.1117	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,242; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
68	Rural Multi-Lane Segment Four- lane Divided	808+30.00 0	808+80.00 0	50.00	0.0095	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,662; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
69	Rural Multi-Lane Segment Four- lane Undivided	808+80.00 0	809+00.00 0	20.00	0.0038	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,242; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	l
70	Rural Multi-Lane Segment Four- lane Undivided	809+00.00 0	809+60.00 0	60.00	0.0114	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	ı
71	Rural Multi-Lane Segment Four- lane Divided	809+60.00 0	810+00.00 0	40.00	0.0076	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	0.00	0.00	8.00	Traversable Median	20.00	false	false		
72	Rural Multi-Lane Segment Four- lane Divided	810+00.00 0	810+20.00 0	20.00	0.0038	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
73	Rural Multi-Lane Segment Four- lane Divided	810+20.00 0	816+00.00 0	580.00	0.1098	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
74	Rural Multi-Lane Segment Four- lane Divided	816+00.00 0	816+70.00 0	70.00	0.0133	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	11.50	Non-Traversable Median	11.50	false	false		

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
75	Rural Multi-Lane Segment Four- lane Divided	816+70.00 0	817+20.00 0	50.00	0.0095	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600: 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	17.50	Non-Traversable Median	17.50	false	false		
76	Rural Multi-Lane Segment Four- lane Divided	817+20.00 0	853+70.00 0	3,650.0 0	0.6913	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
77	Rural Multi-Lane Segment Four- lane Divided	853+70.00 0	854+00.00 0	30.00	0.0057	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	18.36	Non-Traversable Median	18.36	false	false		
78	Rural Multi-Lane Segment Four- lane Divided	854+00.00 0	854+16.00 0	16.00	0.0030	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	15.85	Non-Traversable Median	15.85	false	false		
79	Rural Multi-Lane Segment Four- lane Divided	854+16.00 0	854+80.00 0	64.00	0.0121	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	11.49	Non-Traversable Median	11.49	false	false		
80	Rural Multi-Lane Segment Four- lane Divided	854+80.00 0	860+90.00 0	610.00	0.1155	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
81	Rural Multi-Lane Segment Four- lane Divided	860+90.00 0	861+85.00 0	95.00	0.0180	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110: 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
82	Rural Multi-Lane Segment Four- lane Undivided	861+85.00 0	862+00.00 0	15.00	0.0028	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	0.00	0.00	None	0.00	false	false	0:1	0:1
83	Rural Multi-Lane Segment Four- lane Undivided	862+00.00 0	862+50.00 0	50.00	0.0095	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	0.00	0.00	None	0.00	false	false	0:1	0:1
84	Rural Multi-Lane Segment Four- lane Undivided	862+50.00 0	862+60.00 0	10.00	0.0019	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:
85	Rural Multi-Lane Segment Four- lane Divided	862+60.00 0	863+10.00 0	50.00	0.0095	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	8.00	Traversable Median	19.00	false	false		
86	Rural Multi-Lane Segment Four- lane Divided	863+10.00 0	869+00.00 0	590.00	0.1117	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	19.00	false	false		
87	Rural Multi-Lane Segment Four- lane Divided	869+00.00 0	869+70.00 0	70.00	0.0133	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	11.50	Non-Traversable Median	11.50	false	false		
88	Rural Multi-Lane Segment Four- lane Divided	869+70.00 0	870+20.00 0	50.00	0.0095	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	17.50	Non-Traversable Median	17.50	false	false		
89	Rural Multi-Lane Segment Four- lane Divided	870+20.00 0	881+80.00 0	1,160.0 0	0.2197	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
90	Rural Multi-Lane Segment Four- lane Divided	881+80.00 0	882+31.00 0	51.00	0.0097	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	17.45	Non-Traversable Median	17.45	false	false		
91	Rural Multi-Lane Segment Four- lane Divided	882+31.00 0	883+00.00 0	69.00	0.0131	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222: 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	11.45	Non-Traversable Median	11.45	false	false		
92	Rural Multi-Lane Segment Four- lane Divided	883+00.00 0	887+90.00 0	490.00	0.0928	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222: 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
93	Rural Multi-Lane Segment Four- lane Divided	887+90.00 0	888+20.00 0	30.00	0.0057	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222: 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964: 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
94	Rural Multi-Lane Segment Four- lane Undivided	888+20.00 0	889+30.00 0	110.00	0.0208	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222: 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964: 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
95	Rural Multi-Lane Segment Four- lane Divided	889+30.00 0	889+50.00 0	20.00	0.0038	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222: 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964: 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
96	Rural Multi-Lane Segment Four- lane Divided	889+50.00 0	894+50.00 0	500.00	0.0947	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222: 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
97	Rural Multi-Lane Segment Four- lane Divided	894+50.00 0	895+15.00 0	65.00	0.0123	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222: 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	11.55	Non-Traversable Median	11.55	false	false		
98	Rural Multi-Lane Segment Four- lane Divided	895+15.00 0	895+60.00 0	45.00	0.0085	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	17.55	Non-Traversable Median	17.55	false	false		
99	Rural Multi-Lane Segment Four- lane Divided	895+60.00 0	898+00.00 0	240.00	0.0455	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
100	Rural Multi-Lane Segment Four- lane Divided	898+00.00 0	906+70.00 0	870.00	0.1648	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222: 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	11.50	11.50	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
101	Rural Multi-Lane Segment Four- lane Divided	906+70.00 0	907+21.00 0	51.00	0.0097	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222: 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964: 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	11.50	11.50	8.00	8.00	17.45	Non-Traversable Median	17.45	false	false		
102	Rural Multi-Lane Segment Four- lane Divided	907+21.00 0	907+80.00 0	59.00	0.0112	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222: 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,609; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	11.50	11.50	8.00	8.00	11.95	Non-Traversable Median	11.95	false	false		
103	Rural Multi-Lane Segment Four- lane Divided	907+80.00 0	907+90.00 0	10.00	0.0019	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222: 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,609; 2047: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	11.50	11.50	8.00	8.00	8.50	Non-Traversable Median	19.50	false	false		
104	Rural Multi-Lane Segment Four- lane Divided	907+90.00	913+70.00 0	580.00	0.1098	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	11.50	11.50	8.00	8.00	8.00	Non-Traversable Median	19.00	false	false		

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Side
105	Rural Multi-Lane Segment Four- lane Divided	913+70.00 0	914+00.00 0	30.00	0.0057	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	11.50	11.50	8.00	8.00	8.00	Traversable Median	19.00	false	false		
106	Rural Multi-Lane Segment Four- lane Divided	914+00.00 0	914+30.00 0	30.00	0.0057	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,949; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	11.50	11.50	8.00	8.00	8.00	Traversable Median	19.00	false	false		
107	Rural Multi-Lane Segment Four- lane Undivided	914+30.00 0	914+40.00 0	10.00	0.0019	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,949; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	11.50	11.50	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
108	Rural Multi-Lane Segment Four- lane Undivided	914+40.00 0	915+40.00 0	100.00	0.0189	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,949; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	11.50	11.50	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1
109	Rural Multi-Lane Segment Four- lane Divided	915+40.00 0	916+00.00 0	60.00	0.0114	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,949; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	11.50	11.50	8.00	8.00	8.00	Traversable Median	19.00	false	false		
110	Rural Multi-Lane Segment Four- lane Divided	916+00.00 0	921+00.00 0	500.00	0.0947	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,949; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	11.50	11.50	8.00	8.00	8.00	Non-Traversable Median	19.00	false	false		
111	Rural Multi-Lane Segment Four- lane Divided	921+00.00 0	921+90.00 0	90.00	0.0170	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,949; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	11.50	11.50	8.00	8.00	8.00	Non-Traversable Median	19.00	false	false		
112	Rural Multi-Lane Segment Four- lane Divided	921+90.00 0	922+00.00 0	10.00	0.0019	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,949; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	11.50	11.50	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
113	Rural Multi-Lane Segment Four- lane Divided	922+00.00 0	922+59.00 0	59.00	0.0112	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,949; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	11.50	11.50	8.00	8.00	11.54	Non-Traversable Median	11.54	false	false		
114	Rural Multi-Lane Segment Four- lane Divided	922+59.00 0	923+00.00 0	41.00	0.0078	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,949; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	11.50	11.50	8.00	8.00	17.54	Non-Traversable Median	17.54	false	false		
115	Rural Multi-Lane Segment Four- lane Divided	923+00.00 0	941+70.00 0	1,870.0 0	0.3542	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,949; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	11.50	11.50	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
116	Rural Multi-Lane Segment Four- lane Divided	941+70.00 0	948+50.00 0	680.00	0.1288	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,949; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	11.50	11.50	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		

Table 3. User Defined CMF Used in the Eval Segment CPM Evaluation (Section 3)

Name	Description	Start Loc. (Sta. ft)	End Loc. (Sta. ft)	Start CMF Year	End CMF Year	Severity	CMF Value
1	TWLTL	585+00.000	600+00.000	2025	2050	Total	0.6900
1	TWLTL	612+50.000	639+00.000	2025	2050	Total	0.6900
1	TWLTL	640+00.000	680+90.000	2025	2050	Total	0.6900
1	TWLTL	682+20.000	699+20.000	2025	2050	Total	0.6900

**Table 4. Crash History Highway - Homogeneous Segments (Section 3)** 

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Width (ft)	Right Lane Width (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
1	Rural Multi-Lane Segment Four-lane Undivided	585+00.000	594+84.940	984.94	0.1865	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
2	Rural Multi-Lane Segment Four-lane Undivided	594+84.940	600+00.000	515.06	0.0975	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
3	Rural Multi-Lane Segment Four-lane Undivided	600+00.000	601+00.000	100.00	0.0189	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
4	Rural Multi-Lane Segment Four-lane Undivided	601+00.000	602+00.000	100.00	0.0189	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
5	Rural Multi-Lane Segment Four-lane Undivided	602+00.000	605+00.000	300.00	0.0568	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
6	Rural Multi-Lane Segment Four-lane Undivided	605+00.000	605+60.000	60.00	0.0114	2018-2022: 4,325	12.00	12.00	8.00	0.00	0.00	None	0.00	false	false	0:1	0:1
7	Rural Multi-Lane Segment Four-lane Undivided	605+60.000	605+70.000	10.00	0.0019	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
8	Rural Multi-Lane Segment Four-lane Undivided	605+70.000	605+75.000	5.00	0.0009	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
9	Rural Multi-Lane Segment Four-lane Undivided	605+75.000	609+00.000	325.00	0.0616	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
10	Rural Multi-Lane Segment Four-lane Undivided	609+00.000	609+21.930	21.93	0.0042	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
11	Rural Multi-Lane Segment Four-lane Undivided	609+21.930	611+40.000	218.07	0.0413	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
12	Rural Multi-Lane Segment Four-lane Undivided	611+40.000	612+50.000	110.00	0.0208	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
13	Rural Multi-Lane Segment Four-lane Undivided	612+50.000	624+64.530	1,214.53	0.2300	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
14	Rural Multi-Lane Segment Four-lane Undivided	624+64.530	636+92.820	1,228.29	0.2326	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
15	Rural Multi-Lane Segment Four-lane Undivided	636+92.820	639+00.000	207.18	0.0392	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
16	Rural Multi-Lane Segment Four-lane Undivided	639+00.000	640+00.000	100.00	0.0189	2018-2022: 4,325	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1
17	Rural Multi-Lane Segment Four-lane Undivided	640+00.000	647+26.050	726.05	0.1375	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
18	Rural Multi-Lane Segment Four-lane Undivided	647+26.050	667+80.000	2,053.95	0.3890	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
19	Rural Multi-Lane Segment Four-lane Undivided	667+80.000	668+80.000	100.00	0.0189	2018-2022: 4,325	12.00	12.00	8.00	0.00	0.00	None	0.00	false	false	0:1	0:1
20	Rural Multi-Lane Segment Four-lane Undivided	668+80.000	672+86.110	406.11	0.0769	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
21	Rural Multi-Lane Segment Four-lane Undivided	672+86.110	676+00.000	313.89	0.0594	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
22	Rural Multi-Lane Segment Four-lane Undivided	676+00.000	677+50.000	150.00	0.0284	2018-2022: 4,325	12.00	12.00	0.00	8.00	0.00	None	0.00	false	false	0:1	0:1
23	Rural Multi-Lane Segment Four-lane Undivided	677+50.000	679+00.000	150.00	0.0284	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
24	Rural Multi-Lane Segment Four-lane Undivided	679+00.000	680+80.000	180.00	0.0341	2018-2022: 4,150	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
25	Rural Multi-Lane Segment Four-lane Undivided	680+80.000	680+90.000	10.00	0.0019	2018-2022: 4,150	12.00	12.00	0.00	8.00	0.00	None	0.00	false	false	0:1	0:1
26	Rural Multi-Lane Segment Four-lane Undivided	680+90.000	682+20.000	130.00	0.0246	2018-2022: 4,150	12.00	12.00	0.00	8.00	0.00	None	0.00	false	false	0:1	0:1
27	Rural Multi-Lane Segment Four-lane Undivided	682+20.000	683+82.710	162.71	0.0308	2018-2022: 4,150	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
28	Rural Multi-Lane Segment Four-lane Undivided	683+82.710	691+50.000	767.29	0.1453	2018-2022: 4,150	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
29	Rural Multi-Lane Segment Four-lane Undivided	691+50.000	692+70.000	120.00	0.0227	2018-2022: 4,150	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
30	Rural Multi-Lane Segment Four-lane Undivided	692+70.000	693+85.010	115.01	0.0218	2018-2022: 4,150	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
31	Rural Multi-Lane Segment Four-lane Undivided	693+85.010	698+70.000	484.99	0.0919	2018-2022: 4,150	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
32	Rural Multi-Lane Segment Four-lane Undivided	698+70.000	699+00.000	30.00	0.0057	2018-2022: 4,150	12.00	12.00	0.00	8.00	0.00	None	0.00	false	false	0:1	0:1
33	Rural Multi-Lane Segment Four-lane Undivided	699+00.000	699+20.000	20.00	0.0038	2018-2022: 4,150	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1
34	Rural Multi-Lane Segment Four-lane Undivided	699+20.000	700+40.000	120.00	0.0227	2018-2022: 4,150	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1
35	Rural Multi-Lane Segment Four-lane Divided	700+40.000	700+50.000	10.00	0.0019	2018-2022: 4,150	12.00	12.00	0.00	0.00	8.00	Traversable Median	8.00	false	false		
36	Rural Multi-Lane Segment Four-lane Divided	700+50.000	701+10.000	60.00	0.0114	2018-2022: 4,150	12.00	12.00	8.00	0.00	8.00	Traversable Median	8.00	false	false		
37	Rural Multi-Lane Segment Four-lane Divided	701+10.000	702+00.000	90.00	0.0170	2018-2022: 4,150	12.00	12.00	8.00	0.00	8.00	Non-Traversable Median	8.00	false	false		
38	Rural Multi-Lane Segment Four-lane Divided	702+00.000	702+50.000	50.00	0.0095	2018-2022: 4,900	12.00	12.00	8.00	0.00	8.00	Non-Traversable Median	8.00	false	false		
39	Rural Multi-Lane Segment Four-lane Divided	702+50.000	707+00.000	450.00	0.0852	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
40	Rural Multi-Lane Segment Four-lane Divided	707+00.000	708+00.000	100.00	0.0189	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Width (ft)	Right Lane Width (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
41	Rural Multi-Lane Segment Four-lane Divided	708+00.000	708+80.000	80.00	0.0152	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
42	Rural Multi-Lane Segment Four-lane Divided	708+80.000	709+00.000	20.00	0.0038	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
43	Rural Multi-Lane Segment Four-lane Undivided	709+00.000	710+30.000	130.00	0.0246	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
44	Rural Multi-Lane Segment Four-lane Divided	710+30.000	710+47.850	17.85	0.0034	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
45	Rural Multi-Lane Segment Four-lane Divided	710+47.850	725+00.000	1,452.15	0.2750	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
46	Rural Multi-Lane Segment Four-lane Divided	725+00.000	727+52.350	252.35	0.0478	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
47	Rural Multi-Lane Segment Four-lane Divided	727+52.350	735+00.000	747.65	0.1416	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
48	Rural Multi-Lane Segment Four-lane Divided	735+00.000	755+50.000	2,050.00	0.3883	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
49	Rural Multi-Lane Segment Four-lane Undivided	755+50.000	756+90.000	140.00	0.0265	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
50	Rural Multi-Lane Segment Four-lane Divided	756+90.000	757+00.000	10.00	0.0019	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
51	Rural Multi-Lane Segment Four-lane Divided	757+00.000	763+30.000	630.00	0.1193	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
52	Rural Multi-Lane Segment Four-lane Divided	763+30.000	764+00.000	70.00	0.0133	2018-2022: 4,900	12.00	12.00	8.00	8.00	11.50	Non-Traversable Median	11.50	false	false		
53	Rural Multi-Lane Segment Four-lane Divided	764+00.000	764+50.000	50.00	0.0095	2018-2022: 4,900	12.00	12.00	8.00	8.00	17.50	Non-Traversable Median	17.50	false	false		
54	Rural Multi-Lane Segment Four-lane Divided	764+50.000	765+52.550	102.55	0.0194	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
55	Rural Multi-Lane Segment Four-lane Divided	765+52.550	777+80.000	1,227.45	0.2325	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
56	Rural Multi-Lane Segment Four-lane Undivided	777+80.000	778+80.000	100.00	0.0189	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
57	Rural Multi-Lane Segment Four-lane Divided	778+80.000	779+00.000	20.00	0.0038	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
58	Rural Multi-Lane Segment Four-lane Divided	779+00.000	780+45.930	145.93	0.0276	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
59	Rural Multi-Lane Segment Four-lane Divided	780+45.930	785+40.000	494.07		2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
60	Rural Multi-Lane Segment Four-lane Divided	785+40.000	785+50.000	10.00		2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
61	Rural Multi-Lane Segment Four-lane Divided	785+50.000	786+09.000	59.00	0.0112	2018-2022: 4,900	12.00	12.00	8.00	8.00	11.54	Non-Traversable Median	11.54	false	false		
62	Rural Multi-Lane Segment Four-lane Divided	786+09.000	786+50.000	41.00		2018-2022: 4,900	12.00	12.00	8.00	8.00	17.54	Non-Traversable Median	17.54	false	false		
63	Rural Multi-Lane Segment Four-lane Divided	786+50,000	801+10.000	1.460.00		2018-2022: 4.900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
64	Rural Multi-Lane Segment Four-lane Divided	801+10.000	801+61.000	51.00	0.0097	2018-2022: 4,900	12.00	12.00	8.00	8.00	17.45	Non-Traversable Median	17.45	false	false		
65	Rural Multi-Lane Segment Four-lane Divided	801+61.000	802+30.000	69.00	0.0131	2018-2022: 4,900	12.00	12.00	8.00	8.00	11.45	Non-Traversable Median	11.45	false	false		
66	Rural Multi-Lane Segment Four-lane Divided	802+30.000	802+40.000	10.00	0.0019	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
67	Rural Multi-Lane Segment Four-lane Divided	802+40.000	808+30.000	590.00		2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
68	Rural Multi-Lane Segment Four-lane Divided	808+30.000	808+80.000	50.00		2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
69	Rural Multi-Lane Segment Four-lane Undivided	808+80.000	809+00.000	20.00		2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
70	Rural Multi-Lane Segment Four-lane Undivided	809+00.000	809+60 000	60.00		2018-2022: 4,900	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1
71	Rural Multi-Lane Segment Four-lane Divided	809+60.000	810+00.000	40.00		2018-2022: 4,900	12.00	12.00	0.00	0.00	8.00	Traversable Median	20.00	false	false	0.1	
72	Rural Multi-Lane Segment Four-lane Divided	810+00.000	810+20.000	20.00	0.0038	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
73	Rural Multi-Lane Segment Four-lane Divided	810+20.000	816+00.000	580.00		2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
74	Rural Multi-Lane Segment Four-lane Divided	816+00.000	816+70.000	70.00		2018-2022: 4,900	12.00	12.00	8.00	8.00	11.50	Non-Traversable Median	11.50	false	false		
75	Rural Multi-Lane Segment Four-lane Divided	816+70.000		50.00		2018-2022: 4,900	12.00	12.00	8.00	8.00	17.50	Non-Traversable Median	17.50	false	false		
76	Rural Multi-Lane Segment Four-lane Divided	817+20.000	853+70.000	3,650.00		2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
77	Rural Multi-Lane Segment Four-lane Divided	853+70.000	854+00.000	30.00		2018-2022: 4,900	12.00	12.00	8.00	8.00	18.36	Non-Traversable Median	18.36	false	false		
78	Rural Multi-Lane Segment Four-lane Divided	854+00.000	854+16.000	16.00		2018-2022: 4,900	12.00	12.00	8.00	8.00	15.85	Non-Traversable Median	15.85	false	false		
79	Rural Multi-Lane Segment Four-lane Divided	854+16,000	854+80.000	64.00		2018-2022: 4,900	12.00	12.00	8.00	8.00	11.49	Non-Traversable Median	11.49	false	false		
80		854+80.000		610.00		2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
81	Rural Multi-Lane Segment Four-lane Divided	854+80.000 860+90.000	861+85.000	95.00		2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
81	Rural Multi-Lane Segment Four-lane Divided	861+85.000	861+85.000 862+00.000				_	12.00	8.00	0.00	0.00		0.00	false	false	0:1	0:1
	Rural Multi-Lane Segment Four-lane Undivided			15.00		2018-2022: 4,900	12.00				0.00	None					
83	Rural Multi-Lane Segment Four-lane Undivided	862+00.000	862+50.000	50.00	0.0095	2018-2022: 4,900	12.00	12.00	8.00	0.00	0.00	None	0.00	false	false	0:1	0:1

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Width (ft)	Right Lane Width (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
84	Rural Multi-Lane Segment Four-lane Undivided	862+50.000	862+60.000	10.00	0.0019	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
85	Rural Multi-Lane Segment Four-lane Divided	862+60.000	863+10.000	50.00	0.0095	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Traversable Median	19.00	false	false		
86	Rural Multi-Lane Segment Four-lane Divided	863+10.000	869+00.000	590.00	0.1117	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	19.00	false	false		
87	Rural Multi-Lane Segment Four-lane Divided	869+00.000	869+70.000	70.00	0.0133	2018-2022: 4,900	12.00	12.00	8.00	8.00	11.50	Non-Traversable Median	11.50	false	false		
88	Rural Multi-Lane Segment Four-lane Divided	869+70.000	870+20.000	50.00	0.0095	2018-2022: 4,900	12.00	12.00	8.00	8.00	17.50	Non-Traversable Median	17.50	false	false		
89	Rural Multi-Lane Segment Four-lane Divided	870+20.000	881+80.000	1,160.00	0.2197	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
90	Rural Multi-Lane Segment Four-lane Divided	881+80.000	882+31.000	51.00	0.0097	2018-2022: 4,900	12.00	12.00	8.00	8.00	17.45	Non-Traversable Median	17.45	false	false		
91	Rural Multi-Lane Segment Four-lane Divided	882+31.000	883+00.000	69.00	0.0131	2018-2022: 4,900	12.00	12.00	8.00	8.00	11.45	Non-Traversable Median	11.45	false	false		
92	Rural Multi-Lane Segment Four-lane Divided	883+00.000	887+90.000	490.00	0.0928	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
93	Rural Multi-Lane Segment Four-lane Divided	887+90.000	888+20.000	30.00	0.0057	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
94	Rural Multi-Lane Segment Four-lane Undivided	888+20.000	889+30.000	110.00	0.0208	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
95	Rural Multi-Lane Segment Four-lane Divided	889+30.000	889+50.000	20.00	0.0038	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
96	Rural Multi-Lane Segment Four-lane Divided	889+50.000	894+50.000	500.00	0.0947	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
97	Rural Multi-Lane Segment Four-lane Divided	894+50.000	895+15.000	65.00	0.0123	2018-2022: 4,900	12.00	12.00	8.00	8.00	11.55	Non-Traversable Median	11.55	false	false		
98	Rural Multi-Lane Segment Four-lane Divided	895+15.000	895+60.000	45.00	0.0085	2018-2022: 4,900	12.00	12.00	8.00	8.00	17.55	Non-Traversable Median	17.55	false	false		
99	Rural Multi-Lane Segment Four-lane Divided	895+60.000	898+00.000	240.00	0.0455	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
100	Rural Multi-Lane Segment Four-lane Divided	898+00.000	906+70.000	870.00	0.1648	2018-2022: 4,900	11.50	11.50	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
101	Rural Multi-Lane Segment Four-lane Divided	906+70.000	907+21.000	51.00	0.0097	2018-2022: 4,900	11.50	11.50	8.00	8.00	17.45	Non-Traversable Median	17.45	false	false		
102	Rural Multi-Lane Segment Four-lane Divided	907+21.000	907+80.000	59.00	0.0112	2018-2022: 4,900	11.50	11.50	8.00	8.00	11.95	Non-Traversable Median	11.95	false	false		
103	Rural Multi-Lane Segment Four-lane Divided	907+80.000	907+90.000	10.00	0.0019	2018-2022: 4,900	11.50	11.50	8.00	8.00	8.50	Non-Traversable Median	19.50	false	false		
104	Rural Multi-Lane Segment Four-lane Divided	907+90.000	913+70.000	580.00	0.1098	2018-2022: 4,900	11.50	11.50	8.00	8.00	8.00	Non-Traversable Median	19.00	false	false		
105	Rural Multi-Lane Segment Four-lane Divided	913+70.000	914+00.000	30.00	0.0057	2018-2022: 4,900	11.50	11.50	8.00	8.00	8.00	Traversable Median	19.00	false	false		
106	Rural Multi-Lane Segment Four-lane Divided	914+00.000	914+30.000	30.00	0.0057	2018-2022: 4,900	11.50	11.50	8.00	8.00	8.00	Traversable Median	19.00	false	false		
107	Rural Multi-Lane Segment Four-lane Undivided	914+30.000	914+40.000	10.00	0.0019	2018-2022: 4,900	11.50	11.50	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
108	Rural Multi-Lane Segment Four-lane Undivided	914+40.000	915+40.000	100.00	0.0189	2018-2022: 4,900	11.50	11.50	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1
109	Rural Multi-Lane Segment Four-lane Divided	915+40.000	916+00.000	60.00	0.0114	2018-2022: 4,900	11.50	11.50	8.00	8.00	8.00	Traversable Median	19.00	false	false		
110	Rural Multi-Lane Segment Four-lane Divided	916+00.000	921+00.000	500.00	0.0947	2018-2022: 4,900	11.50	11.50	8.00	8.00	8.00	Non-Traversable Median	19.00	false	false		
111	Rural Multi-Lane Segment Four-lane Divided	921+00.000	921+90.000	90.00	0.0170	2018-2022: 4,900	11.50	11.50	8.00	8.00	8.00	Non-Traversable Median	19.00	false	false		
112	Rural Multi-Lane Segment Four-lane Divided	921+90.000	922+00.000	10.00	0.0019	2018-2022: 4,900	11.50	11.50	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
113	Rural Multi-Lane Segment Four-lane Divided	922+00.000	922+59.000	59.00	0.0112	2018-2022: 4,900	11.50	11.50	8.00	8.00	11.54	Non-Traversable Median	11.54	false	false		
114	Rural Multi-Lane Segment Four-lane Divided	922+59.000	923+00.000	41.00	0.0078	2018-2022: 4,900	11.50	11.50	8.00	8.00	17.54	Non-Traversable Median	17.54	false	false		
115	Rural Multi-Lane Segment Four-lane Divided	923+00.000	941+70.000	1,870.00	0.3542	2018-2022: 4,900	11.50	11.50	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
116	Rural Multi-Lane Segment Four-lane Divided	941+70.000	948+50.000	680.00	0.1288	2018-2022: 4,900	11.50	11.50	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		

 Table 5. Evaluation Intersection (Section 3)

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
1	I90EBRamp_S466th_SD38 (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	699+20.000	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,292; 2032: 8,453; 2033: 8,678; 2035: 9,172; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418: 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	2025: 630; 2026: 644; 2027: 657; 2028: 671; 2029: 685; 2030: 700; 2031: 716; 2032: 732; 2033: 748; 2034: 764; 2035: 780; 2036: 796; 2037: 812; 2038: 828; 2039: 844; 2040: 860; 2041: 1,166; 2042: 1,473; 2043: 1,779; 2044: 2,086; 2045: 2,392; 2046: 2,699; 2047: 3,005; 2048: 3,312; 2049: 3,618; 2050: 3,925	4	Stop-Controlled	1	0	4.64	4.27	false
2	SD38/260th_St (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	605+70.000	12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654;	2025: 1,508; 2026: 1,706; 2027: 1,904; 2028: 2,102; 2029: 2,300; 2030: 2,472; 2031: 2,645; 2032: 2,818; 2033: 2,909; 2034: 3,163: 2035: 3,336; 2,036: 3,509; 2,037: 3,681; 2,038: 3,854; 2,039: 4,027; 2040: 4,200; 2041: 4,260; 2042: 4,320; 2043: 4,380; 2044: 4,440; 2045: 4,500; 2046: 4,560; 2047: 4,620; 2048: 4,680; 2049: 4,740; 2050: 4,800	4	Stop-Controlled	2	1	16.83	13.71	false
3	466thN/SD38 (v1)	Rural Multi-Lane Intersection Three-Legged w/STOP control	676+50.000	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,42; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	2025: 118; 2026: 121; 2027: 124; 2028: 127; 2029: 130; 2030: 133; 2031: 136; 2032: 139; 2033: 142; 2034: 145; 2035: 149; 2036: 152; 2037: 155; 2038: 158; 2039: 161; 2040: 165; 2041: 168; 2042: 186; 2047: 189; 2048: 193; 2049: 196; 2050: 200	3	Stop-Controlled	0	0	6.61		false

### **Table 6. Evaluation Intersection (Section 3)**

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
5	468th Ave (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	809+00.000	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,690; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,95; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	2025: 667; 2026: 682; 2027: 696; 2028: 710; 2029: 725; 2030: 741; 2031: 758; 2032: 775; 2033: 792; 2034: 809; 2035: 825; 2036: 842; 2037: 859; 2038: 876; 2039: 893; 2040: 910; 2041: 1,052; 2042: 1,195; 2043: 1,337; 2044: 1480; 2045: 1,622; 2046: 1,765; 2047: 1,907; 2048: 2,050; 2049: 2,192; 2050: 2,335	4	Stop-Controlled	1	0	0.00	0.00	false
6	SD38/Hwy 139 (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	862+00.000	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,795; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	2025: 2,990; 2026: 3,054; 2027: 3,117; 2028: 3,181; 2029: 3,245; 2030: 3,321; 2031: 3,397; 2032: 3,474; 2033: 3,550; 2034: 3,626; 2035: 3,703; 2036: 3,779; 2037: 3,855; 2038: 3,932; 2039: 4,008; 2040: 4,085; 2041: 4,178; 2042: 4,271; 2043: 4,364; 2044: 4,457; 2045: 4,550; 2046: 4,643; 2047: 4,736; 2048: 4,829; 2049: 4,922; 2050: 5,015	4	Stop-Controlled	1	0	0.00	0.00	false
7	LaMesa/SD 38 (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	915+00.000	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	2025: 1,266; 2026: 1,293; 2027: 1,320; 2028: 1,347; 2029: 1,375; 2030: 1,407; 2031: 1,439; 2032: 1,471; 2033: 1,504; 2034: 1,536; 2035: 1,568; 2036: 1,725; 2037: 1,949; 2038: 2,172; 2039; 2,396; 2040: 2,620; 2041: 2,940; 2042: 3,261; 2043: 3,581; 2044: 3,902; 2045: 4,222; 2046: 4,543; 2047: 4,863; 2048: 5,184; 2049: 5,504; 2050: 5,825	4	Stop-Controlled	0	0	0.00	0.00	false

 Table 7. Evaluation Ramp Terminal - Site (Section 3)

Inter. No.	Title	Туре	Area Type	Legs	Location (Sta. ft)	Traffic Control	AADT
	4 190WB Ramps (v2)	Freeway Ramp Terminal A2 - Three-Leg at Two-Quadrant Parclo A	Rural	4	681+00.000		Inside: 2025: 5.705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,078; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,029: 7,780; 2030: 8,004; 2031: 12,878; 2050: 13,170; Outside: 2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 2,229; 2032: 8,453; 2038: 6,851; 2039: 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170: Entrace 2025: 856; 2026: 875; 2027: 893; 2028: 11,2029: 90; 2039: 951; 2031: 973; 2032: 995; 2033: 1,017; 2034: 1,039; 2035: 1,060; 2036: 1,082; 2037: 1,104; 2038: 1,126; 2039: 1,148; 2040: 1,170; 2041: 1,339; 2042: 1,508; 2043: 1,677; 2044: 1,846; 2045: 2,015; 2046: 2,184; 2047: 2,353; 2048: 2,522; 2049: 2,691; 2050: 2,691; 2050: 2,641; 1,677; 2044: 1,846; 2045: 2,015; 2046: 2,184; 2047: 2,353; 2048: 2,522; 2049: 2,691; 2050: 2,601

## $Table \ 8. \ Crash \ History \ Intersection \ (Section \ 3)$

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
1	I90EBRamp_S466th_SD38 (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	699+20.000	2018-2022: 4,150	2018-2022: 590	4	Stop-Controlled	1	0	4.64	4.27	false
2	SD38/260th_St (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	605+70.000	2018-2022: 4,325	2018-2022: 915	4	Stop-Controlled	2	1	16.83	13.71	false
3	466thN/SD38 (v1)	Rural Multi-Lane Intersection Three-Legged w/STOP control	676±50.000	2018-2022: 4.325	2018-2022: 110	3	Ston-Controlled	0	0	6.61		false

## Table 9. Crash History Intersection (Section 3)

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
5	468th Ave (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	809+00.000	2018-2022: 4,900	2018-2022: 625	4	Stop-Controlled	1	0	0.00	0.00	false
6	SD38/Hwy139 (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	862+00.000	2018-2022: 4,900	2018-2022: 2,800	4	Stop-Controlled	1	0	0.00	0.00	false
7	LaMesa/SD38 (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	915+00,000	2018-2022: 4.900	2018-2022: 1.185	4	Stop-Controlled	0	0	0.00	0.00	false

### Table 10. Crash Highway Ramp Terminal - Site (Highway with Crash History)

Inter. No.	Title	Туре	Area Type	Legs	Location (Sta. ft)	Traffic Control	AADT
4	I90WB Ramps (v2)	Freeway Ramp Terminal A2 - Three-Leg at Two-Quadrant Parclo A	Rural	4	681+00.000	Stop-Controlled	Inside: 2018-2022: 4,150; Outside: 2018-2022: 4,150 :: Entrance: 2018-2022: 802

Table 11. Expected Highway Crash Rates and Frequencies Summary (Section 3)

Last Year of Analysis   2050		
Evaluated Length (mi)   6.8845     Average Future Road AADT (vpd)   10,372     Expected Crashes	First Year of Analysis	2025
Average Future Road AADT (vpd)   10,372	Last Year of Analysis	2050
Total Crashes   621.51	Evaluated Length (mi)	6.8845
Total Crashes   621.51     Fatal and Injury Crashes   327.86     Fatal and Serious Injury Crashes   204.46     Property-Damage-Only Crashes   293.65     Percent of Total Expected Crashes     Percent Fatal and Injury Crashes (%)   53     Percent Fatal and Serious Injury Crashes (%)   33     Percent Property-Damage-Only Crashes (%)   47     Expected Crash Rate     Crash Rate (crashes/mi/yr)   3.4722     FI Crash Rate (crashes/mi/yr)   1.8316     FI no/C Crash Rate (crashes/mi/yr)   1.1422     PDO Crash Rate (crashes/mi/yr)   1.6405     Expected Travel Crash Rate     Total Travel (million veh-mi)   677.62     Travel FI Crash Rate (crashes/million veh-mi)   0.92     Travel FI Crash Rate (crashes/million veh-mi)   0.48     Travel FI Crash Rate (crashes/million veh-mi)   0.48     Travel FI Crash Rate (crashes/million veh-mi)   0.48     Travel FI Crash Rate (crashes/million veh-mi)   0.30	Average Future Road AADT (vpd)	10,372
Fatal and Injury Crashes  Fatal and Serious Injury Crashes  Property-Damage-Only Crashes  Percent of Total Expected Crashes  Percent Fatal and Injury Crashes (%)  Percent Fatal and Injury Crashes (%)  Percent Fatal and Serious Injury Crashes (%)  Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  1.8316  FI no/C Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  O.92  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  O.30	Expected Crashes	
Fatal and Serious Injury Crashes   204.46     Property-Damage-Only Crashes   293.65     Percent of Total Expected Crashes     Percent Fatal and Injury Crashes (%)   53     Percent Fatal and Serious Injury Crashes (%)   33     Percent Property-Damage-Only Crashes (%)   47     Expected Crash Rate     Crash Rate (crashes/mi/yr)   3.4722     FI Crash Rate (crashes/mi/yr)   1.8316     FI no/C Crash Rate (crashes/mi/yr)   1.1422     PDO Crash Rate (crashes/mi/yr)   1.6405     Expected Travel Crash Rate     Total Travel (million veh-mi)   677.62     Travel FI Crash Rate (crashes/million veh-mi)   0.92     Travel FI Crash Rate (crashes/million veh-mi)   0.48     Travel FI Crash Rate (crashes/million veh-mi)   0.30	Total Crashes	621.51
Property-Damage-Only Crashes  Percent of Total Expected Crashes  Percent Fatal and Injury Crashes (%)  Percent Fatal and Serious Injury Crashes (%)  Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  1.8316  FI no/C Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  O.38  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  O.38  Travel FI Crash Rate (crashes/million veh-mi)  O.39  Travel FI Crash Rate (crashes/million veh-mi)  O.30	Fatal and Injury Crashes	327.86
Percent of Total Expected Crashes	Fatal and Serious Injury Crashes	204.46
Percent Fatal and Injury Crashes (%)  Percent Fatal and Serious Injury Crashes (%)  Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  1.8316  FI no/C Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  1.1422  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  0.92  Travel FI Crash Rate (crashes/million veh-mi)  0.48  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.30	Property-Damage-Only Crashes	293.65
Percent Fatal and Serious Injury Crashes (%)  Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  1.8316  FI no/C Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  1.1422  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  0.48  Travel FI no/C Crash Rate (crashes/million veh-mi)	Percent of Total Expected Crashes	
Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  5.4722  FI Crash Rate (crashes/mi/yr)  FI no/C Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  1.6405  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  0.48  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.30	Percent Fatal and Injury Crashes (%)	53
Expected Crash Rate  Crash Rate (crashes/mi/yr) 3.4722  FI Crash Rate (crashes/mi/yr) 1.8316  FI no/C Crash Rate (crashes/mi/yr) 1.1422  PDO Crash Rate (crashes/mi/yr) 1.6405  Expected Travel Crash Rate  Total Travel (million veh-mi) 677.62  Travel Crash Rate (crashes/million veh-mi) 0.92  Travel FI Crash Rate (crashes/million veh-mi) 0.48  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.30	Percent Fatal and Serious Injury Crashes (%)	33
Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  1.8316  FI no/C Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  1.6405  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  0.92  Travel FI Crash Rate (crashes/million veh-mi)  0.48  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.30	Percent Property-Damage-Only Crashes (%)	47
FI Crash Rate (crashes/mi/yr)  1.8316  FI no/C Crash Rate (crashes/mi/yr)  1.1422  PDO Crash Rate (crashes/mi/yr)  1.6405  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  0.92  Travel FI Crash Rate (crashes/million veh-mi)  0.48  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.30	Expected Crash Rate	
FI no/C Crash Rate (crashes/mi/yr)  1.1422  PDO Crash Rate (crashes/mi/yr)  1.6405  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  0.48  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.30	Crash Rate (crashes/mi/yr)	3.4722
PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.30	FI Crash Rate (crashes/mi/yr)	1.8316
Expected Travel Crash Rate  Total Travel (million veh-mi) 677.62  Travel Crash Rate (crashes/million veh-mi) 0.92  Travel FI Crash Rate (crashes/million veh-mi) 0.48  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.30	FI no/C Crash Rate (crashes/mi/yr)	1.1422
Total Travel (million veh-mi) 677.62  Travel Crash Rate (crashes/million veh-mi) 0.92  Travel FI Crash Rate (crashes/million veh-mi) 0.48  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.30	PDO Crash Rate (crashes/mi/yr)	1.6405
Travel Crash Rate (crashes/million veh-mi) 0.92  Travel FI Crash Rate (crashes/million veh-mi) 0.48  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.30	Expected Travel Crash Rate	
Travel FI Crash Rate (crashes/million veh-mi) 0.48  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.30	Total Travel (million veh-mi)	677.62
Travel FI no/C Crash Rate (crashes/million veh-mi) 0.30	Travel Crash Rate (crashes/million veh-mi)	0.92
	Travel FI Crash Rate (crashes/million veh-mi)	0.48
Travel PDO Crash Rate (crashes/million veh-mi) 0.43	Travel FI no/C Crash Rate (crashes/million veh-mi)	0.30
,	Travel PDO Crash Rate (crashes/million veh-mi)	0.43

Table 12. Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 3)

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr	Expected FI Crash Frequency (crashes/yr	Expected FI no/C Crash Frequency (crashes/yr	Expected PDO Crash Frequency (crashes/yr	Predicted Total Crash Frequency (crashes/yr	Predicted FI Crash Frequency (crashes/yr	Predicted FI no/C Crash Frequency (crashes/yr	Predicted PDO Crash Frequency (crashes/yr	(Expected - Predicted) Total Crash Frequency (crashes/yr	(Expected - Predicted) FI Crash Frequency (crashes/yr	(Expected - Predicted) FI no/C Crash Frequency (crashes/yr	(Expected - Predicted) PDO Crash Frequency (crashes/yr	Expected Crash Rate (crashes/ mi/yr)	Expected Travel Crash Rate (crashes/ million veh-mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
1	585+00.000	594+84.940	0.1865	12.419	12.987	0.4777	0.3553	0.2214	0.1223	0.4995	0.2945	0.1563	0.2050	-0.0218	0.0608	0.0652	-0.0826	2.5607	0.59	
2	594+84.940	600+00.000	0.0975	3.241	6.791	0.1246	0.0755	0.0471	0.0491	0.2612	0.1540	0.0817	0.1072	-0.1366	-0.0785	-0.0347	-0.0581	1.2777	0.29	
3	600+00.000	601+00.000	0.0189	0.912	1.911	0.0351	0.0212	0.0132	0.0138	0.0735	0.0433	0.0230	0.0302	-0.0384	-0.0221	-0.0098	-0.0163	1.8518	0.42	
4	601+00.000	602+00.000	0.0189	0.912	1.911	0.0351	0.0212	0.0132	0.0138	0.0735	0.0433	0.0230	0.0302	-0.0384	-0.0221	-0.0098	-0.0163	1.8518	0.42	
5	602+00.000	605+00.000	0.0568	15.348	7.486	0.5903	0.1555	0.0969	0.4348	0.2879	0.1665	0.0852	0.1214	0.3024	-0.0110	0.0117	0.3134	10.3895	1.90	
6	605+00.000	605+60.000	0.0114	0.743	1.629	0.0286	0.0173	0.0108	0.0113	0.0627	0.0362	0.0185	0.0264	-0.0341	-0.0190	-0.0078	-0.0151	2.5153	0.46	
7	605+60.000	605+70.000	0.0019	0.119	0.249	0.0046	0.0028	0.0017	0.0018	0.0096	0.0056	0.0028	0.0040	-0.0050	-0.0028	-0.0011	-0.0022	2.4181	0.44	
SD38/260th_St (v1)	605+70.000			35.375	96.924	1.3606	0.5456	0.3469	0.8150	3.7279	1.7583	0.8841	1.9696	-2.3673	-1.2127	-0.5372	-1.1546			0.20
8	605+70.000	605+75.000	0.0009	0.059	0.125	0.0023	0.0014	0.0009	0.0009	0.0048	0.0028	0.0014	0.0020	-0.0025	-0.0014	-0.0006	-0.0011	2.4181	0.44	
9	605+75.000	609+00.000	0.0616	15.646	8.110	0.6018	0.5117	0.3188	0.0901	0.3119	0.1804	0.0923	0.1315	0.2898	0.3313	0.2266	-0.0415	9.7763	1.79	
10	609+00.000	609+21.930	0.0042	12.037	0.547	0.4630	0.0149	0.0093	0.4480	0.0210	0.0122	0.0062	0.0089	0.4419	0.0028	0.0031	0.4392	111.4663	20.38	
11	609+21.930	611+40.000	0.0413	2.597	5.442	0.0999	0.0605	0.0377	0.0394	0.2093	0.1210	0.0619	0.0883	-0.1094	-0.0605	-0.0242	-0.0489	2.4181	0.44	
12	611+40.000	612+50.000	0.0208	1.310	2.745	0.0504	0.0305	0.0190	0.0199	0.1056	0.0611	0.0312	0.0445	-0.0552	-0.0305	-0.0122	-0.0247	2.4181	0.44	
13	612+50.000	624+64.530	0.2300	18.104	20.912	0.6963	0.3194	0.1990	0.3770	0.8043	0.4651	0.2379	0.3391	-0.1080	-0.1458	-0.0389	0.0378	3.0271	0.55	
14	624+64.530	636+92.820	0.2326	10.091	21.149	0.3881	0.2351	0.1465	0.1530	0.8134	0.4704	0.2406	0.3430	-0.4253	-0.2353	-0.0941	-0.1900	1.6685	0.30	
15	636+92.820	639+00.000	0.0392	1.702	3.567	0.0655	0.0397	0.0247	0.0258	0.1372	0.0793	0.0406	0.0579	-0.0717	-0.0397	-0.0159	-0.0320	1.6685	0.30	
16	639+00.000	640+00.000	0.0189	1.282	2.935	0.0493	0.0297	0.0185	0.0196	0.1129	0.0653	0.0334	0.0476	-0.0636	-0.0356	-0.0149	-0.0280	2.6043	0.48	
17	640+00.000	647+26.050	0.1375	14.091	12.501	0.5419	0.2136	0.1331	0.3283	0.4808	0.2781	0.1422	0.2027	0.0611	-0.0644	-0.0091	0.1256	3.9411	0.72	
18	647+26.050	667+80.000	0.3890	33.126	35.365	1.2741	0.5595	0.3486	0.7146	1.3602	0.7866	0.4024	0.5735	-0.0861	-0.2271	-0.0538	0.1410	3.2752	0.60	
19	667+80.000	668+80.000	0.0189	0.855	1.874	0.0329	0.0198	0.0124	0.0130	0.0721	0.0417	0.0213	0.0304	-0.0392	-0.0218	-0.0090	-0.0174	1.7355	0.32	
20	668+80.000	672+86.110	0.0769	3.337	6.992	0.1283	0.0777	0.0484	0.0506	0.2689	0.1555	0.0796	0.1134	-0.1406	-0.0778	-0.0311	-0.0628	1.6685	0.30	
21	672+86.110	676+00.000	0.0594	18.830	5.404	0.7242	0.6591	0.4107	0.0651	0.2079	0.1202	0.0615	0.0877	0.5164	0.5389	0.3492	-0.0225	12.1822	2.23	
22	676+00.000	677+50.000	0.0284	18.186	2.810	0.6995	0.6651	0.4144	0.0344	0.1081	0.0625	0.0320	0.0456	0.5914	0.6025	0.3824	-0.0112	24.6213	4.50	
466thN/SD38 (v1)	676+50.000			22.161	36.857	0.8523	0.4087	0.2971	0.4437	1.4176	0.6242	0.3958	0.7934	-0.5652	-0.2155	-0.0987	-0.3497			0.15
23	677+50.000	679+00.000	0.0284	25.609	2.583	0.9849	0.4193	0.2613	0.5656	0.0993	0.0574	0.0294	0.0419	0.8856	0.3619	0.2319	0.5238	34.6702	6.34	
24	679+00.000	680+80.000	0.0341	0.911	1.863	0.0351	0.0213	0.0134	0.0137	0.0716	0.0429	0.0235	0.0287	-0.0366	-0.0216	-0.0101	-0.0150	1.0282	0.29	
25	680+80.000	680+90.000	0.0019	0.053	0.113	0.0020	0.0012	0.0008	0.0008	0.0043	0.0026	0.0014	0.0017	-0.0023	-0.0014	-0.0006	-0.0009	1.0706	0.30	
26	680+90.000	682+20.000	0.0246	0.993	2.121	0.0382	0.0232	0.0145	0.0150	0.0816	0.0489	0.0267	0.0327	-0.0434	-0.0257	-0.0122	-0.0177	1.5517	0.44	
I90WB Ramps (v2)	681+00.000			20.225	15.236	0.7779	0.4251		0.3528	0.5860	0.1969		0.3891	0.1919	0.2282		-0.0363			0.20
27	682+20.000	683+82.710	0.0308	0.824	1.684	0.0317	0.0193	0.0121	0.0124	0.0648	0.0388	0.0212	0.0260	-0.0331	-0.0195	-0.0091	-0.0136	1.0282	0.29	
28	683+82.710	691+50.000	0.1453	3.885	7.940	0.1494	0.0909	0.0570	0.0585	0.3054	0.1830	0.1001	0.1224	-0.1560	-0.0920	-0.0431	-0.0639	1.0282	0.29	
29	691+50.000	692+70.000	0.0227	0.608	1.242	0.0234	0.0142	0.0089	0.0091	0.0478	0.0286	0.0157	0.0191	-0.0244	-0.0144	-0.0067	-0.0100	1.0282	0.29	
30	692+70.000	693+85.010	0.0218	0.582	1.190	0.0224	0.0136	0.0085	0.0088	0.0458	0.0274	0.0150	0.0183	-0.0234	-0.0138	-0.0065	-0.0096	1.0282	0.29	
31	693+85.010	698+70.000	0.0919	2.456	5.019	0.0944	0.0575	0.0360	0.0370	0.1930	0.1156	0.0633	0.0774	-0.0986	-0.0582	-0.0272	-0.0404	1.0282	0.29	
32	698+70.000	699+00.000	0.0057	0.158	0.338	0.0061	0.0037	0.0023	0.0024	0.0130	0.0078	0.0043	0.0052	-0.0069	-0.0041	-0.0019	-0.0028	1.0706	0.30	
33	699+00.000	699+20.000	0.0038	0.109	0.243	0.0042	0.0025	0.0016	0.0017	0.0094	0.0056	0.0031	0.0038	-0.0052	-0.0031	-0.0015	-0.0021	1.1096	0.31	
I90EBRamp_S466th_SD38 (v1)	699+20.000			20.777	59.524	0.7991	0.3462	0.2319	0.4529	2.2894	1.2112	0.6869	1.0782	-1.4903	-0.8649	-0.4550	-0.6253			0.20
34	699+20.000	700+40.000	0.0227	0.950	2.117	0.0365	0.0221	0.0139	0.0145	0.0814	0.0488	0.0267	0.0326	-0.0449	-0.0267	-0.0128	-0.0182	1.6081	0.45	
35	700+40.000	700+50.000	0.0019	0.056	0.111	0.0022	0.0012	0.0008	0.0010	0.0043	0.0022	0.0014	0.0020	-0.0021	-0.0010	-0.0006	-0.0011	1.1385	0.32	

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr	Expected FI Crash Frequency (crashes/yr	Expected FI no/C Crash Frequency (crashes/yr	Expected PDO Crash Frequency (crashes/yr	Predicted Total Crash Frequency (crashes/yr	Predicted FI Crash Frequency (crashes/yr	Predicted FI no/C Crash Frequency (crashes/yr	Predicted PDO Crash Frequency (crashes/yr	(Expected - Predicted) Total Crash Frequency (crashes/yr	(Expected - Predicted) FI Crash Frequency (crashes/yr	(Expected - Predicted) FI no/C Crash Frequency (crashes/yr	(Expected - Predicted) PDO Crash Frequency (crashes/yr	Expected Crash Rate (crashes/ mi/yr)	Expected Travel Crash Rate (crashes/ million veh-mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
36	700+50.000	701+10.000	0.0114	0.323	0.615	0.0124	0.0069	0.0048	0.0055	0.0237	0.0124	0.0079	0.0113	-0.0112	-0.0054	-0.0032	-0.0058	1.0929	0.31	
37	701+10.000	702+00.000	0.0170	0.474	0.887	0.0182	0.0102	0.0070	0.0081	0.0341	0.0178	0.0115	0.0163	-0.0159	-0.0077	-0.0044	-0.0082	1.0704	0.30	
38	702+00.000	702+50.000	0.0095	0.199	0.404	0.0076	0.0042	0.0029	0.0034	0.0155	0.0083	0.0054	0.0073	-0.0079	-0.0041	-0.0025	-0.0038	0.8065	0.27	
39	702+50.000	707+00.000	0.0852	1.712	3.338	0.0658	0.0362	0.0247	0.0296	0.1284	0.0683	0.0447	0.0601	-0.0625	-0.0321	-0.0200	-0.0305	0.7724	0.26	
40	707+00.000	708+00.000	0.0189	0.380	0.742	0.0146	0.0080	0.0055	0.0066	0.0285	0.0152	0.0099	0.0134	-0.0139	-0.0071	-0.0044	-0.0068	0.7724	0.26	
41	708+00.000	708+80.000	0.0152	0.304	0.593	0.0117	0.0064	0.0044	0.0053	0.0228	0.0121	0.0079	0.0107	-0.0111	-0.0057	-0.0035	-0.0054	0.7724	0.26	
42	708+80.000	709+00.000	0.0038	0.077	0.151	0.0030	0.0016	0.0011	0.0013	0.0058	0.0031	0.0020	0.0027	-0.0029	-0.0015	-0.0009	-0.0014	0.7802	0.27	
43	709+00.000	710+30.000	0.0246	0.687	1.558	0.0264	0.0158	0.0096	0.0106	0.0599	0.0365	0.0206	0.0234	-0.0335	-0.0207	-0.0110	-0.0128	1.0727	0.36	
44	710+30.000	710+47.850	0.0034	0.068	0.132	0.0026	0.0014	0.0010	0.0012	0.0051	0.0027	0.0018	0.0024	-0.0025	-0.0013	-0.0008	-0.0012	0.7724	0.26	
45	710+47.850	725+00.000	0.2750	18.323	10.771	0.7047	0.3140	0.2144	0.3907	0.4143	0.2203	0.1442	0.1939	0.2905	0.0937	0.0702	0.1968	2.5624	0.87	
46	725+00.000	727+52.350	0.0478	0.960	1.872	0.0369	0.0203	0.0139	0.0166	0.0720	0.0383	0.0251	0.0337	-0.0351	-0.0180	-0.0112	-0.0171	0.7724	0.26	
47	727+52.350	735+00.000	0.1416	2.844	5.545	0.1094	0.0602	0.0411	0.0492	0.2133	0.1134	0.0742	0.0998	-0.1039	-0.0533	-0.0332	-0.0506	0.7724	0.26	
48	735+00.000	755+50.000	0.3883	7.797	15.205	0.2999	0.1650	0.1126	0.1349	0.5848	0.3111	0.2035	0.2737	-0.2849	-0.1461	-0.0909	-0.1388	0.7724	0.26	
49	755+50.000	756+90.000	0.0265	0.740	1.678	0.0284	0.0170	0.0104	0.0114	0.0645	0.0393	0.0222	0.0252	-0.0361	-0.0223	-0.0118	-0.0138	1.0727	0.36	
50	756+90.000	757+00.000	0.0019	0.038	0.076	0.0015	0.0008	0.0006	0.0007	0.0029	0.0015	0.0010	0.0014	-0.0014	-0.0007	-0.0005	-0.0007	0.7802	0.27	
51	757+00.000	763+30.000	0.1193	2.396	4.673	0.0922	0.0507	0.0346	0.0415	0.1797	0.0956	0.0626	0.0841	-0.0876	-0.0449	-0.0279	-0.0427	0.7724	0.26	
52	763+30.000	764+00.000	0.0133	0.266	0.519	0.0102	0.0056	0.0038	0.0046	0.0200	0.0106	0.0070	0.0093	-0.0097	-0.0050	-0.0031	-0.0047	0.7724	0.26	
53	764+00.000	764+50.000	0.0095	0.190	0.371	0.0073	0.0040	0.0027	0.0033	0.0143	0.0076	0.0050	0.0067	-0.0069	-0.0036	-0.0022	-0.0034	0.7724	0.26	
54	764+50.000	765+52.550	0.0194	0.390	0.761	0.0150	0.0083	0.0056	0.0067	0.0293	0.0156	0.0102	0.0137	-0.0143	-0.0073	-0.0045	-0.0069	0.7724	0.26	
55	765+52.550	777+80.000	0.2325	8.935	9.104	0.3437	0.1340	0.0915	0.2097	0.3502	0.1862	0.1219	0.1639	-0.0065	-0.0523	-0.0304	0.0458	1.4783	0.50	
56	777+80.000	778+80.000	0.0189	0.528	1.198	0.0203	0.0121	0.0074	0.0082	0.0461	0.0281	0.0159	0.0180	-0.0258	-0.0159	-0.0084	-0.0098	1.0727	0.36	
57	778+80.000	779+00.000	0.0038	0.077	0.151	0.0030	0.0016	0.0011	0.0013	0.0058	0.0031	0.0020	0.0027	-0.0029	-0.0015	-0.0009	-0.0014	0.7802	0.27	
58	779+00.000	780+45.930	0.0276	0.555	1.082	0.0213	0.0117	0.0080	0.0096	0.0416	0.0221	0.0145	0.0195	-0.0203	-0.0104	-0.0065	-0.0099	0.7724	0.26	
59	780+45.930	785+40.000	0.0936	1.879	3.664	0.0723	0.0398	0.0271	0.0325	0.1409	0.0750	0.0491	0.0660	-0.0687	-0.0352	-0.0219	-0.0335	0.7724	0.26	
60	785+40.000	785+50.000	0.0019	0.038	0.074	0.0015	0.0008	0.0005	0.0007	0.0029	0.0015	0.0010	0.0013	-0.0014	-0.0007	-0.0004	-0.0007	0.7724	0.26	
61	785+50.000	786+09.000	0.0112	0.224	0.438	0.0086	0.0047	0.0032	0.0039	0.0168	0.0090	0.0059	0.0079	-0.0082	-0.0042	-0.0026	-0.0040	0.7724	0.26	
62	786+09.000	786+50.000	0.0078	0.156	0.304	0.0060	0.0033	0.0023	0.0027	0.0117	0.0062	0.0041	0.0055	-0.0057	-0.0029	-0.0018	-0.0028	0.7724	0.26	
63	786+50.000	801+10.000	0.2765	14.086	10.829	0.5418	0.2903	0.1982	0.2515	0.4165	0.2215	0.1450	0.1950	0.1253	0.0688	0.0532	0.0565	1.9593	0.67	
64	801+10.000	801+61.000	0.0097	0.194	0.378	0.0075	0.0041	0.0028	0.0034	0.0145	0.0077	0.0051	0.0068	-0.0071	-0.0036	-0.0023	-0.0035	0.7724	0.26	
65	801+61.000	802+30.000	0.0131	4.529	0.512	0.1742	0.1653	0.1128	0.0089	0.0197	0.0105	0.0069	0.0092	0.1545	0.1548	0.1060	-0.0003	13.3297	4.54	
66	802+30.000	802+40.000	0.0019	0.038	0.074	0.0015	0.0008	0.0005	0.0007	0.0029	0.0015	0.0010	0.0013	-0.0014	-0.0007	-0.0004	-0.0007	0.7724	0.26	
67	802+40.000	808+30.000	0.1117	2.244	4.376	0.0863	0.0475	0.0324	0.0388	0.1683	0.0895	0.0586	0.0788	-0.0820	-0.0420	-0.0262	-0.0400	0.7724	0.26	
68	808+30.000	808+80.000	0.0095	0.192	0.378	0.0074	0.0041	0.0028	0.0033	0.0145	0.0077	0.0051	0.0068	-0.0072	-0.0037	-0.0023	-0.0035	0.7802	0.27	
69	808+80.000	809+00.000	0.0038	0.106	0.240	0.0041	0.0024	0.0015	0.0016	0.0092	0.0056	0.0032	0.0036	-0.0052	-0.0032	-0.0017	-0.0020	1.0727	0.36	
468th Ave (v1)	809+00.000			30.877	45.501	1.1876	0.7630	0.5027	0.4246	1.7500	0.8386	0.4933	0.9115	-0.5624	-0.0756	0.0094	-0.4869			0.35
70	809+00.000	809+60.000	0.0114	0.400	0.998	0.0154	0.0091	0.0056	0.0063	0.0384	0.0231	0.0128	0.0153	-0.0230	-0.0140	-0.0072	-0.0090	1.3553	0.40	
71	809+60.000	810+00.000	0.0076	0.193	0.414	0.0074	0.0041	0.0028	0.0033	0.0159	0.0084	0.0054	0.0076	-0.0085	-0.0043	-0.0026	-0.0042	0.9798	0.29	
72	810+00.000	810+20.000	0.0038	0.089	0.175	0.0034	0.0019	0.0013	0.0015	0.0067	0.0035	0.0023	0.0032	-0.0033	-0.0017	-0.0010	-0.0017	0.9039	0.27	
73	810+20.000	816+00.000	0.1098	7.498	4.984	0.2884	0.0848	0.0579	0.2036	0.1917	0.1006	0.0651	0.0910	0.0967	-0.0158	-0.0071	0.1125	2.6254	0.78	
74	816+00.000	816+70.000	0.0133	0.308	0.602	0.0119	0.0065	0.0045	0.0053	0.0231	0.0121	0.0079	0.0110	-0.0113	-0.0056	-0.0034	-0.0057	0.8948	0.27	
75	816+70.000	817+20.000	0.0095	0.220	0.430	0.0085	0.0047	0.0032	0.0038	0.0165	0.0087	0.0056	0.0078	-0.0081	-0.0040	-0.0024	-0.0040	0.8948	0.27	

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr	Expected FI Crash Frequency (crashes/yr	Expected FI no/C Crash Frequency (crashes/yr	Expected PDO Crash Frequency (crashes/yr	Predicted Total Crash Frequency (crashes/yr	Predicted FI Crash Frequency (crashes/yr	Predicted FI no/C Crash Frequency (crashes/yr	Predicted PDO Crash Frequency (crashes/yr	(Expected - Predicted) Total Crash Frequency (crashes/yr	(Expected - Predicted) FI Crash Frequency (crashes/yr	(Expected - Predicted) FI no/C Crash Frequency (crashes/yr	(Expected - Predicted) PDO Crash Frequency (crashes/yr	Expected Crash Rate (crashes/ mi/yr)	Expected Travel Crash Rate (crashes/ million veh-mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
76	817+20.000	853+70.000	0.6913	40.796	31.361	1.5691	0.9058	0.6183	0.6633	1.2062	0.6332	0.4094	0.5730	0.3629	0.2725	0.2089	0.0903	2.2698	0.67	
77	853+70.000	854+00.000	0.0057	0.132	0.258	0.0051	0.0028	0.0019	0.0023	0.0099	0.0052	0.0034	0.0047	-0.0048	-0.0024	-0.0015	-0.0024	0.8948	0.27	
78	854+00.000	854+16.000	0.0030	0.070	0.138	0.0027	0.0015	0.0010	0.0012	0.0053	0.0028	0.0018	0.0025	-0.0026	-0.0013	-0.0008	-0.0013	0.8948	0.27	
79	854+16.000	854+80.000	0.0121	0.282	0.550	0.0108	0.0060	0.0041	0.0049	0.0211	0.0111	0.0072	0.0100	-0.0103	-0.0051	-0.0031	-0.0052	0.8948	0.27	
80	854+80.000	860+90.000	0.1155	2.688	5.241	0.1034	0.0569	0.0388	0.0465	0.2016	0.1058	0.0684	0.0958	-0.0982	-0.0490	-0.0296	-0.0493	0.8948	0.27	
81	860+90.000	861+85.000	0.0180	0.423	0.833	0.0163	0.0089	0.0061	0.0073	0.0320	0.0168	0.0109	0.0152	-0.0158	-0.0079	-0.0048	-0.0079	0.9039	0.27	
82	861+85.000	862+00.000	0.0028	0.097	0.231	0.0037	0.0022	0.0014	0.0015	0.0089	0.0053	0.0030	0.0035	-0.0051	-0.0031	-0.0016	-0.0020	1.3127	0.39	
SD38/Hwy139 (v1)	862+00.000			37.280	89.593	1.4338	0.6694	0.3723	0.7644	3.4459	1.8214	0.9272	1.6245	-2.0120	-1.1520	-0.5549	-0.8600			0.28
83	862+00.000	862+50.000	0.0095	0.379	0.903	0.0146	0.0087	0.0053	0.0059	0.0347	0.0207	0.0112	0.0141	-0.0201	-0.0120	-0.0059	-0.0082	1.5407	0.40	
84	862+50.000	862+60.000	0.0019	0.073	0.166	0.0028	0.0017	0.0010	0.0011	0.0064	0.0038	0.0021	0.0026	-0.0036	-0.0021	-0.0010	-0.0015	1.4857	0.38	
85	862+60.000	863+10.000	0.0095	0.257	0.505	0.0099	0.0054	0.0037	0.0044	0.0194	0.0101	0.0064	0.0094	-0.0096	-0.0046	-0.0027	-0.0049	1.0426	0.27	
86	863+10.000	869+00.000	0.1117	8.700	5.848	0.3346	0.0992	0.0677	0.2354	0.2249	0.1166	0.0745	0.1083	0.1097	-0.0174	-0.0068	0.1271	2.9945	0.77	
87	869+00.000	869+70.000	0.0133	0.356	0.694	0.0137	0.0075	0.0051	0.0062	0.0267	0.0138	0.0088	0.0128	-0.0130	-0.0063	-0.0037	-0.0067	1.0321	0.27	
88	869+70.000	870+20.000	0.0095	0.254	0.495	0.0098	0.0054	0.0037	0.0044	0.0191	0.0099	0.0063	0.0092	-0.0093	-0.0045	-0.0026	-0.0048	1.0321	0.27	
89	870+20.000	881+80.000	0.2197	11.597	11.497	0.4460	0.3089	0.2109	0.1371	0.4422	0.2293	0.1465	0.2129	0.0039	0.0796	0.0644	-0.0758	2.0302	0.52	
90	881+80.000	882+31.000	0.0097	0.259	0.505	0.0100	0.0055	0.0037	0.0045	0.0194	0.0101	0.0064	0.0094	-0.0095	-0.0046	-0.0027	-0.0049	1.0321	0.27	
91	882+31.000	883+00.000	0.0131	0.351	0.684	0.0135	0.0074	0.0051	0.0061	0.0263	0.0136	0.0087	0.0127	-0.0128	-0.0062	-0.0037	-0.0066	1.0321	0.27	
92	883+00.000	887+90.000	0.0928	8.192	4.856	0.3151	0.2475	0.1689	0.0676	0.1868	0.0968	0.0619	0.0899	0.1283	0.1506	0.1070	-0.0223	3.3950	0.88	
93	887+90.000	888+20.000	0.0057	0.154	0.303	0.0059	0.0033	0.0022	0.0027	0.0117	0.0060	0.0039	0.0056	-0.0057	-0.0028	-0.0016	-0.0030	1.0426	0.27	
94	888+20.000	889+30.000	0.0208	0.805	1.826	0.0310	0.0185	0.0113	0.0124	0.0702	0.0418	0.0226	0.0284	-0.0393	-0.0233	-0.0113	-0.0160	1.4857	0.38	
95	889+30.000	889+50,000	0.0038	0.103	0.202	0.0039	0.0022	0.0015	0.0018	0.0078	0.0040	0.0026	0.0037	-0.0038	-0.0019	-0.0011	-0.0020	1.0426	0.27	
96	889+50.000	894+50.000	0.0947	2.541	4.955	0.0977	0.0538	0.0367	0.0440	0.1906	0.0988	0.0632	0.0918	-0.0929	-0.0451	-0.0265	-0.0478	1.0321	0.27	
97	894+50.000	895+15.000	0.0123	0.330	0.644	0.0127	0.0070	0.0048	0.0057	0.0248	0.0128	0.0032	0.0119	-0.0121	-0.0059	-0.0233	-0.0062	1.0321	0.27	
98	895+15.000	895+60,000	0.0123	0.229	0.446	0.0027	0.0048	0.0033	0.0037	0.0172	0.0089	0.0052	0.0083	-0.0084	-0.0037	-0.0034	-0.0043	1.0321	0.27	
99	895+60.000	898+00.000	0.0455	1.220	2.379	0.0469	0.0048	0.0033	0.0211	0.0172	0.0474	0.0303	0.0441	-0.0084	-0.0216	-0.0024	-0.0043	1.0321	0.27	
100	898+00.000	906+70.000	0.1648	4.439	8.687	0.1707	0.0238	0.0170	0.0768	0.3341	0.1732	0.0303	0.1609	-0.1634	-0.0210	-0.0127	-0.0229	1.0321	0.27	
	906+70.000	907+21.000	0.1048	0.260	0.509	0.1707	0.0959	0.0041	0.0768	0.0196	0.1732	0.1107	0.1009	-0.1034	-0.0793	-0.0466	-0.0041	1.0361	0.27	
101	908+70.000	907+21.000		0.260	0.589	0.0100	0.0055	0.0038	0.0043	0.0196	0.0102	0.0065		-0.0096	-0.0047	-0.0027	-0.0049	1.0361	0.27	
102	907+21.000	907+80.000	0.0112	0.301	0.100	0.0116	0.0064	0.0043	0.0052	0.0227	0.0117	0.0075	0.0109	-0.0111	-0.0054	-0.0032	-0.0057	1.0361		
	1																		0.27	
104	907+90.000	913+70.000	0.1098	2.959	5.792	0.1138	0.0626	0.0427	0.0512	0.2227	0.1155	0.0738	0.1073	-0.1089	-0.0529	-0.0311	-0.0561	1.0361	0.27	
105	913+70.000	914+00.000	0.0057	0.155	0.306	0.0059	0.0033	0.0022	0.0027	0.0118	0.0061	0.0039	0.0057	-0.0058	-0.0028	-0.0017	-0.0030	1.0466	0.27	
106	914+00.000	914+30.000	0.0057	0.119	0.235	0.0046	0.0025	0.0017	0.0021	0.0090	0.0048	0.0031	0.0042	-0.0045	-0.0023	-0.0014	-0.0022	0.8058	0.27	
107	914+30.000	914+40.000	0.0019	0.055	0.124	0.0021	0.0013	0.0008	0.0008	0.0048	0.0029	0.0016	0.0019	-0.0027	-0.0017	-0.0009	-0.0010	1.1098	0.37	
108	914+40.000	915+40.000	0.0189	0.585	1.463	0.0225	0.0134	0.0082	0.0091	0.0563	0.0342	0.0192	0.0221	-0.0338	-0.0209	-0.0111	-0.0129	1.1880	0.39	
LaMesa/SD38 (v1)	915+00.000	016.00.000	0.0114	45.389	83.561	1.7457	0.7046	0.4318	1.0411	3.2139	1.8143	0.9753	1.3996	-1.4682	-1.1097	-0.5434	-0.3585	0.0050	0.22	0.47
109	915+40.000	916+00.000	0.0114	0.238	0.470	0.0092		0.0034	0.0041	0.0181	0.0096	0.0063	0.0085	-0.0089	-0.0046	-0.0028	-0.0044	0.8058	0.27	
110	916+00.000	921+00.000	0.0947	1.964	3.844	0.0755	0.0416	0.0284	0.0340	0.1479	0.0785	0.0512	0.0694	-0.0723	-0.0369	-0.0229	-0.0354	0.7977	0.26	
111	921+00.000	921+90.000	0.0170	0.353	0.692	0.0136	0.0075	0.0051	0.0061	0.0266	0.0141	0.0092	0.0125	-0.0130	-0.0066	-0.0041	-0.0064	0.7977	0.26	
112	921+90.000	922+00.000	0.0019	0.039	0.077	0.0015	0.0008	0.0006	0.0007	0.0030	0.0016	0.0010	0.0014	-0.0014	-0.0007	-0.0005	-0.0007	0.7977	0.26	
113	922+00.000	922+59.000	0.0112	0.232	0.454	0.0089	0.0049	0.0033	0.0040	0.0174	0.0093	0.0060	0.0082	-0.0085	-0.0044	-0.0027	-0.0042	0.7977	0.26	
114	922+59.000	923+00.000	0.0078	0.161	0.315	0.0062	0.0034	0.0023	0.0028	0.0121	0.0064	0.0042	0.0057	-0.0059	-0.0030	-0.0019	-0.0029	0.7977	0.26	

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period		Expected Total Crash Frequency (crashes/yr	Expected FI Crash Frequency (crashes/yr	Expected FI no/C Crash Frequency (crashes/yr	Expected PDO Crash Frequency (crashes/yr	Predicted Total Crash Frequency (crashes/yr	FI Crash	Crash	Predicted PDO Crash Frequency (crashes/yr	Total Crash	FI Crash Frequency	FI no/C Crash	Predicted) PDO Crash	Expected Crash Rate (crashes/ mi/yr)	Crash Rate	Expected Intersection Travel Crash Rate (crashes/million veh)
115	923+00.000	941+70.000	0.3542	11.752	14.377	0.4520	0.2940	0.2007	0.1580	0.5530	0.2935	0.1916	0.2595	-0.1009	0.0006	0.0091	-0.1015	1.2763	0.42	
116	941+70.000	948+50.000	0.1288	2.671	5.228	0.1027	0.0565	0.0386	0.0462	0.2011	0.1067	0.0697	0.0944	-0.0983	-0.0502	-0.0311	-0.0481	0.7977	0.26	
All Segments			6.8845	409.421	399.083	15.7469	8.7474	5.6810	6.9996	15.3493	8.5222	4.9517	6.8271	0.3976	0.2252	0.7293	0.1724	2.2873	0.60	
All Intersections				212.085	427.197	8.1571	3.8625	2.1827	4.2946	16.4306	8.2648	4.3625	8.1659	-8.2735	-4.4022	-2.1798	-3.8713			0.25
Total		·	6.8845	621.506	826.279	23.9041	12.6099	7.8637	11.2942	31.7800	16.7869	9.3142	14.9930	-7.8759	-4.1770	-1.4505	-3.6989	3.4722		

Table 13. Expected Crash Frequencies and Rates by Horizontal Design Element (Section 3)

Title	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected FI no/C Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted FI no/C Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) FI no/C Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi llion veh- mi)
Tangent	585+00.000	594+84.940	0.1865	12.419	12.987	0.4777	0.3553	0.2214	0.1223	0.4995	0.2945	0.1563	0.2050	-0.0218	0.0608	0.0652	-0.0826	2.5607	0.59
Simple Curve 1	594+84.940	609+21.930	0.2722	49.017	28.760	1.8853	0.8216	0.5119	1.0637	1.1062	0.6443	0.3342	0.4618	0.7791	0.1773	0.1778	0.6019	6.9271	1.30
Tangent	609+21.930	624+64.300	0.2921	22.007	29.094	0.8464	0.4103	0.2557	0.4361	1.1190	0.6472	0.3310	0.4719	-0.2726	-0.2368	-0.0754	-0.0358	2.8975	0.53
Simple Curve 2	624+64.300	624+64.530	0.0000	0.003	0.004	0.0001	0.0001	0.0000	0.0001	0.0002	0.0001	0.0000	0.0001	-0.0000	-0.0000	-0.0000	0.0000	3.0271	0.55
Tangent	624+64.530	636+92.820	0.2326	10.091	21.149	0.3881	0.2351	0.1465	0.1530	0.8134	0.4704	0.2406	0.3430	-0.4253	-0.2353	-0.0941	-0.1900	1.6685	0.30
Simple Curve 3	636+92.820	647+26.050	0.1957	17.075	19.003	0.6567	0.2830	0.1763	0.3738	0.7309	0.4227	0.2162	0.3082	-0.0742	-0.1397	-0.0399	0.0656	3.3560	0.61
Tangent	647+26.050	672+86.110	0.4849	37.317	44.231	1.4353	0.6571	0.4094	0.7782	1.7012	0.9838	0.5033	0.7173	-0.2659	-0.3267	-0.0938	0.0608	2.9602	0.54
Simple Curve 4	672+86.110	683+82.710	0.2077	65.406	16.578	2.5156	1.8085	1.1271	0.7071	0.6376	0.3734	0.1957	0.2642	1.8780	1.4351	0.9314	0.4429	12.1123	2.27
Tangent	683+82.710	693+85.010	0.1898	5.075	10.371	0.1952	0.1188	0.0745	0.0764	0.3989	0.2390	0.1308	0.1599	-0.2037	-0.1202	-0.0563	-0.0835	1.0282	0.29
Simple Curve 5	693+85.010	710+47.850	0.3149	7.953	16.248	0.3059	0.1778	0.1157	0.1281	0.6249	0.3564	0.2105	0.2685	-0.3191	-0.1786	-0.0949	-0.1405	0.9712	0.30
Tangent	710+47.850	727+51.450	0.3227	19.279	12.636	0.7415	0.3343	0.2282	0.4072	0.4860	0.2585	0.1691	0.2275	0.2555	0.0758	0.0590	0.1798	2.2982	0.78
Simple Curve 6	727+51.450	727+52.350	0.0002	0.003	0.007	0.0001	0.0001	0.0000	0.0001	0.0003	0.0001	0.0001	0.0001	-0.0001	-0.0001	-0.0000	-0.0001	0.7724	0.26
Tangent	727+52.350	765+52.550	0.7197	14.661	28.827	0.5639	0.3116	0.2115	0.2523	1.1087	0.5947	0.3856	0.5140	-0.5448	-0.2832	-0.1742	-0.2617	0.7835	0.27
Simple Curve 7	765+52.550	780+45.930	0.2828	10.095	11.536	0.3883	0.1595	0.1080	0.2288	0.4437	0.2396	0.1542	0.2041	-0.0554	-0.0801	-0.0462	0.0247	1.3728	0.47
Tangent	780+45.930	948+50.000	3.1826	139.017	147.652	5.3468	3.0744	2.0947	2.2724	5.6789	2.9974	1.9239	2.6815	-0.3321	0.0770	0.1708	-0.4091	1.6800	0.50

Table 14. Predicted Crash Frequencies by Year (Section 3)

Year	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)
2025	17.10	9.05	52.953	5.50	32.182	8.04	47.047
2026	18.92	9.99	52.834	6.00	31.718	8.92	47.166
2027	20.77	10.95	52.723	6.50	31.292	9.82	47.277
2028	22.64	11.91	52.621	7.00	30.897	10.73	47.379
2029	24.55	12.90	52.526	7.50	30.531	11.66	47.474
2030	25.25	13.26	52.510	7.68	30.401	11.99	47.490
2031	25.95	13.62	52.496	7.86	30.274	12.33	47.504
2032	26.66	13.99	52.483	8.04	30.152	12.67	47.517
2033	27.37	14.36	52.473	8.22	30.033	13.01	47.527
2034	28.08	14.73	52.463	8.40	29.919	13.35	47.537
2035	28.79	15.10	52.455	8.58	29.808	13.69	47.545
2036	29.60	15.53	52.480	8.79	29.696	14.06	47.520
2037	30.44	15.99	52.522	9.01	29.586	14.45	47.478
2038	31.29	16.45	52.563	9.22	29.480	14.84	47.437
2039	32.13	16.90	52.602	9.44	29.377	15.23	47.398
2040	32.97	17.36	52.641	9.65	29.277	15.61	47.359
2041	34.37	18.12	52.725	10.01	29.109	16.25	47.275
2042	35.74	18.88	52.809	10.35	28.949	16.87	47.191
2043	37.09	19.62	52.889	10.68	28.796	17.47	47.111
2044	38.42	20.35	52.968	11.01	28.651	18.07	47.032
2045	39.74	21.08	53.043	11.33	28.511	18.66	46.957
2046	41.06	21.81	53.116	11.65	28.377	19.25	46.884
2047	42.37	22.54	53.186	11.97	28.248	19.84	46.814
2048	43.69	23.26	53.253	12.29	28.124	20.42	46.747
2049	44.99	23.99	53.319	12.60	28.005	21.00	46.681
2050	46.30	24.72	53.382	12.91	27.889	21.59	46.618
Total	826.28	436.46	52.822	242.17	29.308	389.82	47.178
Average	31.78	16.79	52.822	9.31	29.308	14.99	47.178

Table 15. Expected Crash Frequencies by Year (Section 3)

Year	Total Crashes	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)
2025	12.86	6.80	52.883	4.64	36.123	6.06	47.117
2026	14.23	7.51	52.764	5.07	35.602	6.72	47.236
2027	15.62	8.22	52.653	5.49	35.123	7.39	47.347
2028	17.03	8.95	52.551	5.91	34.680	8.08	47.450
2029	18.47	9.69	52.457	6.33	34.269	8.78	47.544
2030	18.99	9.96	52.440	6.48	34.123	9.03	47.561
2031	19.52	10.23	52.426	6.63	33.981	9.29	47.575
2032	20.05	10.51	52.414	6.79	33.843	9.54	47.587
2033	20.59	10.79	52.403	6.94	33.711	9.80	47.598
2034	21.12	11.06	52.393	7.09	33.582	10.05	47.608
2035	21.66	11.35	52.385	7.25	33.457	10.31	47.616
2036	22.26	11.67	52.410	7.42	33.332	10.59	47.590
2037	22.90	12.01	52.452	7.60	33.209	10.89	47.549
2038	23.53	12.35	52.493	7.79	33.089	11.18	47.508
2039	24.16	12.69	52.532	7.97	32.974	11.47	47.468
2040	24.80	13.04	52.571	8.15	32.862	11.76	47.429
2041	25.85	13.61	52.655	8.45	32.673	12.24	47.345
2042	26.89	14.18	52.739	8.74	32.493	12.71	47.261
2043	27.90	14.73	52.819	9.02	32.322	13.16	47.181
2044	28.90	15.29	52.897	9.29	32.158	13.61	47.102
2045	29.89	15.84	52.972	9.57	32.002	14.06	47.027
2046	30.89	16.38	53.045	9.84	31.852	14.50	46.954
2047	31.87	16.93	53.115	10.11	31.707	14.94	46.884
2048	32.86	17.48	53.183	10.37	31.568	15.38	46.816
2049	33.84	18.02	53.248	10.64	31.434	15.82	46.751
2050	34.83	18.57	53.311	10.90	31.304	16.26	46.687
Total	621.51	327.86	52.752	204.46	32.897	293.65	47.248
Average	23.90	12.61	52.752	7.86	32.897	11.29	47.248

Table 16. Comparing Predicted and Expected Crashes for the Evaluation Period (Section 3)

Scope	Total Crashes	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)
Predicted	826.28	436.46	52.822	242.17	29.308	389.82	47.178
Expected	621.51	327.86	52.752	204.46	32.897	293.65	47.248
Expected - Predicted	-204.77	-108.60		-37.71		-96.17	
Percent Difference	-32.95	-33.12		-18.45		-32.75	

Table 17. Expected Crash Severity by Ramp Terminal or Roundabout (Section 3)

Seg. No.	Туре	Type Crashes (crashes) Incapacitating Injury (A) Crashes (crashes) Injury		Non-Incapacitating Injury (B) Crashes (crashes)	Possible Injury (C) Crashes (crashes)	No Injury (O) Crashes (crashes)
4	FRERampTerminal	0.1307	0.6864	2.2706	7.9648	9.1729

 Table 18. Expected
 Crash Type Distribution (Section 3)

Element Type	Crash Type	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)	Total Crashes	Percent Total (%)
Highway Segment	Single	103.33	16.6	77.46	12.5	88.79	14.3	194.37	31.3
Highway Segment	Total Single Vehicle Crashes	103.33	16.6	77.46	12.5	88.79	14.3	194.37	31.3
Highway Segment	Angle Collision	49.47	8.0	30.59	4.9	39.07	6.3	88.51	14.2
Highway Segment	Head-on Collision	4.99	0.8	4.63	0.7	0.26	0.0	3.14	0.5
Highway Segment	Rear-end Collision	55.08	8.9	24.98	4.0	29.18	4.7	76.94	12.4
Highway Segment	Sideswipe	8.80	1.4	4.99	0.8	16.33	2.6	30.07	4.8
Highway Segment	Total Multiple Vehicle Crashes	118.34	19.0	65.19	10.5	84.84	13.7	198.66	32.0
Highway Segment	Total Highway Segment Crashes	227.43	36.6	147.71	23.8	181.99	29.3	409.42	65.9
Highway Segment	Other Collision	5.76	0.9	5.06	0.8	8.36	1.3	16.40	2.6
Intersection	Single	13.98	2.2	11.95	1.9	24.92	4.0	39.47	6.4
Intersection	Total Single Vehicle Crashes	13.98	2.2	11.95	1.9	24.92	4.0	39.47	6.4
Intersection	Angle Collision	45.97	7.4	30.94	5.0	28.84	4.6	72.86	11.7
Intersection	Head-on Collision	1.87	0.3	1.53	0.2	1.59	0.3	3.36	0.5
Intersection	Rear-end Collision	19.40	3.1	6.39	1.0	25.46	4.1	45.10	7.3
Intersection	Sideswipe	3.92	0.6	2.40	0.4	16.14	2.6	21.11	3.4
Intersection	Total Multiple Vehicle Crashes	71.17	11.5	41.26	6.6	72.04	11.6	142.42	22.9
Intersection	Total Intersection Crashes	89.45	14.4	56.75	9.1	102.46	16.5	191.69	30.9
Intersection	Other Collision	4.30	0.7	3.54	0.6	5.51	0.9	9.81	1.6
Ramp Terminal	Collision with Animal	0.00	0.0	0.00	0.0	0.00	0.0	0.00	0.0
Ramp Terminal	Collision with Fixed Object	0.86	0.1	0.00	0.0	1.45	0.2	2.31	0.4
Ramp Terminal	Collision with Other Object	0.00	0.0	0.00	0.0	0.05	0.0	0.05	0.0
Ramp Terminal	Other Single-vehicle Collision	0.72	0.1	0.00	0.0	0.24	0.0	0.96	0.2
Ramp Terminal	Collision with Parked Vehicle	0.08	0.0	0.00	0.0	0.14	0.0	0.21	0.0
Ramp Terminal	Total Single Vehicle Crashes	1.66	0.3	0.00	0.0	1.87	0.3	3.53	0.6
Ramp Terminal	Angle Collision	5.77	0.9	0.00	0.0	3.41	0.5	9.18	1.5

Element Type	Crash Type	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)	Total Crashes	Percent Total (%)
Ramp Terminal	Head-on Collision	0.22	0.0	0.00	0.0	0.14	0.0	0.36	0.1
Ramp Terminal	Other Multiple-vehicle Collision	0.14	0.0	0.00	0.0	0.24	0.0	0.38	0.1
Ramp Terminal	Rear-end Collision	3.04	0.5	0.00	0.0	2.53	0.4	5.57	0.9
Ramp Terminal	Sideswipe, Same Direction Collision	0.22	0.0	0.00	0.0	0.98	0.2	1.20	8 0.1 7 0.9 0 0.2
Ramp Terminal	Total Multiple Vehicle Crashes	9.39	1.5	0.00	0.0	7.30	1.2	16.70	2.7
Ramp Terminal	Total Ramp Terminal Crashes	11.05	1.8	0.00	0.0	9.17	1.5	20.23	3.3
	Total Crashes	327.94	52.8	204.46	32.9	293.62	47.3	621.34	100.0

**Table 19. Evaluation Message** 

Start Location (Sta. ft)	End Location (Sta. ft)	Message
585+00.000	594+84.940	Warning: for segment #1 (585+00.000 to 594+84.940 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
585+00.000	594+84.940	Warning: for segment #1 (585+00.000 to 594+84.940 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
594+84.940	600+00.000	Warning: for segment #2 (594+84.940 to 600+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
594+84.940	600+00.000	Warning: for segment #2 (594+84.940 to 600+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
600+00.000	601+00.000	Warning: for segment #3 (600+00.000 to 601+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
600+00.000	601+00.000	Warning: for segment #3 (600+00.000 to 601+00.000), no foreslope data available for right side of road for use by AFM3ru, using 1.0
601+00.000	602+00.000	Warning: for segment #4 (601+00.000 to 602+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
601+00.000	602+00.000	Warning: for segment #4 (601+00.000 to 602+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
602+00.000	605+00.000	Warning: for segment #5 (602+00.000 to 605+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
602+00.000	605+00.000	Warning: for segment #5 (602+00.000 to 605+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+00.000	605+60.000	Warning: for segment #6 (605+00.000 to 605+60.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+00.000	605+60.000	Warning: for segment #6 (605+00.000 to 605+60.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+60.000	605+70.000	Warning: for segment #7 (605+60.000 to 605+70.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+60.000	605+70.000	Warning: for segment #7 (605+60.000 to 605+70.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+70.000	605+75.000	Warning: for segment #8 (605+70.000 to 605+75.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+70.000	605+75.000	Warning: for segment #8 (605+70.000 to 605+75.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+75.000	609+00.000	Warning: for segment #9 (605+75.000 to 609+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+75.000	609+00.000	Warning: for segment #9 (605+75.000 to 609+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
609+00.000	609+21.930	Warning: for segment #10 (609+00.000 to 609+21.930 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
609+00.000	609+21.930	Warning: for segment #10 (609+00.000 to 609+21.930 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
609+21.930	611+40.000	Warning: for segment #11 (609+21.930 to 611+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
609+21.930	611+40.000	Warning: for segment #11 (609+21.930 to 611+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
611+40.000	612+50.000	Warning: for segment #12 (611+40.000 to 612+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
611+40.000	612+50.000	Warning: for segment #12 (611+40.000 to 612+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
612+50.000	624+64.530	Warning: for segment #13 (612+50.000 to 624+64.530 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
612+50.000	624+64.530	Warning: for segment #13 (612+50.000 to 624+64.530 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
624+64.530	636+92.820	Warning: for segment #14 (624+64.530 to 636+92.820 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
624+64.530	636+92.820	Warning: for segment #14 (624+64.530 to 636+92.820 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
636+92.820	639+00.000	Warning: for segment #15 (636+92.820 to 639+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
636+92.820	639+00.000	Warning: for segment #15 (636+92.820 to 639+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0

Start Location (Sta. ft)	End Location (Sta. ft)	Message
639+00.000	640+00.000	Warning: for segment #16 (639+00.000 to 640+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
639+00.000	640+00.000	Warning: for segment #16 (639+00.000 to 640+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
640+00.000	647+26.050	Warning: for segment #17 (640+00.000 to 647+26.050 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
640+00.000	647+26.050	Warning: for segment #17 (640+00.000 to 647+26.050 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
647+26.050	667+80.000	Warning: for segment #18 (647+26.050 to 667+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
647+26.050	667+80.000	Warning: for segment #18 (647+26.050 to 667+80.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
667+80.000	668+80.000	Warning: for segment #19 (667+80.000 to 668+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
667+80.000	668+80.000	Warning: for segment #19 (667+80.000 to 668+80.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
668+80.000	672+86.110	Warning: for segment #20 (668+80.000 to 672+86.110 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
668+80.000	672+86.110	Warning: for segment #20 (668+80.000 to 672+86.110 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
672+86.110	676+00.000	Warning: for segment #21 (672+86.110 to 676+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
672+86.110	676+00.000	Warning: for segment #21 (672+86.110 to 676+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
676+00.000	677+50.000	Warning: for segment #22 (676+00.000 to 677+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
676+00.000	677+50.000	Warning: for segment #22 (676+00.000 to 677+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
677+50.000	679+00.000	Warning: for segment #23 (677+50.000 to 679+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
677+50.000	679+00.000	Warning: for segment #23 (677+50.000 to 679+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
679+00.000	680+80.000	Warning: for segment #24 (679+00.000 to 680+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
679+00.000	680+80.000	Warning: for segment #24 (679+00.000 to 680+80.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
680+80.000	680+90.000	Warning: for segment #25 (680+80.000 to 680+90.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
680+80.000	680+90.000	Warning: for segment #25 (680+80.000 to 680+90.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
680+90.000	682+20.000	Warning: for segment #26 (680+90.000 to 682+20.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
680+90.000	682+20.000	Warning: for segment #26 (680+90.000 to 682+20.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
682+20.000	683+82.710	Warning: for segment #27 (682+20.000 to 683+82.710 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
682+20.000	683+82.710	Warning: for segment #27 (682+20.000 to 683+82.710 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
683+82.710	691+50.000	Warning: for segment #28 (683+82.710 to 691+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
683+82.710	691+50.000	Warning: for segment #28 (683+82.710 to 691+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
691+50.000	692+70.000	Warning: for segment #29 (691+50.000 to 692+70.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
691+50.000	692+70.000	Warning: for segment #29 (691+50.000 to 692+70.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
692+70.000	693+85.010	Warning: for segment #30 (692+70.000 to 693+85.010 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
692+70.000	693+85.010	Warning: for segment #30 (692+70.000 to 693+85.010 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
693+85.010	698+70.000	Warning: for segment #31 (693+85.010 to 698+70.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
693+85.010	698+70.000	Warning: for segment #31 (693+85.010 to 698+70.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0

Start Location (Sta. ft)	End Location (Sta. ft)	Message
698+70.000	699+00.000	Warning: for segment #32 (698+70.000 to 699+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
698+70.000	699+00.000	Warning: for segment #32 (698+70.000 to 699+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
699+00.000	699+20.000	Warning: for segment #33 (699+00.000 to 699+20.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
699+00.000	699+20.000	Warning: for segment #33 (699+00.000 to 699+20.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
699+20.000	700+40.000	Warning: for segment #34 (699+20.000 to 700+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
699+20.000	700+40.000	Warning: for segment #34 (699+20.000 to 700+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
709+00.000	710+30.000	Warning: for segment #43 (709+00.000 to 710+30.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
709+00.000	710+30.000	Warning: for segment #43 (709+00.000 to 710+30.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
755+50.000	756+90.000	Warning: for segment #49 (755+50.000 to 756+90.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
755+50.000	756+90.000	Warning: for segment #49 (755+50.000 to 756+90.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
777+80.000	778+80.000	Warning: for segment #56 (777+80.000 to 778+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
777+80.000	778+80.000	Warning: for segment #56 (777+80.000 to 778+80.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
808+80.000	809+00.000	Warning: for segment #69 (808+80.000 to 809+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
808+80.000	809+00.000	Warning: for segment #69 (808+80.000 to 809+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
809+00.000	809+60.000	Warning: for segment #70 (809+00.000 to 809+60.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
809+00.000	809+60.000	Warning: for segment #70 (809+00.000 to 809+60.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
861+85.000	862+00.000	Warning: for segment #82 (861+85.000 to 862+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
861+85.000	862+00.000	Warning: for segment #82 (861+85.000 to 862+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
862+00.000	862+50.000	Warning: for segment #83 (862+00.000 to 862+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
862+00.000	862+50.000	Warning: for segment #83 (862+00.000 to 862+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
862+50.000	862+60.000	Warning: for segment #84 (862+50.000 to 862+60.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
862+50.000	862+60.000	Warning: for segment #84 (862+50.000 to 862+60.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
888+20.000	889+30.000	Warning: for segment #94 (888+20.000 to 889+30.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
888+20.000	889+30.000	Warning: for segment #94 (888+20.000 to 889+30.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
914+30.000	914+40.000	Warning: for segment #107 (914+30.000 to 914+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
914+30.000	914+40.000	Warning: for segment #107 (914+30.000 to 914+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
914+40.000	915+40.000	Warning: for segment #108 (914+40.000 to 915+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
914+40.000	915+40.000	Warning: for segment #108 (914+40.000 to 915+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
585+00.000	594+84.940	Warning: for segment #1 (585+00.000 to 594+84.940 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
585+00.000	594+84.940	Warning: for segment #1 (585+00.000 to 594+84.940 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
594+84.940	600+00.000	Warning: for segment #2 (594+84.940 to 600+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
594+84.940	600+00.000	Warning: for segment #2 (594+84.940 to 600+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0

Start Location (Sta. ft)	End Location (Sta. ft)	Message
600+00.000	601+00.000	Warning: for segment #3 (600+00.000 to 601+00.000), no foreslope data available for left side of road for use by AFM3ru, using 1.0
600+00.000	601+00.000	Warning: for segment #3 (600+00.000 to 601+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
601+00.000	602+00.000	Warning: for segment #4 (601+00.000 to 602+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
601+00.000	602+00.000	Warning: for segment #4 (601+00.000 to 602+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
602+00.000	605+00.000	Warning: for segment #5 (602+00.000 to 605+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
602+00.000	605+00.000	Warning: for segment #5 (602+00.000 to 605+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+00.000	605+60.000	Warning: for segment #6 (605+00.000 to 605+60.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+00.000	605+60.000	Warning: for segment #6 (605+00.000 to 605+60.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+60.000	605+70.000	Warning: for segment #7 (605+60.000 to 605+70.000), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+60.000	605+70.000	Warning: for segment #7 (605+60.000 to 605+70.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+70.000	605+75.000	Warning: for segment #8 (605+70.000 to 605+75.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+70.000	605+75.000	Warning: for segment #8 (605+70.000 to 605+75.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+75.000	609+00.000	Warning: for segment #9 (605+75.000 to 609+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+75.000	609+00.000	Warning: for segment #9 (605+75.000 to 609+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
609+00.000	609+21.930	Warning: for segment #10 (609+00.000 to 609+21.930 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
609+00.000	609+21.930	Warning: for segment #10 (609+00.000 to 609+21.930 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
609+21.930	611+40.000	Warning: for segment #11 (609+21.930 to 611+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
609+21.930	611+40.000	Warning: for segment #11 (609+21.930 to 611+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
611+40.000	612+50.000	Warning: for segment #12 (611+40.000 to 612+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
611+40.000	612+50.000	Warning: for segment #12 (611+40.000 to 612+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
612+50.000	624+64.530	Warning: for segment #13 (612+50.000 to 624+64.530 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
612+50.000	624+64.530	Warning: for segment #13 (612+50.000 to 624+64.530 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
624+64.530	636+92.820	Warning: for segment #14 (624+64.530 to 636+92.820 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
624+64.530	636+92.820	Warning: for segment #14 (624+64.530 to 636+92.820 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
636+92.820	639+00.000	Warning: for segment #15 (636+92.820 to 639+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
636+92.820	639+00.000	Warning: for segment #15 (636+92.820 to 639+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
639+00.000	640+00.000	Warning: for segment #16 (639+00.000 to 640+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
639+00.000	640+00.000	Warning: for segment #16 (639+00.000 to 640+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
640+00.000	647+26.050	Warning: for segment #17 (640+00.000 to 647+26.050 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
640+00.000	647+26.050	Warning: for segment #17 (640+00.000 to 647+26.050 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
647+26.050	667+80.000	Warning: for segment #18 (647+26.050 to 667+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
647+26.050	667+80.000	Warning: for segment #18 (647+26.050 to 667+80.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0

Start Location (Sta. ft)	End Location (Sta. ft)	Message
667+80.000	668+80.000	Warning: for segment #19 (667+80.000 to 668+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
667+80.000	668+80.000	Warning: for segment #19 (667+80.000 to 668+80.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
668+80.000	672+86.110	Warning: for segment #20 (668+80.000 to 672+86.110 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
668+80.000	672+86.110	Warning: for segment #20 (668+80.000 to 672+86.110 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
672+86.110	676+00.000	Warning: for segment #21 (672+86.110 to 676+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
672+86.110	676+00.000	Warning: for segment #21 (672+86.110 to 676+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
676+00.000	677+50.000	Warning: for segment #22 (676+00.000 to 677+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
676+00.000	677+50.000	Warning: for segment #22 (676+00.000 to 677+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
677+50.000	679+00.000	Warning: for segment #23 (677+50.000 to 679+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
677+50.000	679+00.000	Warning: for segment #23 (677+50.000 to 679+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
679+00.000	680+80.000	Warning: for segment #24 (679+00.000 to 680+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
679+00.000	680+80.000	Warning: for segment #24 (679+00.000 to 680+80.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
680+80.000	680+90.000	Warning: for segment #25 (680+80.000 to 680+90.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
680+80.000	680+90.000	Warning: for segment #25 (680+80.000 to 680+90.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
680+90.000	682+20.000	Warning: for segment #26 (680+90.000 to 682+20.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
680+90.000	682+20.000	Warning: for segment #26 (680+90.000 to 682+20.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
682+20.000	683+82.710	Warning: for segment #27 (682+20.000 to 683+82.710 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
682+20.000	683+82.710	Warning: for segment #27 (682+20.000 to 683+82.710 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
683+82.710	691+50.000	Warning: for segment #28 (683+82.710 to 691+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
683+82.710	691+50.000	Warning: for segment #28 (683+82.710 to 691+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
691+50.000	692+70.000	Warning: for segment #29 (691+50.000 to 692+70.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
691+50.000	692+70.000	Warning: for segment #29 (691+50.000 to 692+70.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
692+70.000	693+85.010	Warning: for segment #30 (692+70.000 to 693+85.010 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
692+70.000	693+85.010	Warning: for segment #30 (692+70.000 to 693+85.010 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
693+85.010	698+70.000	Warning: for segment #31 (693+85.010 to 698+70.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
693+85.010	698+70.000	Warning: for segment #31 (693+85.010 to 698+70.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
698+70.000	699+00.000	Warning: for segment #32 (698+70.000 to 699+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
698+70.000	699+00.000	Warning: for segment #32 (698+70.000 to 699+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
699+00.000	699+20.000	Warning: for segment #33 (699+00.000 to 699+20.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
699+00.000	699+20.000	Warning: for segment #33 (699+00.000 to 699+20.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
699+20.000	700+40.000	Warning: for segment #34 (699+20.000 to 700+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
699+20.000	700+40.000	Warning: for segment #34 (699+20.000 to 700+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0

Start Location (Sta. ft)	End Location (Sta. ft)	Message
709+00.000	710+30.000	Warning: for segment #43 (709+00.000 to 710+30.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
709+00.000	710+30.000	Warning: for segment #43 (709+00.000 to 710+30.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
755+50.000	756+90.000	Warning: for segment #49 (755+50.000 to 756+90.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
755+50.000	756+90.000	Warning: for segment #49 (755+50.000 to 756+90.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
777+80.000	778+80.000	Warning: for segment #56 (777+80.000 to 778+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
777+80.000	778+80.000	Warning: for segment #56 (777+80.000 to 778+80.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
808+80.000	809+00.000	Warning: for segment #69 (808+80.000 to 809+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
808+80.000	809+00.000	Warning: for segment #69 (808+80.000 to 809+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
809+00.000	809+60.000	Warning: for segment #70 (809+00.000 to 809+60.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
809+00.000	809+60.000	Warning: for segment #70 (809+00.000 to 809+60.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
861+85.000	862+00.000	Warning: for segment #82 (861+85.000 to 862+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
861+85.000	862+00.000	Warning: for segment #82 (861+85.000 to 862+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
862+00.000	862+50.000	Warning: for segment #83 (862+00.000 to 862+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
862+00.000	862+50.000	Warning: for segment #83 (862+00.000 to 862+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
862+50.000	862+60.000	Warning: for segment #84 (862+50.000 to 862+60.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
862+50.000	862+60.000	Warning: for segment #84 (862+50.000 to 862+60.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
888+20.000	889+30.000	Warning: for segment #94 (888+20.000 to 889+30.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
888+20.000	889+30.000	Warning: for segment #94 (888+20.000 to 889+30.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
914+30.000	914+40.000	Warning: for segment #107 (914+30.000 to 914+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
914+30.000	914+40.000	Warning: for segment #107 (914+30.000 to 914+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
914+40.000	915+40.000	Warning: for segment #108 (914+40.000 to 915+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
914+40.000	915+40.000	Warning: for segment #108 (914+40.000 to 915+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0

## **Section 4 Evaluation**

Section: Section 4

**Evaluation Start Location:** 948+50.000 **Evaluation End Location:** 974+11.000

Area Type: Urban

Functional Class: Arterial

**Type of Alignment:** Undivided, Multilane **Model Category:** Urban/Suburban Arterial **Calibration Factor:** 4D=1.0; 4SG=1.0; 4U=1.0;

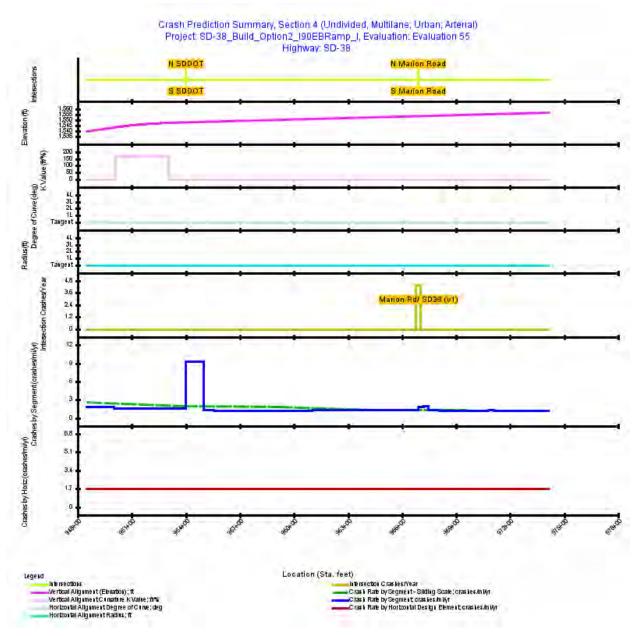


Figure 2. Crash Prediction Summary (Section 4)

**Table 20. Observed Crashes Used in the Evaluation (Section 4)** 

Year	Observed Crashes	Total Crashes Used	FI Crashes	FI no/C Crashes	PDO Crashes
2018	5	5	4	0	1
2019	1	1	0	0	1
2020	2	2	0	0	2
2021	2	2	2	0	0
2022	2	2	0	0	2
All Years	12 <sup>[1]</sup>	12	6	0	6

#### **Footnotes**

<sup>[1]</sup> Note: Observed crash data that does not comply with the associated CPM model requirements may not be used in EB processing.

**Table 21. Evaluation Highway - Homogeneous Segments (Section 4)** 

Se g. N o.	Type	Start Locati on (Sta. ft)	End Locati on (Sta. ft)	Len gth (ft)	gth		Number Major Commer icial Drivewa ys	Number Minor Commer icial Drivewa ys	Number Major Industial/I nstitutiona I	Number Minor Industial/I nstitutiona l	Number Major Resident ial Drivewa ys	Number Minor Resident ial Drivewa ys	Numbe r Other Drivew ays	Lighti ng	Automat ed Speed Enforce ment	Dens ity (fixe d objec ts/mi	Me dian Wid th (ft)	Туре	Effecti ve Media n Width (ft)	Speed Level	Numbe r Rail Highw ay Crossi ngs	Average Shou der Widti (ft)	Il Ave
11 7	Urban/Suburban Arterial Segment Four-lane Undivided	948+5 0.000	950+0 0.000	150.			0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediat e/High	0	8.0	00 11.0
11 8	Urban/Suburban Arterial Segment Four-lane Undivided	950+0 0.000	954+0 0.000			2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediat e/High	0	8.0	00 11.0
11 9	Urban/Suburban Arterial Segment Four-lane Undivided	954+0 0.000	955+0 0.000				0	0	2	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediat e/High	0	4.0	00 11.0
12 0	Urban/Suburban Arterial Segment Four-lane Divided	955+0 0.000	955+5 5.000		0.0		0	0	0	0	0	0	0	false	false	0.0	4.01	Non- Traversable Median	4.01	Intermediat e/High	0	8.0	00 11.0
12	Urban/Suburban Arterial Segment Four-lane Divided	955+5 5.000	958+2 8.000				0	0	0	0	0	0	0	false	false	0.0	10.0	Non- Traversable Median	10.02	Intermediat e/High	0	8.0	00 11.0
12 2	Urban/Suburban Arterial Segment Four-lane Divided	958+2 8.000	961+0 1.000	273.			0	0	0	0	0	0	0	false	false	0.0		Non- Traversable Median	19.99	Intermediat e/High	0	8.0	00 11.0
12	Urban/Suburban Arterial Segment Four-lane Divided	961+0 1.000	962+0 0.000		0.0		0	0	0	0	0	0	0	false	false	0.0		Traversable Median	23.12	Intermediat e/High	0	8.0	00 11.0
12	Urban/Suburban Arterial Segment Four-lane Divided	962+0 0.000	963+6 9.000				0	0	0	0	0	0	0	false	false	0.0		Traversable Median	30.13	Intermediat e/High	0	8.0	00 11.0
12	Urban/Suburban Arterial Segment Four-lane Divided	963+6 9.000	965+0 0.000				0	0	0	0	0	0	0	false	false	0.0		Traversable Median	24.55	Intermediat e/High	0	8.0	00 11.0
12	Urban/Suburban Arterial Segment Four-lane Divided	965+0 0.000	966+3 8.000				0	0	0	0	0	0	0	false	false	0.0	7.54	Traversable Median	19.54	Intermediat e/High	0	4.0	00 11.0
12 7	Urban/Suburban Arterial Segment Four-lane Divided	966+3 8.000	966+7 0.000	32.0	0.00		0	0	0	0	0	0	0	false	false	0.0		Traversable Median	16.38	Intermediat e/High	0	4.0	00 11.0

Se g. N	Туре	Start Locati on (Sta. ft)	End Locat on (Sta. ft)	i Len gth (ft)		th	AADT	Number Major Commer icial Drivewa ys	Number Minor Commer icial Drivewa ys	Number Major Industial/I nstitutiona I	Number Minor Industial/I nstitutiona I	Number Major Resident ial Drivewa ys	Number Minor Resident ial Drivewa ys	Numbe r Other Drivew ays	Lighti ng	Automat ed Speed Enforce ment	Dens ity (fixe d objec ts/mi		Туре	Effecti ve Media n Width (ft)	Speed Level	Numbe r Rail Highw ay Crossi ngs	Avera ge Shoul der Width (ft)	Aver age Lane Widt h (ft)
12	Urban/Suburban Arterial Segment Four-lane Divided	966+7 0.000	966+9 1.000		0.0	00 40	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,441; 2043: 8,066; 2044: 9,085; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	3.39	Traversable Median	15.39	Intermediat e/High	0	0.00	11.00
12	Urban/Suburban Arterial Segment Four-lane Undivided	966+9 1.000	967+1 4.000		0.0	00 44	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,666; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	true	false	0.0	0.00	None	0.00	Intermediat e/High	0	0.00	11.00
13	Urban/Suburban Arterial Segment Four-lane Undivided	967+1 4.000	967+2 0.000		0.0	00 11	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	true	false	0.0	0.00	None	0.00	Intermediat e/High	0	8.00	11.00
13 1	Urban/Suburban Arterial Segment Four-lane Undivided	967+2 0.000	967+4 5.000	25.0		00 47	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediat e/High	0	8.00	11.00
13	Urban/Suburban Arterial Segment Four-lane Divided	967+4 5.000	968+0 6.000	61.0		01 16	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,444; 2043: 8,066; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	4.01	Non- Traversable Median	18.01	Intermediat e/High	0	8.00	11.00
13	Urban/Suburban Arterial Segment Four-lane Divided	968+0 6.000	970+1 9.000			05 17	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,444; 2043: 8,066; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	9.52	Non- Traversable Median	23.52	Intermediat e/High	0	8.00	11.00
13 4	Urban/Suburban Arterial Segment Four-lane Divided	970+7 9.000				00 57	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,441; 2043: 8,066; 2044: 9,085; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	14.5	Non- Traversable Median	14.53	Intermediat e/High	0	8.00	11.00
13 5	Urban/Suburban Arterial Segment Four-lane Divided	971+0 9.000			0.0	05 72	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	20.0	Non- Traversable Median	20.01	Intermediat e/High	0	8.00	11.00

 Table 22. Crash Highway Highway - Homogeneous Segments (Section 4)

Se g. No	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Leng th (ft)	Leng th (mi)	AADT	Number Major Commerici al Driveways	Number Minor Commerici al Driveways	Number Major Industial/Inst itutional	Number Minor Industial/Inst itutional	Number Major Residential Driveways	Number Minor Residential Driveways	Number Other Drivewa ys	Lightin g	Automated Speed Enforceme nt	Densit y (fixed object s/mi)	Medi an Widt h (ft)	Туре	Effective Median Width (ft)	Speed Level	Number Rail Highwa y Crossing s	Averag e Should er Width (ft)	Avera ge Lane Width (ft)
11 7	Urban/Suburban Arterial Segment Four-lane Undivided	948+50. 000	950+00. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediate/ High	0	8.00	11.00
11 8	Urban/Suburban Arterial Segment Four-lane Undivided	950+00. 000	954+00. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediate/ High	0	8.00	11.00
11 9	Urban/Suburban Arterial Segment Four-lane Undivided	954+00. 000	955+00. 000			2018-2022: 4,900	0	0	2	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediate/ High	0	4.00	11.00
12 0	Urban/Suburban Arterial Segment Four-lane Divided	955+00. 000	955+55. 000	55.00		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	4.01	Non-Traversable Median	4.01	Intermediate/ High	0	8.00	11.00
12 1	Urban/Suburban Arterial Segment Four-lane Divided	955+55. 000	958+28. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	10.02	Non-Traversable Median	10.02	Intermediate/ High	0	8.00	11.00
12 2	Urban/Suburban Arterial Segment Four-lane Divided	958+28. 000	961+01. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	20.03	Non-Traversable Median	19.99	Intermediate/ High	0	8.00	11.00
12 3	Urban/Suburban Arterial Segment Four-lane Divided	961+01. 000	962+00. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	23.12	Traversable Median	23.12	Intermediate/ High	0	8.00	11.00
12 4	Urban/Suburban Arterial Segment Four-lane Divided	962+00. 000	963+69. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	18.13	Traversable Median	30.13	Intermediate/ High	0	8.00	11.00
12 5	Urban/Suburban Arterial Segment Four-lane Divided	963+69. 000	965+00. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	12.55	Traversable Median	24.55	Intermediate/ High	0	8.00	11.00
12 6	Urban/Suburban Arterial Segment Four-lane Divided	965+00. 000	966+38. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	7.54	Traversable Median	19.54	Intermediate/ High	0	4.00	11.00
12 7	Urban/Suburban Arterial Segment Four-lane Divided	966+38. 000	966+70. 000	32.00		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	4.38	Traversable Median	16.38	Intermediate/ High	0	4.00	11.00
12 8	Urban/Suburban Arterial Segment Four-lane Divided	966+70. 000	966+91. 000	21.00		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	3.39	Traversable Median	15.39	Intermediate/ High	0	0.00	11.00
12 9	Urban/Suburban Arterial Segment Four-lane Undivided	966+91. 000	967+14. 000			2018-2022: 4,900	0	0	0	0	0	0	0	true	false	0.0	0.00	None	0.00	Intermediate/ High	0	0.00	11.00
13 0	Urban/Suburban Arterial Segment Four-lane Undivided	967+14. 000	967+20. 000			2018-2022: 4,900	0	0	0	0	0	0	0	true	false	0.0	0.00	None	0.00	Intermediate/ High	0	8.00	11.00
13 1	Urban/Suburban Arterial Segment Four-lane Undivided	967+20. 000	967+45. 000	25.00		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediate/ High	0	8.00	11.00
13 2	Urban/Suburban Arterial Segment Four-lane Divided	967+45. 000	968+06. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	4.01	Non-Traversable Median	18.01	Intermediate/ High	0	8.00	11.00
13 3	Urban/Suburban Arterial Segment Four-lane Divided	968+06. 000	970+79. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	9.52	Non-Traversable Median	23.52	Intermediate/ High	0	8.00	11.00
13 4	Urban/Suburban Arterial Segment Four-lane Divided	970+79. 000	971+09. 000	30.00		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	14.53	Non-Traversable Median	14.53	Intermediate/ High	0	8.00	11.00
13 5	Urban/Suburban Arterial Segment Four-lane Divided	971+09. 000	974+11. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	20.01	Non-Traversable Median	20.01	Intermediate/ High	0	8.00	11.00

### **Table 23. Evaluation Intersection (Section 4)**

nter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Le gs	Traffic Control	Approache s w/Left Turn Lanes	Approache s w/Right Turn Lanes	Right	Pedestrian Volume (crossings/ day)	d at	Light	l Near	Num ber of Bus Stops	Number of	Max Lanes Crosse d	
8	Marion Rd/ SD38 (v1)	Urban/Suburban Arterial Intersection Four-Legged Signalized	966+91. 000	6,255; 2030: 6,660; 2031: 7,065; 2032: 7,470; 2035: 7,875; 2034: 8,280; 2035: 8,685; 2036: 9,900; 2037: 9,495; 2038: 9,900; 2039: 10,305; 2040: 10,710; 2041: 11,861; 2042: 13,012; 2043: 14,163; 2044: 15,314; 2045: 16,465; 2046: 17,516; 2047: 18,767; 2038: 10,918; 2049: 10,908; 2069: 2069	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	4	Signalized	4	3	0	20	false	false	false	0	0	6	j

## **Table 24. Crash History Intersection (Section 4)**

nter. No.	Title	Type	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Approaches w/Left Turn Lanes		m/o Diobt	Pedestrian Volume (crossings/day	Lighted at Night	Red Light Camer a	School	n of Duo		Max Lanes Crossed	
8	Marion Rd/ SD38 (v1)	Urban/Suburban Arterial Intersection Four-Legged Signalized	966+91.000	2018-2022: 5.400	2018-2022: 4,900	4	Signalized	4	3	0	20	false	false	false	0	0		6

Table 25. Expected Highway Crash Rates and Frequencies Summary (Section 4)

First Year of Analysis	2025
Last Year of Analysis	2050
Evaluated Length (mi)	0.4850
Average Future Road AADT (vpd)	8,272
Expected Crashes	
Total Crashes	135.85
Fatal and Injury Crashes	46.51
Property-Damage-Only Crashes	89.34
Percent of Total Expected Crashes	
Percent Fatal and Injury Crashes (%)	34
Percent Property-Damage-Only Crashes (%)	66
Expected Crash Rate	
Crash Rate (crashes/mi/yr)	10.7725
FI Crash Rate (crashes/mi/yr)	3.6878
PDO Crash Rate (crashes/mi/yr)	7.0847
Expected Travel Crash Rate	
Total Travel (million veh-mi)	38.08
Travel Crash Rate (crashes/million veh-mi)	3.57
Travel FI Crash Rate (crashes/million veh-mi)	1.22
Travel PDO Crash Rate (crashes/million veh-mi)	2.35

Table 26. Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 4)

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi/ yr)	Expected Travel Crash Rate (crashes/mil lion veh-mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
117	948+50.000	950+00.000	0.0284	1.343	1.459	0.0517	0.0179	0.0337	0.0561	0.0181	0.0380	-0.0044	-0.0001	-0.0043	1.8184	0.60	
118	950+00.000	954+00.000	0.0758	3.172	3.889	0.1220	0.0437	0.0783	0.1496	0.0481	0.1014	-0.0276	-0.0044	-0.0232	1.6103	0.53	
119	954+00.000	955+00.000	0.0189	4.560	6.162	0.1754	0.0651	0.1103	0.2370	0.0817	0.1553	-0.0616	-0.0167	-0.0449	9.2608	3.07	
120	955+00.000	955+55.000	0.0104	0.373	0.382	0.0143	0.0039	0.0104	0.0147	0.0039	0.0108	-0.0004	-0.0000	-0.0003	1.3757	0.46	
121	955+55.000	958+28.000	0.0517	1.685	1.897	0.0648	0.0182	0.0466	0.0730	0.0195	0.0535	-0.0082	-0.0013	-0.0069	1.2532	0.42	
122	958+28.000	961+01.000	0.0517	1.685	1.897	0.0648	0.0182	0.0466	0.0730	0.0195	0.0535	-0.0082	-0.0013	-0.0069	1.2532	0.42	
123	961+01.000	962+00.000	0.0187	0.651	0.681	0.0251	0.0069	0.0182	0.0262	0.0070	0.0192	-0.0011	-0.0001	-0.0010	1.3360	0.44	
124	962+00.000	963+69.000	0.0320	1.069	1.151	0.0411	0.0114	0.0297	0.0443	0.0118	0.0325	-0.0032	-0.0004	-0.0027	1.2842	0.42	
125	963+69.000	965+00.000	0.0248	0.850	0.901	0.0327	0.0090	0.0237	0.0347	0.0092	0.0254	-0.0020	-0.0002	-0.0017	1.3176	0.44	
126	965+00.000	966+38.000	0.0261	0.893	0.949	0.0343	0.0095	0.0249	0.0365	0.0097	0.0268	-0.0022	-0.0003	-0.0019	1.3136	0.43	
127	966+38.000	966+70.000	0.0061	0.217	0.220	0.0083	0.0023	0.0061	0.0085	0.0023	0.0062	-0.0001	0.0000	-0.0001	1.3765	0.46	
128	966+70.000	966+91.000	0.0040	0.143	0.144	0.0055	0.0015	0.0040	0.0056	0.0015	0.0041	-0.0001	0.0000	-0.0001	1.3834	0.46	
Marion Rd/ SD38 (v1)	966+91.000			114.572	49.969	4.4066	1.5303	2.8763	1.9219	0.6310	1.2909	2.4847	0.8993	1.5854			0.65
129	966+91.000	967+14.000	0.0044	0.203	0.205	0.0078	0.0027	0.0051	0.0079	0.0025	0.0054	-0.0001	0.0001	-0.0002	1.7894	0.59	
130	967+14.000	967+20.000	0.0011	0.053	0.053	0.0021	0.0007	0.0014	0.0021	0.0007	0.0014	-0.0000	0.0000	-0.0000	1.8054	0.60	
131	967+20.000	967+45.000	0.0047	0.240	0.243	0.0092	0.0031	0.0061	0.0093	0.0030	0.0063	-0.0001	0.0001	-0.0003	1.9466	0.65	
132	967+45.000	968+06.000	0.0116	0.412	0.424	0.0159	0.0043	0.0115	0.0163	0.0043	0.0120	-0.0005	-0.0000	-0.0004	1.3720	0.45	
133	968+06.000	970+79.000	0.0517	1.685	1.897	0.0648	0.0182	0.0466	0.0730	0.0195	0.0535	-0.0082	-0.0013	-0.0069	1.2532	0.42	
134	970+79.000	971+09.000	0.0057	0.206	0.208	0.0079	0.0021	0.0058	0.0080	0.0021	0.0059	-0.0001	0.0000	-0.0001	1.3915	0.46	
135	971+09.000	974+11.000	0.0572	1.842	2.098	0.0708	0.0199	0.0509	0.0807	0.0215	0.0592	-0.0099	-0.0016	-0.0083	1.2387	0.41	
All Segments			0.4850	21.280	24.862	0.8185	0.2585	0.5600	0.9562	0.2860	0.6702	-0.1378	-0.0275	-0.1102	1.6874	0.56	
All Intersections				114.572	49.969	4.4066	1.5303	2.8763	1.9219	0.6310	1.2909	2.4847	0.8993	1.5854			0.65
Total			0.4850	135.852	74.831	5.2251	1.7887	3.4363	2.8781	0.9170	1.9611	2.3470	0.8718	1.4752	10.7725		

50

Table 27. Expected Crash Frequencies and Rates by Horizontal Design Element (Section 4)

Tit	Start Location (Sta. ft)	End Location (Sta. ft)		Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	Frequency	PDO Crash	Expected Crash Rate (crashes/mi /yr)	
Tange	nt 948+50.000	974+11.000	0.4850	21.280	24.862	0.8185	0.2585	0.5600	0.9562	0.2860	0.6702	-0.1378	-0.0275	-0.1102	1.6874	0.56

Table 28. Predicted Crash Frequencies by Year (Section 4)

Year	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
2025	1.54	0.47	30.768	1.07	69.232
2026	1.62	0.50	30.791	1.12	69.209
2027	1.69	0.52	30.812	1.17	69.188
2028	1.77	0.55	30.830	1.22	69.170
2029	1.85	0.57	30.847	1.28	69.153
2030	1.93	0.60	30.902	1.33	69.098
2031	2.01	0.62	30.960	1.39	69.040
2032	2.10	0.65	31.018	1.45	68.982
2033	2.18	0.68	31.078	1.50	68.922
2034	2.27	0.71	31.139	1.56	68.862
2035	2.35	0.73	31.199	1.62	68.801
2036	2.44	0.76	31.260	1.68	68.740
2037	2.53	0.79	31.321	1.74	68.679
2038	2.61	0.82	31.381	1.79	68.618
2039	2.70	0.85	31.442	1.85	68.558
2040	2.79	0.88	31.502	1.91	68.498
2041	3.01	0.95	31.680	2.06	68.320
2042	3.24	1.03	31.854	2.21	68.146
2043	3.46	1.11	32.026	2.35	67.975
2044	3.69	1.19	32.192	2.50	67.808
2045	3.92	1.27	32.354	2.65	67.646
2046	4.15	1.35	32.511	2.80	67.489
2047	4.39	1.43	32.664	2.95	67.336
2048	4.62	1.52	32.811	3.11	67.189
2049	4.86	1.60	32.954	3.26	67.046
2050	5.10	1.69	33.093	3.41	66.907
Total	74.83	23.84	31.860	50.99	68.140
Average	2.88	0.92	31.860	1.96	68.140

Table 29. Expected Crash Frequencies by Year (Section 4)

Year	Total Crashes	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
2025	2.80	0.93	33.060	1.87	66.820
2026	2.94	0.97	33.085	1.96	66.798
2027	3.07	1.02	33.107	2.05	66.778
2028	3.21	1.06	33.127	2.15	66.760
2029	3.35	1.11	33.145	2.24	66.744
2030	3.50	1.16	33.205	2.34	66.691
2031	3.65	1.22	33.266	2.44	66.635
2032	3.81	1.27	33.329	2.54	66.579
2033	3.96	1.32	33.394	2.63	66.521
2034	4.12	1.38	33.458	2.74	66.463
2035	4.27	1.43	33.524	2.84	66.404
2036	4.43	1.49	33.589	2.94	66.346
2037	4.59	1.54	33.654	3.04	66.287
2038	4.75	1.60	33.719	3.14	66.228
2039	4.91	1.66	33.784	3.25	66.170
2040	5.07	1.72	33.849	3.35	66.112
2041	5.47	1.86	34.040	3.61	65.941
2042	5.88	2.01	34.228	3.86	65.772
2043	6.29	2.16	34.411	4.12	65.607
2044	6.70	2.32	34.590	4.38	65.446
2045	7.12	2.47	34.765	4.65	65.289
2046	7.54	2.63	34.934	4.91	65.138
2047	7.96	2.79	35.097	5.18	64.991
2048	8.39	2.96	35.256	5.44	64.848
2049	8.82	3.12	35.409	5.71	64.710
2050	9.26	3.29	35.558	5.98	64.577
Total	135.85	46.51	34.234	89.34	65.766
Average	5.22	1.79	34.234	3.44	65.766

Table 30. Comparing Predicted and Expected Crashes for the Evaluation Period (Section 4)

Scope	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
Predicted	74.83	23.84	31.860	50.99	68.140
Expected	135.85	46.51	34.234	89.34	65.766
Expected - Predicted	61.02	22.67		38.35	
Percent Difference	44.92	48.74		42.93	

Table 31. Expected Five Lane or Fewer Crash Type Distribution (Section 4)

Element Type	Crash Type	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)	Total Crashes	Percent Total (%)
Highway Segment	Collision with Animal	0.00	0.0	0.21	0.2	0.21	0.2
Highway Segment	Collision with Bicycle	0.09	0.1	0.00	0.0	0.09	0.1
Highway Segment	Collision with Fixed Object	0.57	0.4	3.57	2.6	4.14	3.0
Highway Segment	Collision with Other Object	0.03	0.0	0.09	0.1	0.11	0.1
Highway Segment	Other Single-vehicle Collision	0.43	0.3	0.53	0.4	0.97	0.7
Highway Segment	Collision with Pedestrian	0.34	0.3	0.00	0.0	0.34	0.3
Highway Segment	Total Single Vehicle Crashes	1.47	1.1	4.39	3.2	5.86	4.3
Highway Segment	Angle Collision	0.37	0.3	0.54	0.4	0.91	0.7
Highway Segment	Driveway-related Collision	1.32	1.0	2.27	1.7	3.58	2.6
Highway Segment	Head-on Collision	0.17	0.1	0.05	0.0	0.21	0.2
Highway Segment	Other Multi-vehicle Collision	0.20	0.1	0.58	0.4	0.79	0.6
Highway Segment	Rear-end Collision	2.79	2.1	4.80	3.5	7.59	5.6
Highway Segment	Sideswipe, Opposite Direction Collision	0.15	0.1	0.09	0.1	0.24	0.2
Highway Segment	Sideswipe, Same Direction Collision	0.26	0.2	1.83	1.3	2.09	1.5
Highway Segment	Total Multiple Vehicle Crashes	5.25	3.9	10.16	7.5	15.42	11.3
Highway Segment	Total Highway Segment Crashes	6.72	4.9	14.56	10.7	21.28	15.7
Intersection	Collision with Animal	0.00	0.0	0.00	0.0	0.01	0.0
Intersection	Collision with Bicycle	0.73	0.5	0.00	0.0	0.73	0.5
Intersection	Collision with Fixed Object	0.90	0.7	1.71	1.3	2.61	1.9
Intersection	Non-Collision	0.17	0.1	0.07	0.0	0.24	0.2
Intersection	Collision with Other Object	0.09	0.1	0.14	0.1	0.23	0.2
Intersection	Other Single-vehicle Collision	0.05	0.0	0.04	0.0	0.09	0.1
Intersection	Collision with Parked Vehicle	0.00	0.0	0.00	0.0	0.00	0.0
Intersection	Collision with Pedestrian	0.44	0.3	0.00	0.0	0.44	0.3
Intersection	Total Intersection Single Vehicle Crashes	2.38	1.7	1.97	1.4	4.34	3.2
Intersection	Angle Collision	12.98	9.6	17.77	13.1	30.75	22.6
Intersection	Head-on Collision	1.83	1.3	2.18	1.6	4.02	3.0
Intersection	Other Multi-vehicle Collision	2.06	1.5	15.36	11.3	17.42	12.8
Intersection	Rear-end Collision	16.84	12.4	35.17	25.9	52.01	38.3
Intersection	Sideswipe	3.70	2.7	2.33	1.7	6.03	4.4
Intersection	Total Intersection Multiple Vehicle Crashes	37.41	27.5	72.82	53.6	110.23	81.1
Intersection	Total Intersection Crashes	39.79	29.3	74.78	55.0	114.57	84.3
	Total Crashes	46.51	34.2	89.34	65.8	135.85	100.0

## Interactive Highway Safety Design Model

# **Crash Prediction Evaluation Report**

#### Disclaimer

The Interactive Highway Design Model (IHSDM) software is disseminated under the sponsorship of the Department of Transportation in the interest of information exchange. The United States Government assumes no liability for its content or use thereof. This document does not constitute a standard, specification, or regulation.

The United States Government does not endorse products or manufacturers. Trade and manufacturers' names may appear in this software and documentation only because they are considered essential to the objective of the software.

#### **Limited Warranty and Limitations of Remedies**

This software product is provided "as-is," without warranty of any kind-either expressed or implied (but not limited to the implied warranties of merchantability and fitness for a particular purpose). The FHWA do not warrant that the functions contained in the software will meet the end-user's requirements or that the operation of the software will be uninterrupted and error-free.

Under no circumstances will the FHWA be liable to the end-user for any damages or claimed lost profits, lost savings, or other incidental or consequential damages rising out of the use or inability to use the software (even if these organizations have been advised of the possibility of such damages), or for any claim by any other party.

#### **Notice**

The use of the IHSDM software is being done strictly on a voluntary basis. In exchange for provision of IHSDM, the user agrees that the Federal Highway Administration (FHWA), U.S. Department of Transportation and any other agency of the Federal Government shall not be responsible for any errors, damage or other liability that may result from any and all use of the software, including installation and testing of the software. The user further agrees to hold the FHWA and the Federal Government harmless from any resulting liability. The user agrees that this hold harmless provision shall flow to any person to whom or any entity to which the user provides the IHSDM software. It is the user's full responsibility to inform any person to whom or any entity to which it provides the IHSDM software of this hold harmless provision.

## **Table of Contents**

Report Overview	1
Disclaimer Regarding Crash Prediction Method	2
Section Types	3
Section 1 Evaluation	3
List of Tables	
Table Observed Crashes Used in the Evaluation (Section 1)	5
Table Evaluation Highway - Homogeneous Segments (Section 1)	6
Table Crash History Highway - Homogeneous Segments (Section 1)	17
Table Evaluation Intersection - Section 1	23
Table Crash History Intersection - Section 1	24
Table Expected Highway Crash Rates and Frequencies Summary (Section 1)	25
Table Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 1)	26
Table Expected Crash Frequencies and Rates by Horizontal Design Element (Section 1)	30
Table Predicted Crash Frequencies by Year (Section 1)	31
Table Expected Crash Frequencies by Year (Section 1)	32
Table Comparing Predicted and Expected Crashes for the Evaluation Period (Section 1)	33
Table Expected Crash Type Distribution (Section 1)	34
Table Evaluation Message	35
List of Figures	
Figure Crash Prediction Summary (Section 1)	4

## **Report Overview**

Report Generated: Jun 10, 2024 10:00 AM

Report Template: System: Single Page, 508 Compliant [System] (mlcpm5, Dec 5, 2019 2:16 PM)

Evaluation Date: Mon Jun 10 10:00:02 CDT 2024

**IHSDM Version:** v17.0.0 (Sep 22, 2021)

Crash Prediction Module: v12.0.0 (Sep 22, 2021)

User Name: naveen.mallipaddi

**Organization Name:** 

Phone: E-Mail:

Project Title: SD-38\_Build\_Option3\_I90EBRamp\_I

Project Comment: Created Mon Mar 27 16:47:43 CDT 2023

Project Unit System: U.S. Customary

**Highway Title: SD-38** 

Highway Comment: Created Mon Mar 27 16:49:47 CDT 2023

**Highway Version:** 17

Evaluation Title: Option3\_SD19\_4644th

Evaluation Comment: Created Mon Jun 10 09:58:59 CDT 2024

**Minimum Location:** 171+44.000 **Maximum Location:** 580+10.000

Policy for Superelevation: AASHTO 2011 U.S. Customary

Calibration: HSM Configuration

Crash Distribution: HSM Configuration Model/CMF: HSM Configuration

First Year of Analysis: 2025 Last Year of Analysis: 2050

**Empirical-Bayes Analysis:** Site-Specific **Highway with Crash History:** SD-38

Highway with Crash History Comment: Created Mon Mar 27 16:49:47 CDT 2023

Highway with Crash History Version: 17 First Year of Observed Crashes: 2018 Last Year of Observed Crashes: 2022

# **Disclaimer Regarding Crash Prediction Method**

IMPORTANT NOTICE ABOUT COMPARING RESULTS FROM HIGHWAY SAFETY MANUAL FIRST EDITION (2010) MODELS TO RESULTS FROM NEW MODELS DEVELOPED UNDER NCHRP PROJECTS 17-70, 17-58, AND 17-68

Since the publication of the Highway Safety Manual - First Edition (HSM-1), in 2010 by the American Association of State Highway and Transportation Officials (AASHTO), multiple research efforts have been undertaken through the National Cooperative Highway Research Program (NCHRP) to develop safety performance models for road segment and intersection facility types that were not initially reflected in the HSM-1, in order to expand the breadth and depth of the HSM in the future.

The IHSDM Crash Prediction Module (CPM) is intended as a faithful implementation of HSM Part C predictive methods. As NCHRP projects to develop new predictive methods for the HSM are completed, FHWA works to incorporate the new methods into IHSDM, sometimes in advance of publication in the HSM. The following new crash predictive methods have been accepted by NCHRP project panels and incorporated into IHSDM, while pending AASHTO's approval for incorporation into a future edition of the HSM:

- Roundabouts: completed in 2018 under NCHRP Project 17-70, the new methods will provide improved outcomes for the safety analysis of roundabouts.
- 6+ lane and one-way urban/suburban arterials (including models for segments and intersections): completed under NCHRP Project 17-58.
- Intersection crash prediction methods for some intersection configurations and traffic control types not currently addressed in the HSM (e.g., all-way stop; rural 3-leg signalized; 3-leg stop-controlled where the major leg turns; urban 5-leg signalized; urban high-speed intersections): completed in 2021 under NCHRP Project 17-68.

However, in the absence of local calibration factors (see HSM-1 Part C, Appendix A for guidance on calibration of the predictive models), it is neither appropriate nor advisable to directly compare the results from new models (from NCHRP Projects 17-58, 17-68, and 17-70) to results from HSM-1 models, as the models were not calibrated to the same base state data sets, and consequently can produce unexpected results. If local calibration factors are available and applied to both new models and HSM-1 models, then it may be appropriate to directly compare the results. [Note: Work being performed under NCHRP Project 17-72 (Update of Crash Modification Factors for the Highway Safety Manual) is expected to re-calibrate many of the old (HSM-1) and new (e.g., NCHRP 17-70) models to data from a single (or small number of) states, that would allow results from all models to be directly compared.]

The models produced for NCHRP Project 17-70 have independent value in terms of informing the design of a roundabout and assessing the effects of different design characteristics on the expected safety performance of a roundabout.

The HSM-1 interim method previously included in IHSDM for evaluating roundabouts on urban/suburban arterials (i.e., evaluating an existing intersection and then applying a Crash Modification Factor for replacing the existing intersection with a roundabout) has been deactivated in IHSDM, to minimize any confusion with the new roundabout methodology.

# **Section Types**

### **Section 1 Evaluation**

Section: Section 1

**Evaluation Start Location:** 171+44.000 **Evaluation End Location:** 580+10.000

Area Type: Rural

Functional Class: Arterial

Type of Alignment: Undivided, Two Lane

Model Category: Rural, Two Lane

Calibration Factor: 2U=1.0; 3ST=1.0; 4ST=1.0;

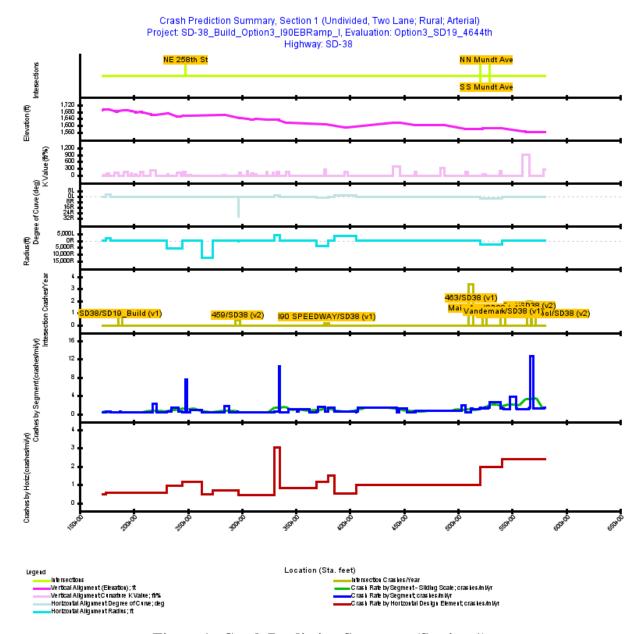


Figure 1. Crash Prediction Summary (Section 1)

**Table 1. Observed Crashes Used in the Evaluation (Section 1)** 

Year	Observed Crashes	Total Crashes Used	FI Crashes	FI no/C Crashes	PDO Crashes
2018	9	9	2	1	7
2019	5	4	1	0	3
2020	9	9	5	1	4
2021	8	7	3	1	4
2022	6	6	3	1	3
All Years	37 <sup>[1]</sup>	35	14	4	21

#### **Footnotes**

[1] Note: Observed crash data that does not comply with the associated CPM model requirements may not be used in EB processing.

 Table 2. Evaluation Highway - Homogeneous Segments (Section 1)

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	rd	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
1	Rural Two-Lane Segment Two-lane Undivided	171+44. 000	172+42. 000	98.00	0.018 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,362; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	4.25	6.2	3	false	0	false	false	false				
2	Rural Two-Lane Segment Two-lane Undivided	172+42. 000	174+52. 690	210.6 9	0.039 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.17	6.2	3	false	0	false	false	false				
3	Rural Two-Lane Segment Two-lane Undivided	174+52. 690	176+25. 000	172.3 1	0.032 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.17	6.2	3	false	0	false	false	false	2,074. 80	2.0	true	40
4	Rural Two-Lane Segment Two-lane Undivided	176+25. 000	178+85. 250	260.2 5		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.88	6.2	3	false	0	false	false	false	2,074. 80	2.0	true	40
5	Rural Two-Lane Segment Two-lane Undivided	178+85. 250	183+75. 370	490.1 2	0.092	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,000; 2041: 3,122; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.88	6.2	3	false	0	false	false	false				
6	Rural Two-Lane Segment Two-lane Undivided	183+75. 370	184+00. 000	24.63	0.004 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,000; 2041: 3,122; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
7	Rural Two-Lane Segment Two-lane Undivided	184+00. 000	184+45. 000	45.00	0.008	2025; 2,232; 2026; 2,282; 2027; 2,331; 2028; 2,380; 2029; 2,430; 2030; 2,490; 2031; 2,550; 2032; 2,610; 2033; 2,670; 2034; 2,730; 2035; 2,790; 2036; 2,850; 2037; 2,910; 2038; 2,970; 2039; 3,030; 2040; 3,090; 2041; 3,162; 2042; 3,234; 2043; 3,306; 2044; 3,378; 2045; 3,450; 2046; 3,522; 2047; 3,594; 2048; 3,666; 2049; 3,738; 2050; 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
8	Rural Two-Lane Segment Two-lane Undivided	184+45. 000	185+20. 000	75.00	0.014	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
9	Rural Two-Lane Segment Two-lane Undivided	185+20. 000	186+60. 000	140.0 0		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
10	Rural Two-Lane Segment Two-lane Undivided	186+60. 000	187+20. 000	60.00	0.011	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
11	Rural Two-Lane Segment Two-lane Undivided	187+20. 000	187+60. 000	40.00	0.007 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	1.13	6.2	3	false	0	false	false	false				
12	Rural Two-Lane Segment Two-lane Undivided	187+60. 000	190+00. 000	240.0 0	0.045 5	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
13	Rural Two-Lane Segment Two-lane Undivided	190+00. 000	192+00. 000	200.0	0.037 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
14	Rural Two-Lane Segment Two-lane Undivided	192+00. 000	192+39. 270	39.27	0.007 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,362; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.13	6.2	3	false	0	false	false	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
15	Rural Two-Lane Segment Two-lane Undivided	192+39. 270	193+60. 000	120.7 3	0.022 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.94	6.2	3	false	0	false	false	false				
16	Rural Two-Lane Segment Two-lane Undivided	193+60. 000	197+65. 000	405.0 0	0.076 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 0.94	6.2	3	false	0	false	false	false				
17	Rural Two-Lane Segment Two-lane Undivided	197+65. 000	199+00. 000		0.025 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 1.94	6.2	3	false	0	false	false	false				
18	Rural Two-Lane Segment Two-lane Undivided	199+00. 000	201+63. 750		0.050 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 1.94	6.2	3	false	0	false	false	false				
19	Rural Two-Lane Segment Two-lane Undivided	201+63. 750	202+00. 000	36.25	0.006 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
20	Rural Two-Lane Segment Two-lane Undivided	202+00. 000	207+00. 000	500.0 0	0.094 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,362; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
21	Rural Two-Lane Segment Two-lane Undivided	207+00. 000	207+49. 760	49.76	0.009 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
22	Rural Two-Lane Segment Two-lane Undivided	207+49. 760	217+74. 250		0.194 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 1.70	6.2	3	false	0	false	false	false				
23	Rural Two-Lane Segment Two-lane Undivided	217+74. 250	221+00. 000	325.7 5	0.061 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
24	Rural Two-Lane Segment Two-lane Undivided	221+00. 000	226+00. 000	500.0	0.094 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
25	Rural Two-Lane Segment Two-lane Undivided	226+00. 000	230+66. 250	466.2 5	0.088	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
26	Rural Two-Lane Segment Two-lane Undivided	230+66. 250	231+39. 700	73.45	0.013 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.77	6.2	3	false	0	false	false	false	5,644. 64	2.0	true	70
27	Rural Two-Lane Segment Two-lane Undivided	231+39. 700	235+00. 000		0.068 2	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	2.00	6.2	3	false	0	false	false	false	5,644. 64	2.0	true	70
28	Rural Two-Lane Segment Two-lane Undivided	235+00. 000	241+61. 390	661.3 9	0.125	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,096; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	2.00	6.2	3	false	0	false	false	false	5,644. 64	2.0	true	70
29	Rural Two-Lane Segment Two-lane Undivided	241+61. 390	242+00. 000	38.61	0.007	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.16	6.2	3	false	0	false	false	false	5,644. 64	2.0	true	70

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
30	Rural Two-Lane Segment Two-lane Undivided	242+00. 000	245+14. 280	314.2 8		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,096; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.16	6.2	3	false	0	false	false	false	5,644. 64	2.0	true	70
31	Rural Two-Lane Segment Two-lane Undivided	245+14. 280	246+55. 100	140.8 2		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.16	6.2	3	false	0	false	false	false				
32	Rural Two-Lane Segment Two-lane Undivided	246+55. 100	248+00. 000	144.9 0		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2044: 3,046; 2044: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.42	6.2	3	false	0	false	false	false				
33	Rural Two-Lane Segment Two-lane Undivided	248+00. 000	249+00. 000	100.0 0		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	0.42	6.2	3	false	0	false	false	false				
34	Rural Two-Lane Segment Two-lane Undivided	249+00. 000	251+21. 980	221.9 8		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.42	6.2	3	false	0	false	false	false				
35	Rural Two-Lane Segment Two-lane Undivided	251+21. 980	252+40. 240	118.2 6	0.022 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	3.43	6.2	3	false	0	false	false	false				
36	Rural Two-Lane Segment Two-lane Undivided	252+40. 240	263+22. 600	1,082. 36		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
37	Rural Two-Lane Segment Two-lane Undivided	263+22. 600	272+66. 740	944.1 4		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	false	false	false	12,237 .00	2.0	true	70
38	Rural Two-Lane Segment Two-lane Undivided	272+66. 740	280+00. 000	733.2 6		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
39	Rural Two-Lane Segment Two-lane Undivided	280+00. 000	283+15. 050	315.0 5		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
40	Rural Two-Lane Segment Two-lane Undivided	283+15. 050	284+08. 540	93.49	0.017 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	4.47	6.2	3	false	0	false	false	false				
41	Rural Two-Lane Segment Two-lane Undivided	284+08. 540	288+50. 000	441.4 6		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.47	6.2	3	false	0	false	false	false				
42	Rural Two-Lane Segment Two-lane Undivided	288+50. 000	289+00. 000	50.00		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.47	6.2	3	false	0	false	false	false				
43	Rural Two-Lane Segment Two-lane Undivided	289+00. 000	295+90. 000	690.0 0		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,096; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.47	6.2	3	false	0	false	false	false				
44	Rural Two-Lane Segment Two-lane Undivided	295+90. 000	296+00. 000	10.00		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	1.47	6.2	3	false	0	false	false	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
45	Rural Two-Lane Segment Two-lane Undivided	296+00. 000	296+10. 000	10.00	0.001 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,334; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	1.47	6.2	3	false	0	false	false	false				
46	Rural Two-Lane Segment Two-lane Undivided	296+10. 000	296+96. 520	86.52	0.016 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.47	6.2	3	false	0	false	false	false				
47	Rural Two-Lane Segment Two-lane Undivided	296+96. 520	298+33. 660	137.1 4	0.026 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.47	6.2	3	false	0	false	false	false				
48	Rural Two-Lane Segment Two-lane Undivided	298+33. 660	303+50. 000	516.3 4	0.097 8	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.61	6.2	3	false	0	false	false	false				
49	Rural Two-Lane Segment Two-lane Undivided	303+50. 000	304+50. 000		0.018 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 2,324; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.61	6.2	3	false	0	false	false	false				
50	Rural Two-Lane Segment Two-lane Undivided	304+50. 000	305+02. 039	52.04	0.009 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.61	6.2	3	false	0	false	false	false				
51	Rural Two-Lane Segment Two-lane Undivided	305+02. 039	309+35. 490	433.4 5	0.082 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.15	6.2	3	false	0	false	false	false				
52	Rural Two-Lane Segment Two-lane Undivided	309+35. 490	311+70. 000	234.5 1	0.044 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.24	6.2	3	false	0	false	false	false				
53	Rural Two-Lane Segment Two-lane Undivided	311+70. 000	313+25. 000	155.0 0	0.029 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.24	6.2	3	false	0	false	false	false				
54	Rural Two-Lane Segment Two-lane Undivided	313+25. 000	323+00. 000		0.184 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.33	6.2	3	false	0	false	false	false				
55	Rural Two-Lane Segment Two-lane Undivided	323+00. 000	323+26. 980	26.98	0.005 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.33	6.2	3	false	0	false	false	false				
56	Rural Two-Lane Segment Two-lane Undivided	323+26. 980	328+89. 230	562.2 5	0.106 5	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,030; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.26	6.2	3	false	0	false	false	false				
57	Rural Two-Lane Segment Two-lane Undivided	328+89. 230	329+81. 740	92.51	0.017 5	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,343; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.52	6.2	3	false	0	false	false	false				
58	Rural Two-Lane Segment Two-lane Undivided	329+81. 740	333+24. 920	343.1 8	0.065 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,343; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.52	6.2	3	false	0	false	false	false	4,010. 13	2.0	true	70
59	Rural Two-Lane Segment Two-lane Undivided	333+24. 920	334+00. 000	75.08	0.014 2	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,343; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	2.17	6.2	3	false	0	false	false	false	4,010. 13	2.0	true	70

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
60	Rural Two-Lane Segment Two-lane Undivided	334+00. 000	335+39. 960	139.9 6	0.026 5	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	2.17	6.2	3	false	O	) false	false	false	4,010. 13	2.0	true	e 70
61	Rural Two-Lane Segment Two-lane Undivided	335+39. 960	342+39. 000	699.0 4	0.132 4	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	2.17	6.2	3	false	C	false	false	false				
62	Rural Two-Lane Segment Two-lane Undivided	342+39. 000	343+00. 000	61.00	0.011 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.24	6.2	3	false	C	false	false	false				
63	Rural Two-Lane Segment Two-lane Undivided	343+00. 000	351+20. 000	820.0 0	0.155	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.24	6.2	3	false	C	false	false	false				
64	Rural Two-Lane Segment Two-lane Undivided	351+20. 000	352+00. 000	80.00	0.015	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	0.00	0.24	6.2	3	false	C	) false	false	false				
65	Rural Two-Lane Segment Two-lane Undivided	352+00. 000	352+20. 000	20.00	0.003	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	0.00	0.00	0.24	6.2	3	false	C	false	false	false				
66	Rural Two-Lane Segment Two-lane Undivided	352+20. 000	362+50. 000	1,030. 00	0.195	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	0.24	6.2	3	false	O	false	false	false				
67	Rural Two-Lane Segment Two-lane Undivided	362+50. 000	369+14. 990	664.9 9	0.125	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	0.24	6.2	3	false	O	false	false	false				
68	Rural Two-Lane Segment Two-lane Undivided	369+14. 990	370+30. 000	115.0 1	0.021	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	0.24	6.2	3	false	O	false	false	false	4,023. 18	2.0	true	e 70
69	Rural Two-Lane Segment Two-lane Undivided	370+30. 000	370+60. 000	30.00	0.005 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	0.24	6.2	3	false	O	false	false	false	4,023. 18	2.0	true	e 70
70	Rural Two-Lane Segment Two-lane Undivided	370+60. 000	376+83. 610	623.6 1	0.118	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	0.24	6.2	3	false	O	false	false	false	4,023. 18	2.0	true	e 70
71	Rural Two-Lane Segment Two-lane Undivided	376+83. 610	378+00. 000	116.3 9		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	1.04	6.2	3	false	O	false	false	false	4,023. 18	2.0	true	e 70
72	Rural Two-Lane Segment Two-lane Undivided	378+00. 000	378+40. 000	40.00	0.007	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	8.00	1.04	6.2	3	false	0	) false	false	false	4,023. 18	2.0	true	е 70
73	Rural Two-Lane Segment Two-lane Undivided	378+40. 000	378+60. 000	20.00		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	8.00	1.04	6.2	3	false	0	) false	false	false	4,023. 18	2.0	true	е 70
74	Rural Two-Lane Segment Two-lane Undivided	378+60. 000	379+00. 000	40.00	0.007	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,12; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,252; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	0.00	8.00	1.04	6.2	3	false	C	) false	false	false	4,023. 18	2.0	true	e 70

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	y Density	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
75	Rural Two-Lane Segment Two-lane Undivided	379+00. 000	379+62. 690	62.69	0.011 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	4,023. 18	2.0	true	e 70
76	Rural Two-Lane Segment Two-lane Undivided	379+62. 690	385+22. 970	560.2 8	0.106 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false				
77	Rural Two-Lane Segment Two-lane Undivided	385+22. 970	386+60. 000	137.0 3	0.026 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
78	Rural Two-Lane Segment Two-lane Undivided	386+60. 000	389+50. 000	290.0 0	0.054 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
79	Rural Two-Lane Segment Two-lane Undivided	389+50. 000	394+00. 000	450.0 0	0.085 2	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
80	Rural Two-Lane Segment Two-lane Undivided	394+00. 000	396+46. 150	246.1 5	0.046 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.04	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
81	Rural Two-Lane Segment Two-lane Undivided	396+46. 150	397+00. 000	53.85	0.010 2	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
82	Rural Two-Lane Segment Two-lane Undivided	397+00. 000	399+00. 000	200.0 0	0.037 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	e 70
83	Rural Two-Lane Segment Two-lane Undivided	399+00. 000	405+75. 410	675.4 1	0.127 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0	12.0 0	8.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856. 89	2.0	true	70
84	Rural Two-Lane Segment Two-lane Undivided	405+75. 410	406+00. 000	24.59	0.004 7	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.72	6.2	3	false	0	false	false	false				
85	Rural Two-Lane Segment Two-lane Undivided	406+00. 000	407+00. 000	100.0	0.018 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910: 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	0.00	8.00	0.72	6.2	3	false	0	false	false	false				
86	Rural Two-Lane Segment Two-lane Undivided	407+00. 000	443+25. 000	3,625. 00	0.686 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.72	6.2	3	false	0	false	false	false				
87	Rural Two-Lane Segment Two-lane Undivided	443+25. 000	445+50. 000	225.0 0	0.042 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 0.96	6.2	3	false	0	false	false	false				
88	Rural Two-Lane Segment Two-lane Undivided	445+50. 000	452+50. 000	700.0 0	0.132 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,096; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	- 0.96	6.2	3	false	0	false	false	false				
89	Rural Two-Lane Segment Two-lane Undivided	452+50. 000	459+00. 000	650.0 0	0.123 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,040; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
90	Rural Two-Lane Segment Two-lane Undivided	459+00. 000	460+00. 000	100.0	0.018 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	0.00	0.96	6.2	3	false	0	false	false	false				
91	Rural Two-Lane Segment Two-lane Undivided	460+00. 000	460+58. 580	58.58	0.011 1	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
92	Rural Two-Lane Segment Two-lane Undivided	460+58. 580	485+61. 230	2,502. 65		2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.01	6.2	3	false	0	false	false	false				
93	Rural Two-Lane Segment Two-lane Undivided	485+61. 230	503+00. 000	1,738. 77	0.329 3	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.07	6.2	3	false	0	false	false	false				
94	Rural Two-Lane Segment Two-lane Undivided	503+00. 000	507+00. 000		0.075 8	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.07	6.2	3	false	0	false	false	false				
95	Rural Two-Lane Segment Two-lane Undivided	507+00. 000	508+00. 000		0.018 9	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.07	6.2	3	false	0	false	true	false				
96	Rural Two-Lane Segment Two-lane Undivided	508+00. 000	508+08. 240	8.24	0.001 6	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	1.07	6.2	3	false	0	false	true	false				
97	Rural Two-Lane Segment Two-lane Undivided	508+08. 240	510+30. 000	221.7 6	0.042 0	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,366; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.21	6.2	3	false	0	false	true	false				
98	Rural Two-Lane Segment Two-lane Undivided	510+30. 000	512+00. 000	170.0 0	0.032	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,362; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,66; 2049: 3,738; 2050: 3,810	12.0 0	12.0 0	8.00	8.00	0.21	6.2	3	false	0	false	false	false				
99	Rural Two-Lane Segment Two-lane Undivided	512+00. 000	513+00. 000	100.0 0	0.018 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.21	6.2	3	false	0	false	true	false				
100	Rural Two-Lane Segment Two-lane Undivided	513+00. 000	515+00. 000		0.037 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.21	6.2	3	false	0	false	true	false				
101	Rural Two-Lane Segment Two-lane Undivided	515+00. 000	520+00. 000	500.0 0	0.094 7	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.21	6.2	3	false	0	true	true	false				
102	Rural Two-Lane Segment Two-lane Undivided	520+00. 000	520+49. 150	49.15	0.009	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.21	6.2	3	false	0	false	true	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
103	Rural Two-Lane Segment Two-lane Undivided	520+49. 150	521+00. 000	50.85	0.009 6	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0	0.00	0.00	0.21	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
104	Rural Two-Lane Segment Two-lane Undivided	521+00. 000	523+38. 600	238.6 0	0.045 2	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.21	6.2	3	false	0	true	true	false	2,458. 49	2.0	true	e 45
105	Rural Two-Lane Segment Two-lane Undivided	523+38. 600	524+00. 000	61.40	0.011 6	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	1.90	6.2	3	false	0	true	true	false	2,458. 49	2.0	true	e 45
106	Rural Two-Lane Segment Two-lane Undivided	524+00. 000			0.018 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	1.90	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
107	Rural Two-Lane Segment Two-lane Undivided	525+00. 000	525+18. 580	18.58	0.003 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	1.90	6.2	3	false	0	true	true	false	2,458. 49	2.0	true	e 45
108	Rural Two-Lane Segment Two-lane Undivided	525+18. 580	528+00. 000		0.053 3	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	true	true	false	2,458. 49	2.0	true	е 45
109	Rural Two-Lane Segment Two-lane Undivided	528+00. 000	529+00. 000	100.0 0	0.018 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.02	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
110	Rural Two-Lane Segment Two-lane Undivided	529+00. 000	539+00. 000	1,000. 00	0.189 4	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	true	true	false	2,458. 49	2.0	true	e 45
111	Rural Two-Lane Segment Two-lane Undivided	539+00. 000	539+50. 000	50.00	0.009 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0 0	8.00	8.00	0.02	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
112	Rural Two-Lane Segment Two-lane Undivided	539+50. 000	540+00. 000	50.00	0.009 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
113	Rural Two-Lane Segment Two-lane Undivided	540+00. 000	540+50. 000	50.00	0.009 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45
114	Rural Two-Lane Segment Two-lane Undivided	540+50. 000	540+74. 370	24.37	0.004 6	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0 0	0.00	0.00	0.96	6.2	3	false	0	false	true	false	2,458. 49	2.0	true	e 45

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	rd	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
115	Rural Two-Lane Segment Two-lane Undivided	540+74. 370	541+00. 000	25.63	0.004 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0	0.00	0.00	0.96	6.2	3	false	0	false	true	false				
116	Rural Two-Lane Segment Two-lane Undivided	541+00. 000	541+50. 000	50.00	0.009 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.96	6.2	3	false	0	false	true	false				
117	Rural Two-Lane Segment Two-lane Undivided	541+50. 000	541+70. 000	20.00	0.003 8	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.96	6.2	3	false	0	false	true	false				
118	Rural Two-Lane Segment Two-lane Undivided	541+70. 000	542+30. 000	60.00	0.011 4	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	0.00	0.00	0.96	6.2	3	false	0	false	true	false				
119	Rural Two-Lane Segment Two-lane Undivided	542+30. 000	542+64. 000	34.00	0.006 4	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	true	false				
120	Rural Two-Lane Segment Two-lane Undivided	542+64. 000	543+34. 000	70.00	0.013 3	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	true	false				
121	Rural Two-Lane Segment Two-lane Undivided	543+34. 000	544+00. 000	66.00	0.012 5	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	true	false				
122	Rural Two-Lane Segment Two-lane Undivided	544+00. 000	545+00. 000	100.0	0.018 9	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
123	Rural Two-Lane Segment Two-lane Undivided	545+00. 000	548+23. 000	323.0 0	0.061 2	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
124	Rural Two-Lane Segment Two-lane Undivided	548+23. 000	553+70. 000	547.0 0	0.103 6	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
125	Rural Two-Lane Segment Two-lane Undivided	553+70. 000	554+00. 000	30.00	0.005 7	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0	0.00	0.00	0.96	6.2	3	false	0	false	false	false				
126	Rural Two-Lane Segment Two-lane Undivided	554+00. 000	554+20. 000	20.00	0.003	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0	12.0	0.00	0.00	0.96	6.2	3	false	0	false	false	false				

Seg No.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)	Lengt h (mi)	AADT	Left Lan e Wid th (ft)	Rig ht Lan e Wid th (ft)	Left Shoulde r Width (ft)	Right Shoulde r Width (ft)	Gra de (%)	Drivewa y Density (drivewa ys/mi)	Haza rd Ratin g	Centerline Rumble Strip	Passin g Lanes	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Radiu s (ft)	Superelevation (%)	Adver se	Desig n Spee d (mph
127	Rural Two-Lane Segment Two-lane Undivided	554+20. 000	560+00. 000	580.0 0	0.109	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
128	Rural Two-Lane Segment Two-lane Undivided	560+00. 000	562+58. 560	258.5 6	0.049	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.96	6.2	3	false	0	false	false	false				
129	Rural Two-Lane Segment Two-lane Undivided	562+58. 560	564+00. 000	141.4 4	0.026	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				
130	Rural Two-Lane Segment Two-lane Undivided	564+00. 000	565+00. 000	100.0 0	0.018	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,833; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				
131	Rural Two-Lane Segment Two-lane Undivided	565+00. 000	565+77. 000	77.00	0.014 6	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				
132	Rural Two-Lane Segment Two-lane Undivided	565+77. 000	566+10. 000	33.00	0.006	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	0.00	0.00	0.20	6.2	3	false	0	false	false	false				
133	Rural Two-Lane Segment Two-lane Undivided	566+10. 000	566+50. 000	40.00	0.007 6	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,771; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	0.00	0.00	0.20	6.2	3	false	0	false	false	false				
134	Rural Two-Lane Segment Two-lane Undivided	566+50. 000	569+37. 000	287.0 0	0.054 4	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				
135	Rural Two-Lane Segment Two-lane Undivided	569+37. 000	569+70. 000	33.00	0.006	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,3394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	8.00	0.00	0.20	6.2	3	false	0	false	false	false				
136	Rural Two-Lane Segment Two-lane Undivided	569+70. 000	570+00. 000	30.00	0.005	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,3394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				
137	Rural Two-Lane Segment Two-lane Undivided	570+00. 000	575+00. 000	500.0 0	0.094	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,3394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	true	false	false				
138	Rural Two-Lane Segment Two-lane Undivided	575+00. 000	579+50. 000	450.0 0	0.085	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,3394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0 0	12.0 0	8.00	8.00	0.20	6.2	3	false	0	false	false	false				

Se N	g o.	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Lengt h (ft)		AADT	Lan e Wid th		Left Shoulde r Width (ft)	r Width	Gra	Drivewa y Density (drivewa ys/mi)	rd	Rumble	Passin	TW LT Lan e	Lightin g	Automated Speed Enforceme nt	Superelevation (%)	Desig n Spee d (mph
13		ural Two-Lane Segment Two-lane Undivided	579+50. 000	579+70. 000	20.00	0.003 8	2025; 7,087; 2026; 8,007; 2027; 8,928; 2028; 9,849; 2029; 10,770; 2030; 10,937; 2031; 11,104; 2032; 11,271; 2033; 11,439; 2034; 11,606; 2035; 11,773; 2036; 11,940; 2037; 12,108; 2038; 12,275; 2039; 12,442; 2040; 12,610; 2041; 12,806; 2042; 13,002; 2043; 13,198; 2044; 13,394; 2045; 13,590; 2046; 13,786; 2047; 13,982; 2048; 14,178; 2049; 14,374; 2050; 14,570	12.0	12.0	0.00	0.00	0.20	6.2	3	false	0	false	false	false		
14		ural Two-Lane Segment Two-lane Undivided	579+70. 000	580+10. 000		0.007 6	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	12.0	12.0	0.00	0.00	0.20	6.2	3	false	0	false	false	false		

**Table 3. Crash History Highway - Homogeneous Segments (Section 1)** 

g		Start	End	T4h	T41			Right	Left	Right	C1	Driveway	п	Centerline	D'.	TWL		Automated	D. P			Design
Seg. No.	Туре	Location (Sta. ft)	Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Lane Widt h (ft)	Lane Widt h (ft)	Shoulder Width (ft)	Shoulder Width (ft)	Grad e (%)	Density (driveways/ mi)	Hazard Rating	Rumble Strip	Passing Lanes	T Lane	Lighting	Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Speed (mph)
1	Rural Two-Lane Segment Two-lane Undivided	171+44.00 0	172+42.00 0	98.00	0.0186	2018-2022: 2,085	12.00	12.00	8.00	8.00	4.25	6.2	3	false	0	false	false	false				
2	Rural Two-Lane Segment Two-lane Undivided	172+42.00 0	174+52.69 0	210.69	0.0399	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.17	6.2	3	false	0	false	false	false				
3	Rural Two-Lane Segment Two-lane Undivided	174+52.69 0	176+25.00 0	172.31	0.0326	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.17	6.2	3	false	0	false	false	false	2,074.80	2.0	true	40
4	Rural Two-Lane Segment Two-lane Undivided	176+25.00 0	178+85.25 0	260.25	0.0493	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.88	6.2	3	false	0	false	false	false	2,074.80	2.0	true	40
5	Rural Two-Lane Segment Two-lane Undivided	178+85.25 0	183+75.37 0	490.12	0.0928	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.88	6.2	3	false	0	false	false	false				
6	Rural Two-Lane Segment Two-lane Undivided	183+75.37 0	184+00.00 0	24.63	0.0047	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
7	Rural Two-Lane Segment Two-lane Undivided	184+00.00 0	184+45.00 0	45.00	0.0085	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
8	Rural Two-Lane Segment Two-lane Undivided	184+45.00 0	185+20.00 0	75.00	0.0142	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
9	Rural Two-Lane Segment Two-lane Undivided	185+20.00 0	186+60.00 0	140.00	0.0265	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
10	Rural Two-Lane Segment Two-lane Undivided	186+60.00 0	187+20.00 0	60.00	0.0114	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
11	Rural Two-Lane Segment Two-lane Undivided	187+20.00 0	187+60.00 0	40.00	0.0076	2018-2022: 2,085	12.00	12.00	0.00	0.00	1.13	6.2	3	false	0	false	false	false				
12	Rural Two-Lane Segment Two-lane Undivided	187+60.00 0	190+00.00 0	240.00	0.0455	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
13	Rural Two-Lane Segment Two-lane Undivided	190+00.00 0	192+00.00 0	200.00	0.0379	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
14	Rural Two-Lane Segment Two-lane Undivided	192+00.00 0	192+39.27 0	39.27	0.0074	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.13	6.2	3	false	0	false	false	false				
15	Rural Two-Lane Segment Two-lane Undivided	192+39.27 0	193+60.00 0	120.73	0.0229	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.94	6.2	3	false	0	false	false	false				
16	Rural Two-Lane Segment Two-lane Undivided	193+60.00 0	197+65.00 0	405.00	0.0767	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.94	6.2	3	false	0	false	false	false				
17	Rural Two-Lane Segment Two-lane Undivided	197+65.00 0	199+00.00 0	135.00	0.0256	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.94	6.2	3	false	0	false	false	false				
18	Rural Two-Lane Segment Two-lane Undivided	199+00.00 0	201+63.75	263.75	0.0500	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.94	6.2	3	false	0	false	false	false				
19	Rural Two-Lane Segment Two-lane Undivided	201+63.75 0	202+00.00 0	36.25	0.0069	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
20	Rural Two-Lane Segment Two-lane Undivided	202+00.00 0	207+00.00	500.00	0.0947	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
21	Rural Two-Lane Segment Two-lane Undivided	207+00.00	207+49.76 0	49.76	0.0094	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.13	6.2	3	false	0	false	false	false				
22	Rural Two-Lane Segment Two-lane Undivided	207+49.76 0	217+74.25 0	1,024.49	0.1940	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.70	6.2	3	false	0	false	false	false				
23	Rural Two-Lane Segment Two-lane Undivided	217+74.25 0	221+00.00	325.75	0.0617	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
24	Rural Two-Lane Segment Two-lane Undivided	221+00.00	226+00.00	500.00	0.0947	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
25	Rural Two-Lane Segment Two-lane Undivided	226+00.00	230+66.25	466.25	0.0883	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.77	6.2	3	false	0	false	false	false				
			•	-	-	•					-		-		•							

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazard Rating	Centerline Rumble Strip	Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
26	Rural Two-Lane Segment Two-lane Undivided	230+66.25 0	231+39.70 0	73.45	0.0139	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.77	6.2	3	false	0	false	false	false	5,644.64	2.0	true	70
27	Rural Two-Lane Segment Two-lane Undivided	231+39.70 0	235+00.00 0	360.30	0.0682	2018-2022: 2,085	12.00	12.00	8.00	8.00	-2.00	6.2	3	false	0	false	false	false	5,644.64	2.0	true	70
28	Rural Two-Lane Segment Two-lane Undivided	235+00.00 0	241+61.39 0	661.39	0.1253	2018-2022: 2,085	12.00	12.00	8.00	8.00	-2.00	6.2	3	false	0	false	false	false	5,644.64	2.0	true	70
29	Rural Two-Lane Segment Two-lane Undivided	241+61.39 0	242+00.00 0	38.61	0.0073	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.16	6.2	3	false	0	false	false	false	5,644.64	2.0	true	70
30	Rural Two-Lane Segment Two-lane Undivided	242+00.00 0	245+14.28 0	314.28	0.0595	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.16	6.2	3	false	0	false	false	false	5,644.64	2.0	true	70
31	Rural Two-Lane Segment Two-lane Undivided	245+14.28 0	246+55.10 0	140.82	0.0267	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.16	6.2	3	false	0	false	false	false				
32	Rural Two-Lane Segment Two-lane Undivided	246+55.10 0	248+00.00 0	144.90	0.0274	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.42	6.2	3	false	0	false	false	false				
33	Rural Two-Lane Segment Two-lane Undivided	248+00.00 0	249+00.00 0	100.00	0.0189	2018-2022: 2,085	12.00	12.00	0.00	0.00	-0.42	6.2	3	false	0	false	false	false				
34	Rural Two-Lane Segment Two-lane Undivided	249+00.00 0	251+21.98 0	221.98	0.0420	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.42	6.2	3	false	0	false	false	false				
35	Rural Two-Lane Segment Two-lane Undivided	251+21.98 0	252+40.24 0	118.26	0.0224	2018-2022: 2,085	12.00	12.00	8.00	8.00	3.43	6.2	3	false	0	false	false	false				
36	Rural Two-Lane Segment Two-lane Undivided	252+40.24 0	263+22.60 0	1,082.36	0.2050	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
37	Rural Two-Lane Segment Two-lane Undivided	263+22.60 0	272+66.74 0	944.14	0.1788	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.02	6.2	3	false	0	false	false	false	12,237.00	2.0	true	70
38	Rural Two-Lane Segment Two-lane Undivided	272+66.74 0	280+00.00 0	733.26	0.1389	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
39	Rural Two-Lane Segment Two-lane Undivided	280+00.00 0	283+15.05 0	315.05	0.0597	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.02	6.2	3	false	0	false	false	false				
40	Rural Two-Lane Segment Two-lane Undivided	283+15.05 0	284+08.54 0	93.49	0.0177	2018-2022: 2,085	12.00	12.00	8.00	8.00	4.47	6.2	3	false	0	false	false	false				
41	Rural Two-Lane Segment Two-lane Undivided	284+08.54 0	288+50.00 0	441.46	0.0836	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.47	6.2	3	false	0	false	false	false				
42	Rural Two-Lane Segment Two-lane Undivided	288+50.00 0	289+00.00 0	50.00	0.0095	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.47	6.2	3	false	0	false	false	false				
43	Rural Two-Lane Segment Two-lane Undivided	289+00.00 0	295+90.00 0	690.00	0.1307	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.47	6.2	3	false	0	false	false	false				
44	Rural Two-Lane Segment Two-lane Undivided	295+90.00 0	296+00.00 0	10.00	0.0019	2018-2022: 2,085	12.00	12.00	0.00	0.00	-1.47	6.2	3	false	0	false	false	false				
45	Rural Two-Lane Segment Two-lane Undivided	296+00.00 0	296+10.00 0	10.00	0.0019	2018-2022: 2,085	12.00	12.00	0.00	0.00	-1.47	6.2	3	false	0	false	false	false				
46	Rural Two-Lane Segment Two-lane Undivided	296+10.00 0	296+96.52 0	86.52	0.0164	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.47	6.2	3	false	0	false	false	false				
47	Rural Two-Lane Segment Two-lane Undivided	296+96.52 0	298+33.66 0	137.14	0.0260	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.47	6.2	3	false	0	false	false	false				
48	Rural Two-Lane Segment Two-lane Undivided	298+33.66 0	303+50.00 0	516.34	0.0978	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.61	6.2	3	false	0	false	false	false				
49	Rural Two-Lane Segment Two-lane Undivided	303+50.00 0	304+50.00 0	100.00	0.0189	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.61	6.2	3	false	0	false	false	false				
50	Rural Two-Lane Segment Two-lane Undivided	304+50.00 0	305+02.03 9	52.04	0.0099	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.61	6.2	3	false	0	false	false	false				
51	Rural Two-Lane Segment Two-lane Undivided	305+02.03 9	309+35.49 0	433.45	0.0821	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.15	6.2	3	false	0	false	false	false				
52	Rural Two-Lane Segment Two-lane Undivided	309+35.49 0	311+70.00 0	234.51	0.0444	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.24	6.2	3	false	0	false	false	false				

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Lane Widt	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazard Rating	Centerline Rumble Strip	Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
53	Rural Two-Lane Segment Two-lane Undivided	311+70.00 0	313+25.00 0	155.00	0.0294	2018-2022: 2,085	12.00	12.00	8.00	8.00	1.24	6.2	3	false	0	false	false	false				
54	Rural Two-Lane Segment Two-lane Undivided	313+25.00 0	323+00.00 0	975.00	0.1847	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.33	6.2	3	false	0	false	false	false				
55	Rural Two-Lane Segment Two-lane Undivided	323+00.00 0	323+26.98 0	26.98	0.0051	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.33	6.2	3	false	0	false	false	false				
56	Rural Two-Lane Segment Two-lane Undivided	323+26.98 0	328+89.23 0	562.25	0.1065	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.26	6.2	3	false	0	false	false	false				
57	Rural Two-Lane Segment Two-lane Undivided	328+89.23 0	329+81.74 0	92.51	0.0175	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.52	6.2	3	false	0	false	false	false				
58	Rural Two-Lane Segment Two-lane Undivided	329+81.74 0	333+24.92 0	343.18	0.0650	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.52	6.2	3	false	0	false	false	false	4,010.13	2.0	true	70
59	Rural Two-Lane Segment Two-lane Undivided	333+24.92 0	334+00.00 0	75.08	0.0142	2018-2022: 2,085	12.00	12.00	8.00	8.00	-2.17	6.2	3	false	0	false	false	false	4,010.13	2.0	true	70
60	Rural Two-Lane Segment Two-lane Undivided	334+00.00 0	335+39.96 0	139.96	0.0265	2018-2022: 2,085	12.00	12.00	8.00	8.00	-2.17	6.2	3	false	0	false	false	false	4,010.13	2.0	true	70
61	Rural Two-Lane Segment Two-lane Undivided	335+39.96 0	342+39.00 0	699.04	0.1324	2018-2022: 2,085	12.00	12.00	8.00	8.00	-2.17	6.2	3	false	0	false	false	false				
62	Rural Two-Lane Segment Two-lane Undivided	342+39.00 0	343+00.00 0	61.00	0.0116	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false				
63	Rural Two-Lane Segment Two-lane Undivided	343+00.00 0	351+20.00 0	820.00	0.1553	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false				
64	Rural Two-Lane Segment Two-lane Undivided	351+20.00 0	352+00.00 0	80.00	0.0152	2018-2022: 2,085	12.00	12.00	0.00	0.00	-0.24	6.2	3	false	0	false	false	false				
65	Rural Two-Lane Segment Two-lane Undivided	352+00.00 0	352+20.00 0	20.00	0.0038	2018-2022: 2,085	12.00	12.00	0.00	0.00	-0.24	6.2	3	false	0	false	false	false				
66	Rural Two-Lane Segment Two-lane Undivided	352+20.00 0	362+50.00 0	1,030.00	0.1951	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false				
67	Rural Two-Lane Segment Two-lane Undivided	362+50.00 0	369+14.99 0	664.99	0.1259	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false				
68	Rural Two-Lane Segment Two-lane Undivided	369+14.99 0	370+30.00 0	115.01	0.0218	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
69	Rural Two-Lane Segment Two-lane Undivided	370+30.00 0	370+60.00 0	30.00	0.0057	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
70	Rural Two-Lane Segment Two-lane Undivided	370+60.00 0	376+83.61 0	623.61	0.1181	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.24	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
71	Rural Two-Lane Segment Two-lane Undivided	376+83.61 0	378+00.00 0	116.39	0.0220	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
72	Rural Two-Lane Segment Two-lane Undivided	378+00.00 0	378+40.00 0	40.00	0.0076	2018-2022: 2,085	12.00	12.00	0.00	8.00	-1.04	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
73	Rural Two-Lane Segment Two-lane Undivided	378+40.00 0	378+60.00 0	20.00	0.0038	2018-2022: 2,085	12.00	12.00	0.00	8.00	-1.04	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
74	Rural Two-Lane Segment Two-lane Undivided	378+60.00 0	379+00.00 0	40.00	0.0076	2018-2022: 2,085	12.00	12.00	0.00	8.00	-1.04	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
75	Rural Two-Lane Segment Two-lane Undivided	379+00.00 0	379+62.69 0	62.69	0.0119	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	4,023.18	2.0	true	70
76	Rural Two-Lane Segment Two-lane Undivided	379+62.69 0	385+22.97 0	560.28	0.1061	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false				
77	Rural Two-Lane Segment Two-lane Undivided	385+22.97 0	386+60.00 0	137.03	0.0260	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
78	Rural Two-Lane Segment Two-lane Undivided	386+60.00 0	389+50.00 0	290.00	0.0549	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
79	Rural Two-Lane Segment Two-lane Undivided	389+50.00 0	394+00.00 0	450.00	0.0852	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazard Rating	Centerline Rumble Strip	Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
80	Rural Two-Lane Segment Two-lane Undivided	394+00.00 0	396+46.15 0	246.15	0.0466	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.04	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
81	Rural Two-Lane Segment Two-lane Undivided	396+46.15 0	397+00.00 0	53.85	0.0102	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
82	Rural Two-Lane Segment Two-lane Undivided	397+00.00 0	399+00.00 0	200.00	0.0379	2018-2022: 2,085	12.00	12.00	0.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
83	Rural Two-Lane Segment Two-lane Undivided	399+00.00 0	405+75.41 0	675.41	0.1279	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.72	6.2	3	false	0	false	false	false	3,856.89	2.0	true	70
84	Rural Two-Lane Segment Two-lane Undivided	405+75.41 0	406+00.00 0	24.59	0.0047	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.72	6.2	3	false	0	false	false	false				
85	Rural Two-Lane Segment Two-lane Undivided	406+00.00 0	407+00.00 0	100.00	0.0189	2018-2022: 2,085	12.00	12.00	0.00	8.00	0.72	6.2	3	false	0	false	false	false				
86	Rural Two-Lane Segment Two-lane Undivided	407+00.00 0	443+25.00 0	3,625.00	0.6866	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.72	6.2	3	false	0	false	false	false				
87	Rural Two-Lane Segment Two-lane Undivided	443+25.00 0	445+50.00 0	225.00	0.0426	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
88	Rural Two-Lane Segment Two-lane Undivided	445+50.00 0	452+50.00 0	700.00	0.1326	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
89	Rural Two-Lane Segment Two-lane Undivided	452+50.00 0	459+00.00 0	650.00	0.1231	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
90	Rural Two-Lane Segment Two-lane Undivided	459+00.00 0	460+00.00 0	100.00	0.0189	2018-2022: 2,085	12.00	12.00	8.00	0.00	-0.96	6.2	3	false	0	false	false	false				
91	Rural Two-Lane Segment Two-lane Undivided	460+00.00 0	460+58.58 0	58.58	0.0111	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
92	Rural Two-Lane Segment Two-lane Undivided	460+58.58 0	485+61.23 0	2,502.65	0.4740	2018-2022: 2,085	12.00	12.00	8.00	8.00	-0.01	6.2	3	false	0	false	false	false				
93	Rural Two-Lane Segment Two-lane Undivided	485+61.23 0	503+00.00 0	1,738.77	0.3293	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.07	6.2	3	false	0	false	false	false				
94	Rural Two-Lane Segment Two-lane Undivided	503+00.00 0	507+00.00 0	400.00	0.0758	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.07	6.2	3	false	0	false	false	false				
95	Rural Two-Lane Segment Two-lane Undivided	507+00.00 0	508+00.00 0	100.00	0.0189	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.07	6.2	3	false	0	false	true	false				
96	Rural Two-Lane Segment Two-lane Undivided	508+00.00 0	508+08.24 0	8.24	0.0016	2018-2022: 2,085	12.00	12.00	8.00	8.00	-1.07	6.2	3	false	0	false	true	false				
97	Rural Two-Lane Segment Two-lane Undivided	508+08.24 0	510+30.00 0	221.76	0.0420	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.21	6.2	3	false	0	false	true	false				
98	Rural Two-Lane Segment Two-lane Undivided	510+30.00 0	512+00.00 0	170.00	0.0322	2018-2022: 2,085	12.00	12.00	8.00	8.00	0.21	6.2	3	false	0	false	false	false				
99	Rural Two-Lane Segment Two-lane Undivided	512+00.00 0	513+00.00 0	100.00	0.0189	2018-2022: 4,325	12.00	12.00	0.00	0.00	0.21	6.2	3	false	0	false	true	false				
100	Rural Two-Lane Segment Two-lane Undivided	513+00.00 0	515+00.00 0	200.00	0.0379	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.21	6.2	3	false	0	false	true	false				
101	Rural Two-Lane Segment Two-lane Undivided	515+00.00 0	520+00.00 0	500.00	0.0947	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.21	6.2	3	false	0	true	true	false				
102	Rural Two-Lane Segment Two-lane Undivided	520+00.00 0	520+49.15 0	49.15	0.0093	2018-2022: 4,325	12.00	12.00	0.00	0.00	0.21	6.2	3	false	0	false	true	false				
103	Rural Two-Lane Segment Two-lane Undivided	520+49.15 0	521+00.00 0	50.85	0.0096	2018-2022: 4,325	12.00	12.00	0.00	0.00	0.21	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
104	Rural Two-Lane Segment Two-lane Undivided	521+00.00 0	523+38.60 0	238.60	0.0452	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.21	6.2	3	false	0	true	true	false	2,458.49	2.0	true	45
105	Rural Two-Lane Segment Two-lane Undivided	523+38.60 0	524+00.00 0	61.40	0.0116	2018-2022: 4,325	12.00	12.00	8.00	8.00	1.90	6.2	3	false	0	true	true	false	2,458.49	2.0	true	45
106	Rural Two-Lane Segment Two-lane Undivided	524+00.00 0	525+00.00 0	100.00	0.0189	2018-2022: 4,325	12.00	12.00	0.00	0.00	1.90	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazard Rating	Centerline Rumble Strip	Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Adverse	Design Speed (mph)
107	Rural Two-Lane Segment Two-lane Undivided	525+00.00 0	525+18.58 0	18.58	0.0035	2018-2022: 4,325	12.00	12.00	8.00	8.00	1.90	6.2	3	false	0	true	true	false	2,458.49	2.0	true	45
108	Rural Two-Lane Segment Two-lane Undivided	525+18.58 0	528+00.00 0	281.42	0.0533	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.02	6.2	3	false	0	true	true	false	2,458.49	2.0	true	45
109	Rural Two-Lane Segment Two-lane Undivided	528+00.00 0	529+00.00 0	100.00	0.0189	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.02	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
110	Rural Two-Lane Segment Two-lane Undivided	529+00.00 0	539+00.00 0	1,000.00	0.1894	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.02	6.2	3	false	0	true	true	false	2,458.49	2.0	true	45
111	Rural Two-Lane Segment Two-lane Undivided	539+00.00 0	539+50.00 0	50.00	0.0095	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.02	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
112	Rural Two-Lane Segment Two-lane Undivided	539+50.00 0	540+00.00 0	50.00	0.0095	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
113	Rural Two-Lane Segment Two-lane Undivided	540+00.00 0	540+50.00 0	50.00	0.0095	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
114	Rural Two-Lane Segment Two-lane Undivided	540+50.00 0	540+74.37 0	24.37	0.0046	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	true	false	2,458.49	2.0	true	45
115	Rural Two-Lane Segment Two-lane Undivided	540+74.37 0	541+00.00 0	25.63	0.0049	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	true	false				
116	Rural Two-Lane Segment Two-lane Undivided	541+00.00 0	541+50.00 0	50.00	0.0095	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	true	false				
117	Rural Two-Lane Segment Two-lane Undivided	541+50.00 0	541+70.00 0	20.00	0.0038	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	true	false				
118	Rural Two-Lane Segment Two-lane Undivided	541+70.00 0	542+30.00 0	60.00	0.0114	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	true	false				
119	Rural Two-Lane Segment Two-lane Undivided	542+30.00 0	542+64.00 0	34.00	0.0064	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	true	false				
120	Rural Two-Lane Segment Two-lane Undivided	542+64.00 0	543+34.00 0	70.00	0.0133	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	true	false				
121	Rural Two-Lane Segment Two-lane Undivided	543+34.00 0	544+00.00 0	66.00	0.0125	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	true	false				
122	Rural Two-Lane Segment Two-lane Undivided	544+00.00 0	545+00.00 0	100.00	0.0189	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
123	Rural Two-Lane Segment Two-lane Undivided	545+00.00 0	548+23.00 0	323.00	0.0612	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
124	Rural Two-Lane Segment Two-lane Undivided	548+23.00 0	553+70.00 0	547.00	0.1036	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
125	Rural Two-Lane Segment Two-lane Undivided	553+70.00 0	554+00.00 0	30.00	0.0057	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	false	false				
126	Rural Two-Lane Segment Two-lane Undivided	554+00.00 0	554+20.00 0	20.00	0.0038	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.96	6.2	3	false	0	false	false	false				
127	Rural Two-Lane Segment Two-lane Undivided	554+20.00 0	560+00.00 0	580.00	0.1098	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
128	Rural Two-Lane Segment Two-lane Undivided	560+00.00 0	562+58.56 0	258.56	0.0490	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.96	6.2	3	false	0	false	false	false				
129	Rural Two-Lane Segment Two-lane Undivided	562+58.56 0	564+00.00 0	141.44	0.0268	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false				
130	Rural Two-Lane Segment Two-lane Undivided	564+00.00 0	565+00.00 0	100.00	0.0189	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false				
131	Rural Two-Lane Segment Two-lane Undivided	565+00.00 0	565+77.00 0	77.00	0.0146	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false				
132	Rural Two-Lane Segment Two-lane Undivided	565+77.00 0	566+10.00 0	33.00	0.0063	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.20	6.2	3	false	0	false	false	false				
133	Rural Two-Lane Segment Two-lane Undivided	566+10.00 0	566+50.00 0	40.00	0.0076	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.20	6.2	3	false	0	false	false	false				

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT			Shoulder	Right Shoulder Width (ft)	Grad e (%)	Driveway Density (driveways/ mi)	Hazard Rating		Passing Lanes	TWL T Lane	Lighting	Automated Speed Enforcement	Radius (ft)	Superelevation (%)	Design Speed (mph)
134	Rural Two-Lane Segment Two-lane Undivided	566+50.00 0	569+37.00 0	287.00	0.0544	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false			
135	Rural Two-Lane Segment Two-lane Undivided	569+37.00 0	569+70.00 0	33.00	0.0063	2018-2022: 4,325	12.00	12.00	8.00	0.00	-0.20	6.2	3	false	0	false	false	false			
136	Rural Two-Lane Segment Two-lane Undivided	569+70.00 0	570+00.00 0	30.00	0.0057	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false			
137	Rural Two-Lane Segment Two-lane Undivided	570+00.00 0	575+00.00 0	500.00	0.0947	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	true	false	false			
138	Rural Two-Lane Segment Two-lane Undivided	575+00.00 0	579+50.00 0	450.00	0.0852	2018-2022: 4,325	12.00	12.00	8.00	8.00	-0.20	6.2	3	false	0	false	false	false			
139	Rural Two-Lane Segment Two-lane Undivided	579+50.00 0	579+70.00 0	20.00	0.0038	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.20	6.2	3	false	0	false	false	false			
140	Rural Two-Lane Segment Two-lane Undivided	579+70.00 0	580+10.00 0	40.00	0.0076	2018-2022: 4,325	12.00	12.00	0.00	0.00	-0.20	6.2	3	false	0	false	false	false			

**Table 4. Evaluation Intersection - Section 1** 

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
1	West Central School/SD38 (v2)	Rural Two-Lane Intersection Three-Legged w/STOP control	569+50.000	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,773; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	2025: 912; 2026: 932; 2027: 951; 2028: 970; 2029: 990; 2030: 1,013; 2031: 1,036; 2032: 1,059; 2033: 1,082; 2034: 1,105; 2035: 1,129; 2036: 1,152; 2037: 1,175; 2038: 1,198; 2039: 1,221; 2040: 1,245; 2041: 1,273; 2042: 1,302; 2043: 1,330; 2044: 1,359; 2045: 1,387; 2046: 1,416; 2047: 1,444; 2048: 1,473; 2049: 1,501; 2050: 1,530	3	Stop-Controlled	0	0	1.37		false
2	2nd/SD38 (v2)	Rural Two-Lane Intersection Four-Legged w/STOP control	566+00.000	2025: 7,087; 2026: 8,007; 2027: 8,928; 2028: 9,849; 2029: 10,770; 2030: 10,937; 2031: 11,104; 2032: 11,271; 2033: 11,439; 2034: 11,606; 2035: 11,735; 2036: 11,940; 2037: 12,108; 2038: 12,275; 2039: 12,442; 2040: 12,610; 2041: 12,806; 2042: 13,002; 2043: 13,198; 2044: 13,394; 2045: 13,590; 2046: 13,786; 2047: 13,982; 2048: 14,178; 2049: 14,374; 2050: 14,570	2025: 1,338; 2026: 1,366; 2027: 1,394; 2028: 1,422; 2029: 1,450; 2030: 1,484; 2031: 1,518; 2032: 1,552; 2033: 1,586; 2034: 1,620; 2035: 1,654; 2036: 1,688; 2037: 1,722; 2038: 1,756; 2039: 1,790; 2040: 1,825; 2041: 1,867; 2042: 1,909; 2043: 1,951; 2044: 1,993; 2045: 2,035; 2046: 2,077; 2047: 2,119; 2048: 2,161; 2049: 2,203; 2050: 2,245	4	Stop-Controlled	1	0	41.37	41.37	false
4	459/SD38 (v2)	Rural Two-Lane Intersection Four-Legged w/STOP control	296+00.000	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	2025: 320; 2026: 329; 2027: 337; 2028: 346; 2029: 355; 2030: 363; 2031: 371; 2032: 379; 2033: 387; 2034: 395; 2035: 404; 2036: 412; 2037: 420; 2038: 428: 2039: 436; 2040: 445; 2041: 455; 2042: 465; 2043: 475; 2044: 485; 2044: 485; 2046: 505; 2047: 515; 2048: 525; 2049: 535; 2050: 545	4	Stop-Controlled	2	0	0.04	0.04	false
5	SD38/SD19_Build (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	187+50.000	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378: 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	2025: 2,094; 2026: 2,140; 2027: 2,187; 2028: 2,233; 2029: 2,280; 2030: 2,336; 2031: 2,392; 2032: 2,499; 2033: 2,505; 2034: 2,561; 2035: 2,618; 2036: 2,674; 2037: 2,730; 2038: 2,787; 2039: 2,843; 2040: 2,900; 2041: 2,967; 2042: 3,034; 2043: 3,101; 2044: 3,168; 2045: 3,235; 2046: 3,302; 2047: 3,369; 2048: 3,436; 2049: 3,503; 2050: 3,570	4	Stop-Controlled	2	0	5.84	5.84	false
6	I90 SPEEDWAY/SD38 (v1)	Rural Two-Lane Intersection Three-Legged w/STOP control	378+50.000	2025: 2,232; 2026: 2,282; 2027: 2,331; 2028: 2,380; 2029: 2,430; 2030: 2,490; 2031: 2,550; 2032: 2,610; 2033: 2,670; 2034: 2,730; 2035: 2,790; 2036: 2,850; 2037: 2,910; 2038: 2,970; 2039: 3,030; 2040: 3,090; 2041: 3,162; 2042: 3,234; 2043: 3,306; 2044: 3,378; 2045: 3,450; 2046: 3,522; 2047: 3,594; 2048: 3,666; 2049: 3,738; 2050: 3,810	2025: 260; 2026: 264; 2027: 267; 2028: 271; 2029: 275; 2030: 281; 2031: 288; 2032: 295; 2033: 302; 2034: 309; 2035: 315; 2036: 322; 2037: 329; 2038: 336; 2039: 333; 2040: 350; 2041: 372; 2042: 395; 2043: 417; 2044: 440; 2045: 462; 2046: 485; 2047: 507; 2048: 530; 2049: 552; 2050: 575	3	Stop-Controlled	1	1	5.46		false
7	463/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	512+00.000	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	2025: 3,802; 2026: 3,882; 2027: 3,963; 2028: 4,044; 2029: 4,125; 2030: 4,221; 2031: 4,318; 2032: 4,415; 2033: 4,512; 2034: 4,609; 2035: 4,705; 2036: 4,802; 2037: 4,899; 2038: 4,996; 2039: 5,093; 2040: 5,190; 2041: 5,308; 2042: 5,427; 2043: 5,545; 2044: 5,664; 2045: 5,782; 2046: 5,901; 2047: 6,019; 2048: 6,138; 2049: 6,256; 2050: 6,375	4	Stop-Controlled	1	0	1.43	1.43	false
8	Main Ave/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	524+50.000	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	2025: 1,367; 2026: 1,397; 2027: 1,426; 2028: 1,455; 2029: 1,485; 2030: 1,520; 2031: 1,555; 2032: 1,590; 2033: 1,625; 2034: 1,660; 2035: 1,695; 2036: 1,730; 2037: 1,765; 2038: 1,800; 2039: 1,835; 2040: 1,870; 2041: 1,912; 2042: 1,955; 2043: 1,997; 2044: 2,040; 2045: 2,082; 2046: 2,125; 2047: 2,167; 2048: 2,210; 2049: 2,252; 2050: 2,295	4	Stop-Controlled	0	0	11.00	10.54	false
9	Vandemark/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	541+50.000	2025: 6,594; 2026: 7,350; 2027: 8,107; 2028: 8,863; 2029: 9,620; 2030: 9,751; 2031: 9,883; 2032: 10,015; 2033: 10,147; 2034: 10,279; 2035: 10,410; 2036: 10,542; 2037: 10,674; 2038: 10,806; 2039: 10,938; 2040: 11,070; 2041: 11,221; 2042: 11,372; 2043: 11,523; 2044: 11,674; 2045: 11,825; 2046: 11,976; 2047: 12,127; 2048: 12,278; 2049: 12,429; 2050: 12,580	2025: 657; 2026: 672; 2027: 686; 2028: 700; 2029: 715; 2030: 731; 2031: 747; 2032: 764; 2033: 780; 2034: 796; 2035: 813; 2036: 829; 2037: 845; 2038: 862; 2039: 878; 2040: 895; 2041: 915; 2042: 936; 2043: 956; 2044: 977; 2045: 997; 2046: 1,018; 2047: 1,038; 2048: 1,059; 2049: 1,079; 2050: 1,100	4	Stop-Controlled	1	0	48.63	48.63	false

 $\begin{tabular}{ll} \textbf{Table 5.} & \textbf{Crash History Intersection - Section 1} \\ \end{tabular}$ 

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
1	West Central School/SD38 (v2)	Rural Two-Lane Intersection Three-Legged w/STOP control	569+50.000	2018-2022: 4,325	2018-2022: 855	3	Stop-Controlled	0	0	1.37		false
2	2nd/SD38 (v2)	Rural Two-Lane Intersection Four-Legged w/STOP control	566+00.000	2018-2022: 4,325	2018-2022: 1,255	4	Stop-Controlled	1	0	41.37	41.37	false
4	459/SD38 (v2)	Rural Two-Lane Intersection Four-Legged w/STOP control	296+00.000	2018-2022: 2,085	2018-2022: 295	4	Stop-Controlled	2	0	0.04	0.04	false
5	SD38/SD19_Build (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	187+50.000	2018-2022: 2,085	2018-2022: 1,955	4	Stop-Controlled	2	0	5.84	5.84	false
6	I90 SPEEDWAY/SD38 (v1)	Rural Two-Lane Intersection Three-Legged w/STOP control	378+50.000	2018-2022: 2,085	2018-2022: 250	3	Stop-Controlled	1	1	5.46		false
7	463/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	512+00.000	2018-2022: 4,325	2018-2022: 3,560	4	Stop-Controlled	1	0	1.43	1.43	false
8	Main Ave/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	524+50.000	2018-2022: 4,325	2018-2022: 1,280	4	Stop-Controlled	0	0	11.00	10.54	false
9	Vandemark/SD38 (v1)	Rural Two-Lane Intersection Four-Legged w/STOP control	541+50.000	2018-2022: 4,325	2018-2022: 615	4	Stop-Controlled	1	0	48.63	48.63	false

Table 6. Expected Highway Crash Rates and Frequencies Summary (Section 1)

	i
First Year of Analysis	2025
Last Year of Analysis	2050
Evaluated Length (mi)	7.7398
Average Future Road AADT (vpd)	4,284
Expected Crashes	
Total Crashes	476.74
Fatal and Injury Crashes	193.60
Property-Damage-Only Crashes	283.14
Percent of Total Expected Crashes	
Percent Fatal and Injury Crashes (%)	41
Percent Property-Damage-Only Crashes (%)	59
Expected Crash Rate	
Crash Rate (crashes/mi/yr)	2.3691
FI Crash Rate (crashes/mi/yr)	0.9621
PDO Crash Rate (crashes/mi/yr)	1.4070
Expected Travel Crash Rate	
Total Travel (million veh-mi)	314.63
Travel Crash Rate (crashes/million veh-mi)	1.51
Travel FI Crash Rate (crashes/million veh-mi)	0.61
Travel PDO Crash Rate (crashes/million veh-mi)	0.90

Table 7. Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 1)

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi llion veh- mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
1	171+44.000	172+42.000	0.0186	0.237	0.402	0.0091	0.0033	0.0058	0.0155	0.0050	0.0105	-0.0063	-0.0017	-0.0047	0.4920	0.45	
2	172+42.000	174+52.690	0.0399	0.482	0.786	0.0185	0.0067	0.0119	0.0302	0.0097	0.0205	-0.0117	-0.0030	-0.0087	0.4646	0.43	
3	174+52.690	176+25.000	0.0326	0.499	0.980	0.0192	0.0072	0.0120	0.0377	0.0121	0.0256	-0.0185	-0.0049	-0.0136	0.5886	0.54	
4	176+25.000	178+85.250	0.0493	0.754	1.479	0.0290	0.0108	0.0182	0.0569	0.0183	0.0386	-0.0279	-0.0074	-0.0205	0.5886	0.54	
5	178+85.250	183+75.370	0.0928	1.121	1.829	0.0431	0.0155	0.0276	0.0703	0.0226	0.0478	-0.0272	-0.0071	-0.0201	0.4646	0.43	
6	183+75.370	184+00.000	0.0047	0.056	0.092	0.0022	0.0008	0.0014	0.0035	0.0011	0.0024	-0.0014	-0.0004	-0.0010	0.4646	0.43	
7	184+00.000	184+45.000	0.0085	0.103	0.168	0.0040	0.0014	0.0025	0.0065	0.0021	0.0044	-0.0025	-0.0006	-0.0018	0.4646	0.43	
8	184+45.000	185+20.000	0.0142	0.172	0.280	0.0066	0.0024	0.0042	0.0108	0.0035	0.0073	-0.0042	-0.0011	-0.0031	0.4646	0.43	
9	185+20.000	186+60.000	0.0265	0.320	0.522	0.0123	0.0044	0.0079	0.0201	0.0064	0.0136	-0.0078	-0.0020	-0.0058	0.4646	0.43	
10	186+60.000	187+20.000	0.0114	0.137	0.224	0.0053	0.0019	0.0034	0.0086	0.0028	0.0058	-0.0033	-0.0009	-0.0025	0.4646	0.43	
11	187+20.000	187+60.000	0.0076	0.111	0.208	0.0043	0.0016	0.0027	0.0080	0.0026	0.0054	-0.0037	-0.0010	-0.0027	0.5613	0.52	
SD38/SD19_Build (v1)	187+50.000			18.404	41.067	0.7079	0.3235	0.3844	1.5795	0.6808	0.8987	-0.8716	-0.3573	-0.5144			0.3
12	187+60.000	190+00.000	0.0455	0.549	0.895	0.0211	0.0076	0.0135	0.0344	0.0111	0.0234	-0.0133	-0.0035	-0.0099	0.4646	0.43	
13	190+00.000	192+00.000	0.0379	0.458	0.746	0.0176	0.0063	0.0113	0.0287	0.0092	0.0195	-0.0111	-0.0029	-0.0082	0.4646	0.43	
14	192+00.000	192+39.270	0.0074	0.090	0.146	0.0035	0.0012	0.0022	0.0056	0.0018	0.0038	-0.0022	-0.0006	-0.0016	0.4646	0.43	
15	192+39.270	193+60.000	0.0229	0.276	0.451	0.0106	0.0038	0.0068	0.0173	0.0056	0.0118	-0.0067	-0.0017	-0.0050	0.4646	0.43	
16	193+60.000	197+65.000	0.0767	0.927	1.511	0.0356	0.0128	0.0228	0.0581	0.0187	0.0395	-0.0225	-0.0058	-0.0166	0.4646	0.43	
17	197+65.000	199+00.000	0.0256	0.309	0.504	0.0119	0.0043	0.0076	0.0194	0.0062	0.0132	-0.0075	-0.0019	-0.0055	0.4646	0.43	
18	199+00.000	201+63.750	0.0500	0.603	0.984	0.0232	0.0083	0.0149	0.0379	0.0121	0.0257	-0.0146	-0.0038	-0.0108	0.4646	0.43	
19	201+63.750	202+00.000	0.0069	0.083	0.135	0.0032	0.0011	0.0020	0.0052	0.0017	0.0035	-0.0020	-0.0005	-0.0015	0.4646	0.43	
20	202+00.000	207+00.000	0.0947	1.144	1.866	0.0440	0.0158	0.0282	0.0718	0.0230	0.0487	-0.0278	-0.0072	-0.0205	0.4646	0.43	
21	207+00.000	207+49.760	0.0094	0.114	0.186	0.0044	0.0016	0.0028	0.0071	0.0023	0.0048	-0.0028	-0.0007	-0.0020	0.4646	0.43	
22	207+49.760	217+74.250	0.1940	2.344	3.823	0.0902	0.0324	0.0577	0.1470	0.0472	0.0998	-0.0569	-0.0148	-0.0421	0.4646	0.43	
23	217+74.250	221+00.000	0.0617	3.596	1.215	0.1383	0.0144	0.1239	0.0467	0.0150	0.0317	0.0916	-0.0006	0.0922	2.2419	2.07	
24	221+00.000	226+00.000	0.0947	1.144	1.866	0.0440	0.0158	0.0282	0.0718	0.0230	0.0487	-0.0278	-0.0072	-0.0205	0.4646	0.43	
25	226+00.000	230+66.250	0.0883	1.067	1.740	0.0410	0.0148	0.0263	0.0669	0.0215	0.0454	-0.0259	-0.0067	-0.0192	0.4646	0.43	
26	230+66.250	231+39.700	0.0139	0.188	0.333	0.0072	0.0026	0.0046	0.0128	0.0041	0.0087	-0.0055	-0.0015	-0.0041	0.5207	0.48	
27	231+39.700	235+00.000	0.0682	0.924	1.631	0.0355	0.0130	0.0225	0.0627	0.0201	0.0426	-0.0272	-0.0071	-0.0201	0.5207	0.48	
28	235+00.000	241+61.390	0.1253	4.891	2.994	0.1881	0.1175	0.0706	0.1152	0.0370	0.0782	0.0730	0.0805	-0.0076	1.5018	1.39	
29	241+61.390	242+00.000	0.0073	0.099	0.175	0.0038	0.0014	0.0024	0.0067	0.0022	0.0046	-0.0029	-0.0008	-0.0021	0.5207	0.48	
30	242+00.000	245+14.280	0.0595	0.806	1.423	0.0310	0.0113	0.0197	0.0547	0.0176	0.0372	-0.0237	-0.0062	-0.0175	0.5207	0.48	
31	245+14.280	246+55.100	0.0267	0.322	0.525	0.0124	0.0045	0.0079	0.0202	0.0065	0.0137	-0.0078	-0.0020	-0.0058	0.4646	0.43	
32	246+55.100	248+00.000	0.0274	0.332	0.541	0.0128	0.0046	0.0082	0.0208	0.0067	0.0141	-0.0080	-0.0021	-0.0060	0.4646	0.43	
33	248+00.000	249+00.000	0.0189	3.721	0.519	0.1431	0.0060	0.1371	0.0200	0.0064	0.0136	0.1231	-0.0004	0.1236	7.5559	6.97	
34	249+00.000	251+21.980	0.0420	0.508	0.828	0.0195	0.0070	0.0125	0.0319	0.0102	0.0216	-0.0123	-0.0032	-0.0091	0.4646	0.43	
35	251+21.980	252+40.240	0.0224	0.286	0.485	0.0110	0.0040	0.0070	0.0187	0.0060	0.0127	-0.0076	-0.0020	-0.0057	0.4920	0.45	
36	252+40.240	263+22.600	0.2050	5.327	4.038	0.2049	0.0424	0.1625	0.1553	0.0499	0.1055	0.0496	-0.0074	0.0570	0.9995	0.92	
37	263+22.600	272+66.740	0.1788	2.351	4.061	0.0904	0.0329	0.0575	0.1562	0.0501	0.1060	-0.0657	-0.0172	-0.0485	0.5057	0.47	
38	272+66.740	280+00.000	0.1389	1.678	2.736	0.0645	0.0232	0.0413	0.1052	0.0338	0.0715	-0.0407	-0.0106	-0.0301	0.4646	0.43	

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi llion veh- mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
39	280+00.000	283+15.050	0.0597	0.721	1.175	0.0277	0.0100	0.0178	0.0452	0.0145	0.0307	-0.0175	-0.0045	-0.0129	0.4646	0.43	
40	283+15.050	284+08.540	0.0177	0.227	0.384	0.0087	0.0032	0.0056	0.0148	0.0047	0.0100	-0.0060	-0.0016	-0.0045	0.4920	0.45	
41	284+08.540	288+50.000	0.0836	3.861	1.647	0.1485	0.0190	0.1295	0.0634	0.0203	0.0430	0.0851	-0.0013	0.0865	1.7761	1.64	
42	288+50.000	289+00.000	0.0095	0.114	0.187	0.0044	0.0016	0.0028	0.0072	0.0023	0.0049	-0.0028	-0.0007	-0.0021	0.4646	0.43	
43	289+00.000	295+90.000	0.1307	1.579	2.575	0.0607	0.0218	0.0389	0.0990	0.0318	0.0672	-0.0383	-0.0100	-0.0284	0.4646	0.43	
44	295+90.000	296+00.000	0.0019	0.028	0.052	0.0011	0.0004	0.0007	0.0020	0.0006	0.0014	-0.0009	-0.0002	-0.0007	0.5613	0.52	
459/SD38 (v2)	296+00.000			11.436	12.695	0.4399	0.1714	0.2685	0.4883	0.2104	0.2778	-0.0484	-0.0391	-0.0093			0.37
45	296+00.000	296+10.000	0.0019	0.028	0.052	0.0011	0.0004	0.0007	0.0020	0.0006	0.0014	-0.0009	-0.0002	-0.0007	0.5613	0.52	
46	296+10.000	296+96.520	0.0164	0.198	0.323	0.0076	0.0027	0.0049	0.0124	0.0040	0.0084	-0.0048	-0.0012	-0.0036	0.4646	0.43	
47	296+96.520	298+33.660	0.0260	0.314	0.512	0.0121	0.0043	0.0077	0.0197	0.0063	0.0134	-0.0076	-0.0020	-0.0056	0.4646	0.43	
48	298+33.660	303+50.000	0.0978	1.181	1.927	0.0454	0.0163	0.0291	0.0741	0.0238	0.0503	-0.0287	-0.0074	-0.0212	0.4646	0.43	
49	303+50.000	304+50.000	0.0189	0.229	0.373	0.0088	0.0032	0.0056	0.0144	0.0046	0.0097	-0.0056	-0.0014	-0.0041	0.4646	0.43	
50	304+50.000	305+02.039	0.0099	0.119	0.194	0.0046	0.0016	0.0029	0.0075	0.0024	0.0051	-0.0029	-0.0008	-0.0021	0.4646	0.43	
51	305+02.039	309+35.490	0.0821	0.992	1.617	0.0381	0.0137	0.0244	0.0622	0.0200	0.0422	-0.0241	-0.0063	-0.0178	0.4646	0.43	
52	309+35.490	311+70.000	0.0444	0.536	0.875	0.0206	0.0074	0.0132	0.0337	0.0108	0.0229	-0.0130	-0.0034	-0.0096	0.4646	0.43	
53	311+70.000	313+25.000	0.0294	0.355	0.578	0.0136	0.0049	0.0087	0.0222	0.0071	0.0151	-0.0086	-0.0022	-0.0064	0.4646	0.43	
54	313+25.000	323+00.000	0.1847	2.231	3.638	0.0858	0.0309	0.0549	0.1399	0.0449	0.0950	-0.0541	-0.0141	-0.0401	0.4646	0.43	
55	323+00.000	323+26.980	0.0051	0.062	0.101	0.0024	0.0009	0.0015	0.0039	0.0012	0.0026	-0.0015	-0.0004	-0.0011	0.4646	0.43	
56	323+26.980	328+89.230	0.1065	1.286	2.098	0.0495	0.0178	0.0317	0.0807	0.0259	0.0548	-0.0312	-0.0081	-0.0231	0.4646	0.43	
57	328+89.230	329+81.740	0.0175	0.212	0.345	0.0081	0.0029	0.0052	0.0133	0.0043	0.0090	-0.0051	-0.0013	-0.0038	0.4646	0.43	
58	329+81.740	333+24.920	0.0650	0.936	1.738	0.0360	0.0133	0.0227	0.0669	0.0215	0.0454	-0.0308	-0.0081	-0.0227	0.5541	0.51	
59	333+24.920	334+00.000	0.0142	0.205	0.380	0.0079	0.0029	0.0050	0.0146	0.0047	0.0099	-0.0067	-0.0018	-0.0050	0.5541	0.51	
60	334+00.000	335+39.960	0.0265	7.182	0.709	0.2762	0.0084	0.2679	0.0273	0.0088	0.0185	0.2490	-0.0004	0.2494	10.4210	9.62	
61	335+39.960	342+39.000	0.1324	1.599	2.608	0.0615	0.0221	0.0394	0.1003	0.0322	0.0681	-0.0388	-0.0101	-0.0287	0.4646	0.43	
62	342+39.000	343+00.000	0.0116	0.140	0.228	0.0054	0.0019	0.0034	0.0088	0.0028	0.0059	-0.0034	-0.0009 0.0687	-0.0025	0.4646	0.43	
63	343+00.000	351+20.000 352+00.000	0.1553	4.727 0.221	3.060 0.415	0.1818	0.1065 0.0032	0.0753 0.0054	0.1177 0.0160	0.0378	0.0799	0.0641	-0.0020	-0.0046 -0.0055	1.1706 0.5613	0.52	
65	351+20.000 352+00.000	352+00.000 352+20.000	0.0152	0.221	0.415	0.0085	0.0032	0.0054	0.0160	0.0051	0.0108	-0.0075 -0.0019	-0.0020	-0.0055	0.5613	0.52	
66	352+20.000	362+50.000	0.0038	5.207	3.843	0.2003	0.0008	0.0013	0.1478	0.0013	0.1004	0.0525	-0.0003	0.0593	1.0267	0.32	
67	362+50.000	362+30.000 369+14.990	0.1951	1.521	2.481	0.2003	0.0406	0.0375	0.0954	0.0474	0.1004	-0.0369	-0.0069	-0.0273	0.4646	0.43	
68	369+14.990	370+30.000	0.0218	0.305	0.553	0.0383	0.0210	0.0074	0.0213	0.0068	0.0144	-0.0309	-0.0030	-0.0273	0.5385	0.50	
69	370+30.000	370+60,000	0.0218	0.080	0.144	0.00117	0.0043	0.0019	0.0213	0.0018	0.0038	-0.0093	-0.0023	-0.0018	0.5385	0.50	
70	370+60.000	376+83.610	0.1181	4.958	2.998	0.1907	0.1212	0.0695	0.1153	0.0370	0.0783	0.0754	0.0842	-0.0018	1.6146	1.49	
71	376+83.610	378+00.000	0.0220	0.309	0.559	0.0119	0.0044	0.0075	0.0215	0.0069	0.0146	-0.0096	-0.0025	-0.0071	0.5385	0.50	
72	378+00.000	378+40,000	0.0076	0.117	0.230	0.0045	0.0017	0.0075	0.0088	0.0028	0.0060	-0.0044	-0.0023	-0.0032	0.5919	0.55	
73	378+40.000	378+60.000	0.0078	0.058	0.115	0.0022	0.0008	0.0014	0.0044	0.0014	0.0030	-0.0022	-0.0012	-0.0032	0.5919	0.55	
I90 SPEEDWAY/SD38 (v1)	378+50.000	2.2.00.000	2.3030	4.683	6.719	0.1801	0.0774	0.1027	0.2584	0.1073	0.1512	-0.0783	-0.0298	-0.0485	3.3717	0.55	0.16
74	378+60.000	379+00.000	0.0076	0.117	0.230	0.0045	0.0017	0.0028	0.0088	0.0028	0.0060	-0.0044	-0.0012	-0.0032	0.5919	0.55	0.10
75	379+00.000	379+62.690	0.0119	0.166	0.301	0.0064	0.0024	0.0040	0.0116	0.0020	0.0079	-0.0052	-0.0012	-0.0032	0.5385	0.50	
76	379+62.690	385+22.970	0.1061	4.133	2.091	0.1590	0.1024	0.0566	0.0804	0.0258	0.0546	0.0785	0.0766	0.0020	1.4979	1.38	
77	385+22.970	386+60.000	0.0260	0.359	0.643	0.0138	0.0051	0.0087	0.0247	0.0079	0.0168	-0.0109	-0.0029	-0.0081	0.5314	0.49	
78	386+60.000	389+50.000	0.0549	0.759	1.361	0.0292	0.0107	0.0185	0.0524	0.0168	0.0355	-0.0232	-0.0061	-0.0171	0.5314	0.49	

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi Ilion veh- mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
79	389+50.000	394+00.000	0.0852	1.178	2.112	0.0453	0.0166	0.0287	0.0812	0.0261	0.0552	-0.0359	-0.0095	-0.0265	0.5314	0.49	
80	394+00.000	396+46.150	0.0466	0.644	1.155	0.0248	0.0091	0.0157	0.0444	0.0143	0.0302	-0.0197	-0.0052	-0.0145	0.5314	0.49	
81	396+46.150	397+00.000	0.0102	0.141	0.253	0.0054	0.0020	0.0034	0.0097	0.0031	0.0066	-0.0043	-0.0011	-0.0032	0.5314	0.49	
82	397+00.000	399+00.000	0.0379	0.576	1.122	0.0221	0.0083	0.0139	0.0432	0.0139	0.0293	-0.0210	-0.0056	-0.0154	0.5847	0.54	
83	399+00.000	405+75.410	0.1279	1.768	3.170	0.0680	0.0249	0.0430	0.1219	0.0391	0.0828	-0.0539	-0.0142	-0.0398	0.5314	0.49	
84	405+75.410	406+00.000	0.0047	0.056	0.092	0.0022	0.0008	0.0014	0.0035	0.0011	0.0024	-0.0014	-0.0004	-0.0010	0.4646	0.43	
85	406+00.000	407+00.000	0.0189	0.254	0.446	0.0098	0.0036	0.0062	0.0172	0.0055	0.0116	-0.0074	-0.0019	-0.0054	0.5164	0.48	
86	407+00.000	443+25.000	0.6866	25.399	13.526	0.9769	0.4312	0.5456	0.5202	0.1670	0.3532	0.4567	0.2642	0.1924	1.4229	1.31	
87	443+25.000	445+50.000	0.0426	0.515	0.840	0.0198	0.0071	0.0127	0.0323	0.0104	0.0219	-0.0125	-0.0032	-0.0092	0.4646	0.43	
88	445+50.000	452+50.000	0.1326	4.452	2.612	0.1712	0.1044	0.0669	0.1005	0.0322	0.0682	0.0708	0.0721	-0.0013	1.2917	1.19	
89	452+50.000	459+00.000	0.1231	1.487	2.425	0.0572	0.0206	0.0366	0.0933	0.0299	0.0633	-0.0361	-0.0094	-0.0267	0.4646	0.43	
90	459+00.000	460+00.000	0.0189	0.254	0.446	0.0098	0.0036	0.0062	0.0172	0.0055	0.0116	-0.0074	-0.0019	-0.0054	0.5164	0.48	
91	460+00.000	460+58.580	0.0111	0.134	0.219	0.0052	0.0019	0.0033	0.0084	0.0027	0.0057	-0.0033	-0.0008	-0.0024	0.4646	0.43	
92	460+58.580	485+61.230	0.4740	8.577	9.338	0.3299	0.0899	0.2399	0.3592	0.1153	0.2439	-0.0293	-0.0253	-0.0039	0.6959	0.64	
93	485+61.230	503+00.000	0.3293	6.829	6.488	0.2627	0.0647	0.1979	0.2495	0.0801	0.1694	0.0131	-0.0154	0.0285	0.7976	0.74	
94	503+00.000	507+00.000	0.0758	3.766	1.492	0.1448	0.0174	0.1275	0.0574	0.0184	0.0390	0.0874	-0.0010	0.0885	1.9120	1.76	
95	507+00.000	508+00.000	0.0189	0.217	0.344	0.0084	0.0030	0.0054	0.0132	0.0042	0.0090	-0.0049	-0.0013	-0.0036	0.4416	0.41	
96	508+00.000	508+08.240	0.0016	0.018	0.028	0.0007	0.0002	0.0004	0.0011	0.0003	0.0007	-0.0004	-0.0001	-0.0003	0.4416	0.41	
97	508+08.240	510+30.000	0.0420	0.482	0.762	0.0185	0.0066	0.0119	0.0293	0.0094	0.0199	-0.0108	-0.0028	-0.0080	0.4416	0.41	
98	510+30.000	512+00.000	0.0322	0.389	0.634	0.0150	0.0054	0.0096	0.0244	0.0078	0.0166	-0.0094	-0.0025	-0.0070	0.4646	0.43	
463/SD38 (v1)	512+00.000			87.655	169.483	3.3714	1.6033	1.7681	6.5186	2.8095	3.7091	-3.1472	-1.2062	-1.9410			0.88
99	512+00.000	513+00.000	0.0189	0.626	1.666	0.0241	0.0095	0.0145	0.0641	0.0206	0.0435	-0.0400	-0.0110	-0.0290	1.2714	0.33	
100	513+00.000	515+00.000	0.0379	1.092	2.395	0.0420	0.0161	0.0259	0.0921	0.0296	0.0626	-0.0501	-0.0135	-0.0366	1.1086	0.29	
101	515+00.000	520+00.000	0.0947	2.689	5.796	0.1034	0.0394	0.0640	0.2229	0.0716	0.1514	-0.1195	-0.0322	-0.0873	1.0920	0.28	
102	520+00.000	520+49.150	0.0093	0.308	0.819	0.0118	0.0047	0.0071	0.0315	0.0101	0.0214	-0.0197	-0.0054	-0.0142	1.2714	0.33	
103	520+49.150	521+00.000	0.0096	0.343	1.049	0.0132	0.0054	0.0078	0.0403	0.0130	0.0274	-0.0271	-0.0076	-0.0195	1.3705	0.36	
104	521+00.000	523+38.600	0.0452	1.409	3.425	0.0542	0.0211	0.0331	0.1317	0.0423	0.0895	-0.0776	-0.0212	-0.0564	1.1991	0.31	
105	523+38.600	524+00.000	0.0116	0.362	0.881	0.0139	0.0054	0.0085	0.0339	0.0109	0.0230	-0.0200	-0.0054	-0.0145	1.1991	0.31	
106	524+00.000 524+50.000	525+00.000	0.0189	0.675 42.110	2.063	0.0260 1.6196	0.0105	0.0154 0.9418	0.0793 5.1069	0.0255 2.2011	0.0539 2.9058	-0.0534 -3.4872	-0.0149 -1.5232	-0.0384 -1.9640	1.3705	0.36	0.37
Main Ave/SD38 (v1) 107	525+00.000	525+18.580	0.0035	0.110	0.267	0.0042	0.0778	0.9418	0.0103	0.0033	0.0070	-0.0060	-0.0016	-0.0044	1.1991	0.31	0.37
108	525+18.580	528+00.000	0.0033	1.662	4.040	0.0639	0.0016	0.0026	0.0103	0.0033	0.1055	-0.0000	-0.0016	-0.0665	1.1991	0.31	
109	528+00.000	529+00.000	0.0333	0.675	2.063	0.0260	0.0249	0.0390	0.1334	0.0499	0.1033	-0.0534	-0.0230	-0.0663	1.3705	0.31	
110	529+00.000	539+00.000	0.01894	13.262	14.355	0.0200	0.0103	0.3971	0.0793	0.0233	0.0339	-0.0334	-0.0149	0.0223	2.6933	0.36	
111	539+00.000	539+50.000	0.1894	0.299	0.742	0.0115	0.1129	0.3971	0.0285	0.1772	0.3749	-0.0420	-0.0643	-0.0124	1.2152	0.70	
111	539+50.000	540+00.000	0.0095	0.299	0.742	0.0115	0.0045	0.0070	0.0285	0.0092	0.0194	-0.0170	-0.0047	-0.0124	1.2152	0.32	
113	540+00.000	540+50.000	0.0095	0.299	0.742	0.0115	0.0045	0.0070	0.0285	0.0092	0.0194	-0.0170	-0.0047	-0.0124	1.2152	0.32	
114	540+50.000	540+74.370	0.0046	0.165	0.503	0.0063	0.0026	0.0070	0.0193	0.0062	0.0134	-0.0170	-0.0036	-0.0094	1.3705	0.32	
115	540+74.370	541+00.000	0.0049	0.161	0.427	0.0062	0.0024	0.0037	0.0164	0.0053	0.0131	-0.0130	-0.0038	-0.0074	1.2714	0.33	
116	541+00.000	541+50.000	0.0095	0.313	0.833	0.0120	0.0024	0.0037	0.0320	0.0103	0.0218	-0.0200	-0.0025	-0.0145	1.2714	0.33	
Vandemark/SD38 (v1)	541+50.000	2.12.20.000	5.5075	28.334	74.904	1.0898	0.5033	0.5865	2.8809	1.2417	1.6393	-1.7912	-0.7384	-1.0527	1.2.14	0.33	0.27
117	541+50.000	541+70.000	0.0038	0.125	0.333	0.0048	0.0019	0.0029	0.0128	0.0041	0.0087	-0.0080	-0.0022	-0.0058	1.2714	0.33	

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi Ilion veh- mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
118	541+70.000	542+30.000	0.0114	0.376	1.000	0.0144	0.0057	0.0087	0.0384	0.0123	0.0261	-0.0240	-0.0066	-0.0174	1.2714	0.33	
119	542+30.000	542+64.000	0.0064	0.186	0.407	0.0071	0.0027	0.0044	0.0157	0.0050	0.0106	-0.0085	-0.0023	-0.0062	1.1086	0.29	
120	542+64.000	543+34.000	0.0133	0.382	0.838	0.0147	0.0056	0.0091	0.0322	0.0104	0.0219	-0.0175	-0.0047	-0.0128	1.1086	0.29	
121	543+34.000	544+00.000	0.0125	0.360	0.790	0.0139	0.0053	0.0086	0.0304	0.0098	0.0206	-0.0165	-0.0045	-0.0121	1.1086	0.29	
122	544+00.000	545+00.000	0.0189	0.566	1.300	0.0218	0.0084	0.0134	0.0500	0.0160	0.0339	-0.0282	-0.0076	-0.0206	1.1497	0.30	
123	545+00.000	548+23.000	0.0612	1.829	4.198	0.0703	0.0271	0.0432	0.1615	0.0518	0.1096	-0.0911	-0.0247	-0.0664	1.1497	0.30	
124	548+23.000	553+70.000	0.1036	10.151	7.109	0.3904	0.2627	0.1277	0.2734	0.0878	0.1857	0.1170	0.1750	-0.0580	3.7686	0.98	
125	553+70.000	554+00.000	0.0057	0.194	0.542	0.0074	0.0030	0.0045	0.0209	0.0067	0.0142	-0.0134	-0.0037	-0.0097	1.3100	0.34	
126	554+00.000	554+20.000	0.0038	0.129	0.361	0.0050	0.0020	0.0030	0.0139	0.0045	0.0094	-0.0089	-0.0025	-0.0065	1.3100	0.34	
127	554+20.000	560+00.000	0.1098	3.284	7.538	0.1263	0.0487	0.0776	0.2899	0.0931	0.1969	-0.1636	-0.0444	-0.1193	1.1497	0.30	
128	560+00.000	562+58.560	0.0490	1.464	3.360	0.0563	0.0217	0.0346	0.1292	0.0415	0.0878	-0.0729	-0.0198	-0.0532	1.1497	0.30	
129	562+58.560	564+00.000	0.0268	0.801	1.838	0.0308	0.0119	0.0189	0.0707	0.0227	0.0480	-0.0399	-0.0108	-0.0291	1.1497	0.30	
130	564+00.000	565+00.000	0.0189	0.566	1.300	0.0218	0.0084	0.0134	0.0500	0.0160	0.0339	-0.0282	-0.0076	-0.0206	1.1497	0.30	
131	565+00.000	565+77.000	0.0146	0.494	1.133	0.0190	0.0073	0.0117	0.0436	0.0140	0.0296	-0.0246	-0.0067	-0.0179	1.3020	0.30	
132	565+77.000	566+10.000	0.0063	0.241	0.675	0.0093	0.0037	0.0056	0.0260	0.0083	0.0176	-0.0167	-0.0046	-0.0121	1.4835	0.34	
2nd/SD38 (v2)	566+00.000			51.588	119.976	1.9842	0.7408	1.2433	4.6145	1.9888	2.6256	-2.6303	-1.2480	-1.3823			0.41
133	566+10.000	566+50.000	0.0076	0.292	0.819	0.0112	0.0045	0.0067	0.0315	0.0101	0.0214	-0.0203	-0.0056	-0.0146	1.4835	0.34	
134	566+50.000	569+37.000	0.0544	17.818	4.224	0.6853	0.2643	0.4210	0.1625	0.0522	0.1103	0.5228	0.2121	0.3107	12.6074	2.89	
135	569+37.000	569+70.000	0.0063	0.228	0.581	0.0088	0.0034	0.0053	0.0223	0.0072	0.0152	-0.0136	-0.0037	-0.0098	1.4018	0.32	
West Central School/SD38 (v2)	569+50.000			18.933	73.624	0.7282	0.3383	0.3899	2.8317	1.1752	1.6565	-2.1035	-0.8368	-1.2667			0.16
136	569+70.000	570+00.000	0.0057	0.192	0.442	0.0074	0.0029	0.0045	0.0170	0.0055	0.0115	-0.0096	-0.0026	-0.0070	1.3020	0.30	
137	570+00.000	575+00.000	0.0947	3.160	7.122	0.1215	0.0467	0.0748	0.2739	0.0879	0.1860	-0.1524	-0.0412	-0.1112	1.2834	0.29	
138	575+00.000	579+50.000	0.0852	2.885	6.623	0.1110	0.0428	0.0682	0.2547	0.0818	0.1730	-0.1438	-0.0390	-0.1048	1.3020	0.30	
139	579+50.000	579+70.000	0.0038	0.146	0.409	0.0056	0.0022	0.0034	0.0157	0.0051	0.0107	-0.0101	-0.0028	-0.0073	1.4835	0.34	
140	579+70.000	580+10.000	0.0076	0.292	0.819	0.0112	0.0045	0.0067	0.0315	0.0101	0.0214	-0.0203	-0.0056	-0.0146	1.4835	0.34	
All Segments			7.7398	213.597	230.845	8.2153	3.0105	5.2047	8.8786	2.8500	6.0286	-0.6634	0.1605	-0.8239	1.0614	0.68	
All Intersections				263.144	631.248	10.1209	4.4358	5.6852	24.2788	10.4147	13.8641	-14.1579	-5.9790	-8.1789			0.39
Total			7.7398	476.741	862.093	18.3362	7.4463	10.8899	33.1574	13.2648	19.8927	-14.8212	-5.8185	-9.0028	2.3691		

**Table 8. Expected Crash Frequencies and Rates by Horizontal Design Element (Section 1)** 

Title	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	Expected Travel Crash Rate (crashes/mi Ilion veh- mi)
Tangent	171+44.000	174+52.690	0.0585	0.720	1.188	0.0277	0.0100	0.0177	0.0457	0.0147	0.0310	-0.0180	-0.0047	-0.0133	0.4733	0.44
Simple Curve 1	174+52.690	178+85.250	0.0819	1.254	2.459	0.0482	0.0180	0.0302	0.0946	0.0304	0.0642	-0.0464	-0.0123	-0.0340	0.5886	0.54
Tangent	178+85.250	230+66.250	0.9812	14.723	19.390	0.5663	0.1684	0.3979	0.7458	0.2394	0.5064	-0.1795	-0.0710	-0.1085	0.5771	0.53
Simple Curve 2	230+66.250	245+14.280	0.2742	6.908	6.555	0.2657	0.1459	0.1198	0.2521	0.0809	0.1712	0.0136	0.0649	-0.0513	0.9688	0.89
Tangent	245+14.280	263+22.600	0.3425	10.496	6.937	0.4037	0.0685	0.3352	0.2668	0.0856	0.1812	0.1369	-0.0172	0.1541	1.1787	1.09
Simple Curve 3	263+22.600	272+66.740	0.1788	2.351	4.061	0.0904	0.0329	0.0575	0.1562	0.0501	0.1060	-0.0657	-0.0172	-0.0485	0.5057	0.47
Tangent	272+66.740	296+96.470	0.4602	8.432	9.130	0.3243	0.0823	0.2420	0.3511	0.1127	0.2384	-0.0268	-0.0304	0.0036	0.7047	0.65
Simple Curve 4	296+96.470	296+96.520	0.0000	0.000	0.000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	-0.0000	-0.0000	-0.0000	0.4646	0.43
Tangent	296+96.520	329+81.740	0.6222	7.516	12.258	0.2891	0.1040	0.1851	0.4715	0.1513	0.3201	-0.1824	-0.0474	-0.1350	0.4646	0.43
Simple Curve 5	329+81.740	335+39.960	0.1057	8.323	2.828	0.3201	0.0246	0.2956	0.1088	0.0349	0.0738	0.2114	-0.0103	0.2217	3.0280	2.79
Tangent	335+39.960	369+14.990	0.6392	13.471	12.739	0.5181	0.1961	0.3220	0.4900	0.1573	0.3327	0.0282	0.0389	-0.0107	0.8106	0.75
Simple Curve 6	369+14.990	379+62.690	0.1984	6.109	5.131	0.2350	0.1376	0.0974	0.1973	0.0633	0.1340	0.0376	0.0742	-0.0366	1.1841	1.09
Tangent	379+62.690	385+22.970	0.1061	4.133	2.091	0.1590	0.1024	0.0566	0.0804	0.0258	0.0546	0.0785	0.0766	0.0020	1.4979	1.38
Simple Curve 7	385+22.970	405+75.410	0.3887	5.424	9.817	0.2086	0.0767	0.1319	0.3776	0.1212	0.2564	-0.1690	-0.0445	-0.1245	0.5366	0.50
Tangent	405+75.410	520+49.150	2.1731	57.544	50.368	2.2132	0.8301	1.3832	1.9372	0.6218	1.3154	0.2760	0.2082	0.0678	1.0185	0.88
Simple Curve 8	520+49.150	540+74.370	0.3836	19.560	30.871	0.7523	0.2085	0.5438	1.1874	0.3811	0.8062	-0.4350	-0.1726	-0.2624	1.9614	0.51
Tangent	540+74.370	580+10.000	0.7454	46.632	55.023	1.7936	0.8047	0.9889	2.1163	0.6793	1.4370	-0.3227	0.1253	-0.4481	2.4062	0.58

Table 9. Predicted Crash Frequencies by Year (Section 1)

Year	Total Crashes	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
2025	21.56	8.58	39.798	12.98	60.202
2026	23.15	9.22	39.843	13.93	60.157
2027	24.72	9.86	39.881	14.86	60.119
2028	26.28	10.49	39.913	15.79	60.087
2029	27.83	11.12	39.940	16.71	60.060
2030	28.45	11.37	39.948	17.09	60.052
2031	29.08	11.62	39.955	17.46	60.045
2032	29.71	11.87	39.963	17.84	60.037
2033	30.34	12.13	39.971	18.21	60.029
2034	30.97	12.38	39.978	18.59	60.022
2035	31.61	12.64	39.985	18.97	60.015
2036	32.24	12.89	39.992	19.35	60.008
2037	32.88	13.15	39.999	19.73	60.001
2038	33.52	13.41	40.006	20.11	59.994
2039	34.16	13.67	40.012	20.49	59.988
2040	34.80	13.93	40.019	20.87	59.981
2041	35.57	14.24	40.027	21.33	59.973
2042	36.35	14.55	40.035	21.80	59.965
2043	37.12	14.87	40.043	22.26	59.957
2044	37.90	15.18	40.051	22.72	59.949
2045	38.68	15.49	40.058	23.19	59.942
2046	39.47	15.81	40.066	23.65	59.934
2047	40.25	16.13	40.073	24.12	59.927
2048	41.03	16.45	40.080	24.59	59.920
2049	41.82	16.76	40.087	25.05	59.913
2050	42.61	17.09	40.094	25.53	59.906
Total	862.09	344.88	40.005	517.21	59.995
Average	33.16	13.27	40.005	19.89	59.995

**Note:** Fatal and Injury Crashes and Property Damage Only Crashes do not necessarily sum up to Total Crashes because the distribution of these three crashes had been derived independently.

**Table 10. Expected Crash Frequencies by Year (Section 1)** 

Year	Total Crashes	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
2025	11.92	4.82	40.399	7.11	59.596
2026	12.80	5.18	40.445	7.62	59.551
2027	13.67	5.53	40.483	8.14	59.514
2028	14.53	5.89	40.516	8.64	59.482
2029	15.39	6.24	40.543	9.15	59.455
2030	15.73	6.38	40.551	9.35	59.447
2031	16.08	6.52	40.559	9.56	59.440
2032	16.43	6.67	40.567	9.77	59.432
2033	16.78	6.81	40.574	9.97	59.425
2034	17.13	6.95	40.582	10.18	59.418
2035	17.48	7.09	40.589	10.38	59.410
2036	17.83	7.24	40.596	10.59	59.404
2037	18.18	7.38	40.603	10.80	59.397
2038	18.54	7.53	40.610	11.01	59.390
2039	18.89	7.67	40.617	11.22	59.384
2040	19.25	7.82	40.624	11.43	59.377
2041	19.67	7.99	40.632	11.68	59.369
2042	20.10	8.17	40.640	11.93	59.361
2043	20.53	8.34	40.648	12.18	59.353
2044	20.96	8.52	40.656	12.44	59.345
2045	21.39	8.70	40.664	12.69	59.338
2046	21.82	8.88	40.671	12.95	59.330
2047	22.26	9.05	40.678	13.20	59.323
2048	22.69	9.23	40.686	13.46	59.316
2049	23.13	9.41	40.693	13.72	59.309
2050	23.56	9.59	40.700	13.97	59.302
Total	476.74	193.60	40.610	283.14	59.390
Average	18.34	7.45	40.610	10.89	59.390

**Note:** Fatal and Injury Crashes and Property Damage Only Crashes do not necessarily sum up to Total Crashes because the distribution of these three crashes had been derived independently.

Table 11. Comparing Predicted and Expected Crashes for the Evaluation Period (Section 1)

Scope	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
Predicted	862.09	344.88	40.005	517.21	59.995
Expected	476.74	193.60	40.610	283.14	59.390
Expected - Predicted	-385.35	-151.28		-234.07	
Percent Difference	-80.83	-78.14		-82.67	

**Note:** Fatal and Injury Crashes and Property Damage Only Crashes do not necessarily sum up to Total Crashes because the distribution of these three crashes had been derived independently.

**Table 12. Expected Crash Type Distribution (Section 1)** 

Element Type	Crash Type	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)	Total Crashes	Percent Total (%)
Highway Segment	Collision with Animal	2.97	0.6	24.90	5.2	25.84	5.4
Highway Segment	Collision with Bicycle	0.31	0.1	0.14	0.0	0.43	0.1
Highway Segment	Other Single-vehicle Collision	0.55	0.1	3.92	0.8	4.49	0.9
Highway Segment	Overturned	2.90	0.6	2.03	0.4	5.34	1.1
Highway Segment	Collision with Pedestrian	0.55	0.1	0.14	0.0	0.64	0.1
Highway Segment	Run Off Road	42.66	9.0	68.34	14.3	111.28	23.4
Highway Segment	Total Single Vehicle Crashes	49.94	10.5	99.46	20.9	148.02	31.1
Highway Segment	Angle Collision	7.91	1.7	9.74	2.0	18.16	3.8
Highway Segment	Head-on Collision	2.66	0.6	0.41	0.1	3.42	0.7
Highway Segment	Other Multiple-vehicle Collision	2.04	0.4	4.06	0.9	5.77	1.2
Highway Segment	Rear-end Collision	12.91	2.7	16.51	3.5	30.33	6.4
Highway Segment	Sideswipe	2.97	0.6	5.14	1.1	7.90	1.7
Highway Segment	Total Multiple Vehicle Crashes	28.49	6.0	35.86	7.5	65.57	13.8
Highway Segment	Total Highway Segment Crashes	78.43	16.5	135.32	28.4	213.60	44.8
Intersection	Collision with Animal	0.71	0.1	2.22	0.5	2.84	0.6
Intersection	Collision with Bicycle	0.12	0.0	0.15	0.0	0.26	0.1
Intersection	Other Single-vehicle Collision	0.54	0.1	1.61	0.3	2.10	0.4
Intersection	Overturned	0.86	0.2	0.63	0.1	1.50	0.3
Intersection	Collision with Pedestrian	0.12	0.0	0.15	0.0	0.26	0.1
Intersection	Run Off Road	12.42	2.6	22.60	4.7	34.98	7.3
Intersection	Total Single Vehicle Crashes	14.77	3.1	27.36	5.7	41.97	8.8
Intersection	Angle Collision	58.58	12.3	50.48	10.6	108.83	22.8
Intersection	Head-on Collision	7.15	1.5	3.79	0.8	10.81	2.3
Intersection	Other Multiple-vehicle Collision	4.93	1.0	5.42	1.1	10.33	2.2
Intersection	Rear-end Collision	24.76	5.2	39.65	8.3	64.53	13.5
Intersection	Sideswipe	5.15	1.1	21.12	4.4	26.48	5.6
Intersection	Total Multiple Vehicle Crashes	100.56	21.1	120.46	25.3	220.99	46.4
Intersection	Total Intersection Crashes	115.33	24.2	147.81	31.0	262.95	55.2
	Total Crashes	193.76	40.7	283.14	59.4	476.55	100.0

**Note:** Fatal and Injury Crashes and Property Damage Only Crashes do not necessarily sum up to Total Crashes because the distribution of these three crashes had been derived independently.

**Table 13. Evaluation Message** 

Start Location (Sta. ft)	End Location (Sta. ft)	Message
580+00.000	580+00.000	Warning: for intersection #3 (580+00.000 to 580+00.000 ), SE SD-38 at 580+00.000 has more than one lane exiting. No intersection crash prediction computed.
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,560 vpd) for 2018 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,560 vpd) for 2019 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,560 vpd) for 2020 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,560 vpd) for 2021 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,560 vpd) for 2022 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
580+00.000	580+00.000	Warning: for intersection #3 (580+00.000 to 580+00.000 ), SE SD-38 at 580+00.000 has more than one lane exiting. No intersection crash prediction computed.
187+50.000	187+50.000	Warning: for intersection #5 (187+50.000 to 187+50.000), minor road traffic volume (3,503 vpd) for 2049 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
187+50.000	187+50.000	Warning: for intersection #5 (187+50.000 to 187+50.000), minor road traffic volume (3,570 vpd) for 2050 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,802 vpd) for 2025 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,882 vpd) for 2026 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (3,963 vpd) for 2027 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,044 vpd) for 2028 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,125 vpd) for 2029 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,221 vpd) for 2030 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,318 vpd) for 2031 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,415 vpd) for 2032 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000), minor road traffic volume (4,512 vpd) for 2033 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST

Start Location (Sta. ft)	End Location (Sta. ft)	Message
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,609 vpd) for 2034 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,705 vpd) for 2035 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,802 vpd) for 2036 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,899 vpd) for 2037 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (4,996 vpd) for 2038 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,093 vpd) for 2039 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,190 vpd) for 2040 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,308 vpd) for 2041 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,427 vpd) for 2042 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,545 vpd) for 2043 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,664 vpd) for 2044 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,782 vpd) for 2045 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (5,901 vpd) for 2046 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (6,019 vpd) for 2047 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (6,138 vpd) for 2048 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (6,256 vpd) for 2049 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST
512+00.000	512+00.000	Warning: for intersection #7 (512+00.000 to 512+00.000 ), minor road traffic volume (6,375 vpd) for 2050 is not within the model limit (3,500 vpd) for reliable results for intersection type 4ST

# Interactive Highway Safety Design Model

# **Crash Prediction Evaluation Report**

#### Disclaimer

The Interactive Highway Design Model (IHSDM) software is disseminated under the sponsorship of the Department of Transportation in the interest of information exchange. The United States Government assumes no liability for its content or use thereof. This document does not constitute a standard, specification, or regulation.

The United States Government does not endorse products or manufacturers. Trade and manufacturers' names may appear in this software and documentation only because they are considered essential to the objective of the software.

#### **Limited Warranty and Limitations of Remedies**

This software product is provided "as-is," without warranty of any kind-either expressed or implied (but not limited to the implied warranties of merchantability and fitness for a particular purpose). The FHWA do not warrant that the functions contained in the software will meet the end-user's requirements or that the operation of the software will be uninterrupted and error-free.

Under no circumstances will the FHWA be liable to the end-user for any damages or claimed lost profits, lost savings, or other incidental or consequential damages rising out of the use or inability to use the software (even if these organizations have been advised of the possibility of such damages), or for any claim by any other party.

#### **Notice**

The use of the IHSDM software is being done strictly on a voluntary basis. In exchange for provision of IHSDM, the user agrees that the Federal Highway Administration (FHWA), U.S. Department of Transportation and any other agency of the Federal Government shall not be responsible for any errors, damage or other liability that may result from any and all use of the software, including installation and testing of the software. The user further agrees to hold the FHWA and the Federal Government harmless from any resulting liability. The user agrees that this hold harmless provision shall flow to any person to whom or any entity to which the user provides the IHSDM software. It is the user's full responsibility to inform any person to whom or any entity to which it provides the IHSDM software of this hold harmless provision.

### **Table of Contents**

Report Overview	1
Disclaimer Regarding Crash Prediction Method	2
Section Types	3
Section 3 Evaluation	3
Section 4 Evaluation	41
List of Tables	
Table Observed Crashes Used in the Evaluation (Section 3)	5
Table Evaluation Highway - Homogeneous Segments (Section 3)	6
Table Crash History Highway - Homogeneous Segments (Section 3)	17
Table Evaluation Intersection (Section 3)	21
Table Evaluation Intersection (Section 3)	22
Table Evaluation Ramp Terminal - Site (Section 3)	23
Table Crash History Intersection (Section 3)	24
Table Crash History Intersection (Section 3)	25
Table Crash Highway Ramp Terminal - Site (Highway with Crash History)	26
Table Expected Highway Crash Rates and Frequencies Summary (Section 3)	27
Table Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 3)	28
Table Expected Crash Frequencies and Rates by Horizontal Design Element (Section 3)	33
Table Predicted Crash Frequencies by Year (Section 3)	34
Table Expected Crash Frequencies by Year (Section 3)	35
Table Comparing Predicted and Expected Crashes for the Evaluation Period (Section 3)	36
Table Expected Crash Severity by Ramp Terminal or Roundabout (Section 3)	36
Table Expected Crash Type Distribution (Section 3)	37
Table Evaluation Message	39
Table Observed Crashes Used in the Evaluation (Section 4)	43
Table Evaluation Highway - Homogeneous Segments (Section 4)	44
Table Crash Highway Highway - Homogeneous Segments (Section 4)	46
Table Evaluation Intersection (Section 4)	47
Table Crash History Intersection (Section 4)	48
Table Expected Highway Crash Rates and Frequencies Summary (Section 4)	49
Table Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 4)	50
Table Expected Crash Frequencies and Rates by Horizontal Design Element (Section 4)	51
Table Predicted Crash Frequencies by Year (Section 4)	52
Table Expected Crash Frequencies by Year (Section 4)	53
Table Comparing Predicted and Expected Crashes for the Evaluation Period (Section 4)	54

Table Expected Five Lane or Fewer Crash Type Distribution (Section 4)											
List of Figures											
Figure Crash Prediction Summary (Section 3)	42										

#### **Report Overview**

Report Generated: Jun 10, 2024 10:07 AM

Report Template: System: Single Page, 508 Compliant [System] (mlcpm5, Dec 5, 2019 2:16 PM)

Evaluation Date: Mon Jun 10 10:05:34 CDT 2024

**IHSDM Version:** v17.0.0 (Sep 22, 2021)

Crash Prediction Module: v12.0.0 (Sep 22, 2021)

User Name: naveen.mallipaddi

**Organization Name:** 

Phone: E-Mail:

Project Title: SD-38\_Build\_Option3\_I90EBRamp\_I

Project Comment: Created Mon Mar 27 16:47:43 CDT 2023

Project Unit System: U.S. Customary

**Highway Title: SD-38** 

Highway Comment: Created Mon Mar 27 16:49:47 CDT 2023

**Highway Version: 17** 

**Evaluation Title:** Evaluation 51

Evaluation Comment: Created Mon Jun 10 10:01:57 CDT 2024

**Minimum Location:** 585+00.000 **Maximum Location:** 974+11.000

Policy for Superelevation: AASHTO 2011 U.S. Customary

Calibration: HSM Configuration

Crash Distribution: HSM Configuration Model/CMF: HSM Configuration

First Year of Analysis: 2025 Last Year of Analysis: 2050

**Empirical-Bayes Analysis:** Site-Specific **Highway with Crash History:** SD-38

Highway with Crash History Comment: Created Mon Mar 27 16:49:47 CDT 2023

Highway with Crash History Version: 17 First Year of Observed Crashes: 2018 Last Year of Observed Crashes: 2022

### **Disclaimer Regarding Crash Prediction Method**

IMPORTANT NOTICE ABOUT COMPARING RESULTS FROM HIGHWAY SAFETY MANUAL FIRST EDITION (2010) MODELS TO RESULTS FROM NEW MODELS DEVELOPED UNDER NCHRP PROJECTS 17-70, 17-58, AND 17-68

Since the publication of the Highway Safety Manual - First Edition (HSM-1), in 2010 by the American Association of State Highway and Transportation Officials (AASHTO), multiple research efforts have been undertaken through the National Cooperative Highway Research Program (NCHRP) to develop safety performance models for road segment and intersection facility types that were not initially reflected in the HSM-1, in order to expand the breadth and depth of the HSM in the future.

The IHSDM Crash Prediction Module (CPM) is intended as a faithful implementation of HSM Part C predictive methods. As NCHRP projects to develop new predictive methods for the HSM are completed, FHWA works to incorporate the new methods into IHSDM, sometimes in advance of publication in the HSM. The following new crash predictive methods have been accepted by NCHRP project panels and incorporated into IHSDM, while pending AASHTO's approval for incorporation into a future edition of the HSM:

- Roundabouts: completed in 2018 under NCHRP Project 17-70, the new methods will provide improved outcomes for the safety analysis of roundabouts.
- 6+ lane and one-way urban/suburban arterials (including models for segments and intersections): completed under NCHRP Project 17-58.
- Intersection crash prediction methods for some intersection configurations and traffic control types not currently addressed in the HSM (e.g., all-way stop; rural 3-leg signalized; 3-leg stop-controlled where the major leg turns; urban 5-leg signalized; urban high-speed intersections): completed in 2021 under NCHRP Project 17-68.

However, in the absence of local calibration factors (see HSM-1 Part C, Appendix A for guidance on calibration of the predictive models), it is neither appropriate nor advisable to directly compare the results from new models (from NCHRP Projects 17-58, 17-68, and 17-70) to results from HSM-1 models, as the models were not calibrated to the same base state data sets, and consequently can produce unexpected results. If local calibration factors are available and applied to both new models and HSM-1 models, then it may be appropriate to directly compare the results. [Note: Work being performed under NCHRP Project 17-72 (Update of Crash Modification Factors for the Highway Safety Manual) is expected to re-calibrate many of the old (HSM-1) and new (e.g., NCHRP 17-70) models to data from a single (or small number of) states, that would allow results from all models to be directly compared.]

The models produced for NCHRP Project 17-70 have independent value in terms of informing the design of a roundabout and assessing the effects of different design characteristics on the expected safety performance of a roundabout.

The HSM-1 interim method previously included in IHSDM for evaluating roundabouts on urban/suburban arterials (i.e., evaluating an existing intersection and then applying a Crash Modification Factor for replacing the existing intersection with a roundabout) has been deactivated in IHSDM, to minimize any confusion with the new roundabout methodology.

## **Section Types**

#### **Section 3 Evaluation**

**Section:** Section 3

**Evaluation Start Location:** 585+00.000 **Evaluation End Location:** 948+50.000

Area Type: Rural

Functional Class: Arterial

Type of Alignment: Undivided/Divided Multilane

Model Category: Rural, Multilane

**Calibration Factor:** 3ST=1.0; 4D=1.0; 4ST=1.0; 4U=1.0; RT\_ST\_FI=1.0; RT\_ST\_PDO=1.0;

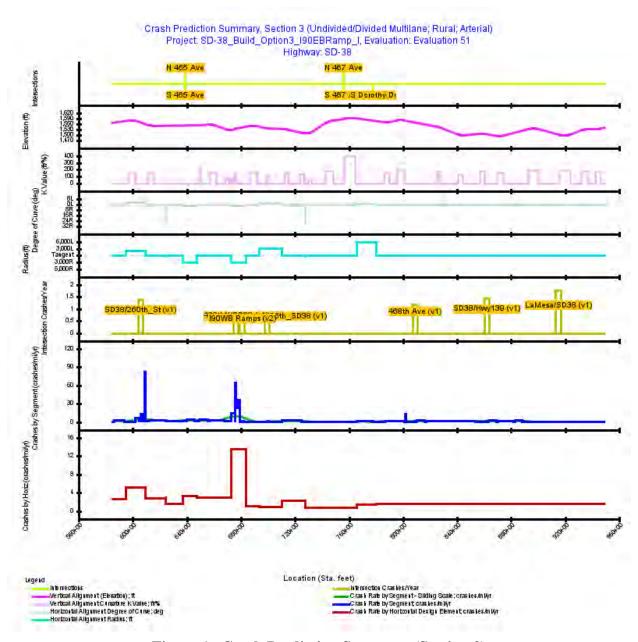


Figure 1. Crash Prediction Summary (Section 3)

**Table 1. Observed Crashes Used in the Evaluation (Section 3)** 

Year	Observed Crashes	Total Crashes Used	FI Crashes	FI no/C Crashes	PDO Crashes
2018	8	8	6	5	2
2019	10	10	4	0	6
2020	7	7	3	2	4
2021	9	9	5	2	4
2022	9	9	5	1	4
All Years	43 <sup>[1]</sup>	43	23	10	20

#### **Footnotes**

<sup>[1]</sup> Note: Observed crash data that does not comply with the associated CPM model requirements may not be used in EB processing.

 Table 2. Evaluation Highway - Homogeneous Segments (Section 3)

leg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Si
1	Rural Multi-Lane Segment Four- lane Divided	585+00.00 0	586+00.00 0	100.00	0.0189	2025; 7,087; 2026; 8,007; 2027; 8,928; 2028; 9,849; 2029; 10,770; 2030; 10,937; 2031; 11,104; 2032; 11,271; 2033; 11,439; 2034; 11,606; 2035; 11,773; 2036; 11,940; 2037; 12,108; 2038; 12,275; 2039; 12,442; 2040; 12,610; 2041; 12,806; 2042; 13,002; 2043; 13,198; 2044; 13,394; 2045; 13,590; 2046; 13,786; 2047; 13,982; 2048; 14,178; 2049; 14,374; 2050; 14,570	12.00	12.00	8.00	8.00	8.40	Non-Traversable Median	8.40	false	false		
2	Rural Multi-Lane Segment Four- lane Divided	586+00.00 0	593+75.00 0	775.00	0.1468	2025; 7,087; 2026; 8,007; 2027; 8,928; 2028; 9,849; 2029; 10,770; 2030; 10,937; 2031; 11,104; 2032; 11,271; 2033; 11,439; 2034; 11,606; 2035; 11,773; 2036; 11,940; 2037; 12,108; 2038; 12,275; 2039; 12,442; 2040; 12,610; 2041; 12,806; 2042; 13,002; 2043; 13,198; 2044; 13,394; 2045; 13,500; 2046; 13,786; 2047; 13,982; 2048; 14,178; 2049; 14,374; 2050; 14,570	12.00	12.00	8.00	8.00	11.90	Non-Traversable Median	11.90	false	false		
3	Rural Multi-Lane Segment Four- lane Divided	593+75.00 0	594+84.94 0	109.94	0.0208	2025; 7,087; 2026; 8,007; 2027; 8,928; 2028; 9,849; 2029; 10,770; 2030; 10,937; 2031; 11,104; 2032; 11,271; 2033; 11,439; 2034; 11,606; 2035; 11,773; 2036; 11,940; 2037; 12,108; 2038; 12,275; 2039; 12,442; 2040; 12,610; 2041; 12,806; 2042; 13,002; 2043; 13,198; 2044; 13,394; 2045; 13,500; 2046; 13,786; 2047; 13,982; 2048; 14,178; 2049; 14,374; 2050; 14,570	12.00	12.00	8.00	8.00	15.44	Non-Traversable Median	15.44	false	false		
4	Rural Multi-Lane Segment Four- lane Divided	594+84.94 0	600+00.00 0	515.06	0.0975	2025; 7,087; 2026; 8,007; 2027; 8,928; 2028; 9,849; 2029; 10,770; 2030; 10,937; 2031; 11,104; 2032; 11,271; 2033; 11,439; 2034; 11,606; 2035; 11,773; 2036; 11,940; 2037; 12,108; 2038; 12,275; 2039; 12,442; 2040; 12,610; 2041; 12,806; 2042; 13,002; 2043; 13,198; 2044; 13,394; 2045; 13,509; 2046; 13,786; 2047; 13,982; 2048; 14,178; 2049; 14,374; 2050; 14,570	12.00	12.00	8.00	8.00	17.94	Non-Traversable Median	17.94	false	false		
5	Rural Multi-Lane Segment Four- lane Divided	600+00.00 0	600+42.00 0	42.00	0.0080	2025; 7,087; 2026; 8,007; 2027; 8,928; 2028; 9,849; 2029; 10,770; 2030; 10,937; 2031; 11,104; 2032; 11,271; 2033; 11,439; 2034; 11,606; 2035; 11,773; 2036; 11,940; 2037; 12,108; 2038; 12,275; 2039; 12,442; 2040; 12,610; 2041; 12,806; 2042; 13,002; 2043; 13,198; 2044; 13,394; 2045; 13,590; 2046; 13,786; 2047; 13,982; 2048; 14,178; 2049; 14,374; 2050; 14,570	12.00	12.00	8.00	8.00	17.48	Non-Traversable Median	17.48	false	false		
6	Rural Multi-Lane Segment Four- lane Divided	600+42.00 0	601+00.00 0	58.00	0.0110	2025; 7,087; 2026; 8,007; 2027; 8,928; 2028; 9,849; 2029; 10,770; 2030; 10,937; 2031; 11,104; 2032; 11,271; 2033; 11,439; 2034; 11,606; 2035; 11,773; 2036; 11,940; 2037; 12,108; 2038; 12,275; 2039; 12,442; 2040; 12,610; 2041; 12,806; 2042; 13,002; 2043; 13,198; 2044; 13,394; 2045; 13,500; 2046; 13,786; 2047; 13,982; 2048; 14,178; 2049; 14,374; 2050; 14,570	12.00	12.00	8.00	8.00	11.48	Non-Traversable Median	11.48	false	false		
7	Rural Multi-Lane Segment Four- lane Divided	601+00.00 0	602+00.00 0	100.00	0.0189	2025; 7,087; 2026; 8,007; 2027; 8,928; 2028; 9,849; 2029; 10,770; 2030; 10,937; 2031; 11,104; 2032; 11,271; 2033; 11,439; 2034; 11,606; 2035; 11,773; 2036; 11,940; 2037; 12,108; 2038; 12,275; 2039; 12,442; 2040; 12,610; 2041; 12,806; 2042; 13,002; 2043; 13,198; 2044; 13,394; 2045; 13,500; 2046; 13,786; 2047; 13,982; 2048; 14,178; 2049; 14,374; 2050; 14,570	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
8	Rural Multi-Lane Segment Four- lane Divided	602+00.00 0	605+00.00 0	300.00	0.0568	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
9	Rural Multi-Lane Segment Four- lane Divided	605+00.00 0	605+10.00 0	10.00	0.0019	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	0.00	8.00	Non-Traversable Median	20.00	false	false		
10	Rural Multi-Lane Segment Four- lane Divided	605+10.00 0	605+40.00 0	30.00	0.0057	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	0.00	8.00	Traversable Median	20.00	false	false		
11	Rural Multi-Lane Segment Four- lane Undivided	605+40.00 0	605+60.00 0	20.00	0.0038	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	0.00	0.00	None	0.00	false	false	0:1	1
12	Rural Multi-Lane Segment Four- lane Undivided	605+60.00 0	605+70.00 0	10.00	0.0019	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1
13	Rural Multi-Lane Segment Four- lane Undivided	605+70.00 0	605+75.00 0	5.00	0.0009	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,323; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1
14	Rural Multi-Lane Segment Four- lane Undivided	605+75.00 0	606+00.00	25.00	0.0047	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,738; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	1

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
15	Rural Multi-Lane Segment Four- lane Divided	606+00.00 0	607+50.00 0	150.00	0.0284	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
16	Rural Multi-Lane Segment Four- lane Divided	607+50.00 0	609+00.00 0	150.00	0.0284	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
17	Rural Multi-Lane Segment Four- lane Divided	609+00.00 0	609+21.93 0	21.93	0.0042	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2048: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
18	Rural Multi-Lane Segment Four- lane Divided	609+21.93 0	611+40.00 0	218.07	0.0413	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2048: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
19	Rural Multi-Lane Segment Four- lane Divided	611+40.00 0	611+50.00 0	10.00	0.0019	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2048: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
20	Rural Multi-Lane Segment Four- lane Divided	611+50.00 0	612+09.00 0	59.00	0.0112	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	11.54	Non-Traversable Median	11.54	false	false		
21	Rural Multi-Lane Segment Four- lane Divided	612+09.00 0	612+50.00 0	41.00	0.0078	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	17.54	Non-Traversable Median	17.54	false	false		
22	Rural Multi-Lane Segment Four- lane Divided	612+50.00 0	624+64.53 0	1,214.5 3	0.2300	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
23	Rural Multi-Lane Segment Four- lane Divided	624+64.53 0	631+30.00 0	665.47	0.1260	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
24	Rural Multi-Lane Segment Four- lane Divided	631+30.00 0	631+81.00 0	51.00	0.0097	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	17.45	Non-Traversable Median	17.45	false	false		
25	Rural Multi-Lane Segment Four- lane Divided	631+81.00 0	632+50.00 0	69.00	0.0131	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,959	12.00	12.00	8.00	8.00	11.45	Non-Traversable Median	11.45	false	false		
26	Rural Multi-Lane Segment Four- lane Divided	632+50.00 0	636+92.82 0	442.82	0.0839	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
27	Rural Multi-Lane Segment Four- lane Divided	636+92.82 0	638+80.00 0	187.18	0.0355	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
28	Rural Multi-Lane Segment Four- lane Divided	638+80.00 0	639+00.00 0	20.00	0.0038	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
29	Rural Multi-Lane Segment Four- lane Undivided	639+00.00 0	640+00.00 0	100.00	0.0189	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1

leg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Righ Side Slop
30	Rural Multi-Lane Segment Four- lane Divided	640+00.00 0	640+20.00 0	20.00	0.0038	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
31	Rural Multi-Lane Segment Four- lane Divided	640+20.00 0	645+50.00 0	530.00	0.1004	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
32	Rural Multi-Lane Segment Four- lane Divided	645+50.00 0	646+30.00 0	80.00	0.0152	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
33	Rural Multi-Lane Segment Four- lane Divided	646+30.00 0	647+00.00 0	70.00	0.0133	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	11.50	Non-Traversable Median	11.50	false	false		
34	Rural Multi-Lane Segment Four- lane Divided	647+00.00 0	647+26.05 0	26.05	0.0049	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2048: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	16.30	Non-Traversable Median	16.30	false	false		
35	Rural Multi-Lane Segment Four- lane Divided	647+26.05 0	647+50.00 0	23.95	0.0045	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	18.80	Non-Traversable Median	18.80	false	false		
36	Rural Multi-Lane Segment Four- lane Divided	647+50.00 0	648+00.00 0	50.00	0.0095	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
37	Rural Multi-Lane Segment Four- lane Divided	648+00.00 0	648+42.00 0	42.00	0.0080	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	17.48	Non-Traversable Median	17.48	false	false		
38	Rural Multi-Lane Segment Four- lane Divided	648+42.00 0	649+00.00 0	58.00	0.0110	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	11.48	Non-Traversable Median	11.48	false	false		
39	Rural Multi-Lane Segment Four- lane Divided	649+00.00 0	649+20.00 0	20.00	0.0038	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
40	Rural Multi-Lane Segment Four- lane Divided	649+20.00 0	655+50.00 0	630.00	0.1193	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
41	Rural Multi-Lane Segment Four- lane Divided	655+50.00 0	655+70.00 0	20.00	0.0038	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
42	Rural Multi-Lane Segment Four- lane Undivided	655+70.00 0	656+50.00 0	80.00	0.0152	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	I
43	Rural Multi-Lane Segment Four- lane Divided	656+50.00 0	667+80.00 0	1,130.0 0	0.2140	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
44	Rural Multi-Lane Segment Four- lane Divided	667+80.00 0	668+50.00 0	70.00	0.0133	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	0.00	20.00	Non-Traversable Median	20.00	false	false		

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
45	Rural Multi-Lane Segment Four- lane Divided	668+50.00 0	668+80.00 0	30.00	0.0057	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	0.00	18.62	Non-Traversable Median	18.62	false	false		
46	Rural Multi-Lane Segment Four- lane Divided	668+80.00 0	669+05.00 0	25.00	0.0047	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	16.08	Non-Traversable Median	16.08	false	false		
47	Rural Multi-Lane Segment Four- lane Divided	669+05.00 0	669+80.00 0	75.00	0.0142	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	11.46	Non-Traversable Median	11.46	false	false		
48	Rural Multi-Lane Segment Four- lane Divided	669+80.00 0	672+86.11 0	306.11	0.0580	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033: 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
49	Rural Multi-Lane Segment Four- lane Divided	672+86.11 0	675+50.00 0	263.89	0.0500	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
50	Rural Multi-Lane Segment Four- lane Divided	675+50.00 0	676+00.00 0	50.00	0.0095	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Traversable Median	8.00	false	false		
51	Rural Multi-Lane Segment Four- lane Divided	676+00.00 0	676+30.00 0	30.00	0.0057	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	0.00	8.00	8.00	Traversable Median	8.00	false	false		
52	Rural Multi-Lane Segment Four- lane Undivided	676+30.00 0	677+50.00 0	120.00	0.0227	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	0.00	8.00	0.00	None	0.00	false	false	0:1	0:1
53	Rural Multi-Lane Segment Four- lane Divided	677+50.00 0	679+00.00 0	150.00	0.0284	2025: 7,901; 2026: 9,093; 2027: 10,285; 2028: 11,477; 2029: 12,670; 2030: 12,965; 2031: 13,260; 2032: 13,556; 2033: 13,851; 2034: 14,147; 2035: 14,442; 2036: 14,738; 2037: 15,033; 2038: 15,329; 2039: 15,624; 2040: 15,920; 2041: 16,287; 2042: 16,654; 2043: 17,021; 2044: 17,388; 2045: 17,755; 2046: 18,122; 2047: 18,489; 2048: 18,856; 2049: 19,223; 2050: 19,590	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
54	Rural Multi-Lane Segment Four- lane Divided	679+00.00 0	680+20.00 0	120.00	0.0227	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
55	Rural Multi-Lane Segment Four- lane Divided	680+20.00 0	680+80.00 0	60.00	0.0114	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
56	Rural Multi-Lane Segment Four- lane Divided	680+80.00 0	681+00.00 0	20.00	0.0038	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	0.00	8.00	8.00	Traversable Median	20.00	false	false		
57	Rural Multi-Lane Segment Four- lane Undivided	681+00.00 0	682+20.00 0	120.00	0.0227	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,259; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	0.00	8.00	0.00	None	0.00	false	false	0:1	0:1
58	Rural Multi-Lane Segment Four- lane Divided	682+20.00 0	683+82.71 0	162.71	0.0308	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,259; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
59	Rural Multi-Lane Segment Four- lane Divided	683+82.71 0	690+00.00 0	617.29	0.1169	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		

Seg. No.	Type	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Righ Side Slope
60	Rural Multi-Lane Segment Four- lane Divided	690+00.00 0	691+50.00 0	150.00	0.0284	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	0.00	20.00	Non-Traversable Median	20.00	false	false		
61	Rural Multi-Lane Segment Four- lane Divided	691+50.00 0	692+01.00 0	51.00	0.0097	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	0.00	17.45	Non-Traversable Median	17.45	false	false		
62	Rural Multi-Lane Segment Four- lane Divided	692+01.00 0	692+70.00 0	69.00	0.0131	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	0.00	11.45	Non-Traversable Median	11.45	false	false		
63	Rural Multi-Lane Segment Four- lane Divided	692+70.00 0	693+85.01 0	115.01	0.0218	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	0.00	8.00	Non-Traversable Median	19.00	false	false		
64	Rural Multi-Lane Segment Four- lane Divided	693+85.01 0	698+50.00 0	464.99	0.0881	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2046: 10,259; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	0.00	8.00	Non-Traversable Median	19.00	false	false		
65	Rural Multi-Lane Segment Four- lane Divided	698+50.00 0	698+70.00 0	20.00	0.0038	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453: 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	0.00	8.00	Traversable Median	19.00	false	false		
66	Rural Multi-Lane Segment Four- lane Divided	698+70.00 0	699+20.00 0	50.00	0.0095	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453: 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	0.00	0.00	8.00	Traversable Median	19.00	false	false		
67	Rural Multi-Lane Segment Four- lane Undivided	699+20.00 0	700+40.00 0	120.00	0.0227	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	1 0
68	Rural Multi-Lane Segment Four- lane Divided	700+40.00 0	700+50.00 0	10.00	0.0019	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453: 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	0.00	0.00	8.00	Traversable Median	8.00	false	false		
69	Rural Multi-Lane Segment Four- lane Divided	700+50.00 0	701+10.00 0	60.00	0.0114	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453: 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	0.00	8.00	Traversable Median	8.00	false	false		
70	Rural Multi-Lane Segment Four- lane Divided	701+10.00 0	702+00.00 0	90.00	0.0170	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453: 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	12.00	12.00	8.00	0.00	8.00	Non-Traversable Median	8.00	false	false		
71	Rural Multi-Lane Segment Four- lane Divided	702+00.00 0	702+50.00 0	50.00	0.0095	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	0.00	8.00	Non-Traversable Median	8.00	false	false		
72	Rural Multi-Lane Segment Four- lane Divided	702+50.00 0	707+00.00 0	450.00	0.0852	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
73	Rural Multi-Lane Segment Four- lane Divided	707+00.00 0	708+00.00 0	100.00	0.0189	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
74	Rural Multi-Lane Segment Four- lane Divided	708+00.00 0	708+80.00 0	80.00	0.0152	2025; 5,804; 2026; 6,105; 2027; 6,407; 2028; 6,708; 2029; 7,010; 2030; 7,134; 2031; 7,259; 2032; 7,383; 2033; 7,508; 2034; 7,632; 2035; 7,757; 2036; 7,881; 2037; 8,006; 2038; 8,130; 2039; 8,255; 2040; 8,380; 2041; 8,528; 2042; 8,676; 2043; 8,824; 2044; 8,972; 2045; 9,120; 2046; 9,268; 2047; 9,416; 2048; 9,564; 2049; 9,712; 2050; 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
75	Rural Multi-Lane Segment Four- lane Divided	708+80.00 0	709+00.00 0	20.00	0.0038	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
76	Rural Multi-Lane Segment Four- lane Undivided	709+00.00 0	710+30.00 0	130.00	0.0246	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2059: 9,860	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
77	Rural Multi-Lane Segment Four- lane Divided	710+30.00 0	710+47.85 0	17.85	0.0034	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2059: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
78	Rural Multi-Lane Segment Four- lane Divided	710+47.85 0	725+00.00 0	1,452.1 5	0.2750	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2006: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
79	Rural Multi-Lane Segment Four- lane Divided	725+00.00 0	727+52.35 0	252.35	0.0478	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2004: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
80	Rural Multi-Lane Segment Four- lane Divided	727+52.35 0	735+00.00 0	747.65	0.1416	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2006: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
81	Rural Multi-Lane Segment Four- lane Divided	735+00.00 0	755+50.00 0	2,050.0 0	0.3883	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
82	Rural Multi-Lane Segment Four- lane Undivided	755+50.00 0	756+90.00 0	140.00	0.0265	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
83	Rural Multi-Lane Segment Four- lane Divided	756+90.00 0	757+00.00 0	10.00	0.0019	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
84	Rural Multi-Lane Segment Four- lane Divided	757+00.00 0	763+30.00 0	630.00	0.1193	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
85	Rural Multi-Lane Segment Four- lane Divided	763+30.00 0	764+00.00 0	70.00	0.0133	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	11.50	Non-Traversable Median	11.50	false	false		
86	Rural Multi-Lane Segment Four- lane Divided	764+00.00 0	764+50.00 0	50.00	0.0095	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2006: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	17.50	Non-Traversable Median	17.50	false	false		
87	Rural Multi-Lane Segment Four- lane Divided	764+50.00 0	765+52.55 0	102.55	0.0194	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
88	Rural Multi-Lane Segment Four- lane Divided	765+52.55 0	777+80.00 0	1,227.4 5	0.2325	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
89	Rural Multi-Lane Segment Four- lane Undivided	777+80.00 0	778+80.00 0	100.00	0.0189	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
90	Rural Multi-Lane Segment Four- lane Divided	778+80.00 0	779+00.00 0	20.00	0.0038	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
91	Rural Multi-Lane Segment Four- lane Divided	779+00.00 0	780+45.93 0	145.93	0.0276	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757: 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
92	Rural Multi-Lane Segment Four- lane Divided	780+45.93 0	785+40.00 0	494.07	0.0936	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757: 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
93	Rural Multi-Lane Segment Four- lane Divided	785+40.00 0	785+50.00 0	10.00	0.0019	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,242; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
94	Rural Multi-Lane Segment Four- lane Divided	785+50.00 0	786+09.00 0	59.00	0.0112	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,242; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	11.54	Non-Traversable Median	11.54	false	false		
95	Rural Multi-Lane Segment Four- lane Divided	786+09.00 0	786+50.00 0	41.00	0.0078	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,242; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	17.54	Non-Traversable Median	17.54	false	false		
96	Rural Multi-Lane Segment Four- lane Divided	786+50.00 0	801+10.00 0	1,460.0 0	0.2765	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,242; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
97	Rural Multi-Lane Segment Four- lane Divided	801+10.00 0	801+61.00 0	51.00	0.0097	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,242; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	17.45	Non-Traversable Median	17.45	false	false		
98	Rural Multi-Lane Segment Four- lane Divided	801+61.00 0	802+30.00 0	69.00	0.0131	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757: 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	11.45	Non-Traversable Median	11.45	false	false		
99	Rural Multi-Lane Segment Four- lane Divided	802+30.00 0	802+40.00 0	10.00	0.0019	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
100	Rural Multi-Lane Segment Four- lane Divided	802+40.00 0	808+30.00 0	590.00	0.1117	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,824; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
101	Rural Multi-Lane Segment Four- lane Divided	808+30.00 0	808+80.00 0	50.00	0.0095	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,676; 2043: 8,242; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
102	Rural Multi-Lane Segment Four- lane Undivided	808+80.00 0	809+00.00 0	20.00	0.0038	2025: 5,804; 2026: 6,105; 2027: 6,407; 2028: 6,708; 2029: 7,010; 2030: 7,134; 2031: 7,259; 2032: 7,383; 2033: 7,508; 2034: 7,632; 2035: 7,757; 2036: 7,881; 2037: 8,006; 2038: 8,130; 2039: 8,255; 2040: 8,380; 2041: 8,528; 2042: 8,662; 2043: 8,242; 2044: 8,972; 2045: 9,120; 2046: 9,268; 2047: 9,416; 2048: 9,564; 2049: 9,712; 2050: 9,860	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
103	Rural Multi-Lane Segment Four- lane Undivided	809+00.00 0	809+60.00 0	60.00	0.0114	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,690; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1
104	Rural Multi-Lane Segment Four- lane Divided	809+60.00 0	810+00.00 0	40.00	0.0076	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	0.00	0.00	8.00	Traversable Median	20.00	false	false		

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
105	Rural Multi-Lane Segment Four- lane Divided	810+00.00 0	810+20.00 0	20.00	0.0038	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
106	Rural Multi-Lane Segment Four- lane Divided	810+20.00 0	816+00.00 0	580.00	0.1098	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
107	Rural Multi-Lane Segment Four- lane Divided	816+00.00 0	816+70.00 0	70.00	0.0133	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	11.50	Non-Traversable Median	11.50	false	false		
108	Rural Multi-Lane Segment Four- lane Divided	816+70.00 0	817+20.00 0	50.00	0.0095	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	17.50	Non-Traversable Median	17.50	false	false		
109	Rural Multi-Lane Segment Four- lane Divided	817+20.00 0	853+70.00 0	3,650.0 0	0.6913	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110: 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
110	Rural Multi-Lane Segment Four- lane Divided	853+70.00 0	854+00.00 0	30.00	0.0057	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	18.36	Non-Traversable Median	18.36	false	false		
111	Rural Multi-Lane Segment Four- lane Divided	854+00.00 0	854+16.00 0	16.00	0.0030	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	15.85	Non-Traversable Median	15.85	false	false		
112	Rural Multi-Lane Segment Four- lane Divided	854+16.00 0	854+80.00 0	64.00	0.0121	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	11.49	Non-Traversable Median	11.49	false	false		
113	Rural Multi-Lane Segment Four- lane Divided	854+80.00 0	860+90.00 0	610.00	0.1155	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
114	Rural Multi-Lane Segment Four- lane Divided	860+90.00 0	861+85.00 0	95.00	0.0180	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
115	Rural Multi-Lane Segment Four- lane Undivided	861+85.00 0	862+00.00 0	15.00	0.0028	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,600; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	12.00	12.00	8.00	0.00	0.00	None	0.00	false	false	0:1	0:1
116	Rural Multi-Lane Segment Four- lane Undivided	862+00.00 0	862+50.00 0	50.00	0.0095	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	0.00	0.00	None	0.00	false	false	0:1	0:1
117	Rural Multi-Lane Segment Four- lane Undivided	862+50.00 0	862+60.00 0	10.00	0.0019	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
118	Rural Multi-Lane Segment Four- lane Divided	862+60.00 0	863+10.00 0	50.00	0.0095	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	8.00	Traversable Median	19.00	false	false		
119	Rural Multi-Lane Segment Four- lane Divided	863+10.00 0	869+00.00 0	590.00	0.1117	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	19.00	false	false		

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Side
120	Rural Multi-Lane Segment Four- lane Divided	869+00.00 0	869+70.00 0	70.00	0.0133	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222: 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,590; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	11.50	Non-Traversable Median	11.50	false	false		
121	Rural Multi-Lane Segment Four- lane Divided	869+70.00 0	870+20.00 0	50.00	0.0095	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2,404: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	17.50	Non-Traversable Median	17.50	false	false		
122	Rural Multi-Lane Segment Four- lane Divided	870+20.00 0	881+80.00 0	1,160.0 0	0.2197	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
123	Rural Multi-Lane Segment Four- lane Divided	881+80.00 0	882+31.00 0	51.00	0.0097	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	17.45	Non-Traversable Median	17.45	false	false		
124	Rural Multi-Lane Segment Four- lane Divided	882+31.00 0	883+00.00 0	69.00	0.0131	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	11.45	Non-Traversable Median	11.45	false	false		
125	Rural Multi-Lane Segment Four- lane Divided	883+00.00 0	887+90.00 0	490.00	0.0928	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
126	Rural Multi-Lane Segment Four- lane Divided	887+90.00 0	888+20.00 0	30.00	0.0057	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
127	Rural Multi-Lane Segment Four- lane Undivided	888+20.00 0	889+30.00 0	110.00	0.0208	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
128	Rural Multi-Lane Segment Four- lane Divided	889+30.00 0	889+50.00 0	20.00	0.0038	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222: 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,257; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
129	Rural Multi-Lane Segment Four- lane Divided	889+50.00 0	894+50.00 0	500.00	0.0947	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
130	Rural Multi-Lane Segment Four- lane Divided	894+50.00 0	895+15.00 0	65.00	0.0123	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222: 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	11.55	Non-Traversable Median	11.55	false	false		
131	Rural Multi-Lane Segment Four- lane Divided	895+15.00 0	895+60.00 0	45.00	0.0085	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2,404: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	17.55	Non-Traversable Median	17.55	false	false		
132	Rural Multi-Lane Segment Four- lane Divided	895+60.00 0	898+00.00 0	240.00	0.0455	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
133	Rural Multi-Lane Segment Four- lane Divided	898+00.00 0	906+70.00 0	870.00	0.1648	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222: 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,609; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
134	Rural Multi-Lane Segment Four- lane Divided	906+70.00 0	907+21.00 0	51.00	0.0097	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	17.45	Non-Traversable Median	17.45	false	false		

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Widt h (ft)	Right Lane Widt h (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
135	Rural Multi-Lane Segment Four- lane Divided	907+21.00 0	907+80.00 0	59.00	0.0112	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	11.95	Non-Traversable Median	11.95	false	false		
136	Rural Multi-Lane Segment Four- lane Divided	907+80.00 0	907+90.00 0	10.00	0.0019	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	8.50	Non-Traversable Median	19.50	false	false		
137	Rural Multi-Lane Segment Four- lane Divided	907+90.00 0	913+70.00 0	580.00	0.1098	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	19.00	false	false		
138	Rural Multi-Lane Segment Four- lane Divided	913+70.00 0	914+00.00 0	30.00	0.0057	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,222; 2036: 10,408; 2037: 10,593; 2038: 10,779; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2045: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	12.00	12.00	8.00	8.00	8.00	Traversable Median	19.00	false	false		
139	Rural Multi-Lane Segment Four- lane Divided	914+00.00 0	914+30.00 0	30.00	0.0057	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	12.00	12.00	8.00	8.00	8.00	Traversable Median	19.00	false	false		
140	Rural Multi-Lane Segment Four- lane Undivided	914+30.00 0	914+40.00 0	10.00	0.0019	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
141	Rural Multi-Lane Segment Four- lane Undivided	914+40.00 0	915+40.00 0	100.00	0.0189	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1
142	Rural Multi-Lane Segment Four- lane Divided	915+40.00 0	916+00.00 0	60.00	0.0114	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	12.00	12.00	8.00	8.00	8.00	Traversable Median	19.00	false	false		
143	Rural Multi-Lane Segment Four- lane Divided	916+00.00 0	921+00.00 0	500.00	0.0947	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	19.00	false	false		
144	Rural Multi-Lane Segment Four- lane Divided	921+00.00 0	921+90.00 0	90.00	0.0170	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,948; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	19.00	false	false		
145	Rural Multi-Lane Segment Four- lane Divided	921+90.00 0	922+00.00 0	10.00	0.0019	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
146	Rural Multi-Lane Segment Four- lane Divided	922+00.00 0	922+59.00 0	59.00	0.0112	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,948; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	12.00	12.00	8.00	8.00	11.54	Non-Traversable Median	11.54	false	false		
147	Rural Multi-Lane Segment Four- lane Divided	922+59.00 0	923+00.00 0	41.00	0.0078	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,949; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	12.00	12.00	8.00	8.00	17.54	Non-Traversable Median	17.54	false	false		
148	Rural Multi-Lane Segment Four- lane Divided	923+00.00 0	941+70.00 0	1,870.0 0	0.3542	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,949; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
149	Rural Multi-Lane Segment Four- lane Divided	941+70.00 0	948+50.00 0	680.00	0.1288	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,949; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		

**Table 3. Crash History Highway - Homogeneous Segments (Section 3)** 

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Width (ft)	Right Lane Width (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
1	Rural Multi-Lane Segment Four-lane Divided	585+00.000	586+00.000	100.00	0.0189	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.40	Non-Traversable Median	8.40	false	false		<u> </u>
2	Rural Multi-Lane Segment Four-lane Divided	586+00.000	593+75.000	775.00	0.1468	2018-2022: 4,325	12.00	12.00	8.00	8.00	11.90	Non-Traversable Median	11.90	false	false		
3	Rural Multi-Lane Segment Four-lane Divided	593+75.000	594+84.940	109.94	0.0208	2018-2022: 4,325	12.00	12.00	8.00	8.00	15.44	Non-Traversable Median	15.44	false	false		
4	Rural Multi-Lane Segment Four-lane Divided	594+84.940	600+00.000	515.06	0.0975	2018-2022: 4,325	12.00	12.00	8.00	8.00	17.94	Non-Traversable Median	17.94	false	false		
5	Rural Multi-Lane Segment Four-lane Divided	600+00.000	600+42.000	42.00	0.0080	2018-2022: 4,325	12.00	12.00	8.00	8.00	17.48	Non-Traversable Median	17.48	false	false		
6	Rural Multi-Lane Segment Four-lane Divided	600+42.000	601+00.000	58.00	0.0110	2018-2022: 4,325	12.00	12.00	8.00	8.00	11.48	Non-Traversable Median	11.48	false	false		
7	Rural Multi-Lane Segment Four-lane Divided	601+00.000	602+00.000	100.00	0.0189	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
8	Rural Multi-Lane Segment Four-lane Divided	602+00.000	605+00.000	300.00	0.0568	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
9	Rural Multi-Lane Segment Four-lane Divided	605+00.000	605+10.000	10.00	0.0019	2018-2022: 4,325	12.00	12.00	8.00	0.00	8.00	Non-Traversable Median	20.00	false	false		
10	Rural Multi-Lane Segment Four-lane Divided	605+10.000	605+40.000	30.00	0.0057	2018-2022: 4,325	12.00	12.00	8.00	0.00	8.00	Traversable Median	20.00	false	false		
11	Rural Multi-Lane Segment Four-lane Undivided	605+40.000	605+60.000	20.00	0.0038	2018-2022: 4,325	12.00	12.00	8.00	0.00	0.00	None	0.00	false	false	0:1	0:1
12	Rural Multi-Lane Segment Four-lane Undivided	605+60.000	605+70.000	10.00	0.0019	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
13	Rural Multi-Lane Segment Four-lane Undivided	605+70.000	605+75.000	5.00	0.0009	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
14	Rural Multi-Lane Segment Four-lane Undivided	605+75.000	606+00.000	25.00	0.0047	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
15	Rural Multi-Lane Segment Four-lane Divided	606+00.000	607+50.000	150.00	0.0284	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
16	Rural Multi-Lane Segment Four-lane Divided	607+50.000	609+00.000	150.00	0.0284	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
17	Rural Multi-Lane Segment Four-lane Divided	609+00.000	609+21.930	21.93	0.0042	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
18	Rural Multi-Lane Segment Four-lane Divided	609+21.930	611+40.000	218.07	0.0413	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
19	Rural Multi-Lane Segment Four-lane Divided	611+40.000	611+50.000	10.00	0.0019	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
20	Rural Multi-Lane Segment Four-lane Divided	611+50.000	612+09.000	59.00	0.0112	2018-2022: 4,325	12.00	12.00	8.00	8.00	11.54	Non-Traversable Median	11.54	false	false		
21	Rural Multi-Lane Segment Four-lane Divided	612+09.000	612+50.000	41.00	0.0078	2018-2022: 4,325	12.00	12.00	8.00	8.00	17.54	Non-Traversable Median	17.54	false	false		
22	Rural Multi-Lane Segment Four-lane Divided	612+50.000	624+64.530	1,214.53	0.2300	2018-2022: 4,325	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
23	Rural Multi-Lane Segment Four-lane Divided	624+64.530	631+30.000	665.47	0.1260	2018-2022: 4,325	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
24	Rural Multi-Lane Segment Four-lane Divided	631+30.000	631+81.000	51.00	0.0097	2018-2022: 4,325	12.00	12.00	8.00	8.00	17.45	Non-Traversable Median	17.45	false	false		
25	Rural Multi-Lane Segment Four-lane Divided	631+81.000	632+50.000	69.00	0.0131	2018-2022: 4,325	12.00	12.00	8.00	8.00	11.45	Non-Traversable Median	11.45	false	false		
26	Rural Multi-Lane Segment Four-lane Divided	632+50.000	636+92.820	442.82	0.0839	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
27	Rural Multi-Lane Segment Four-lane Divided	636+92.820	638+80.000	187.18	0.0355	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
28	Rural Multi-Lane Segment Four-lane Divided	638+80.000	639+00.000	20.00	0.0038	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
29	Rural Multi-Lane Segment Four-lane Undivided	639+00.000	640+00.000	100.00	0.0189	2018-2022: 4,325	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1
30	Rural Multi-Lane Segment Four-lane Divided	640+00.000	640+20.000	20.00	0.0038	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
31	Rural Multi-Lane Segment Four-lane Divided	640+20.000	645+50.000	530.00	0.1004	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
32	Rural Multi-Lane Segment Four-lane Divided	645+50.000	646+30.000	80.00	0.0152	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
33	Rural Multi-Lane Segment Four-lane Divided	646+30.000	647+00.000	70.00	0.0133	2018-2022: 4,325	12.00	12.00	8.00	8.00	11.50	Non-Traversable Median	11.50	false	false		
34	Rural Multi-Lane Segment Four-lane Divided	647+00.000	647+26.050	26.05	0.0049	2018-2022: 4,325	12.00	12.00	8.00	8.00	16.30	Non-Traversable Median	16.30	false	false		
35	Rural Multi-Lane Segment Four-lane Divided	647+26.050	647+50.000	23.95	0.0045	2018-2022: 4,325	12.00	12.00	8.00	8.00	18.80	Non-Traversable Median	18.80	false	false		
36	Rural Multi-Lane Segment Four-lane Divided	647+50.000	648+00.000	50.00	0.0095	2018-2022: 4,325	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		<u> </u>
37	Rural Multi-Lane Segment Four-lane Divided	648+00.000	648+42.000	42.00	0.0080	2018-2022: 4,325	12.00	12.00	8.00	8.00	17.48	Non-Traversable Median	17.48	false	false		<u> </u>
38	Rural Multi-Lane Segment Four-lane Divided	648+42.000	649+00.000	58.00	0.0110	2018-2022: 4,325	12.00	12.00	8.00	8.00	11.48	Non-Traversable Median	11.48	false	false		<u> </u>
39	Rural Multi-Lane Segment Four-lane Divided	649+00.000	649+20.000	20.00	0.0038	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		<u> </u>
40	Rural Multi-Lane Segment Four-lane Divided	649+20.000	655+50.000	630.00	0.1193	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false	<u> </u>	

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Width (ft)	Right Lane Width (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
41	Rural Multi-Lane Segment Four-lane Divided	655+50.000	655+70.000	20.00	0.0038	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
42	Rural Multi-Lane Segment Four-lane Undivided	655+70.000	656+50.000	80.00	0.0152	2018-2022: 4,325	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
43	Rural Multi-Lane Segment Four-lane Divided	656+50.000	667+80.000	1,130.00	0.2140	2018-2022: 4,325	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
44	Rural Multi-Lane Segment Four-lane Divided	667+80.000	668+50.000	70.00	0.0133	2018-2022: 4,325	12.00	12.00	8.00	0.00	20.00	Non-Traversable Median	20.00	false	false		
45	Rural Multi-Lane Segment Four-lane Divided	668+50.000	668+80.000	30.00	0.0057	2018-2022: 4,325	12.00	12.00	8.00	0.00	18.62	Non-Traversable Median	18.62	false	false		
46	Rural Multi-Lane Segment Four-lane Divided	668+80.000	669+05.000	25.00	0.0047	2018-2022: 4,325	12.00	12.00	8.00	8.00	16.08	Non-Traversable Median	16.08	false	false		
47	Rural Multi-Lane Segment Four-lane Divided	669+05.000	669+80.000	75.00	0.0142	2018-2022: 4,325	12.00	12.00	8.00	8.00	11.46	Non-Traversable Median	11.46	false	false		
48	Rural Multi-Lane Segment Four-lane Divided	669+80.000	672+86.110	306.11	0.0580	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
49	Rural Multi-Lane Segment Four-lane Divided	672+86.110	675+50.000	263.89	0.0500	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
50	Rural Multi-Lane Segment Four-lane Divided	675+50.000	676+00.000	50.00	0.0095	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Traversable Median	8.00	false	false		
51	Rural Multi-Lane Segment Four-lane Divided	676+00.000	676+30.000	30.00	0.0057	2018-2022: 4,325	12.00	12.00	0.00	8.00	8.00	Traversable Median	8.00	false	false		
52	Rural Multi-Lane Segment Four-lane Undivided	676+30.000	677+50.000	120.00	0.0227	2018-2022: 4,325	12.00	12.00	0.00	8.00	0.00	None	0.00	false	false	0:1	0:1
53	Rural Multi-Lane Segment Four-lane Divided	677+50.000	679+00.000	150.00	0.0284	2018-2022: 4,325	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
54	Rural Multi-Lane Segment Four-lane Divided	679+00.000	680+20.000	120.00	0.0227	2018-2022: 4,150	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
55	Rural Multi-Lane Segment Four-lane Divided	680+20.000	680+80,000	60.00		2018-2022: 4.150	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
56	Rural Multi-Lane Segment Four-lane Divided	680+80.000	681+00.000	20.00	0.0038	2018-2022: 4,150	12.00	12.00	0.00	8.00	8.00	Traversable Median	20.00	false	false		
57	Rural Multi-Lane Segment Four-lane Undivided	681+00.000	682+20,000	120.00		2018-2022: 4,150	12.00	12.00	0.00	8.00	0.00	None	0.00	false	false	0:1	0:1
58	Rural Multi-Lane Segment Four-lane Divided	682+20.000	683+82.710	162.71		2018-2022: 4,150	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false	0.1	
59	Rural Multi-Lane Segment Four-lane Divided	683+82.710	690+00.000	617.29		2018-2022: 4,150	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
60	Rural Multi-Lane Segment Four-lane Divided	690+00.000	691+50.000	150.00		2018-2022: 4,150	12.00	12.00	8.00	0.00	20.00	Non-Traversable Median	20.00	false	false		
61	Rural Multi-Lane Segment Four-lane Divided	691+50.000		51.00		2018-2022: 4,150	12.00	12.00	8.00	0.00	17.45	Non-Traversable Median	17.45	false	false		
62	-	692+01.000	692+70.000	69.00		2018-2022: 4,150	12.00	12.00	8.00	0.00	11.45		11.45	false	false		
63	Rural Multi-Lane Segment Four-lane Divided	692+01.000 692+70.000	692+70.000	115.01		2018-2022: 4,150	12.00	12.00	8.00	0.00	8.00	Non-Traversable Median	19.00		false		
	Rural Multi-Lane Segment Four-lane Divided													false			
64	Rural Multi-Lane Segment Four-lane Divided	693+85.010	698+50.000	464.99		2018-2022: 4,150	12.00	12.00	8.00	0.00	8.00	Non-Traversable Median	19.00	false	false		$\vdash$
65	Rural Multi-Lane Segment Four-lane Divided	698+50.000	698+70.000	20.00	0.0038	2018-2022: 4,150	12.00	12.00	8.00	0.00	8.00	Traversable Median	19.00	false	false		$\vdash$
66	Rural Multi-Lane Segment Four-lane Divided	698+70.000	699+20.000	50.00		2018-2022: 4,150	12.00	12.00	0.00	0.00	8.00	Traversable Median	19.00	false	false		$\vdash$
67	Rural Multi-Lane Segment Four-lane Undivided	699+20.000	700+40.000	120.00		2018-2022: 4,150	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1
68	Rural Multi-Lane Segment Four-lane Divided	700+40.000		10.00		2018-2022: 4,150	12.00	12.00	0.00	0.00	8.00	Traversable Median	8.00	false	false		<del></del>
69	Rural Multi-Lane Segment Four-lane Divided	700+50.000	701+10.000	60.00		2018-2022: 4,150	12.00	12.00	8.00	0.00	8.00	Traversable Median	8.00	false	false		<del></del>
70	Rural Multi-Lane Segment Four-lane Divided	701+10.000	702+00.000	90.00		2018-2022: 4,150	12.00	12.00	8.00	0.00	8.00	Non-Traversable Median	8.00	false	false		<del></del>
71	Rural Multi-Lane Segment Four-lane Divided	702+00.000	702+50.000	50.00		2018-2022: 4,900	12.00	12.00	8.00	0.00	8.00	Non-Traversable Median	8.00	false	false		<del></del>
72	Rural Multi-Lane Segment Four-lane Divided	702+50.000	707+00.000	450.00	0.0852	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		<u> </u>
73	Rural Multi-Lane Segment Four-lane Divided	707+00.000	708+00.000	100.00	0.0189	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		<u> </u>
74	Rural Multi-Lane Segment Four-lane Divided	708+00.000	708+80.000	80.00	0.0152	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		<u> </u>
75	Rural Multi-Lane Segment Four-lane Divided	708+80.000	709+00.000	20.00	0.0038	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		<u> </u>
76	Rural Multi-Lane Segment Four-lane Undivided	709+00.000	710+30.000	130.00	0.0246	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
77	Rural Multi-Lane Segment Four-lane Divided	710+30.000	710+47.850	17.85	0.0034	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		<u> </u>
78	Rural Multi-Lane Segment Four-lane Divided	710+47.850	725+00.000	1,452.15	0.2750	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		<u> </u>
79	Rural Multi-Lane Segment Four-lane Divided	725+00.000	727+52.350	252.35	0.0478	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		<u> </u>
80	Rural Multi-Lane Segment Four-lane Divided	727+52.350	735+00.000	747.65	0.1416	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		<u> </u>
81	Rural Multi-Lane Segment Four-lane Divided	735+00.000	755+50.000	2,050.00	0.3883	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		<u> </u>
82	Rural Multi-Lane Segment Four-lane Undivided	755+50.000	756+90.000	140.00	0.0265	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
83	Rural Multi-Lane Segment Four-lane Divided	756+90.000	757+00.000	10.00	0.0019	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		

Rural Mathi-Lane Segment Four-lane Divided   764+00.000   764+55.000   765+55.000   765-55.000	alse false alse false alse false alse false alse false alse false alse false alse false	0:1	0:1
Fig.   Rural Mathi-Lame Segment Four-lame Divided   764+00.000   764+50.000   765+52.000   765-52.000   765-52.000   765-52.000   765-52.000   765-52.000   765-52.000   765-52.000   765-52.000   765-52.000   765-52.000   765-52.000   765-52.000   765-52.000   765-52.000   775-80.000   777	alse false alse false alse false alse false alse false alse false alse false false false false false false false false false	0:1	0:1
State   Rumal Madis-Lame Segment Four-lame Divided   764-50,000   765-52.550   777-880,000   1227.55   20225   2018-2022-4900   12.00   12.00   8.00   8.00   20.00   Non-Traversable Median   20.00   fable   765-52.550   777-880,000   2027-800,000   2018-2022-4900   12.00   12.00   8.00   8.00   8.00   20.00   Non-Traversable Median   20.00   fable   765-52.550   777-880,000   778-80,000   7	alse false alse false alse false alse false alse false false false false false false false false false	0:1	0:1
Second Hubbi-Lane Segment Four-lane Univided   765+52-550   777+80000   1227+45   0.2325   2018-2022-4.900   12.00   12.00   12.00   12.00   8.00   8.00   0.00   None   0.00   fable   12.00   12.0	alse false alse false alse false alse false alse false false false false false	0:1	0:1
Rural Multi-Lane Segment Four-lane Divided   778+80.00   179+80.00   100.00   0.0189   2018-2022-4.500   12.00   12.00   8.00   8.00   8.00   8.00   None   0.00   fall   1.00	alse false alse false alse false false false false false false false	0:1	0:1
Part	alse false alse false alse false alse false	0:1	0:1
Second Nation	alse false alse false false false		
Rural Multi-Lane Segment Four-lane Divided   780+45-930   785+40.000   494-07   0.0936   2018-2022-4.900   12.00   12.00   8.00   8.00   8.00   8.00   Non-Traversable Median   8.00   fall   1.00	alse false alse false		
Rural Multi-Lane Segment Four-lane Divided   785+0000   785+0000   786+09.000   7	alse false		
Nural Multi-Lane Segment Four-lane Divided   785+50.000   786+09.000   786+09.000   786+09.000   786+09.000   786+50.000   41.00   0.0078   2018-2022: 4,900   12.00   12.00   8.00   8.00   11.54   Non-Traversable Median   11.54   fab.			
Proceed Segment Four-lane Divided   786+90.000   786+50.0000   786+50.0000   786+50.0000   786+50.0000   786+50.0000   786+50.0000   786+50.0000   786+50.0000   786+50.0000   786+50.000	alse false		
96 Rural Multi-Lane Segment Four-lane Divided 801+10.000 801+61.000 51.00 0.0097 2018-2022: 4,900 12.00 12.00 8.00 8.00 20.00 Non-Traversable Median 17.45 fals 98 Rural Multi-Lane Segment Four-lane Divided 801+61.000 802+30.000 69.00 0.0131 2018-2022: 4,900 12.00 12.00 8.00 8.00 11.45 Non-Traversable Median 11.45 fals 99 Rural Multi-Lane Segment Four-lane Divided 802+30.000 802+40.000 10.00 0.0019 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 8.00 Non-Traversable Median 11.45 fals 10.00 Rural Multi-Lane Segment Four-lane Divided 802+40.000 808+80.000 50.00 0.0019 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 8.00 Non-Traversable Median 20.00 fals 10.00 Rural Multi-Lane Segment Four-lane Divided 808+80.000 808+80.000 50.00 0.005 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 Non-Traversable Median 20.00 fals 10.00 Rural Multi-Lane Segment Four-lane Divided 808+80.000 809+80.000 50.00 0.005 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 Non-Traversable Median 20.00 fals 10.00 Rural Multi-Lane Segment Four-lane Divided 808+80.000 809+80.000 50.00 0.005 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 Non-Traversable Median 20.00 fals 10.00 Rural Multi-Lane Segment Four-lane Divided 808+80.000 809+60.000 60.00 0.0014 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 Non-Traversable Median 20.00 fals 10.00 Rural Multi-Lane Segment Four-lane Divided 809+60.000 810+00.000 60.00 0.0014 2018-2022: 4,900 12.00 12.00 0.00 0.00 0.00 None 0.00 fals 10.00 Rural Multi-Lane Segment Four-lane Divided 809+60.000 810+00.000 58.00 0.0038 2018-2022: 4,900 12.00 12.00 0.00 0.00 8.00 Traversable Median 20.00 fals 10.00 Rural Multi-Lane Segment Four-lane Divided 810+00.000 816+00.000 58.00 0.0038 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 Non-Traversable Median 20.00 fals 10.00 Rural Multi-Lane Segment Four-lane Divided 810+00.000 816+00.			
Proceed No.   Proceedings   Proceeding   Process   Pro	alse false		
8 Rural Multi-Lane Segment Four-lane Divided 801+61.000 802+30.000 69.00 0.0131 2018-2022: 4,900 12.00 12.00 8.00 8.00 11.45 Non-Traversable Median 11.45 fals 100 Rural Multi-Lane Segment Four-lane Divided 808+30.000 808+80.000 50.00 0.0019 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 Non-Traversable Median 20.00 fals 101 Rural Multi-Lane Segment Four-lane Divided 808+80.000 809+00.000 20.00 0.0038 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 Non-Traversable Median 20.00 fals 102 Rural Multi-Lane Segment Four-lane Undivided 808+80.000 809+00.000 809+00.000 60.00 0.0114 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 Non-Traversable Median 20.00 fals 103 Rural Multi-Lane Segment Four-lane Divided 809+00.000 809+00.000 809+00.000 40.00 0.0076 2018-2022: 4,900 12.00 12.00 0.00 0.00 0.00 None 0.00 fals 105 Rural Multi-Lane Segment Four-lane Divided 809+00.000 810+00.000 \$810+20.000 0.0038 2018-2022: 4,900 12.00 12.00 0.00 0.00 8.00 Traversable Median 20.00 fals 105 Rural Multi-Lane Segment Four-lane Divided 810+00.000 810+00.000 \$810+00.000 \$810+20.000 1.000 0.000 \$810+20.000 0.000 0.000 0.000 0.000 0.000 0.000 None 0.000 fals 105 Rural Multi-Lane Segment Four-lane Divided 810+00.000 810+20.000 20.00 0.0038 2018-2022: 4,900 12.00 12.00 0.00 0.00 8.00 Traversable Median 20.00 fals 106 Rural Multi-Lane Segment Four-lane Divided 810+00.000 816+70.000 580.00 0.0038 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 Traversable Median 20.00 fals 106 Rural Multi-Lane Segment Four-lane Divided 810+20.000 816+70.000 580.00 0.0038 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 Non-Traversable Median 20.00 fals 108 Rural Multi-Lane Segment Four-lane Divided 816+70.000 817+20.000 50.00 0.0038 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 11.50 Non-Traversable Median 11.50 fals 108 Rural Multi-Lane Segment Four-lane Divided 854+00.000 854+00.000 854+00.000 854+20.000 10.000 108 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 11.50 Non-Traversable Median 11.50 fals 110 Rural Multi-Lane Segment Four-lane Divided 854+00.000 854+60.000 854+80.000 10.00	alse false		
99 Rural Multi-Lane Segment Four-lane Divided 802+30.000 802+40.000 10.00 0.0019 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 Non-Traversable Median 20.00 fals 101 Rural Multi-Lane Segment Four-lane Divided 808+30.000 808+80.000 50.00 0.0095 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 Non-Traversable Median 20.00 fals 102 Rural Multi-Lane Segment Four-lane Undivided 808+80.000 809+00.000 20.00 0.0038 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 Non-Traversable Median 20.00 fals 103 Rural Multi-Lane Segment Four-lane Undivided 809+00.000 809+60.000 60.00 0.0114 2018-2022: 4,900 12.00 12.00 0.00 0.00 0.00 None 0.00 fals 104 Rural Multi-Lane Segment Four-lane Divided 809+00.000 810+00.000 20.00 0.0038 2018-2022: 4,900 12.00 12.00 0.00 0.00 0.00 None 0.00 fals 105 Rural Multi-Lane Segment Four-lane Divided 810+00.000 810+20.000 20.00 0.0038 2018-2022: 4,900 12.00 12.00 0.00 0.00 0.00 None 0.00 fals 105 Rural Multi-Lane Segment Four-lane Divided 810+00.000 810+20.000 20.00 0.0038 2018-2022: 4,900 12.00 12.00 0.00 0.00 8.00 Traversable Median 20.00 fals 106 Rural Multi-Lane Segment Four-lane Divided 810+20.000 816+00.000 580.00 0.1098 2018-2022: 4,900 12.00 12.00 8.00 8.00 Non-Traversable Median 20.00 fals 106 Rural Multi-Lane Segment Four-lane Divided 810+20.000 816+00.000 580.00 0.1098 2018-2022: 4,900 12.00 12.00 8.00 8.00 Non-Traversable Median 20.00 fals 107 Rural Multi-Lane Segment Four-lane Divided 816+00.000 816+00.000 580.00 0.1098 2018-2022: 4,900 12.00 12.00 8.00 8.00 11.50 Non-Traversable Median 11.50 fals 108 Rural Multi-Lane Segment Four-lane Divided 816+00.000 816+00.00 50.000 50.000 50.000 50.000 50.00000 50.0000 50.0000 50.0000 50.0000 50.0000 50.0000 50.0000 50.0000 50.0000 50.0000 50.00000 50.0000 50.00000 50.0000 50.0000 50.	alse false		
Rural Multi-Lane Segment Four-lane Divided   802+40.000   808+30.000   590.00   0.1117   2018-2022: 4,900   12.00   12.00   8.00   8.00   8.00   8.00   8.00   Non-Traversable Median   20.00   fall	alse false		
Rural Multi-Lane Segment Four-lane Univided   808+30.000   808+80.000   809+00.000   20.00   0.0038   2018-2022: 4,900   12.00   12.00   12.00   8.00   8.00   8.00   Traversable Median   20.00   fals	alse false		
Rural Multi-Lane Segment Four-lane Undivided   808+80.000   809+00.000   20.00   0.0038   2018-2022: 4,900   12.00   12.00   8.00   8.00   0.00   None   0.00   fals	alse false		
Rural Multi-Lane Segment Four-lane Undivided   809+00.000   809+60.0000   809+60.	alse false		
Rural Multi-Lane Segment Four-lane Undivided   809+00.000   809+60.000   60.00   0.0114   2018-2022: 4,900   12.00   12.00   0.00   0.00   0.00   None   0.00   fals	alse false	0:1	0:1
Rural Multi-Lane Segment Four-lane Divided   810+0.000   810+2.000   20.00   0.0038   2018-2022: 4,900   12.00   12.00   8.00   8.00   8.00   R.00   Traversable Median   20.00   fals   106   Rural Multi-Lane Segment Four-lane Divided   810+2.000   816+00.000   816+00.000   580.00   0.1098   2018-2022: 4,900   12.00   12.00   8.00   8.00   8.00   8.00   Non-Traversable Median   20.00   fals   107   Rural Multi-Lane Segment Four-lane Divided   816+00.000   816+70.000   70.00   0.0133   2018-2022: 4,900   12.00   12.00   8.00   8.00   11.50   Non-Traversable Median   11.50   fals   108   Rural Multi-Lane Segment Four-lane Divided   816+70.000   817+20.000   50.00   0.0095   2018-2022: 4,900   12.00   12.00   8.00   8.00   17.50   Non-Traversable Median   17.50   fals   109   Rural Multi-Lane Segment Four-lane Divided   817+20.000   853+70.000   3,650.00   0.6913   2018-2022: 4,900   12.00   12.00   8.00   8.00   8.00   17.50   Non-Traversable Median   20.00   fals   108   Rural Multi-Lane Segment Four-lane Divided   853+70.000   854+00.000   30.00   0.0057   2018-2022: 4,900   12.00   12.00   8.00   8.00   18.36   Non-Traversable Median   18.36   fals   110   Rural Multi-Lane Segment Four-lane Divided   854+00.000   854+0	alse false	0:1	0:1
Rural Multi-Lane Segment Four-lane Divided   810+20.000   816+00.000   816+00.000   810+00.000   800   0.1098   2018-2022: 4,900   12.00   12.00   8.00   8.00   8.00   8.00   Non-Traversable Median   11.50   fals   108   Rural Multi-Lane Segment Four-lane Divided   816+70.000   817+20.000   50.00   0.0095   2018-2022: 4,900   12.00   12.00   8.00   8.00   8.00   17.50   Non-Traversable Median   17.50   fals   109   Rural Multi-Lane Segment Four-lane Divided   817+20.000   853+70.000   3,650.00   0.6913   2018-2022: 4,900   12.00   12.00   8.00   8.00   8.00   17.50   Non-Traversable Median   17.50   fals   109   Rural Multi-Lane Segment Four-lane Divided   817+20.000   853+70.000   3,650.00   0.6913   2018-2022: 4,900   12.00   12.00   8.00   8.00   8.00   18.36   Non-Traversable Median   20.00   fals   109   Rural Multi-Lane Segment Four-lane Divided   853+70.000   854+00.000   30.00   0.0057   2018-2022: 4,900   12.00   12.00   8.00   8.00   18.36   Non-Traversable Median   18.36   fals   111   Rural Multi-Lane Segment Four-lane Divided   854+00.000	alse false		
Rural Multi-Lane Segment Four-lane Divided   816+00.000   816+70.000   70.00   0.0133   2018-2022: 4,900   12.00   12.00   8.00   8.00   11.50   Non-Traversable Median   11.50   fals	alse false		
Rural Multi-Lane Segment Four-lane Divided   816+70.000   817+20.000   50.00   0.0095   2018-2022: 4,900   12.00   12.00   8.00   8.00   17.50   Non-Traversable Median   17.50   fals   17.50   Rural Multi-Lane Segment Four-lane Divided   817+20.000   853+70.000   3,650.00   0.6913   2018-2022: 4,900   12.00   12.00   8.00   8.00   8.00   20.00   Non-Traversable Median   20.00   fals   17.50   Rural Multi-Lane Segment Four-lane Divided   853+70.000   853+70.000   853+70.000   30.00   0.0057   2018-2022: 4,900   12.00   12.00   8.00   8.00   18.36   Non-Traversable Median   18.36   fals   17.50   Rural Multi-Lane Segment Four-lane Divided   854+00.000   854+00	alse false		
Rural Multi-Lane Segment Four-lane Divided   817+20.000   853+70.000   3,650.00   0.6913   2018-2022: 4,900   12.00   12.00   8.00   8.00   20.00   Non-Traversable Median   20.00   fals   110   Rural Multi-Lane Segment Four-lane Divided   853+70.000   854+00.000   854+00.000   30.00   0.0057   2018-2022: 4,900   12.00   12.00   8.00   8.00   18.36   Non-Traversable Median   18.36   fals   111   Rural Multi-Lane Segment Four-lane Divided   854+00.000   85	alse false		
Rural Multi-Lane Segment Four-lane Divided   853+70.000   854+00.000   30.00   0.0057   2018-2022: 4,900   12.00   12.00   8.00   8.00   18.36   Non-Traversable Median   18.36   fals   111   Rural Multi-Lane Segment Four-lane Divided   854+00.000   854+16.000   16.00   0.0030   2018-2022: 4,900   12.00   12.00   12.00   8.00   8.00   15.85   Non-Traversable Median   15.85   fals   112   Rural Multi-Lane Segment Four-lane Divided   854+16.000   854+80.000   64.00   0.0121   2018-2022: 4,900   12.00   12.00   8.00   8.00   11.49   Non-Traversable Median   11.49   fals   113   Rural Multi-Lane Segment Four-lane Divided   854+80.000   860+90.000   610.00   0.1155   2018-2022: 4,900   12.00   12.00   8.00   8.00   8.00   Non-Traversable Median   11.49   fals   113   Rural Multi-Lane Segment Four-lane Divided   854+80.000   860+90.000   610.00   0.1155   2018-2022: 4,900   12.00   12.00   8.00   8.00   8.00   Non-Traversable Median   10.0000   10.000   10.000   10.000   10.000   10.000   10.000   10.0000   10.000   10.000   10.000   10.000   10.000   10.000   10.0000   10.000   10.000   10.000   10.000   10.000   10.000   10.0000   10.000   10.000   10.000   10.000   10.000   10.000   10.0000   10.000   10.000   10.000   10.000   10.000   10.000   10.0000   10.000   10.0000   1	alse false		
111         Rural Multi-Lane Segment Four-lane Divided         854+00.000         854+16.000         16.00         0.0030         2018-2022: 4,900         12.00         12.00         8.00         8.00         15.85         Non-Traversable Median         15.85         fals           112         Rural Multi-Lane Segment Four-lane Divided         854+16.000         854+80.000         64.00         0.0121         2018-2022: 4,900         12.00         12.00         8.00         8.00         11.49         Non-Traversable Median         11.49         fals           113         Rural Multi-Lane Segment Four-lane Divided         854+80.000         860+90.00         610.00         0.1155         2018-2022: 4,900         12.00         12.00         8.00         8.00         Non-Traversable Median         11.49         fals	alse false		
112         Rural Multi-Lane Segment Four-lane Divided         854+16.000         854+80.000         64.00         0.0121         2018-2022: 4,900         12.00         12.00         8.00         8.00         11.49         Non-Traversable Median         11.49         fals           113         Rural Multi-Lane Segment Four-lane Divided         854+80.000         860+90.000         610.00         0.1155         2018-2022: 4,900         12.00         12.00         8.00         8.00         Non-Traversable Median         20.00         fals	alse false		
113 Rural Multi-Lane Segment Four-lane Divided 854+80.000 860+90.000 610.00 0.1155 2018-2022: 4,900 12.00 12.00 8.00 8.00 8.00 Non-Traversable Median 20.00 fals	alse false		
	alse false		
114 Rural Multi-Lana Sammart Four-Jana Divided 860-00 00 861-85 000 95 00 0.0180 2018-2022-4 000 12 00 900 900 900 Tayarakh Madisa	alse false		
11-7 Kurai istanti-Lame Deginent Four-lane Divided   000770.000  001703.000  73.001 0.0100/2010-2022; 4,700   12.00  0.00  0.00  0.00  0.00  11aversable Median   20.00  12.00  12.00  0.00  0.00  0.00  0.00  0.00  0.00	alse false		
115 Rural Multi-Lane Segment Four-lane Undivided 861+85.000 862+00.000 15.00 0.0028 2018-2022: 4,900 12.00 12.00 8.00 0.00 0.00 None 0.00 fals	alse false	0:1	0:1
116 Rural Multi-Lane Segment Four-lane Undivided 862+00.000 862+50.000 50.00 0.0095 2018-2022: 4,900 12.00 12.00 8.00 0.00 0.00 None 0.00 fals	alse false	0:1	0:1
117 Rural Multi-Lane Segment Four-lane Undivided 862+50.000 862+60.000 10.00 0.0019 2018-2022: 4.900 12.00 12.00 8.00 8.00 0.00 None 0.00 fals	alse false	0:1	0:1
118 Rural Multi-Lane Segment Four-lane Divided 862+60.000 863+10.000 50.00 0.0095 2018-2022: 4.900 12.00 12.00 8.00 8.00 Traversable Median 19.00 fals	alse false		$\neg$
119 Rural Multi-Lane Segment Four-lane Divided 863+10.000 869+00.000 590.00 0.1117 2018-2022: 4.900 12.00 12.00 8.00 8.00 8.00 Non-Traversable Median 19.00 fals	alse false		$\neg$
120 Rural Multi-Lane Segment Four-lane Divided 869+00.000 869+70.000 70.00 0.0133 2018-2022: 4.900 12.00 12.00 8.00 8.00 11.50 Non-Traversable Median 11.50 fals	alse false		$\neg$
	alse false		
122 Rural Multi-Lane Segment Four-lane Divided 870+20.000 881+80.000 1,160.00 0.2197 2018-2022: 4,900 12.00 12.00 8.00 8.00 20.00 Non-Traversable Median 20.00 fals			
	alse false		$\neg$
124 Rural Multi-Lane Segment Four-lane Divided 882+31.000 883+00.000 69.00 0.0131 2018-2022: 4,900 12.00 12.00 8.00 8.00 11.45 Non-Traversable Median 11.45 fals		$\neg \vdash$	$\neg$
125 Rural Multi-Lane Segment Four-lane Divided 883+00.000 887+90.000 490.00 0.0928 2018-2022: 4,900 12.00 12.00 8.00 8.00 Non-Traversable Median 20.00 fals	<del>-    </del>		$\neg$
126   Rural Multi-Lane Segment Four-lane Divided   887+90.000   888+20.000   30.00   0.0057   2018-2022: 4,900   12.00   12.00   8.00   8.00   8.00   Traversable Median   20.00   false	alse false		$\neg$

Seg. No.	Туре	Start Location (Sta. ft)	End Location (Sta. ft)	Length (ft)	Length (mi)	AADT	Left Lane Width (ft)	Right Lane Width (ft)	Left Shoulder Width (ft)	Right Shoulder Width (ft)	Median Width (ft)	Median Type	Effective Median Width (ft)	Lighting	Automated Speed Enforcement	Left Side Slope	Right Side Slope
127	Rural Multi-Lane Segment Four-lane Undivided	888+20.000	889+30.000	110.00	0.0208	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
128	Rural Multi-Lane Segment Four-lane Divided	889+30.000	889+50.000	20.00	0.0038	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Traversable Median	20.00	false	false		
129	Rural Multi-Lane Segment Four-lane Divided	889+50.000	894+50.000	500.00	0.0947	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	20.00	false	false		
130	Rural Multi-Lane Segment Four-lane Divided	894+50.000	895+15.000	65.00	0.0123	2018-2022: 4,900	12.00	12.00	8.00	8.00	11.55	Non-Traversable Median	11.55	false	false		
131	Rural Multi-Lane Segment Four-lane Divided	895+15.000	895+60.000	45.00	0.0085	2018-2022: 4,900	12.00	12.00	8.00	8.00	17.55	Non-Traversable Median	17.55	false	false		
132	Rural Multi-Lane Segment Four-lane Divided	895+60.000	898+00.000	240.00	0.0455	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
133	Rural Multi-Lane Segment Four-lane Divided	898+00.000	906+70.000	870.00	0.1648	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
134	Rural Multi-Lane Segment Four-lane Divided	906+70.000	907+21.000	51.00	0.0097	2018-2022: 4,900	12.00	12.00	8.00	8.00	17.45	Non-Traversable Median	17.45	false	false		
135	Rural Multi-Lane Segment Four-lane Divided	907+21.000	907+80.000	59.00	0.0112	2018-2022: 4,900	12.00	12.00	8.00	8.00	11.95	Non-Traversable Median	11.95	false	false		
136	Rural Multi-Lane Segment Four-lane Divided	907+80.000	907+90.000	10.00	0.0019	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.50	Non-Traversable Median	19.50	false	false		
137	Rural Multi-Lane Segment Four-lane Divided	907+90.000	913+70.000	580.00	0.1098	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	19.00	false	false		
138	Rural Multi-Lane Segment Four-lane Divided	913+70.000	914+00.000	30.00	0.0057	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Traversable Median	19.00	false	false		
139	Rural Multi-Lane Segment Four-lane Divided	914+00.000	914+30.000	30.00	0.0057	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Traversable Median	19.00	false	false		
140	Rural Multi-Lane Segment Four-lane Undivided	914+30.000	914+40.000	10.00	0.0019	2018-2022: 4,900	12.00	12.00	8.00	8.00	0.00	None	0.00	false	false	0:1	0:1
141	Rural Multi-Lane Segment Four-lane Undivided	914+40.000	915+40.000	100.00	0.0189	2018-2022: 4,900	12.00	12.00	0.00	0.00	0.00	None	0.00	false	false	0:1	0:1
142	Rural Multi-Lane Segment Four-lane Divided	915+40.000	916+00.000	60.00	0.0114	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Traversable Median	19.00	false	false		
143	Rural Multi-Lane Segment Four-lane Divided	916+00.000	921+00.000	500.00	0.0947	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	19.00	false	false		
144	Rural Multi-Lane Segment Four-lane Divided	921+00.000	921+90.000	90.00	0.0170	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	19.00	false	false		
145	Rural Multi-Lane Segment Four-lane Divided	921+90.000	922+00.000	10.00	0.0019	2018-2022: 4,900	12.00	12.00	8.00	8.00	8.00	Non-Traversable Median	8.00	false	false		
146	Rural Multi-Lane Segment Four-lane Divided	922+00.000	922+59.000	59.00	0.0112	2018-2022: 4,900	12.00	12.00	8.00	8.00	11.54	Non-Traversable Median	11.54	false	false		
147	Rural Multi-Lane Segment Four-lane Divided	922+59.000	923+00.000	41.00	0.0078	2018-2022: 4,900	12.00	12.00	8.00	8.00	17.54	Non-Traversable Median	17.54	false	false		
148	Rural Multi-Lane Segment Four-lane Divided	923+00.000	941+70.000	1,870.00	0.3542	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		
149	Rural Multi-Lane Segment Four-lane Divided	941+70.000	948+50.000	680.00	0.1288	2018-2022: 4,900	12.00	12.00	8.00	8.00	20.00	Non-Traversable Median	20.00	false	false		

**Table 4. Evaluation Intersection (Section 3)** 

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
1	I90EBRamps_S466th_SD38 (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	699+20.000	2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,1710; 2046: 12,2002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170	2025: 630; 2026: 644; 2027: 657; 2028: 671; 2029: 685; 2030; 700; 2031: 716; 2032: 732; 2033: 748; 2034: 764; 2035: 780; 2036: 796; 2037: 812; 2038: 828; 2039: 844; 2040: 860; 2041: 1,166; 2042: 1,473; 2043: 1,779; 2044: 2,086; 2045: 2,392; 2046: 2,699; 2047: 3,005; 2048: 3,312; 2049: 3,618; 2050: 3,925	4	Stop-Controlled	1	0	4.64	4.27	false
2	SD38/260th_St (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	605+70.000	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,782; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	2025: 1,508; 2026: 1,706; 2027: 1,904; 2028: 2,102; 2029: 2,300; 2030: 2,472; 2031: 2,645; 2032: 2,818; 2033: 2,990; 2034: 3,163; 3035: 3,336; 2036: 3,509; 2037: 3,681; 2038: 3,854; 2039: 4,027; 2040: 4,200; 2041: 4,260; 2042: 4,320; 2043: 4,380; 2044: 4,440; 2045: 4,500; 2046: 4,560; 2047: 4,620; 2048: 4,680; 2049: 4,740; 2050: 4,800	4	Stop-Controlled	2	1	16.83	13.71	false
3	466thN/SD38 (v1)	Rural Multi-Lane Intersection Three-Legged w/STOP control	676+50.000	2025; 7,901; 2026; 9,093; 2027; 10,285; 2028; 11,477; 2029; 12,670; 2030; 12,965; 2031; 13,260; 2032; 13,556; 2033; 13,851; 2034; 14,147; 2035; 14,442; 2036; 14,748; 2037; 15,033; 2038; 15,329; 2039; 15,624; 2040; 15,920; 2041; 16,287; 2042; 16,654; 2043; 17,021; 2044; 17,388; 2045; 17,755; 2046; 18,122; 2047; 18,489; 2048; 18,856; 2049; 19,223; 2050; 19,590	2025: 118; 2026: 121; 2027: 124; 2028: 127; 2029: 130; 2030: 133; 2031: 136; 2032: 139; 2033: 142; 2034: 145; 2035: 149; 2036: 152; 2037: 155; 2038: 158; 2039: 161; 2040: 165; 2041: 168; 2042: 172; 2043: 175; 2044: 179; 2045: 182; 2046: 186; 2047: 189; 2048: 193; 2049: 196; 2050: 200	3	Stop-Controlled	0	0	6.61		false

 Table 5. Evaluation Intersection (Section 3)

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
5	468th Ave (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	809+00.000	2025: 6,164; 2026: 6,585; 2027: 7,007; 2028: 7,428; 2029: 7,850; 2030: 8,018; 2031: 8,186; 2032: 8,354; 2033: 8,522; 2034: 8,690; 2035: 8,859; 2036: 9,027; 2037: 9,195; 2038: 9,363; 2039: 9,531; 2040: 9,700; 2041: 9,905; 2042: 10,110; 2043: 10,315; 2044: 10,520; 2045: 10,725; 2046: 10,930; 2047: 11,135; 2048: 11,340; 2049: 11,545; 2050: 11,750	2025: 667: 2026: 682: 2027: 696: 2028: 710: 2029: 725: 2030: 741: 2031: 758; 2032: 775: 2033: 792; 2034: 809; 2035: 825; 2036: 842; 2037: 859; 2038: 876: 2039: 893; 2004: 910; 2041: 1,052: 2042: 1,195: 2043: 1,337; 2044: 1,480; 2045: 1,622; 2046: 1,765; 2047: 1,907; 2048: 2,050; 2049: 2,192; 2050: 2,335	4	Stop-Controlled	1	0	0.00	0.00	false
6	SD38/Hwy 139 (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	862+00.000	2025: 6,704; 2026: 7,305; 2027: 7,907; 2028: 8,508; 2029: 9,110; 2030: 9,295; 2031: 9,480; 2032: 9,666; 2033: 9,851; 2034: 10,037; 2035: 10,272; 2036: 10,408; 2037: 10,593; 2038: 10,799; 2039: 10,964; 2040: 11,150; 2041: 11,375; 2042: 11,600; 2043: 11,825; 2044: 12,050; 2044: 12,275; 2046: 12,500; 2047: 12,725; 2048: 12,950; 2049: 13,175; 2050: 13,400	2025: 2,990; 2026: 3,054; 2027: 3,117; 2028: 3,181; 2029: 3,245; 2030: 3,321; 2031: 3,397; 2032: 3,474; 2033: 3,550; 2034: 3,626; 2035: 3,703; 2036: 3,779; 2037: 3,855; 2038: 3,932; 2039: 4,008; 2040: 4,085; 2041: 4,178; 2042: 4,271; 2043: 4,364; 2044: 4,457; 2045: 4,550; 2046: 4,643; 2047: 4,736; 2048: 4,829; 2049: 4,922; 2050: 5,015	4	Stop-Controlled	1	0	0.00	0.00	false
7	LaMesa/SD 38 (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	915+00.000	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	2025: 1,266; 2026: 1,293; 2027: 1,320; 2028: 1,347; 2029: 1,375; 2030: 1,407; 2031: 1,439; 2032: 1,471; 2033: 1,504; 2034: 1,536; 2035: 1,568; 2036: 1,725; 2037: 1,949; 2038: 2,172; 2039: 2,396; 2040: 2,620; 2041: 2,940; 2042: 3,261; 2043: 3,581; 2044: 3,902; 2045: 4,222; 2046: 4,543; 2047: 4,863; 2048: 5,184; 2049: 5,504; 2050: 5,825	4	Stop-Controlled	0	0	0.00	0.00	false

# **Table 6. Evaluation Ramp Terminal - Site (Section 3)**

Inter. No.	Title	Туре	Area Type	Legs	Location (Sta. ft)	Traffic Control	AADT
4	I90WB Ramps (v2)	Freeway Ramp Terminal A2 - Three-Leg at Two-Quadrant Parclo A	Rural	4	681+00.000	Stop-Controlled	Inside: 2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2033: 8,678; 2034: 8,902; 2035: 9,127; 2036: 9,251; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,148; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2088: 12,858; 2049: 12,878; 2050: 13,170; Outside: 2025: 5,705; 2026: 6,224; 2027: 6,742; 2028: 7,261; 2029: 7,780; 2030: 8,004; 2031: 8,229; 2032: 8,453; 2038: 6,938; 2036: 9,351; 2037: 9,576; 2038: 9,800; 2039: 10,025; 2040: 10,250; 2041: 10,542; 2042: 10,834; 2043: 11,126; 2044: 11,418; 2045: 11,710; 2046: 12,002; 2047: 12,294; 2048: 12,586; 2049: 12,878; 2050: 13,170: Entrance: 2025: 856; 2026: 875; 2027: 893; 2028: 911; 2029: 930; 2030: 951; 2031: 973; 2032: 995; 2033: 1,170: 2034: 1,039; 2035: 1,006; 2036: 1,082; 2037: 1,104; 2038: 1,126; 2039: 91; 148; 2040: 1,170; 2041: 1,339; 2042: 1,508; 2043: 1,677; 2044: 1,846; 2045: 2,015; 2046: 2,184; 2047: 2,353; 2048: 2,522; 2049: 2,69; 2,050:

## Table 7. Crash History Intersection (Section 3)

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
1	I90EBRamps_S466th_SD38 (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	699+20.000	2018-2022: 4,150	2018-2022: 590	4	Stop-Controlled	1	0	4.64	4.27	false
2	SD38/260th_St (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	605+70.000	2018-2022: 4,325	2018-2022: 915	4	Stop-Controlled	2	1	16.83	13.71	false
3	466thN/SD38 (v1)	Rural Multi-Lane Intersection Three-Legged w/STOP control	676+50.000	2018-2022: 4,325	2018-2022: 110	3	Stop-Controlled	0	0	6.61		false

### $Table \ 8. \ Crash \ History \ Intersection \ (Section \ 3)$

Inter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Major road approaches w/Left Turn Lanes	Major road approaches w/Right Turn Lanes	Skew1	Skew2	Lighted at Night
5	468th Ave (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	809+00.000	2018-2022: 4,900	2018-2022: 625	4	Stop-Controlled	1	0	0.00	0.00	false
6	SD38/Hwy139 (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	862+00.000	2018-2022: 4,900	2018-2022: 2,800	4	Stop-Controlled	1	0	0.00	0.00	false
7	LaMesa/SD38 (v1)	Rural Multi-Lane Intersection Four-Legged w/STOP control	915+00.000	2018-2022: 4,900	2018-2022: 1,185	4	Stop-Controlled	0	0	0.00	0.00	false

### Table 9. Crash Highway Ramp Terminal - Site (Highway with Crash History)

Inter. No.	Title	Туре	Area Type	Legs	Location (Sta. ft)	Traffic Control	AADT
4	I90WB Ramps (v2)	Freeway Ramp Terminal A2 - Three-Leg at Two-Quadrant Parclo A	Rural	4	681+00.000	Stop-Controlled	Inside: 2018-2022: 4,150; Outside: 2018-2022: 4,150 :: Entrance: 2018-2022: 802

Table 10. Expected Highway Crash Rates and Frequencies Summary (Section 3)

First Year of Analysis   2025		
Evaluated Length (mi)   6.8845     Average Future Road AADT (vpd)   10,372     Expected Crashes	First Year of Analysis	2025
Average Future Road AADT (vpd)   10,372	Last Year of Analysis	2050
Expected Crashes	Evaluated Length (mi)	6.8845
Total Crashes   611.54     Fatal and Injury Crashes   316.56     Fatal and Serious Injury Crashes   206.04     Property-Damage-Only Crashes   294.99     Percent of Total Expected Crashes     Percent Fatal and Injury Crashes (%)   52     Percent Fatal and Serious Injury Crashes (%)   34     Percent Property-Damage-Only Crashes (%)   48     Expected Crash Rate     Crash Rate (crashes/mi/yr)   3.4165     FI Crash Rate (crashes/mi/yr)   1.7685     FI no/C Crash Rate (crashes/mi/yr)   1.1511     PDO Crash Rate (crashes/mi/yr)   1.6480     Expected Travel Crash Rate     Total Travel (million veh-mi)   677.62     Travel FI Crash Rate (crashes/million veh-mi)   0.90     Travel FI Crash Rate (crashes/million veh-mi)   0.47     Travel FI Crash Rate (crashes/million veh-mi)   0.30	Average Future Road AADT (vpd)	10,372
Fatal and Injury Crashes  Fatal and Serious Injury Crashes  206.04  Property-Damage-Only Crashes  Percent of Total Expected Crashes  Percent Fatal and Injury Crashes (%)  Percent Fatal and Serious Injury Crashes (%)  Percent Property-Damage-Only Crashes (%)  Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  FI Crash Rate (crashes/mi/yr)  1.7685  FI Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel Crash Rate (crashes/million veh-mi)  O.47  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  O.30	Expected Crashes	
Fatal and Serious Injury Crashes   206.04     Property-Damage-Only Crashes   294.99     Percent of Total Expected Crashes     Percent Fatal and Injury Crashes (%)   52     Percent Fatal and Serious Injury Crashes (%)   34     Percent Property-Damage-Only Crashes (%)   48     Expected Crash Rate     Crash Rate (crashes/mi/yr)   3.4165     FI Crash Rate (crashes/mi/yr)   1.7685     FI no/C Crash Rate (crashes/mi/yr)   1.1511     PDO Crash Rate (crashes/mi/yr)   1.6480     Expected Travel Crash Rate     Total Travel (million veh-mi)   677.62     Travel FI Crash Rate (crashes/million veh-mi)   0.90     Travel FI Crash Rate (crashes/million veh-mi)   0.47     Travel FI Crash Rate (crashes/million veh-mi)   0.30	Total Crashes	611.54
Property-Damage-Only Crashes  Percent of Total Expected Crashes  Percent Fatal and Injury Crashes (%) 52  Percent Fatal and Serious Injury Crashes (%) 34  Percent Property-Damage-Only Crashes (%) 48  Expected Crash Rate  Crash Rate (crashes/mi/yr) 3.4165  FI Crash Rate (crashes/mi/yr) 1.7685  FI no/C Crash Rate (crashes/mi/yr) 1.1511  PDO Crash Rate (crashes/mi/yr) 1.6480  Expected Travel Crash Rate  Total Travel (million veh-mi) 677.62  Travel Crash Rate (crashes/million veh-mi) 0.90  Travel FI Crash Rate (crashes/million veh-mi) 0.47  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.30	Fatal and Injury Crashes	316.56
Percent of Total Expected Crashes  Percent Fatal and Injury Crashes (%)  Percent Fatal and Serious Injury Crashes (%)  Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  Travel FI no/C Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI no/C Crash Rate (crashes/million veh-mi)  O.30	Fatal and Serious Injury Crashes	206.04
Percent Fatal and Injury Crashes (%)  Percent Fatal and Serious Injury Crashes (%)  Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  Travel FI no/C Crash Rate (crashes/mi/yr)  Travel FI no/C Crash Rate (crashes/mi/yr)  Travel FI no/C Crash Rate (crashes/mi/yr)  Travel FI no/C Crash Rate (crashes/mi/yr)  Travel FI no/C Crash Rate (crashes/mi/yr)  Travel FI no/C Crash Rate (crashes/mi/yr)  Travel FI no/C Crash Rate (crashes/million veh-mi)  Travel FI no/C Crash Rate (crashes/million veh-mi)  O.30	Property-Damage-Only Crashes	294.99
Percent Fatal and Serious Injury Crashes (%)  Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  3.4165  FI Crash Rate (crashes/mi/yr)  1.7685  FI no/C Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  1.1511  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel FI crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  0.47  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.30	Percent of Total Expected Crashes	
Percent Property-Damage-Only Crashes (%)  Expected Crash Rate  Crash Rate (crashes/mi/yr)  3.4165  FI Crash Rate (crashes/mi/yr)  1.7685  FI no/C Crash Rate (crashes/mi/yr)  PDO Crash Rate (crashes/mi/yr)  1.511  PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  0.47  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.30	Percent Fatal and Injury Crashes (%)	52
Expected Crash Rate  Crash Rate (crashes/mi/yr) 3.4165  FI Crash Rate (crashes/mi/yr) 1.7685  FI no/C Crash Rate (crashes/mi/yr) 1.1511  PDO Crash Rate (crashes/mi/yr) 1.6480  Expected Travel Crash Rate  Total Travel (million veh-mi) 677.62  Travel Crash Rate (crashes/million veh-mi) 0.90  Travel FI Crash Rate (crashes/million veh-mi) 0.47  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.30	Percent Fatal and Serious Injury Crashes (%)	34
Crash Rate (crashes/mi/yr) 3.4165  FI Crash Rate (crashes/mi/yr) 1.7685  FI no/C Crash Rate (crashes/mi/yr) 1.1511  PDO Crash Rate (crashes/mi/yr) 1.6480  Expected Travel Crash Rate  Total Travel (million veh-mi) 677.62  Travel Crash Rate (crashes/million veh-mi) 0.90  Travel FI Crash Rate (crashes/million veh-mi) 0.47  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.30	Percent Property-Damage-Only Crashes (%)	48
FI Crash Rate (crashes/mi/yr)  1.7685  FI no/C Crash Rate (crashes/mi/yr)  1.1511  PDO Crash Rate (crashes/mi/yr)  1.6480  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel Crash Rate (crashes/million veh-mi)  0.90  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  0.47  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.30	Expected Crash Rate	
FI no/C Crash Rate (crashes/mi/yr)  1.1511  PDO Crash Rate (crashes/mi/yr)  1.6480  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.30	Crash Rate (crashes/mi/yr)	3.4165
PDO Crash Rate (crashes/mi/yr)  Expected Travel Crash Rate  Total Travel (million veh-mi)  Travel Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI Crash Rate (crashes/million veh-mi)  Travel FI no/C Crash Rate (crashes/million veh-mi)  0.30	FI Crash Rate (crashes/mi/yr)	1.7685
Expected Travel Crash Rate  Total Travel (million veh-mi) 677.62  Travel Crash Rate (crashes/million veh-mi) 0.90  Travel FI Crash Rate (crashes/million veh-mi) 0.47  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.30	FI no/C Crash Rate (crashes/mi/yr)	1.1511
Total Travel (million veh-mi) 677.62  Travel Crash Rate (crashes/million veh-mi) 0.90  Travel FI Crash Rate (crashes/million veh-mi) 0.47  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.30	PDO Crash Rate (crashes/mi/yr)	1.6480
Travel Crash Rate (crashes/million veh-mi) 0.90  Travel FI Crash Rate (crashes/million veh-mi) 0.47  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.30	Expected Travel Crash Rate	
Travel FI Crash Rate (crashes/million veh-mi) 0.47  Travel FI no/C Crash Rate (crashes/million veh-mi) 0.30	Total Travel (million veh-mi)	677.62
Travel FI no/C Crash Rate (crashes/million veh-mi) 0.30	Travel Crash Rate (crashes/million veh-mi)	0.90
	Travel FI Crash Rate (crashes/million veh-mi)	0.47
Travel PDO Crash Rate (crashes/million veh-mi) 0.43	Travel FI no/C Crash Rate (crashes/million veh-mi)	0.30
	Travel PDO Crash Rate (crashes/million veh-mi)	0.43

Table 11. Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 3)

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr	Expected FI Crash Frequency (crashes/yr	Expected FI no/C Crash Frequency (crashes/yr	Expected PDO Crash Frequency (crashes/yr	Predicted Total Crash Frequency (crashes/yr	Predicted FI Crash Frequency (crashes/yr	Predicted FI no/C Crash Frequency (crashes/yr	Predicted PDO Crash Frequency (crashes/yr	(Expected - Predicted) Total Crash Frequency (crashes/yr	(Expected - Predicted) FI Crash Frequency (crashes/yr	(Expected - Predicted) FI no/C Crash Frequency (crashes/yr	(Expected - Predicted) PDO Crash Frequency (crashes/yr	Expected Crash Rate (crashes/ mi/yr)	Expected Travel Crash Rate (crashes/ million veh-mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
1	585+00.000	586+00.000	0.0189	0.613	1.124	0.0236	0.0131	0.0090	0.0105	0.0432	0.0222	0.0140	0.0211	-0.0197	-0.0091	-0.0050	-0.0106	1.2449	0.28	
2	586+00.000	593+75.000	0.1468	11.628	8.711	0.4472	0.3303	0.2278	0.1170	0.3350	0.1718	0.1087	0.1632	0.1122	0.1584	0.1191	-0.0462	3.0469	0.70	
3	593+75.000	594+84.940	0.0208	0.674	1.236	0.0259	0.0144	0.0099	0.0115	0.0475	0.0244	0.0154	0.0231	-0.0216	-0.0100	-0.0055	-0.0116	1.2449	0.28	
4	594+84.940	600+00.000	0.0975	3.158	5.789	0.1214	0.0674	0.0465	0.0540	0.2227	0.1142	0.0723	0.1085	-0.1012	-0.0468	-0.0257	-0.0545	1.2449	0.28	
5	600+00.000	600+42.000	0.0080	0.258	0.472	0.0099	0.0055	0.0038	0.0044	0.0182	0.0093	0.0059	0.0088	-0.0083	-0.0038	-0.0021	-0.0044	1.2449	0.28	
6	600+42.000	601+00.000	0.0110	0.356	0.652	0.0137	0.0076	0.0052	0.0061	0.0251	0.0129	0.0081	0.0122	-0.0114	-0.0053	-0.0029	-0.0061	1.2449	0.28	
7	601+00.000	602+00.000	0.0189	0.613	1.124	0.0236	0.0131	0.0090	0.0105	0.0432	0.0222	0.0140	0.0211	-0.0197	-0.0091	-0.0050	-0.0106	1.2449	0.28	
8	602+00.000	605+00.000	0.0568	11.046	4.273	0.4249	0.0886	0.0611	0.3362	0.1644	0.0825	0.0512	0.0818	0.2605	0.0061	0.0099	0.2544	7.4774	1.37	
9	605+00.000	605+10.000	0.0019	0.081	0.155	0.0031	0.0017	0.0012	0.0014	0.0060	0.0030	0.0019	0.0030	-0.0028	-0.0013	-0.0007	-0.0016	1.6521	0.30	
10	605+10.000	605+40.000	0.0057	0.247	0.475	0.0095	0.0053	0.0036	0.0042	0.0183	0.0092	0.0057	0.0091	-0.0088	-0.0039	-0.0021	-0.0049	1.6693	0.30	
11	605+40.000	605+60.000	0.0038	0.248	0.543	0.0095	0.0058	0.0036	0.0038	0.0209	0.0121	0.0062	0.0088	-0.0114	-0.0063	-0.0026	-0.0050	2.5153	0.46	
12	605+60.000	605+70.000	0.0019	0.119	0.249	0.0046	0.0028	0.0017	0.0018	0.0096	0.0056	0.0028	0.0040	-0.0050	-0.0028	-0.0011	-0.0022	2.4181	0.44	
SD38/260th_St (v1)	605+70.000			35.375	96.924	1.3606	0.5456	0.3469	0.8150	3.7279	1.7583	0.8841	1.9696	-2.3673	-1.2127	-0.5372	-1.1546			0.20
13	605+70.000	605+75.000	0.0009	0.059	0.125	0.0023	0.0014	0.0009	0.0009	0.0048	0.0028	0.0014	0.0020	-0.0025	-0.0014	-0.0006	-0.0011	2.4181	0.44	
14	605+75.000	606+00.000	0.0047	0.298	0.624	0.0114	0.0069	0.0043	0.0045	0.0240	0.0139	0.0071	0.0101	-0.0125	-0.0069	-0.0028	-0.0056	2.4181	0.44	
15	606+00.000	607+50.000	0.0284	9.988	2.179	0.3841	0.3471	0.2395	0.0370	0.0838	0.0421	0.0261	0.0417	0.3003	0.3050	0.2134	-0.0047	13.5216	2.47	
16	607+50.000	609+00.000	0.0284	1.165	2.137	0.0448	0.0249	0.0172	0.0199	0.0822	0.0413	0.0256	0.0409	-0.0374	-0.0164	-0.0084	-0.0210	1.5777	0.29	
17	609+00.000	609+21.930	0.0042	8.886	0.312	0.3418	0.0080	0.0055	0.3338	0.0120	0.0060	0.0037	0.0060	0.3297	0.0020	0.0018	0.3278	82.2842	15.04	
18	609+21.930	611+40.000	0.0413	1.694	3.106	0.0652	0.0362	0.0250	0.0290	0.1195	0.0600	0.0372	0.0595	-0.0543	-0.0238	-0.0123	-0.0305	1.5777	0.29	
19	611+40.000	611+50.000	0.0019	0.078	0.142	0.0030	0.0017	0.0011	0.0013	0.0055	0.0028	0.0017	0.0027	-0.0025	-0.0011	-0,0006	-0.0014	1.5777	0.29	
20	611+50.000	612+09.000	0.0112	0.458	0.840	0.0176	0.0098	0.0068	0.0078	0.0323	0.0162	0.0101	0.0161	-0.0147	-0.0064	-0.0033	-0.0083	1.5777	0.29	
21	612+09.000	612+50.000	0.0078	0.319	0.584	0.0123	0.0068	0.0047	0.0054	0.0225	0.0113	0.0070	0.0112	-0.0102	-0.0045	-0.0023	-0.0057	1.5777	0.29	
22	612+50.000	624+64.530	0.2300	18.151	17.300	0.6981	0.2748	0.1896	0.4233	0.6654	0.3341	0.2073	0.3313	0.0327	-0.0593	-0.0177	0.0920	3.0350	0.56	
23	624+64.530	631+30.000	0.1260	5.170	9.479	0.1989	0.1104	0.0762	0.0884	0.3646	0.1831	0.1136	0.1815	-0.1657	-0.0726	-0.0374	-0.0931	1.5777	0.29	
24	631+30.000	631+81.000	0.0097	0.396	0.727	0.0152	0.0085	0.0058	0.0068	0.0279	0.0140	0.0087	0.0139	-0.0127	-0.0056	-0.0029	-0.0071	1.5777	0.29	
25	631+81.000	632+50.000	0.0131	0.536	0.983	0.0206	0.0114	0.0079	0.0092	0.0378	0.0190	0.0118	0.0188	-0.0172	-0.0075	-0.0039	-0.0097	1.5777	0.29	
26	632+50.000	636+92.820	0.0839	3.440	6.308	0.1323	0.0735	0.0507	0.0588	0.2426	0.1218	0.0756	0.1208	-0.1103	-0.0483	-0.0249	-0.0620	1.5777	0.29	
27	636+92.820	638+80.000	0.0355	1.454	2.666	0.0559	0.0311	0.0214	0.0249	0.1025	0.0515	0.0320	0.0511	-0.0466	-0.0204	-0.0105	-0.0262	1.5777	0.29	
28	638+80.000	639+00.000	0.0038	0.157	0.291	0.0060	0.0034	0.0023	0.0027	0.0112	0.0056	0.0035	0.0056	-0.0051	-0.0023	-0.0012	-0.0029	1.5948	0.29	
29	639+00.000	640+00.000	0.0189	1.282	2.935	0.0493	0.0297	0.0185	0.0196	0.1129	0.0653	0.0334	0.0476	-0.0636	-0.0356	-0.0149	-0.0280	2.6043	0.48	
30	640+00.000	640+20.000	0.0038	0.157	0.291	0.0060	0.0034	0.0023	0.0027	0.0112	0.0056	0.0035	0.0056	-0.0051	-0.0023	-0.0012	-0.0029	1.5948	0.29	
31	640+20.000	645+50.000	0.1004	12.833	7.550	0.4936	0.1412	0.0974	0.3524	0.2904	0.1458	0.0905	0.1446	0.2032	-0.0046	0.0069	0.2078	4.9172	0.90	
32	645+50.000	646+30.000	0.0152	0.622	1.140	0.0239	0.0133	0.0092	0.0106	0.0438	0.0220	0.0137	0.0218	-0.0199	-0.0087	-0.0045	-0.0112	1.5777	0.29	
33	646+30.000	647+00.000	0.0133	0.544	0.997	0.0209	0.0116	0.0080	0.0093	0.0384	0.0193	0.0119	0.0191	-0.0174	-0.0076	-0.0039	-0.0098	1.5777	0.29	
34	647+00.000	647+26.050	0.0049	0.202	0.371	0.0078	0.0043	0.0030	0.0035	0.0143	0.0072	0.0044	0.0071	-0.0065	-0.0028	-0.0015	-0.0036	1.5777	0.29	
35	647+26.050	647+50.000	0.0045	0.186	0.341	0.0072	0.0040	0.0027	0.0032	0.0131	0.0066	0.0041	0.0065	-0.0060	-0.0026	-0.0013	-0.0034	1.5777	0.29	
36	647+50.000	648+00.000		0.389	0.712	0.0149	0.0083	0.0057	0.0066	0.0274	0.0138	0.0085	0.0136	-0.0125	-0.0055	-0.0028	-0.0070	1.5777	0.29	
37	648+00.000	648+42.000	0.0080	0.326	0.598	0.0126	0.0070	0.0048	0.0056	0.0230	0.0116	0.0072	0.0115	-0.0105	-0.0046	-0.0024	-0.0059	1.5777	0.29	
38	648+42.000	649+00.000	0.0110	0.451	0.826	0.0173	0.0096	0.0066	0.0077	0.0318	0.0160	0.0099	0.0158	-0.0144	-0.0063	-0.0033	-0.0081	1.5777	0.29	

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr	Expected FI Crash Frequency (crashes/yr	Expected FI no/C Crash Frequency (crashes/yr	Expected PDO Crash Frequency (crashes/yr	Predicted Total Crash Frequency (crashes/yr )	Predicted FI Crash Frequency (crashes/yr	Predicted FI no/C Crash Frequency (crashes/yr	Predicted PDO Crash Frequency (crashes/yr	(Expected - Predicted) Total Crash Frequency (crashes/yr	(Expected - Predicted) FI Crash Frequency (crashes/yr	(Expected - Predicted) FI no/C Crash Frequency (crashes/yr	(Expected - Predicted) PDO Crash Frequency (crashes/yr	Expected Crash Rate (crashes/ mi/yr)	Expected Travel Crash Rate (crashes/ million veh-mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
39	649+00.000	649+20.000	0.0038	0.155	0.285	0.0060	0.0033	0.0023	0.0027	0.0110	0.0055	0.0034	0.0055	-0.0050	-0.0022	-0.0011	-0.0028	1.5777	0.29	
40	649+20.000	655+50.000	0.1193	13.610	8.974	0.5235	0.1622	0.1119	0.3612	0.3452	0.1733	0.1075	0.1718	0.1783	-0.0111	0.0044	0.1894	4.3871	0.80	
41	655+50.000	655+70.000	0.0038	0.157	0.291	0.0060	0.0034	0.0023	0.0027	0.0112	0.0056	0.0035	0.0056	-0.0051	-0.0023	-0.0012	-0.0029	1.5948	0.29	
42	655+70.000	656+50.000	0.0152	0.953	1.996	0.0366	0.0222	0.0138	0.0144	0.0768	0.0444	0.0227	0.0324	-0.0401	-0.0222	-0.0089	-0.0179	2.4181	0.44	
43	656+50.000	667+80.000	0.2140	17.494	16.096	0.6729	0.2592	0.1788	0.4136	0.6191	0.3109	0.1929	0.3082	0.0538	-0.0516	-0.0141	0.1054	3.1440	0.57	
44	667+80.000	668+50.000	0.0133	0.570	1.087	0.0219	0.0122	0.0084	0.0098	0.0418	0.0210	0.0130	0.0208	-0.0199	-0.0088	-0.0046	-0.0111	1.6521	0.30	
45	668+50.000	668+80.000	0.0057	0.244	0.466	0.0094	0.0052	0.0036	0.0042	0.0179	0.0090	0.0056	0.0089	-0.0085	-0.0038	-0.0020	-0.0047	1.6521	0.30	
46	668+80.000	669+05.000	0.0047	0.194	0.356	0.0075	0.0041	0.0029	0.0033	0.0137	0.0069	0.0043	0.0068	-0.0062	-0.0027	-0.0014	-0.0035	1.5777	0.29	
47	669+05.000	669+80.000	0.0142	0.583	1.068	0.0224	0.0124	0.0086	0.0100	0.0411	0.0206	0.0128	0.0205	-0.0187	-0.0082	-0.0042	-0.0105	1.5777	0.29	
48	669+80.000	672+86.110	0.0580	2.378	4.360	0.0915	0.0508	0.0350	0.0407	0.1677	0.0842	0.0523	0.0835	-0.0762	-0.0334	-0.0172	-0.0428	1.5777	0.29	
49	672+86.110	675+50.000	0.0500	19.481	3.759	0.7493	0.6841	0.4719	0.0652	0.1446	0.0726	0.0450	0.0720	0.6047	0.6115	0.4269	-0.0068	14.9916	2.74	
50	675+50.000	676+00.000	0.0095	0.397	0.741	0.0153	0.0085	0.0058	0.0068	0.0285	0.0143	0.0089	0.0142	-0.0132	-0.0058	-0.0030	-0.0074	1.6115	0.29	
51	676+00.000	676+30.000	0.0057	9.563	0.484	0.3678	0.3592	0.2478	0.0086	0.0186	0.0094	0.0058	0.0093	0.3492	0.3499	0.2420	-0.0007	64.7353	11.83	
52	676+30.000	677+50.000	0.0227	13.736	3.258	0.5283	0.4896	0.3051	0.0387	0.1253	0.0725	0.0371	0.0528	0.4030	0.4171	0.2680	-0.0141	23.2451	4.25	
466thN/SD38 (v1)	676+50.000			22.161	36.857	0.8523	0.4087	0.2971	0.4437	1.4176	0.6242	0.3958	0.7934	-0.5652	-0.2155	-0.0987	-0.3497			0.15
53	677+50.000	679+00.000	0.0284	27.311	2.137	1.0504	0.3890	0.2684	0.6614	0.0822	0.0413	0.0256	0.0409	0.9683	0.3477	0.2428	0.6205	36.9756	6.76	
54	679+00.000	680+20.000	0.0227	0.604	1.085	0.0232	0.0129	0.0090	0.0103	0.0417	0.0218	0.0140	0.0199	-0.0185	-0.0089	-0.0051	-0.0097	1.0213	0.29	
55	680+20.000	680+80.000	0.0114	0.305	0.553	0.0117	0.0065	0.0045	0.0052	0.0213	0.0111	0.0072	0.0102	-0.0096	-0.0046	-0.0026	-0.0050	1.0325	0.29	
56	680+80.000	681+00.000	0.0038	0.106	0.201	0.0041	0.0023	0.0016	0.0018	0.0077	0.0040	0.0026	0.0037	-0.0036	-0.0018	-0.0010	-0.0019	1.0818	0.30	
I90WB Ramps (v2)	681+00.000			15.671	11.679	0.6027	0.3058		0.2969	0.4492	0.1413		0.3079	0.1535	0.1646		-0.0111			0.16
57	681+00.000	682+20.000	0.0227	0.917	1.958	0.0353	0.0214	0.0134	0.0139	0.0753	0.0451	0.0247	0.0302	-0.0401	-0.0237	-0.0113	-0.0163	1.5517	0.44	
58	682+20.000	683+82.710	0.0308	0.818	1.471	0.0315	0.0175	0.0121	0.0139	0.0566	0.0296	0.0190	0.0270	-0.0251	-0.0120	-0.0069	-0.0131	1.0213	0.29	
59	683+82.710	690+00.000	0.1169	3.104	5.582	0.1194	0.0665	0.0461	0.0529	0.2147	0.1121	0.0721	0.1026	-0.0953	-0.0456	-0.0261	-0.0497	1.0213	0.29	
60	690+00.000	691+50.000	0.0284	0.791	1.478	0.0304	0.0169	0.0117	0.0135	0.0569	0.0297	0.0191	0.0272	-0.0265	-0.0128	-0.0074	-0.0137	1.0704	0.30	
61	691+50.000	692+01.000	0.0097	0.269	0.503	0.0103	0.0058	0.0040	0.0046	0.0193	0.0101	0.0065	0.0092	-0.0090	-0.0043	-0.0025	-0.0047	1.0704	0.30	
62	692+01.000	692+70.000	0.0131	0.364	0.680	0.0140	0.0078	0.0054	0.0062	0.0262	0.0137	0.0088	0.0125	-0.0122	-0.0059	-0.0034	-0.0063	1.0704	0.30	
63	692+70.000	693+85.010	0.0218	0.606	1.134	0.0233	0.0130	0.0090	0.0103	0.0436	0.0228	0.0147	0.0208	-0.0203	-0.0098	-0.0057	-0.0105	1.0704	0.30	
64	693+85.010	698+50.000	0.0881	2.451	4.583	0.0943	0.0525	0.0363	0.0418	0.1763	0.0921	0.0592	0.0842	-0.0820	-0.0396	-0.0229	-0.0424	1.0704	0.30	
65	698+50.000	698+70.000	0.0038	0.106	0.201	0.0041	0.0023	0.0016	0.0018	0.0077	0.0040	0.0026	0.0037	-0.0036	-0.0018	-0.0010	-0.0019	1.0818	0.30	
66	698+70.000	699+20.000	0.0095	0.278	0.544	0.0107	0.0059	0.0041	0.0047	0.0209	0.0109	0.0070	0.0100	-0.0103	-0.0050	-0.0029	-0.0053	1.1273	0.32	
I90EBRamps_S466th_SD38 (v1)	699+20.000			20.777	59.524	0.7991	0.3462	0.2319	0.4529	2.2894	1.2112	0.6869	1.0782	-1.4903	-0.8649	-0.4550	-0.6253			0.20
67	699+20.000	700+40.000	0.0227	0.950	2.117	0.0365	0.0221	0.0139	0.0145	0.0814	0.0488	0.0267	0.0326	-0.0449	-0.0267	-0.0128	-0.0182	1.6081	0.45	
68	700+40.000	700+50.000	0.0019	0.056	0.111	0.0022	0.0012	0.0008	0.0010	0.0043	0.0022	0.0014	0.0020	-0.0021	-0.0010	-0.0006	-0.0011	1.1385	0.32	
69	700+50.000	701+10.000	0.0114	0.323	0.615	0.0124	0.0069	0.0048	0.0055	0.0237	0.0124	0.0079	0.0113	-0.0112	-0.0054	-0.0032	-0.0058	1.0929	0.31	
70	701+10.000	702+00.000	0.0170	0.474	0.887	0.0182	0.0102	0.0070	0.0081	0.0341	0.0178	0.0115	0.0163	-0.0159	-0.0077	-0.0044	-0.0082	1.0704	0.30	
71	702+00.000	702+50.000	0.0095	0.199	0.404	0.0076	0.0042	0.0029	0.0034	0.0155	0.0083	0.0054	0.0073	-0.0079	-0.0041	-0.0025	-0.0038	0.8065	0.27	
72	702+50.000	707+00.000	0.0852	1.712	3.338	0.0658	0.0362	0.0247	0.0296	0.1284	0.0683	0.0447	0.0601	-0.0625	-0.0321	-0.0200	-0.0305	0.7724	0.26	
73	707+00.000	708+00.000	0.0189	0.380	0.742	0.0146	0.0080	0.0055	0.0066	0.0285	0.0152	0.0099	0.0134	-0.0139	-0.0071	-0.0044	-0.0068	0.7724	0.26	
74	708+00.000	708+80.000	0.0152	0.304	0.593	0.0117	0.0064	0.0044	0.0053	0.0228	0.0121	0.0079	0.0107	-0.0111	-0.0057	-0.0035	-0.0054	0.7724	0.26	
75	708+80.000	709+00.000	0.0038	0.077	0.151	0.0030	0.0016	0.0011	0.0013	0.0058	0.0031	0.0020	0.0027	-0.0029	-0.0015	-0.0009	-0.0014	0.7802	0.27	
76	709+00,000	710+30.000	0.0246	0.687	1.558	0.0264	0.0158	0.0096	0.0106	0.0599	0.0365	0.0206	0.0234	-0.0335	-0.0207	-0.0110	-0.0128	1.0727	0.36	

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr	Expected FI Crash Frequency (crashes/yr	Expected FI no/C Crash Frequency (crashes/yr	Expected PDO Crash Frequency (crashes/yr	Predicted Total Crash Frequency (crashes/yr )	Predicted FI Crash Frequency (crashes/yr	Predicted FI no/C Crash Frequency (crashes/yr	Predicted PDO Crash Frequency (crashes/yr	(Expected - Predicted) Total Crash Frequency (crashes/yr	(Expected - Predicted) FI Crash Frequency (crashes/yr	(Expected - Predicted) FI no/C Crash Frequency (crashes/yr	(Expected - Predicted) PDO Crash Frequency (crashes/yr	Expected Crash Rate (crashes/ mi/yr)	Expected Travel Crash Rate (crashes/ million veh-mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
77	710+30.000	710+47.850	0.0034	0.068	0.132	0.0026	0.0014	0.0010	0.0012	0.0051	0.0027	0.0018	0.0024	-0.0025	-0.0013	-0.0008	-0.0012	0.7724	0.26	
78	710+47.850	725+00.000	0.2750	18.323	10.771	0.7047	0.3140	0.2144	0.3907	0.4143	0.2203	0.1442	0.1939	0.2905	0.0937	0.0702	0.1968	2.5624	0.87	
79	725+00.000	727+52.350	0.0478	0.960	1.872	0.0369	0.0203	0.0139	0.0166	0.0720	0.0383	0.0251	0.0337	-0.0351	-0.0180	-0.0112	-0.0171	0.7724	0.26	
80	727+52.350	735+00.000	0.1416	2.844	5.545	0.1094	0.0602	0.0411	0.0492	0.2133	0.1134	0.0742	0.0998	-0.1039	-0.0533	-0.0332	-0.0506	0.7724	0.26	
81	735+00.000	755+50.000	0.3883	7.797	15.205	0.2999	0.1650	0.1126	0.1349	0.5848	0.3111	0.2035	0.2737	-0.2849	-0.1461	-0.0909	-0.1388	0.7724	0.26	
82	755+50.000	756+90.000	0.0265	0.740	1.678	0.0284	0.0170	0.0104	0.0114	0.0645	0.0393	0.0222	0.0252	-0.0361	-0.0223	-0.0118	-0.0138	1.0727	0.36	
83	756+90.000	757+00.000	0.0019	0.038	0.076	0.0015	0.0008	0.0006	0.0007	0.0029	0.0015	0.0010	0.0014	-0.0014	-0.0007	-0.0005	-0.0007	0.7802	0.27	
84	757+00.000	763+30.000	0.1193	2.396	4.673	0.0922	0.0507	0.0346	0.0415	0.1797	0.0956	0.0626	0.0841	-0.0876	-0.0449	-0.0279	-0.0427	0.7724	0.26	
85	763+30.000	764+00.000	0.0133	0.266	0.519	0.0102	0.0056	0.0038	0.0046	0.0200	0.0106	0.0070	0.0093	-0.0097	-0.0050	-0.0031	-0.0047	0.7724	0.26	
86	764+00.000	764+50.000	0.0095	0.190	0.371	0.0073	0.0040	0.0027	0.0033	0.0143	0.0076	0.0050	0.0067	-0.0069	-0.0036	-0.0022	-0.0034	0.7724	0.26	
87	764+50.000	765+52.550	0.0194	0.390	0.761	0.0150	0.0083	0.0056	0.0067	0.0293	0.0156	0.0102	0.0137	-0.0143	-0.0073	-0.0045	-0.0069	0.7724	0.26	
88	765+52.550	777+80.000	0.2325	8.935	9.104	0.3437	0.1340	0.0915	0.2097	0.3502	0.1862	0.1219	0.1639	-0.0065	-0.0523	-0.0304	0.0458	1.4783	0.50	
89	777+80.000	778+80.000	0.0189	0.528	1.198	0.0203	0.0121	0.0074	0.0082	0.0461	0.0281	0.0159	0.0180	-0.0258	-0.0159	-0.0084	-0.0098	1.0727	0.36	
90	778+80.000	779+00.000	0.0038	0.077	0.151	0.0030	0.0016	0.0011	0.0013	0.0058	0.0031	0.0020	0.0027	-0.0029	-0.0015	-0.0009	-0.0014	0.7802	0.27	
91	779+00.000	780+45.930	0.0276	0.555	1.082	0.0213	0.0117	0.0080	0.0096	0.0416	0.0221	0.0145	0.0195	-0.0203	-0.0104	-0.0065	-0.0099	0.7724	0.26	
92	780+45.930	785+40.000	0.0936	1.879	3.664	0.0723	0.0398	0.0271	0.0325	0.1409	0.0750	0.0491	0.0660	-0.0687	-0.0352	-0.0219	-0.0335	0.7724	0.26	
93	785+40.000	785+50.000	0.0019	0.038	0.074	0.0015	0.0008	0.0005	0.0007	0.0029	0.0015	0.0010	0.0013	-0.0014	-0.0007	-0.0004	-0.0007	0.7724	0.26	
94	785+50.000	786+09.000	0.0112	0.224	0.438	0.0086	0.0047	0.0032	0.0039	0.0168	0.0090	0.0059	0.0079	-0.0082	-0.0042	-0.0026	-0.0040	0.7724	0.26	
95	786+09.000	786+50.000	0.0078	0.156	0.304	0.0060	0.0033	0.0023	0.0027	0.0117	0.0062	0.0041	0.0055	-0.0057	-0.0029	-0.0018	-0.0028	0.7724	0.26	
96	786+50.000	801+10.000	0.2765	14.086	10.829	0.5418	0.2903	0.1982	0.2515	0.4165	0.2215	0.1450	0.1950	0.1253	0.0688	0.0532	0.0565	1.9593	0.67	
97	801+10.000	801+61.000	0.0097	0.194	0.378	0.0075	0.0041	0.0028	0.0034	0.0145	0.0077	0.0051	0.0068	-0.0071	-0.0036	-0.0023	-0.0035	0.7724	0.26	
98	801+61.000	802+30.000	0.0131	4.529	0.512	0.1742	0.1653	0.1128	0.0089	0.0197	0.0105	0.0069	0.0092	0.1545	0.1548	0.1060	-0.0003	13.3297	4.54	
99	802+30.000	802+40.000	0.0019	0.038	0.074	0.0015	0.0008	0.0005	0.0007	0.0029	0.0015	0.0010	0.0013	-0.0014	-0.0007	-0.0004	-0.0007	0.7724	0.26	
100	802+40.000	808+30.000	0.1117	2.244	4.376	0.0863	0.0475	0.0324	0.0388	0.1683	0.0895	0.0586	0.0788	-0.0820	-0.0420	-0.0262	-0.0400	0.7724	0.26	
101	808+30.000	808+80.000	0.0095	0.192	0.378	0.0074	0.0041	0.0028	0.0033	0.0145	0.0077	0.0051	0.0068	-0.0072	-0.0037	-0.0023	-0.0035	0.7802	0.27	
102	808+80.000	809+00.000	0.0038	0.106	0.240	0.0041	0.0024	0.0015	0.0016	0.0092	0.0056	0.0032	0.0036	-0.0052	-0.0032	-0.0017	-0.0020	1.0727	0.36	
468th Ave (v1)	809+00.000			30.877	45.501	1.1876	0.7630	0.5027	0.4246	1.7500	0.8386	0.4933	0.9115	-0.5624	-0.0756	0.0094	-0.4869			0.35
103	809+00.000	809+60.000	0.0114	0.400	0.998	0.0154	0.0091	0.0056	0.0063	0.0384	0.0231	0.0128	0.0153	-0.0230	-0.0140	-0.0072	-0.0090	1.3553	0.40	
104	809+60.000	810+00.000	0.0076	0.193	0.414	0.0074	0.0041	0.0028	0.0033	0.0159	0.0084	0.0054	0.0076	-0.0085	-0.0043	-0.0026	-0.0042	0.9798	0.29	
105	810+00.000	810+20.000	0.0038	0.089	0.175	0.0034	0.0019	0.0013	0.0015	0.0067	0.0035	0.0023	0.0032	-0.0033	-0.0017	-0.0010	-0.0017	0.9039	0.27	
106	810+20.000	816+00.000	0.1098	7.498	4.984	0.2884	0.0848	0.0579	0.2036	0.1917	0.1006	0.0651	0.0910	0.0967	-0.0158	-0.0071	0.1125	2.6254	0.78	
107	816+00.000	816+70.000	0.0133	0.308	0.602	0.0119	0.0065	0.0045	0.0053	0.0231	0.0121	0.0079	0.0110	-0.0113	-0.0056	-0.0034	-0.0057	0.8948	0.27	
108	816+70.000	817+20.000	0.0095	0.220	0.430	0.0085	0.0047	0.0032	0.0038	0.0165	0.0087	0.0056	0.0078	-0.0081	-0.0040	-0.0024	-0.0040	0.8948	0.27	
109	817+20.000	853+70.000	0.6913	40.796	31.361	1.5691	0.9058	0.6183	0.6633	1.2062	0.6332	0.4094	0.5730	0.3629	0.2725	0.2089	0.0903	2.2698	0.67	
110	853+70.000	854+00.000	0.0057	0.132	0.258	0.0051	0.0028	0.0019	0.0023	0.0099	0.0052	0.0034	0.0047	-0.0048	-0.0024	-0.0015	-0.0024	0.8948	0.27	
111	854+00.000	854+16.000	0.0030	0.070	0.138	0.0027	0.0015	0.0010	0.0012	0.0053	0.0028	0.0018	0.0025	-0.0026	-0.0013	-0.0008	-0.0013	0.8948	0.27	
112	854+16.000	854+80.000	0.0121	0.282	0.550	0.0108	0.0060	0.0041	0.0049	0.0211	0.0111	0.0072	0.0100	-0.0103	-0.0051	-0.0031	-0.0052	0.8948	0.27	
113	854+80.000	860+90.000	0.1155	2.688	5.241	0.1034	0.0569	0.0388	0.0465	0.2016	0.1058	0.0684	0.0958	-0.0982	-0.0490	-0.0296	-0.0493	0.8948	0.27	
114	860+90.000	861+85.000	0.0180	0.423	0.833	0.0163	0.0089	0.0061	0.0073	0.0320	0.0168	0.0109	0.0152	-0.0158	-0.0079	-0.0048	-0.0079	0.9039	0.27	
115	861+85.000	862+00.000	0.0028	0.097	0.231	0.0037	0.0022	0.0014	0.0015	0.0089	0.0053	0.0030	0.0035	-0.0051	-0.0031	-0.0016	-0.0020	1.3127	0.39	
SD38/Hwy139 (v1)	862+00,000			37.280	89.593	1.4338	0.6694	0.3723	0.7644	3.4459	1.8214	0.9272	1.6245	-2.0120	-1.1520	-0.5549	-0.8600			0.28

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr	Expected FI Crash Frequency (crashes/yr	Expected FI no/C Crash Frequency (crashes/yr	Expected PDO Crash Frequency (crashes/yr	Predicted Total Crash Frequency (crashes/yr	Predicted FI Crash Frequency (crashes/yr	Predicted FI no/C Crash Frequency (crashes/yr	Predicted PDO Crash Frequency (crashes/yr	(Expected - Predicted) Total Crash Frequency (crashes/yr	(Expected - Predicted) FI Crash Frequency (crashes/yr	(Expected - Predicted) FI no/C Crash Frequency (crashes/yr	(Expected - Predicted) PDO Crash Frequency (crashes/yr	Expected Crash Rate (crashes/ mi/yr)	Expected Travel Crash Rate (crashes/ million veh-mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
116	862+00.000	862+50.000	0.0095	0.379	0.903	0.0146	0.0087	0.0053	0.0059	0.0347	0.0207	0.0112	0.0141	-0.0201	-0.0120	-0.0059	-0.0082	1.5407	0.40	
117	862+50.000	862+60.000	0.0019	0.073	0.166	0.0028	0.0017	0.0010	0.0011	0.0064	0.0038	0.0021	0.0026	-0.0036	-0.0021	-0.0010	-0.0015	1.4857	0.38	
118	862+60.000	863+10.000	0.0095	0.257	0.505	0.0099	0.0054	0.0037	0.0044	0.0194	0.0101	0.0064	0.0094	-0.0096	-0.0046	-0.0027	-0.0049	1.0426	0.27	
119	863+10.000	869+00.000	0.1117	8.700	5.848	0.3346	0.0992	0.0677	0.2354	0.2249	0.1166	0.0745	0.1083	0.1097	-0.0174	-0.0068	0.1271	2.9945	0.77	
120	869+00.000	869+70.000	0.0133	0.356	0.694	0.0137	0.0075	0.0051	0.0062	0.0267	0.0138	0.0088	0.0128	-0.0130	-0.0063	-0.0037	-0.0067	1.0321	0.27	
121	869+70.000	870+20.000	0.0095	0.254	0.495	0.0098	0.0054	0.0037	0.0044	0.0191	0.0099	0.0063	0.0092	-0.0093	-0.0045	-0.0026	-0.0048	1.0321	0.27	
122	870+20.000	881+80.000	0.2197	11.597	11.497	0.4460	0.3089	0.2109	0.1371	0.4422	0.2293	0.1465	0.2129	0.0039	0.0796	0.0644	-0.0758	2.0302	0.52	
123	881+80.000	882+31.000	0.0097	0.259	0.505	0.0100	0.0055	0.0037	0.0045	0.0194	0.0101	0.0064	0.0094	-0.0095	-0.0046	-0.0027	-0.0049	1.0321	0.27	
124	882+31.000	883+00.000	0.0131	0.351	0.684	0.0135	0.0074	0.0051	0.0061	0.0263	0.0136	0.0087	0.0127	-0.0128	-0.0062	-0.0037	-0.0066	1.0321	0.27	
125	883+00.000	887+90.000	0.0928	8.192	4.856	0.3151	0.2475	0.1689	0.0676	0.1868	0.0968	0.0619	0.0899	0.1283	0.1506	0.1070	-0.0223	3.3950	0.88	
126	887+90.000	888+20.000	0.0057	0.154	0.303	0.0059	0.0033	0.0022	0.0027	0.0117	0.0060	0.0039	0.0056	-0.0057	-0.0028	-0.0016	-0.0030	1.0426	0.27	
127	888+20.000	889+30.000	0.0208	0.805	1.826	0.0310	0.0185	0.0113	0.0124	0.0702	0.0418	0.0226	0.0284	-0.0393	-0.0233	-0.0113	-0.0160	1.4857	0.38	
128	889+30.000	889+50.000	0.0038	0.103	0.202	0.0039	0.0022	0.0015	0.0018	0.0078	0.0040	0.0026	0.0037	-0.0038	-0.0019	-0.0011	-0.0020	1.0426	0.27	
129	889+50.000	894+50.000	0.0947	2.541	4.955	0.0977	0.0538	0.0367	0.0440	0.1906	0.0988	0.0632	0.0918	-0.0929	-0.0451	-0.0265	-0.0478	1.0321	0.27	
130	894+50.000	895+15.000	0.0123	0.330	0.644	0.0127	0.0070	0.0048	0.0057	0.0248	0.0128	0.0082	0.0119	-0.0121	-0.0059	-0.0034	-0.0062	1.0321	0.27	
131	895+15.000	895+60.000	0.0085	0.229	0.446	0.0088	0.0048	0.0033	0.0040	0.0172	0.0089	0.0057	0.0083	-0.0084	-0.0041	-0.0024	-0.0043	1.0321	0.27	
132	895+60.000	898+00.000	0.0455	1.220	2.379	0.0469	0.0258	0.0176	0.0211	0.0915	0.0474	0.0303	0.0441	-0.0446	-0.0216	-0.0127	-0.0229	1.0321	0.27	
133	898+00.000	906+70.000	0.1648	4.422	8.623	0.1701	0.0936	0.0639	0.0765	0.3316	0.1719	0.1099	0.1597	-0.1616	-0.0784	-0.0460	-0.0832	1.0321	0.27	
134	906+70.000	907+21.000	0.0097	0.259	0.505	0.0100	0.0055	0.0037	0.0045	0.0194	0.0101	0.0064	0.0094	-0.0095	-0.0046	-0.0027	-0.0049	1.0321	0.27	
135	907+21.000	907+80.000	0.0112	0.300	0.585	0.0115	0.0063	0.0043	0.0052	0.0225	0.0117	0.0075	0.0108	-0.0110	-0.0053	-0.0031	-0.0056	1.0321	0.27	
136	907+80.000	907+90.000	0.0019	0.051	0.099	0.0020	0.0011	0.0007	0.0009	0.0038	0.0020	0.0013	0.0018	-0.0019	-0.0009	-0.0005	-0.0010	1.0321	0.27	
137	907+90.000	913+70.000	0.1098	2.948	5.748	0.1134	0.0624	0.0426	0.0510	0.2211	0.1146	0.0733	0.1065	-0.1077	-0.0523	-0.0307	-0.0555	1.0321	0.27	
138	913+70.000	914+00.000	0.0057	0.154	0.303	0.0059	0.0033	0.0022	0.0027	0.0117	0.0060	0.0039	0.0056	-0.0057	-0.0028	-0.0016	-0.0030	1.0426	0.27	
139	914+00.000	914+30.000	0.0057	0.119	0.234	0.0046	0.0025	0.0017	0.0021	0.0090	0.0048	0.0031	0.0042	-0.0044	-0.0023	-0.0014	-0.0022	0.8027	0.27	
140	914+30.000	914+40.000	0.0019	0.054	0.124	0.0021	0.0013	0.0008	0.0008	0.0048	0.0029	0.0016	0.0019	-0.0027	-0.0016	-0.0009	-0.0010	1.1072	0.37	
141	914+40.000	915+40.000	0.0189	0.584	1.455	0.0225	0.0133	0.0081	0.0091	0.0560	0.0340	0.0191	0.0219	-0.0335	-0.0207	-0.0110	-0.0128	1.1855	0.39	
LaMesa/SD38 (v1)	915+00.000			45.389	83.561	1.7457	0.7046	0.4318	1.0411	3.2139	1.8143	0.9753	1.3996	-1.4682	-1.1097	-0.5434	-0.3585			0.47
142	915+40.000	916+00.000	0.0114	0.237	0.467	0.0091	0.0050	0.0034	0.0041	0.0180	0.0095	0.0062	0.0084	-0.0088	-0.0045	-0.0028	-0.0043	0.8027	0.27	
143	916+00.000	921+00.000	0.0947	1.957	3.816	0.0753	0.0414	0.0283	0.0339	0.1467	0.0779	0.0509	0.0689	-0.0715	-0.0365	-0.0226	-0.0350	0.7947	0.26	
144	921+00.000	921+90.000	0.0170	0.352	0.687	0.0135	0.0075	0.0051	0.0061	0.0264	0.0140	0.0092	0.0124	-0.0129	-0.0066	-0.0041	-0.0063	0.7947	0.26	
145	921+90.000	922+00.000	0.0019	0.039	0.076	0.0015	0.0008	0.0006	0.0007	0.0029	0.0016	0.0010	0.0014	-0.0014	-0.0007	-0.0005	-0.0007	0.7947	0.26	
146	922+00.000	922+59.000	0.0112	0.231	0.450	0.0089	0.0049	0.0033	0.0040	0.0173	0.0092	0.0060	0.0081	-0.0084	-0.0043	-0.0027	-0.0041	0.7947	0.26	
147	922+59.000	923+00.000	0.0078	0.160	0.313	0.0062	0.0034	0.0023	0.0028	0.0120	0.0064	0.0042	0.0056	-0.0059	-0.0030	-0.0019	-0.0029	0.7947	0.26	
148	923+00.000	941+70.000	0.3542	11.707	14.270	0.4503	0.2929	0.2000	0.1573	0.5488	0.2913	0.1902	0.2576	-0.0986	0.0017	0.0098	-0.1002	1.2714	0.42	
149	941+70.000	948+50.000	0.1288	2.661	5.189	0.1023	0.0563	0.0384	0.0460	0.1996	0.1059	0.0692	0.0937	-0.0972	-0.0496	-0.0307	-0.0476	0.7947	0.26	
All Segments			6.8845	404.014	364.690	15.5390	8.4321	5.7418	7.1069	14.0265	7.3373	4.6326	6.6892	1.5125	1.0948	1.1092	0.4176	2.2571	0.60	
All Intersections				207.530	423.640	7.9819	3.7433	2.1827	4.2387	16.2938	8.2091	4.3625	8.0847	-8.3119	-4.4659	-2.1798	-3.8461			0.25
Total			6.8845	611.544	788.330	23.5209	12.1754	7.9245	11.3456	30.3204	15.5464	8.9951	14.7740	-6.7994	-3.3710	-1.0706	-3.4284	3.4165		

Table 12. Expected Crash Frequencies and Rates by Horizontal Design Element (Section 3)

Title	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected FI no/C Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted FI no/C Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) FI no/C Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi /yr)	
Tangent	585+00.000	594+84.940	0.1865	12.915	11.070	0.4967	0.3578	0.2468	0.1390	0.4258	0.2184	0.1382	0.2074	0.0709	0.1394	0.1086	-0.0684	2.6628	0.61
Simple Curve 1	594+84.940	609+21.930	0.2722	36.520	19.110	1.4046	0.5861	0.4032	0.8185	0.7350	0.3769	0.2321	0.3581	0.6696	0.2092	0.1711	0.4604	5.1611	0.97
Tangent	609+21.930	624+64.300	0.2921	20.697	21.970	0.7960	0.3292	0.2271	0.4668	0.8450	0.4243	0.2633	0.4207	-0.0490	-0.0951	-0.0362	0.0461	2.7250	0.50
Simple Curve 2	624+64.300	624+64.530	0.0000	0.003	0.003	0.0001	0.0001	0.0000	0.0001	0.0001	0.0001	0.0000	0.0001	0.0000	-0.0000	-0.0000	0.0000	3.0350	0.56
Tangent	624+64.530	636+92.820	0.2326	9.543	17.496	0.3670	0.2038	0.1406	0.1632	0.6729	0.3379	0.2097	0.3350	-0.3059	-0.1341	-0.0691	-0.1718	1.5777	0.29
Simple Curve 3	636+92.820	647+26.050	0.1957	17.252	16.240	0.6635	0.2379	0.1621	0.4257	0.6246	0.3222	0.1928	0.3024	0.0389	-0.0844	-0.0307	0.1233	3.3907	0.62
Tangent	647+26.050	672+86.110	0.4849	37.690	37.458	1.4496	0.5640	0.3876	0.8857	1.4407	0.7292	0.4477	0.7114	0.0089	-0.1653	-0.0601	0.1742	2.9897	0.55
Simple Curve 4	672+86.110	683+82.710	0.2077	73.238	15.648	2.8169	1.9911	1.3396	0.8258	0.6019	0.3216	0.1899	0.2802	2.2150	1.6694	1.1497	0.5456	13.5629	2.53
Tangent	683+82.710	693+85.010	0.1898	5.134	9.377	0.1975	0.1100	0.0761	0.0875	0.3607	0.1883	0.1212	0.1723	-0.1632	-0.0784	-0.0451	-0.0848	1.0402	0.29
Simple Curve 5	693+85.010	710+47.850	0.3149	8.065	15.977	0.3102	0.1748	0.1177	0.1354	0.6145	0.3344	0.2088	0.2801	-0.3043	-0.1596	-0.0911	-0.1447	0.9849	0.30
Tangent	710+47.850	727+51.450	0.3227	19.279	12.636	0.7415	0.3343	0.2282	0.4072	0.4860	0.2585	0.1691	0.2275	0.2555	0.0758	0.0590	0.1798	2.2982	0.78
Simple Curve 6	727+51.450	727+52.350	0.0002	0.003	0.007	0.0001	0.0001	0.0000	0.0001	0.0003	0.0001	0.0001	0.0001	-0.0001	-0.0001	-0.0000	-0.0001	0.7724	0.26
Tangent	727+52.350	765+52.550	0.7197	14.661	28.827	0.5639	0.3116	0.2115	0.2523	1.1087	0.5947	0.3856	0.5140	-0.5448	-0.2832	-0.1742	-0.2617	0.7835	0.27
Simple Curve 7	765+52.550	780+45.930	0.2828	10.095	11.536	0.3883	0.1595	0.1080	0.2288	0.4437	0.2396	0.1542	0.2041	-0.0554	-0.0801	-0.0462	0.0247	1.3728	0.47
Tangent	780+45.930	948+50.000	3.1826	138.918	147.333	5.3430	3.0722	2.0932	2.2708	5.6667	2.9909	1.9197	2.6757	-0.3237	0.0812	0.1734	-0.4049	1.6788	0.50

Table 13. Predicted Crash Frequencies by Year (Section 3)

Year	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)
2025	16.61	8.54	51.412	5.36	32.278	8.07	48.588
2026	18.30	9.37	51.224	5.83	31.866	8.93	48.776
2027	20.01	10.22	51.059	6.30	31.488	9.79	48.941
2028	21.74	11.07	50.913	6.77	31.141	10.67	49.087
2029	23.49	11.93	50.784	7.24	30.818	11.56	49.216
2030	24.14	12.26	50.776	7.41	30.696	11.88	49.224
2031	24.80	12.59	50.771	7.58	30.578	12.21	49.229
2032	25.47	12.93	50.767	7.76	30.463	12.54	49.233
2033	26.13	13.27	50.765	7.93	30.352	12.87	49.235
2034	26.80	13.60	50.764	8.11	30.245	13.19	49.236
2035	27.47	13.94	50.765	8.28	30.141	13.52	49.235
2036	28.23	14.34	50.805	8.48	30.035	13.89	49.195
2037	29.03	14.77	50.865	8.69	29.930	14.26	49.135
2038	29.82	15.19	50.922	8.90	29.828	14.64	49.078
2039	30.62	15.61	50.978	9.10	29.731	15.01	49.022
2040	31.42	16.03	51.033	9.31	29.636	15.38	48.967
2041	32.75	16.75	51.158	9.65	29.478	15.99	48.842
2042	34.05	17.46	51.279	9.98	29.327	16.59	48.721
2043	35.32	18.15	51.394	10.31	29.185	17.17	48.606
2044	36.58	18.84	51.504	10.63	29.048	17.74	48.496
2045	37.83	19.52	51.609	10.94	28.918	18.31	48.391
2046	39.08	20.21	51.711	11.25	28.792	18.87	48.289
2047	40.31	20.89	51.808	11.56	28.672	19.43	48.192
2048	41.55	21.57	51.903	11.86	28.555	19.98	48.097
2049	42.78	22.24	51.993	12.17	28.443	20.54	48.007
2050	44.01	22.92	52.081	12.47	28.335	21.09	47.919
Total	788.33	404.21	51.274	233.87	29.667	384.12	48.726
Average	30.32	15.55	51.274	8.99	29.667	14.77	48.726

Table 14. Expected Crash Frequencies by Year (Section 3)

Year	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)
2025	12.89	6.69	51.904	4.72	36.657	6.20	48.099
2026	14.20	7.34	51.714	5.14	36.189	6.86	48.285
2027	15.52	8.00	51.547	5.55	35.760	7.52	48.449
2028	16.86	8.67	51.400	5.96	35.365	8.19	48.593
2029	18.22	9.34	51.269	6.38	34.999	8.88	48.721
2030	18.73	9.60	51.262	6.53	34.860	9.13	48.729
2031	19.24	9.86	51.256	6.68	34.726	9.38	48.734
2032	19.76	10.12	51.252	6.83	34.595	9.63	48.738
2033	20.27	10.39	51.250	6.99	34.469	9.88	48.740
2034	20.79	10.65	51.249	7.14	34.347	10.13	48.740
2035	21.31	10.92	51.250	7.29	34.229	10.39	48.740
2036	21.90	11.23	51.291	7.47	34.109	10.66	48.700
2037	22.52	11.56	51.351	7.65	33.990	10.95	48.641
2038	23.14	11.89	51.409	7.84	33.875	11.24	48.584
2039	23.75	12.22	51.466	8.02	33.764	11.53	48.528
2040	24.37	12.56	51.520	8.20	33.656	11.81	48.475
2041	25.40	13.12	51.647	8.50	33.476	12.28	48.351
2042	26.41	13.67	51.769	8.80	33.306	12.74	48.231
2043	27.40	14.22	51.885	9.08	33.144	13.18	48.117
2044	28.38	14.76	51.996	9.36	32.989	13.62	48.008
2045	29.35	15.29	52.103	9.64	32.840	14.06	47.904
2046	30.31	15.82	52.205	9.91	32.698	14.49	47.803
2047	31.27	16.36	52.304	10.18	32.561	14.92	47.707
2048	32.23	16.89	52.399	10.45	32.429	15.35	47.614
2049	33.19	17.42	52.490	10.72	32.302	15.77	47.524
2050	34.14	17.95	52.579	10.99	32.179	16.20	47.437
Total	611.54	316.56	51.764	206.04	33.691	294.99	48.236
Average	23.52	12.18	51.764	7.92	33.691	11.35	48.236

Table 15. Comparing Predicted and Expected Crashes for the Evaluation Period (Section 3)

Scope	Total Crashes	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)
Predicted	788.33	404.21	51.274	233.87	29.667	384.12	48.726
Expected	611.54	316.56	51.764	206.04	33.691	294.99	48.236
Expected - Predicted	-176.78	-87.65		-27.84		-89.14	
Percent Difference	-28.91	-27.69		-13.51		-30.22	

**Table 16. Expected Crash Severity by Ramp Terminal or Roundabout (Section 3)** 

Seg. No.	Туре	Fatal (K) Crashes (crashes)	Incapacitating Injury (A) Crashes (crashes)	Non-Incapacitating Injury (B) Crashes (crashes)	Possible Injury (C) Crashes (crashes)	No Injury (O) Crashes (crashes)
4	FRERampTerminal	0.0941	0.4938	1.6335	5.7300	7.7191

 Table 17. Expected
 Crash Type Distribution (Section 3)

Element Type	Crash Type	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)	Total Crashes	Percent Total (%)
Highway Segment	Single	150.43	24.6	110.75	18.1	143.73	23.5	298.08	48.8
Highway Segment	Total Single Vehicle Crashes	150.43	24.6	110.75	18.1	143.73	23.5	298.08	48.8
Highway Segment	Angle Collision	16.09	2.6	10.16	1.7	9.07	1.5	24.58	4.0
Highway Segment	Head-on Collision	3.14	0.5	2.97	0.5	0.36	0.1	2.49	0.4
Highway Segment	Rear-end Collision	38.33	6.3	18.19	3.0	16.88	2.8	49.86	8.2
Highway Segment	Sideswipe	6.30	1.0	3.54	0.6	10.11	1.7	18.64	3.0
Highway Segment	Total Multiple Vehicle Crashes	63.87	10.4	34.86	5.7	36.42	6.0	95.56	15.6
Highway Segment	Total Highway Segment Crashes	219.24	35.9	149.29	24.4	184.78	30.2	404.01	66.1
Highway Segment	Other Collision	4.93	0.8	3.67	0.6	4.62	0.8	10.36	1.7
Intersection	Single	13.98	2.3	11.95	2.0	24.92	4.1	39.47	6.5
Intersection	Total Single Vehicle Crashes	13.98	2.3	11.95	2.0	24.92	4.1	39.47	6.5
Intersection	Angle Collision	45.97	7.5	30.94	5.1	28.84	4.7	72.86	11.9
Intersection	Head-on Collision	1.87	0.3	1.53	0.2	1.59	0.3	3.36	0.5
Intersection	Rear-end Collision	19.40	3.2	6.39	1.0	25.46	4.2	45.10	7.4
Intersection	Sideswipe	3.92	0.6	2.40	0.4	16.14	2.6	21.11	3.5
Intersection	Total Multiple Vehicle Crashes	71.17	11.6	41.26	6.7	72.04	11.8	142.42	23.3
Intersection	Total Intersection Crashes	89.45	14.6	56.75	9.3	102.46	16.8	191.69	31.4
Intersection	Other Collision	4.30	0.7	3.54	0.6	5.51	0.9	9.81	1.6
Ramp Terminal	Collision with Animal	0.00	0.0	0.00	0.0	0.00	0.0	0.00	0.0
Ramp Terminal	Collision with Fixed Object	0.62	0.1	0.00	0.0	1.22	0.2	1.84	0.3
Ramp Terminal	Collision with Other Object	0.00	0.0	0.00	0.0	0.04	0.0	0.04	0.0
Ramp Terminal	Other Single-vehicle Collision	0.52	0.1	0.00	0.0	0.20	0.0	0.72	0.1
Ramp Terminal	Collision with Parked Vehicle	0.06	0.0	0.00	0.0	0.12	0.0	0.17	0.0
Ramp Terminal	Total Single Vehicle Crashes	1.19	0.2	0.00	0.0	1.57	0.3	2.77	0.5
Ramp Terminal	Angle Collision	4.15	0.7	0.00	0.0	2.87	0.5	7.02	1.1

Element Type	Crash Type	FI Crashes	Percent FI (%)	FI/no C Crashes	Percent FI/no C (%)	PDO Crashes	Percent PDO (%)	Total Crashes	Percent Total (%)
Ramp Terminal	Head-on Collision	0.16	0.0	0.00	0.0	0.12	0.0	0.28	0.0
Ramp Terminal	Other Multiple-vehicle Collision	0.10	0.0	0.00	0.0	0.20	0.0	0.30	0.1
Ramp Terminal	Rear-end Collision	2.19	0.4	0.00	0.0	2.13	0.3	4.32	0.7
Ramp Terminal	Sideswipe, Same Direction Collision	0.16	0.0	0.00	0.0	0.83	0.1	0.98	0.2
Ramp Terminal	Total Multiple Vehicle Crashes	6.76	1.1	0.00	0.0	6.14	1.0	12.90	2.1
Ramp Terminal	Total Ramp Terminal Crashes	7.95	1.3	0.00	0.0	7.72	1.3	15.67	2.6
	Total Crashes	316.64	51.8	206.04	33.7	294.96	48.2	611.37	100.0

**Table 18. Evaluation Message** 

Start Location (Sta. ft)	End Location (Sta. ft)	Message
605+40.000	605+60.000	Warning: for segment #11 (605+40.000 to 605+60.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+40.000	605+60.000	Warning: for segment #11 (605+40.000 to 605+60.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+60.000	605+70.000	Warning: for segment #12 (605+60.000 to 605+70.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+60.000	605+70.000	Warning: for segment #12 (605+60.000 to 605+70.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+70.000	605+75.000	Warning: for segment #13 (605+70.000 to 605+75.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+70.000	605+75.000	Warning: for segment #13 (605+70.000 to 605+75.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+75.000	606+00.000	Warning: for segment #14 (605+75.000 to 606+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+75.000	606+00.000	Warning: for segment #14 (605+75.000 to 606+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
639+00.000	640+00.000	Warning: for segment #29 (639+00.000 to 640+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
639+00.000	640+00.000	Warning: for segment #29 (639+00.000 to 640+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
655+70.000	656+50.000	Warning: for segment #42 (655+70.000 to 656+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
655+70.000	656+50.000	Warning: for segment #42 (655+70.000 to 656+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
676+30.000	677+50.000	Warning: for segment #52 (676+30.000 to 677+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
676+30.000	677+50.000	Warning: for segment #52 (676+30.000 to 677+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
681+00.000	682+20.000	Warning: for segment #57 (681+00.000 to 682+20.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
681+00.000	682+20.000	Warning: for segment #57 (681+00.000 to 682+20.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
699+20.000	700+40.000	Warning: for segment #67 (699+20.000 to 700+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
699+20.000	700+40.000	Warning: for segment #67 (699+20.000 to 700+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
709+00.000	710+30.000	Warning: for segment #76 (709+00.000 to 710+30.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
709+00.000	710+30.000	Warning: for segment #76 (709+00.000 to 710+30.000), no foreslope data available for right side of road for use by AFM3ru, using 1.0
755+50.000	756+90.000	Warning: for segment #82 (755+50.000 to 756+90.000), no foreslope data available for left side of road for use by AFM3ru, using 1.0
755+50.000	756+90.000	Warning: for segment #82 (755+50.000 to 756+90.000), no foreslope data available for right side of road for use by AFM3ru, using 1.0
777+80.000	778+80.000	Warning: for segment #89 (777+80.000 to 778+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
777+80.000	778+80.000	Warning: for segment #89 (777+80.000 to 778+80.000), no foreslope data available for right side of road for use by AFM3ru, using 1.0
808+80.000	809+00.000	Warning: for segment #102 (808+80.000 to 809+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
808+80.000	809+00.000	Warning: for segment #102 (808+80.000 to 809+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
809+00.000	809+60.000	Warning: for segment #103 (809+00.000 to 809+60.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
809+00.000	809+60.000	Warning: for segment #103 (809+00.000 to 809+60.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
861+85.000	862+00.000	Warning: for segment #115 (861+85.000 to 862+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
861+85.000	862+00.000	Warning: for segment #115 (861+85.000 to 862+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0

Start Location (Sta. ft)	End Location (Sta. ft)	Message
862+00.000	862+50.000	Warning: for segment #116 (862+00.000 to 862+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
862+00.000	862+50.000	Warning: for segment #116 (862+00.000 to 862+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
862+50.000	862+60.000	Warning: for segment #117 (862+50.000 to 862+60.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
862+50.000	862+60.000	Warning: for segment #117 (862+50.000 to 862+60.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
888+20.000	889+30.000	Warning: for segment #127 (888+20.000 to 889+30.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
888+20.000	889+30.000	Warning: for segment #127 (888+20.000 to 889+30.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
914+30.000	914+40.000	Warning: for segment #140 (914+30.000 to 914+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
914+30.000	914+40.000	Warning: for segment #140 (914+30.000 to 914+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
914+40.000	915+40.000	Warning: for segment #141 (914+40.000 to 915+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
914+40.000	915+40.000	Warning: for segment #141 (914+40.000 to 915+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+40.000	605+60.000	Warning: for segment #11 (605+40.000 to 605+60.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+40.000	605+60.000	Warning: for segment #11 (605+40.000 to 605+60.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+60.000	605+70.000	Warning: for segment #12 (605+60.000 to 605+70.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+60.000	605+70.000	Warning: for segment #12 (605+60.000 to 605+70.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+70.000	605+75.000	Warning: for segment #13 (605+70.000 to 605+75.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+70.000	605+75.000	Warning: for segment #13 (605+70.000 to 605+75.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
605+75.000	606+00.000	Warning: for segment #14 (605+75.000 to 606+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
605+75.000	606+00.000	Warning: for segment #14 (605+75.000 to 606+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
639+00.000	640+00.000	Warning: for segment #29 (639+00.000 to 640+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
639+00.000	640+00.000	Warning: for segment #29 (639+00.000 to 640+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
655+70.000	656+50.000	Warning: for segment #42 (655+70.000 to 656+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
655+70.000	656+50.000	Warning: for segment #42 (655+70.000 to 656+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
676+30.000	677+50.000	Warning: for segment #52 (676+30.000 to 677+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
676+30.000	677+50.000	Warning: for segment #52 (676+30.000 to 677+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
681+00.000	682+20.000	Warning: for segment #57 (681+00.000 to 682+20.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
681+00.000	682+20.000	Warning: for segment #57 (681+00.000 to 682+20.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
699+20.000	700+40.000	Warning: for segment #67 (699+20.000 to 700+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
699+20.000	700+40.000	Warning: for segment #67 (699+20.000 to 700+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
709+00.000	710+30.000	Warning: for segment #76 (709+00.000 to 710+30.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
709+00.000	710+30.000	Warning: for segment #76 (709+00.000 to 710+30.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
755+50.000	756+90.000	Warning: for segment #82 (755+50.000 to 756+90.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
755+50.000	756+90.000	Warning: for segment #82 (755+50.000 to 756+90.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0

Start Location (Sta. ft)	End Location (Sta. ft)	Message
777+80.000	778+80.000	Warning: for segment #89 (777+80.000 to 778+80.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
777+80.000	778+80.000	Warning: for segment #89 (777+80.000 to 778+80.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
808+80.000	809+00.000	Warning: for segment #102 (808+80.000 to 809+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
808+80.000	809+00.000	Warning: for segment #102 (808+80.000 to 809+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
809+00.000	809+60.000	Warning: for segment #103 (809+00.000 to 809+60.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
809+00.000	809+60.000	Warning: for segment #103 (809+00.000 to 809+60.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
861+85.000	862+00.000	Warning: for segment #115 (861+85.000 to 862+00.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
861+85.000	862+00.000	Warning: for segment #115 (861+85.000 to 862+00.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
862+00.000	862+50.000	Warning: for segment #116 (862+00.000 to 862+50.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
862+00.000	862+50.000	Warning: for segment #116 (862+00.000 to 862+50.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
862+50.000	862+60.000	Warning: for segment #117 (862+50.000 to 862+60.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
862+50.000	862+60.000	Warning: for segment #117 (862+50.000 to 862+60.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
888+20.000	889+30.000	Warning: for segment #127 (888+20.000 to 889+30.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
888+20.000	889+30.000	Warning: for segment #127 (888+20.000 to 889+30.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
914+30.000	914+40.000	Warning: for segment #140 (914+30.000 to 914+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
914+30.000	914+40.000	Warning: for segment #140 (914+30.000 to 914+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0
914+40.000	915+40.000	Warning: for segment #141 (914+40.000 to 915+40.000 ), no foreslope data available for left side of road for use by AFM3ru, using 1.0
914+40.000	915+40.000	Warning: for segment #141 (914+40.000 to 915+40.000 ), no foreslope data available for right side of road for use by AFM3ru, using 1.0

#### **Section 4 Evaluation**

**Section:** Section 4

**Evaluation Start Location:** 948+50.000 **Evaluation End Location:** 974+11.000

Area Type: Urban

Functional Class: Arterial

**Type of Alignment:** Undivided, Multilane **Model Category:** Urban/Suburban Arterial **Calibration Factor:** 4D=1.0; 4SG=1.0; 4U=1.0;

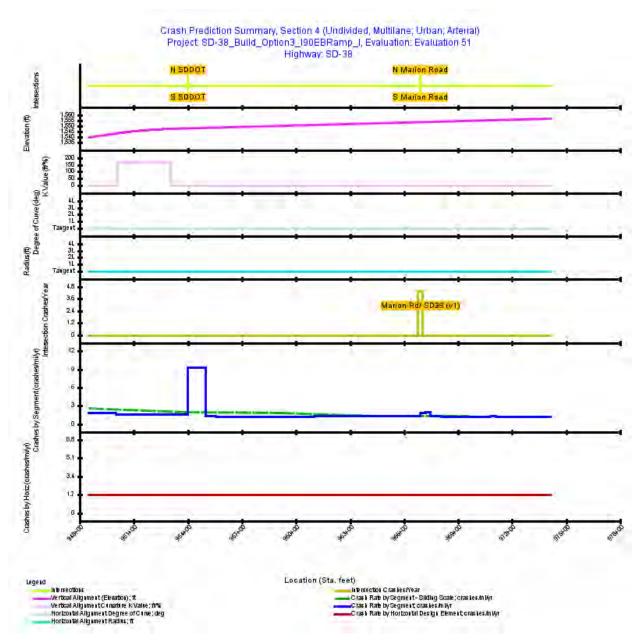


Figure 2. Crash Prediction Summary (Section 4)

**Table 19. Observed Crashes Used in the Evaluation (Section 4)** 

Year	Observed Crashes	Total Crashes Used	FI Crashes	FI no/C Crashes	PDO Crashes
2018	5	5	4	0	1
2019	1	1	0	0	1
2020	2	2	0	0	2
2021	2	2	2	0	0
2022	2	2	0	0	2
All Years	12 <sup>[1]</sup>	12	6	0	6

#### **Footnotes**

<sup>[1]</sup> Note: Observed crash data that does not comply with the associated CPM model requirements may not be used in EB processing.

 Table 20. Evaluation Highway - Homogeneous Segments (Section 4)

Se g. N o.	Туре	Start Locati on (Sta. ft)	End Locati on (Sta. ft)	Len gth (ft)	Len gth (mi)	AADT	Number Major Commer icial Drivewa ys	Number Minor Commer icial Drivewa ys	Number Major Industial/I nstitutiona I	Number Minor Industial/I nstitutiona I	Number Major Resident ial Drivewa ys	Number Minor Resident ial Drivewa ys	Numbe r Other Drivew ays	Lighti ng	Automat ed Speed Enforce ment	Dens ity (fixe d objec ts/mi )	Me dian Wid th (ft)	Туре	Effecti ve Media n Width (ft)	Speed Level	Numbe r Rail Highw ay Crossi ngs	Avera ge Shoul der Width (ft)	Ave age Lan
15 0	Urban/Suburban Arterial Segment Four-lane Undivided	948+5 0.000	950+0 0.000				0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediat e/High	0	8.00	12.0
15 1	Urban/Suburban Arterial Segment Four-lane Undivided	950+0 0.000	954+0 0.000	400.		2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediat e/High	0	8.00	12.0
15 2	Urban/Suburban Arterial Segment Four-lane Undivided	954+0 0.000		100.			0	0	2	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediat e/High	0	4.00	12.0
15	Urban/Suburban Arterial Segment Four-lane Divided	955+0 0.000	955+5 5.000	55.0	0.01 04		0	0	0	0	0	0	0	false	false	0.0	4.01	Non- Traversable Median	4.01	Intermediat e/High	0	8.00	12.0
15 4	Urban/Suburban Arterial Segment Four-lane Divided	955+5 5.000	958+2 8.000			2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0		Non- Traversable Median	10.02	Intermediat e/High	0	8.00	12.0
15 5	Urban/Suburban Arterial Segment Four-lane Divided	958+2 8.000	961+0 1.000	273.			0	0	0	0	0	0	0	false	false	0.0		Non- Traversable Median	19.99	Intermediat e/High	0	8.00	12.0
15 6	Urban/Suburban Arterial Segment Four-lane Divided	961+0 1.000			0.01 87		0	0	0	0	0	0	0	false	false	0.0		Traversable Median	23.12	Intermediat e/High	0	8.00	12.0
15 7	Urban/Suburban Arterial Segment Four-lane Divided	962+0 0.000	963+6 9.000	i 169.	0.03 20		0	0	0	0	0	0	0	false	false	0.0		Traversable Median	30.13	Intermediat e/High	0	8.00	12.0
15 8	Urban/Suburban Arterial Segment Four-lane Divided	963+6 9.000	965+0 0.000				0	0	0	0	0	0	0	false	false	0.0		Traversable Median	24.55	Intermediat e/High	0	8.00	12.0
15 9	Urban/Suburban Arterial Segment Four-lane Divided	965+0 0.000	966+3 8.000	138.	0.02 61		0	0	0	0	0	0	0	false	false	0.0	7.54	Traversable Median	19.54	Intermediat e/High	0	4.00	12.0
16 0	Urban/Suburban Arterial Segment Four-lane Divided	966+3 8.000	966+7 0.000	32.0	0.00		0	0	0	0	0	0	0	false	false	0.0	4.38	Traversable Median	16.38	Intermediat e/High	0	4.00	12.0

Se g. N o.	Туре	Start Locati on (Sta. ft)	End Locati on (Sta. ft)	Len gth (ft)	gtl	th	AADT	Number Major Commer icial Drivewa ys	Number Minor Commer icial Drivewa ys	Number Major Industial/I nstitutiona I	Number Minor Industial/I nstitutiona I	Number Major Resident ial Drivewa ys	Number Minor Resident ial Drivewa ys	Numbe r Other Drivew ays	Lighti ng	Automat ed Speed Enforce ment	Dens ity (fixe d objec ts/mi	Me dian Wid th (ft)	Туре	Effecti ve Media n Width (ft)	Speed Level	Numbe r Rail Highw ay Crossi ngs	Avera ge Shoul der Width (ft)	Aver age Lane Widt h (ft)
16 1	Urban/Suburban Arterial Segment Four-lane Divided	966+7 0.000	966+9 1.000			40	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,441; 2043: 8,066; 2044: 9,085; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	3.39	Traversable Median	15.39	Intermediat e/High	0	0.00	12.00
16 2	Urban/Suburban Arterial Segment Four-lane Undivided	966+9 1.000	967+1 4.000			44	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	true	false	0.0	0.00	None	0.00	Intermediat e/High	0	0.00	12.00
16	Urban/Suburban Arterial Segment Four-lane Undivided	967+1 4.000	967+2 0.000	6.00	0.0	11	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	true	false	0.0	0.00	None	0.00	Intermediat e/High	0	8.00	12.00
16 4	Urban/Suburban Arterial Segment Four-lane Undivided	967+2 0.000				47	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediat e/High	0	8.00	12.00
16 5	Urban/Suburban Arterial Segment Four-lane Divided	967+4 5.000	968+0 6.000	61.0		.01 16	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,441; 2043: 8,066; 2044: 9,085; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	4.01	Non- Traversable Median	18.01	Intermediat e/High	0	8.00	12.00
16 6	Urban/Suburban Arterial Segment Four-lane Divided	968+0 6.000	970+7 9.000			17	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,441; 2043: 8,066; 2044: 9,082; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	9.52	Non- Traversable Median	23.52	Intermediat e/High	0	8.00	12.00
16	Urban/Suburban Arterial Segment Four-lane Divided	970+7 9.000				.00 57	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,441; 2043: 8,066; 2044: 9,085; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	14.5	Non- Traversable Median	14.53	Intermediat e/High	0	8.00	12.00
16 8	Urban/Suburban Arterial Segment Four-lane Divided	971+0 9.000				.05 72	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	0	0	0	0	0	0	0	false	false	0.0	20.0	Non- Traversable Median	20.01	Intermediat e/High	0	8.00	12.00

 Table 21. Crash Highway Highway - Homogeneous Segments (Section 4)

Se g. No	Туре	Start Locatio n (Sta. ft)	End Locatio n (Sta. ft)	Leng th (ft)	Leng th (mi)	AADT	Number Major Commerici al Driveways	al	Number Major Industial/Inst itutional	Number Minor Industial/Inst itutional	Number Major Residential Driveways	Number Minor Residential Driveways	Number Other Drivewa ys	Lightin g	Automated Speed Enforceme nt	Densit y (fixed object s/mi)	Medi an Widt h (ft)	Туре	Effective Median Width (ft)	Speed Level	y	Averag e Should er Width (ft)	Avera ge Lane Width (ft)
15 0	Urban/Suburban Arterial Segment Four-lane Undivided	948+50. 000	950+00. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediate/ High	0	8.00	12.00
15 1	Urban/Suburban Arterial Segment Four-lane Undivided	950+00. 000	954+00. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediate/ High	0	8.00	12.00
15 2	Urban/Suburban Arterial Segment Four-lane Undivided	954+00. 000	955+00. 000			2018-2022: 4,900	0	0	2	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediate/ High	0	4.00	12.00
15 3	Urban/Suburban Arterial Segment Four-lane Divided	955+00. 000	955+55. 000	55.00		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	4.01	Non-Traversable Median	4.01	Intermediate/ High	0	8.00	12.00
15 4	Urban/Suburban Arterial Segment Four-lane Divided	955+55. 000	958+28. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	10.02	Non-Traversable Median	10.02	Intermediate/ High	0	8.00	12.00
15 5	Urban/Suburban Arterial Segment Four-lane Divided	958+28. 000	961+01. 000	273.0 0		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	20.03	Non-Traversable Median	19.99	Intermediate/ High	0	8.00	12.00
15 6	Urban/Suburban Arterial Segment Four-lane Divided	961+01. 000	962+00. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	23.12	Traversable Median	23.12	Intermediate/ High	0	8.00	12.00
15 7	Urban/Suburban Arterial Segment Four-lane Divided	962+00. 000	963+69. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	18.13	Traversable Median	30.13	Intermediate/ High	0	8.00	12.00
15 8	Urban/Suburban Arterial Segment Four-lane Divided	963+69. 000	965+00. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	12.55	Traversable Median	24.55	Intermediate/ High	0	8.00	12.00
15 9	Urban/Suburban Arterial Segment Four-lane Divided	965+00. 000	966+38. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	7.54	Traversable Median	19.54	Intermediate/ High	0	4.00	12.00
16 0	Urban/Suburban Arterial Segment Four-lane Divided	966+38. 000	966+70. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	4.38	Traversable Median	16.38	Intermediate/ High	0	4.00	12.00
16 1	Urban/Suburban Arterial Segment Four-lane Divided	966+70. 000	966+91. 000	21.00		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	3.39	Traversable Median	15.39	Intermediate/ High	0	0.00	12.00
16 2	Urban/Suburban Arterial Segment Four-lane Undivided	966+91. 000	967+14. 000			2018-2022: 4,900	0	0	0	0	0	0	0	true	false	0.0	0.00	None	0.00	Intermediate/ High	0	0.00	12.00
16 3	Urban/Suburban Arterial Segment Four-lane Undivided	967+14. 000	967+20. 000			2018-2022: 4,900	0	0	0	0	0	0	0	true	false	0.0	0.00	None	0.00	Intermediate/ High	0	8.00	12.00
16 4	Urban/Suburban Arterial Segment Four-lane Undivided	967+20. 000	967+45. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	0.00	None	0.00	Intermediate/ High	0	8.00	12.00
16 5	Urban/Suburban Arterial Segment Four-lane Divided	967+45. 000	968+06. 000	61.00		2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	4.01	Non-Traversable Median	18.01	Intermediate/ High	0	8.00	12.00
16 6	Urban/Suburban Arterial Segment Four-lane Divided	968+06. 000	970+79. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	9.52	Non-Traversable Median	23.52	Intermediate/ High	0	8.00	12.00
16 7	Urban/Suburban Arterial Segment Four-lane Divided	970+79. 000	971+09. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	14.53	Non-Traversable Median	14.53	Intermediate/ High	0	8.00	12.00
16 8	Urban/Suburban Arterial Segment Four-lane Divided	971+09. 000	974+11. 000			2018-2022: 4,900	0	0	0	0	0	0	0	false	false	0.0	20.01	Non-Traversable Median	20.01	Intermediate/ High	0	8.00	12.00

## **Table 22. Evaluation Intersection (Section 4)**

I	nter. No.	Title	Туре	Location (Sta. ft)	Major AADT	Minor AADT	Le gs	Traffic Control	Approache s w/Left Turn Lanes	Approache s w/Right Turn Lanes	Right	Pedestrian Volume (crossings/ day)	d at	Light	l Near	Num ber of Bus Stops	Number of	Max Lanes Crosse d	
	8	Marion Rd/ SD38 (v1)	Urban/Suburban Arterial Intersection Four-Legged Signalized	966+91. 000	6,255; 2030: 6,660; 2031: 7,065; 2032: 7,470; 2035: 7,875; 2034: 8,280; 2035: 8,685; 2036: 9,900; 2037: 9,495; 2038: 9,900; 2039: 10,305; 2040: 10,710; 2041: 11,861; 2042: 13,012; 2043: 14,163; 2044: 15,314; 2045: 16,465; 2046: 17,616; 2047: 18,767; 2048: 19,182; 2049: 21,696; 2050: 2050	2025: 5,988; 2026: 6,351; 2027: 6,714; 2028: 7,077; 2029: 7,440; 2030: 7,545; 2031: 7,650; 2032: 7,756; 2033: 7,861; 2034: 7,967; 2035: 8,072; 2036: 8,178; 2037: 8,283; 2038: 8,389; 2039: 8,494; 2040: 8,600; 2041: 8,722; 2042: 8,844; 2043: 8,966; 2044: 9,088; 2045: 9,210; 2046: 9,332; 2047: 9,454; 2048: 9,576; 2049: 9,698; 2050: 9,820	4	Signalized	4	3	0	20	false	false	false	0	0	6	

## **Table 23. Crash History Intersection (Section 4)**

ter. No.	Title	Type	Location (Sta. ft)	Major AADT	Minor AADT	Legs	Traffic Control	Approaches w/Left Turn Lanes		Approaches w/o Right Turn on Red	Pedestrian Volume (crossings/day	Lighted at Night	Red Light Camer a	School	c D	Number of Alcohol Sales Establishments	Max Lanes Crossed
8	Marion Rd/ SD38 (v1)	Urban/Suburban Arterial Intersection Four-Legged Signalized	966+91.000	2018-2022: 5,400	2018-2022: 4,900	4	Signalized	4	3	0	20	false	false	false	0	0	6

Table 24. Expected Highway Crash Rates and Frequencies Summary (Section 4)

First Year of Analysis	2025
Last Year of Analysis	2050
Evaluated Length (mi)	0.4850
Average Future Road AADT (vpd)	8,272
Expected Crashes	
Total Crashes	135.85
Fatal and Injury Crashes	46.51
Property-Damage-Only Crashes	89.34
Percent of Total Expected Crashes	
Percent Fatal and Injury Crashes (%)	34
Percent Property-Damage-Only Crashes (%)	66
Expected Crash Rate	
Crash Rate (crashes/mi/yr)	10.7725
FI Crash Rate (crashes/mi/yr)	3.6878
PDO Crash Rate (crashes/mi/yr)	7.0847
Expected Travel Crash Rate	
Total Travel (million veh-mi)	38.08
Travel Crash Rate (crashes/million veh-mi)	3.57
Travel FI Crash Rate (crashes/million veh-mi)	1.22
Travel PDO Crash Rate (crashes/million veh-mi)	2.35

**Table 25. Expected Crash Frequencies and Rates by Highway Segment/Intersection (Section 4)** 

Segment Number/Intersection Name/Cross Road	Start Location (Sta. ft)	End Location (Sta. ft)	Length (mi)	Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	(Expected - Predicted) PDO Crash Frequency (crashes/yr)	Expected Crash Rate (crashes/mi/ yr)	Expected Travel Crash Rate (crashes/mil lion veh-mi)	Expected Intersection Travel Crash Rate (crashes/million veh)
150	948+50.000	950+00.000	0.0284	1.343	1.459	0.0517	0.0179	0.0337	0.0561	0.0181	0.0380	-0.0044	-0.0001	-0.0043	1.8184	0.60	
151	950+00.000	954+00.000	0.0758	3.172	3.889	0.1220	0.0437	0.0783	0.1496	0.0481	0.1014	-0.0276	-0.0044	-0.0232	1.6103	0.53	
152	954+00.000	955+00.000	0.0189	4.560	6.162	0.1754	0.0651	0.1103	0.2370	0.0817	0.1553	-0.0616	-0.0167	-0.0449	9.2608	3.07	
153	955+00.000	955+55.000	0.0104	0.373	0.382	0.0143	0.0039	0.0104	0.0147	0.0039	0.0108	-0.0004	-0.0000	-0.0003	1.3757	0.46	
154	955+55.000	958+28.000	0.0517	1.685	1.897	0.0648	0.0182	0.0466	0.0730	0.0195	0.0535	-0.0082	-0.0013	-0.0069	1.2532	0.42	
155	958+28.000	961+01.000	0.0517	1.685	1.897	0.0648	0.0182	0.0466	0.0730	0.0195	0.0535	-0.0082	-0.0013	-0.0069	1.2532	0.42	
156	961+01.000	962+00.000	0.0187	0.651	0.681	0.0251	0.0069	0.0182	0.0262	0.0070	0.0192	-0.0011	-0.0001	-0.0010	1.3360	0.44	
157	962+00.000	963+69.000	0.0320	1.069	1.151	0.0411	0.0114	0.0297	0.0443	0.0118	0.0325	-0.0032	-0.0004	-0.0027	1.2842	0.42	
158	963+69.000	965+00.000	0.0248	0.850	0.901	0.0327	0.0090	0.0237	0.0347	0.0092	0.0254	-0.0020	-0.0002	-0.0017	1.3176	0.44	
159	965+00.000	966+38.000	0.0261	0.893	0.949	0.0343	0.0095	0.0249	0.0365	0.0097	0.0268	-0.0022	-0.0003	-0.0019	1.3136	0.43	
160	966+38.000	966+70.000	0.0061	0.217	0.220	0.0083	0.0023	0.0061	0.0085	0.0023	0.0062	-0.0001	0.0000	-0.0001	1.3765	0.46	
161	966+70.000	966+91.000	0.0040	0.143	0.144	0.0055	0.0015	0.0040	0.0056	0.0015	0.0041	-0.0001	0.0000	-0.0001	1.3834	0.46	
Marion Rd/ SD38 (v1)	966+91.000			114.572	49.969	4.4066	1.5303	2.8763	1.9219	0.6310	1.2909	2.4847	0.8993	1.5854			0.65
162	966+91.000	967+14.000	0.0044	0.203	0.205	0.0078	0.0027	0.0051	0.0079	0.0025	0.0054	-0.0001	0.0001	-0.0002	1.7894	0.59	
163	967+14.000	967+20.000	0.0011	0.053	0.053	0.0021	0.0007	0.0014	0.0021	0.0007	0.0014	-0.0000	0.0000	-0.0000	1.8054	0.60	
164	967+20.000	967+45.000	0.0047	0.240	0.243	0.0092	0.0031	0.0061	0.0093	0.0030	0.0063	-0.0001	0.0001	-0.0003	1.9466	0.65	
165	967+45.000	968+06.000	0.0116	0.412	0.424	0.0159	0.0043	0.0115	0.0163	0.0043	0.0120	-0.0005	-0.0000	-0.0004	1.3720	0.45	
166	968+06.000	970+79.000	0.0517	1.685	1.897	0.0648	0.0182	0.0466	0.0730	0.0195	0.0535	-0.0082	-0.0013	-0.0069	1.2532	0.42	
167	970+79.000	971+09.000	0.0057	0.206	0.208	0.0079	0.0021	0.0058	0.0080	0.0021	0.0059	-0.0001	0.0000	-0.0001	1.3915	0.46	
168	971+09.000	974+11.000	0.0572	1.842	2.098	0.0708	0.0199	0.0509	0.0807	0.0215	0.0592	-0.0099	-0.0016	-0.0083	1.2387	0.41	
All Segments			0.4850	21.280	24.862	0.8185	0.2585	0.5600	0.9562	0.2860	0.6702	-0.1378	-0.0275	-0.1102	1.6874	0.56	
All Intersections				114.572	49.969	4.4066	1.5303	2.8763	1.9219	0.6310	1.2909	2.4847	0.8993	1.5854			0.65
Total			0.4850	135.852	74.831	5.2251	1.7887	3.4363	2.8781	0.9170	1.9611	2.3470	0.8718	1.4752	10.7725		

## Table 26. Expected Crash Frequencies and Rates by Horizontal Design Element (Section 4)

Title	Start Location (Sta. ft)	End Location (Sta. ft)		Total Expected Crashes for Evaluation Period	Total Predicted Crashes for Evaluation Period	Expected Total Crash Frequency (crashes/yr)	Expected FI Crash Frequency (crashes/yr)	Expected PDO Crash Frequency (crashes/yr)	Predicted Total Crash Frequency (crashes/yr)	Predicted FI Crash Frequency (crashes/yr)	Predicted PDO Crash Frequency (crashes/yr)	(Expected - Predicted) Total Crash Frequency (crashes/yr)	(Expected - Predicted) FI Crash Frequency (crashes/yr)	PDO Crach		Expected Travel Crash Rate (crashes/mi Ilion veh- mi)
Tangent	948+50.000	974+11.000	0.4850	21.280	24.862	0.8185	0.2585	0.5600	0.9562	0.2860	0.6702	-0.1378	-0.0275	-0.1102	1.6874	0.56

Table 27. Predicted Crash Frequencies by Year (Section 4)

Year	Total Crashes	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
2025	1.54	0.47	30.768	1.07	69.232
2026	1.62	0.50	30.791	1.12	69.209
2027	1.69	0.52	30.812	1.17	69.188
2028	1.77	0.55	30.830	1.22	69.170
2029	1.85	0.57	30.847	1.28	69.153
2030	1.93	0.60	30.902	1.33	69.098
2031	2.01	0.62	30.960	1.39	69.040
2032	2.10	0.65	31.018	1.45	68.982
2033	2.18	0.68	31.078	1.50	68.922
2034	2.27	0.71	31.139	1.56	68.862
2035	2.35	0.73	31.199	1.62	68.801
2036	2.44	0.76	31.260	1.68	68.740
2037	2.53	0.79	31.321	1.74	68.679
2038	2.61	0.82	31.381	1.79	68.618
2039	2.70	0.85	31.442	1.85	68.558
2040	2.79	0.88	31.502	1.91	68.498
2041	3.01	0.95	31.680	2.06	68.320
2042	3.24	1.03	31.854	2.21	68.146
2043	3.46	1.11	32.026	2.35	67.975
2044	3.69	1.19	32.192	2.50	67.808
2045	3.92	1.27	32.354	2.65	67.646
2046	4.15	1.35	32.511	2.80	67.489
2047	4.39	1.43	32.664	2.95	67.336
2048	4.62	1.52	32.811	3.11	67.189
2049	4.86	1.60	32.954	3.26	67.046
2050	5.10	1.69	33.093	3.41	66.907
Total	74.83	23.84	31.860	50.99	68.140
Average	2.88	0.92	31.860	1.96	68.140

Table 28. Expected Crash Frequencies by Year (Section 4)

Year	Total Crashes	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
2025	2.80	0.93	33.060	1.87	66.820
2026	2.94	0.97	33.085	1.96	66.798
2027	3.07	1.02	33.107	2.05	66.778
2028	3.21	1.06	33.127	2.15	66.760
2029	3.35	1.11	33.145	2.24	66.744
2030	3.50	1.16	33.205	2.34	66.691
2031	3.65	1.22	33.266	2.44	66.635
2032	3.81	1.27	33.329	2.54	66.579
2033	3.96	1.32	33.394	2.63	66.521
2034	4.12	1.38	33.458	2.74	66.463
2035	4.27	1.43	33.524	2.84	66.404
2036	4.43	1.49	33.589	2.94	66.346
2037	4.59	1.54	33.654	3.04	66.287
2038	4.75	1.60	33.719	3.14	66.228
2039	4.91	1.66	33.784	3.25	66.170
2040	5.07	1.72	33.849	3.35	66.112
2041	5.47	1.86	34.040	3.61	65.941
2042	5.88	2.01	34.228	3.86	65.772
2043	6.29	2.16	34.411	4.12	65.607
2044	6.70	2.32	34.590	4.38	65.446
2045	7.12	2.47	34.765	4.65	65.289
2046	7.54	2.63	34.934	4.91	65.138
2047	7.96	2.79	35.097	5.18	64.991
2048	8.39	2.96	35.256	5.44	64.848
2049	8.82	3.12	35.409	5.71	64.710
2050	9.26	3.29	35.558	5.98	64.577
Total	135.85	46.51	34.234	89.34	65.766
Average	5.22	1.79	34.234	3.44	65.766

Table 29. Comparing Predicted and Expected Crashes for the Evaluation Period (Section 4)

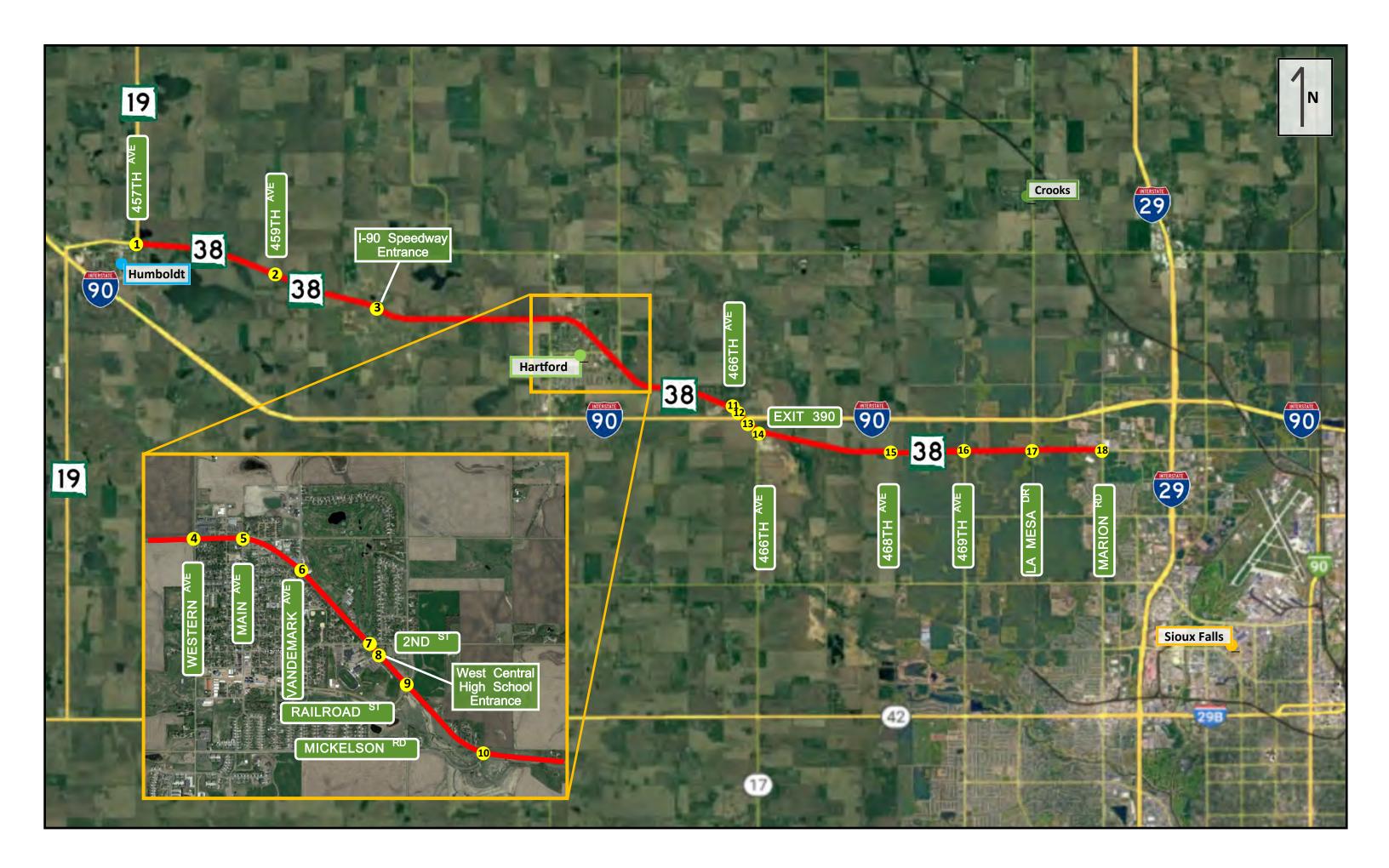
Scope	<b>Total Crashes</b>	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)
Predicted	74.83	23.84	31.860	50.99	68.140
Expected	135.85	46.51	34.234	89.34	65.766
Expected - Predicted	61.02	22.67		38.35	
Percent Difference	44.92	48.74		42.93	

Table 30. Expected Five Lane or Fewer Crash Type Distribution (Section 4)

Element Type	Crash Type	FI Crashes	Percent FI (%)	PDO Crashes	Percent PDO (%)	Total Crashes	Percent Total (%)
Highway Segment	Collision with Animal	0.00	0.0	0.21	0.2	0.21	0.2
Highway Segment	Collision with Bicycle	0.09	0.1	0.00	0.0	0.09	0.1
Highway Segment	Collision with Fixed Object	0.57	0.4	3.57	2.6	4.14	3.0
Highway Segment	Collision with Other Object	0.03	0.0	0.09	0.1	0.11	0.1
Highway Segment	Other Single-vehicle Collision	0.43	0.3	0.53	0.4	0.97	0.7
Highway Segment	Collision with Pedestrian	0.34	0.3	0.00	0.0	0.34	0.3
Highway Segment	Total Single Vehicle Crashes	1.47	1.1	4.39	3.2	5.86	4.3
Highway Segment	Angle Collision	0.37	0.3	0.54	0.4	0.91	0.7
Highway Segment	Driveway-related Collision	1.32	1.0	2.27	1.7	3.58	2.6
Highway Segment	Head-on Collision	0.17	0.1	0.05	0.0	0.21	0.2
Highway Segment	Other Multi-vehicle Collision	0.20	0.1	0.58	0.4	0.79	0.6
Highway Segment	Rear-end Collision	2.79	2.1	4.80	3.5	7.59	5.6
Highway Segment	Sideswipe, Opposite Direction Collision	0.15	0.1	0.09	0.1	0.24	0.2
Highway Segment	Sideswipe, Same Direction Collision	0.26	0.2	1.83	1.3	2.09	1.5
Highway Segment	Total Multiple Vehicle Crashes	5.25	3.9	10.16	7.5	15.42	11.3
Highway Segment	Total Highway Segment Crashes	6.72	4.9	14.56	10.7	21.28	15.7
Intersection	Collision with Animal	0.00	0.0	0.00	0.0	0.01	0.0
Intersection	Collision with Bicycle	0.73	0.5	0.00	0.0	0.73	0.5
Intersection	Collision with Fixed Object	0.90	0.7	1.71	1.3	2.61	1.9
Intersection	Non-Collision	0.17	0.1	0.07	0.0	0.24	0.2
Intersection	Collision with Other Object	0.09	0.1	0.14	0.1	0.23	0.2
Intersection	Other Single-vehicle Collision	0.05	0.0	0.04	0.0	0.09	0.1
Intersection	Collision with Parked Vehicle	0.00	0.0	0.00	0.0	0.00	0.0
Intersection	Collision with Pedestrian	0.44	0.3	0.00	0.0	0.44	0.3
Intersection	Total Intersection Single Vehicle Crashes	2.38	1.7	1.97	1.4	4.34	3.2
Intersection	Angle Collision	12.98	9.6	17.77	13.1	30.75	22.6
Intersection	Head-on Collision	1.83	1.3	2.18	1.6	4.02	3.0
Intersection	Other Multi-vehicle Collision	2.06	1.5	15.36	11.3	17.42	12.8
Intersection	Rear-end Collision	16.84	12.4	35.17	25.9	52.01	38.3
Intersection	Sideswipe	3.70	2.7	2.33	1.7	6.03	4.4
Intersection	Total Intersection Multiple Vehicle Crashes	37.41	27.5	72.82	53.6	110.23	81.1
Intersection	Total Intersection Crashes	39.79	29.3	74.78	55.0	114.57	84.3
	Total Crashes	46.51	34.2	89.34	65.8	135.85	100.0

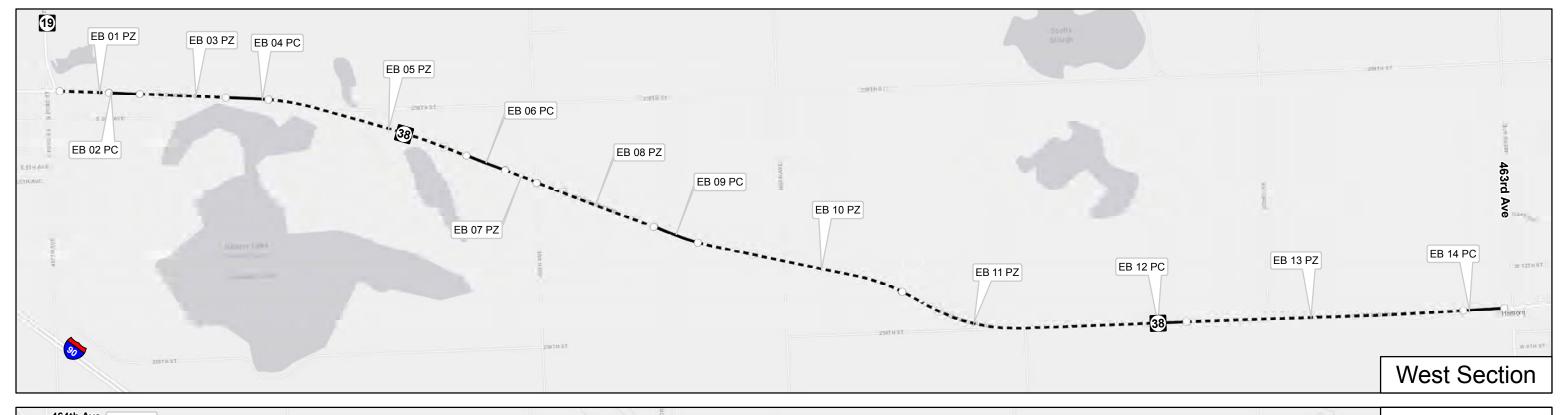


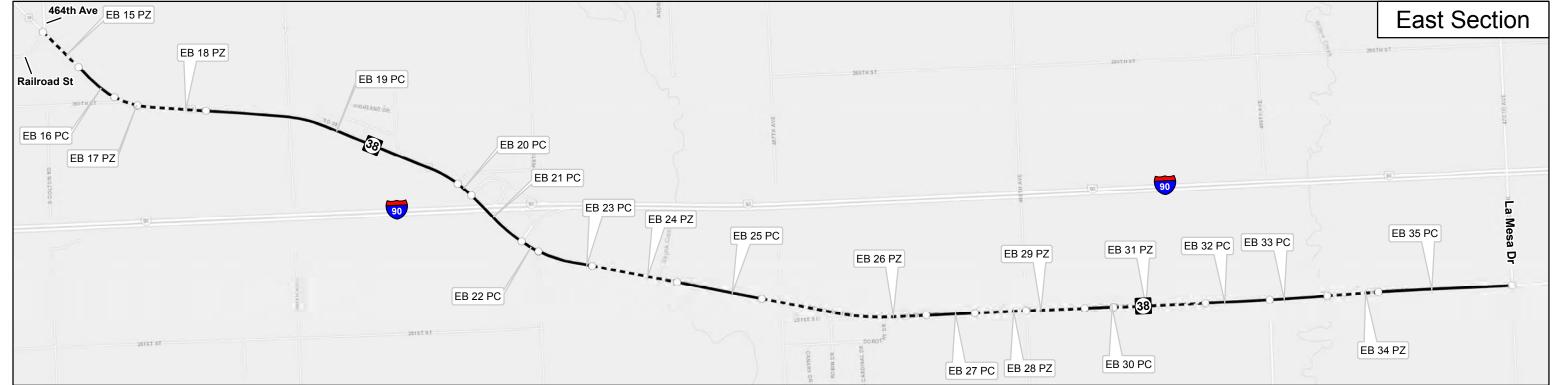
Appendix C – Study Intersections





Appendix D - Study Segments





# **Highway 38 Analysis Segments**

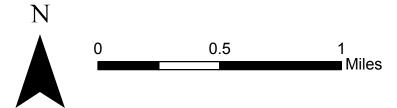
**Eastbound Lanes** 

## Legend

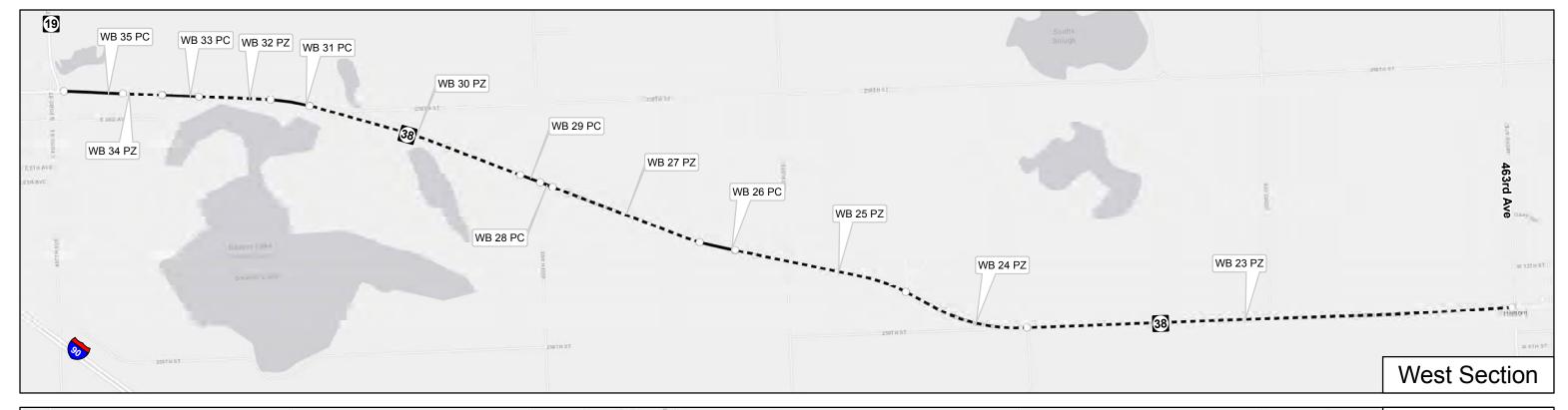
## **Analysis Segments**

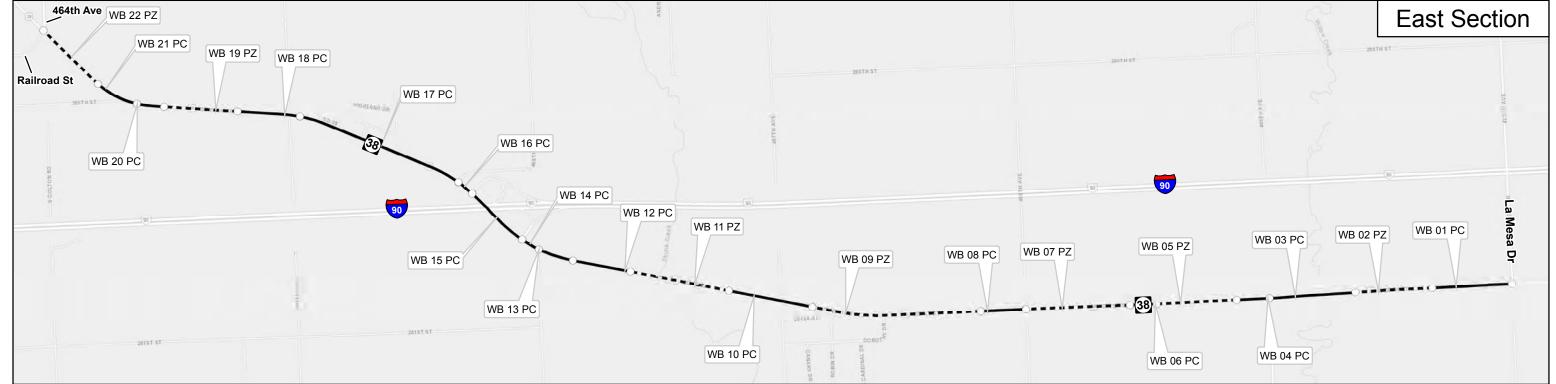
Passing ConstrainedPassing Zones











# **Highway 38 Analysis Segments**

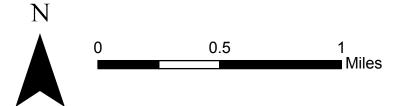
Westbound Lanes

## Legend

## **Analysis Segments**

Passing ConstrainedPassing Zones



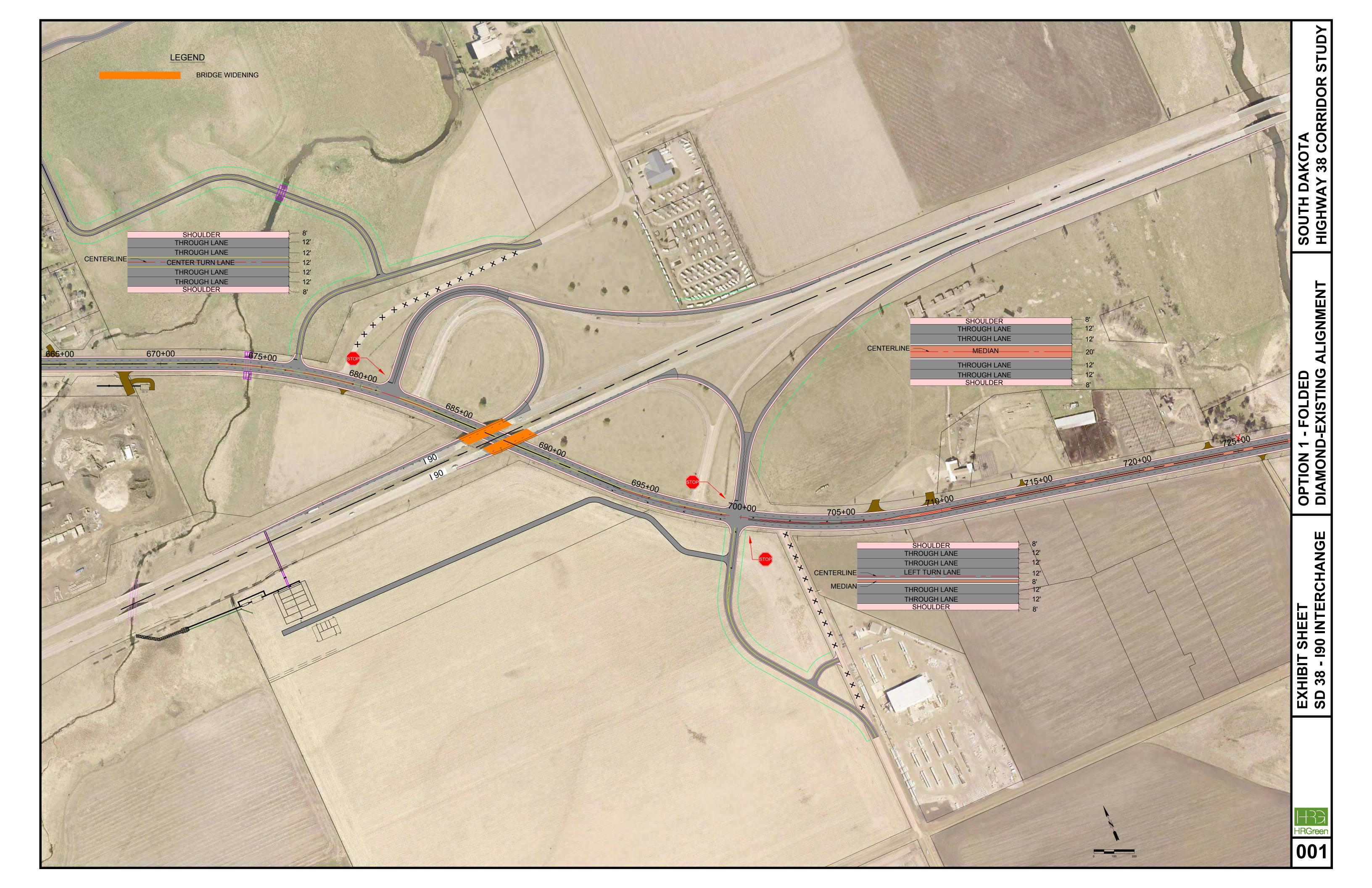


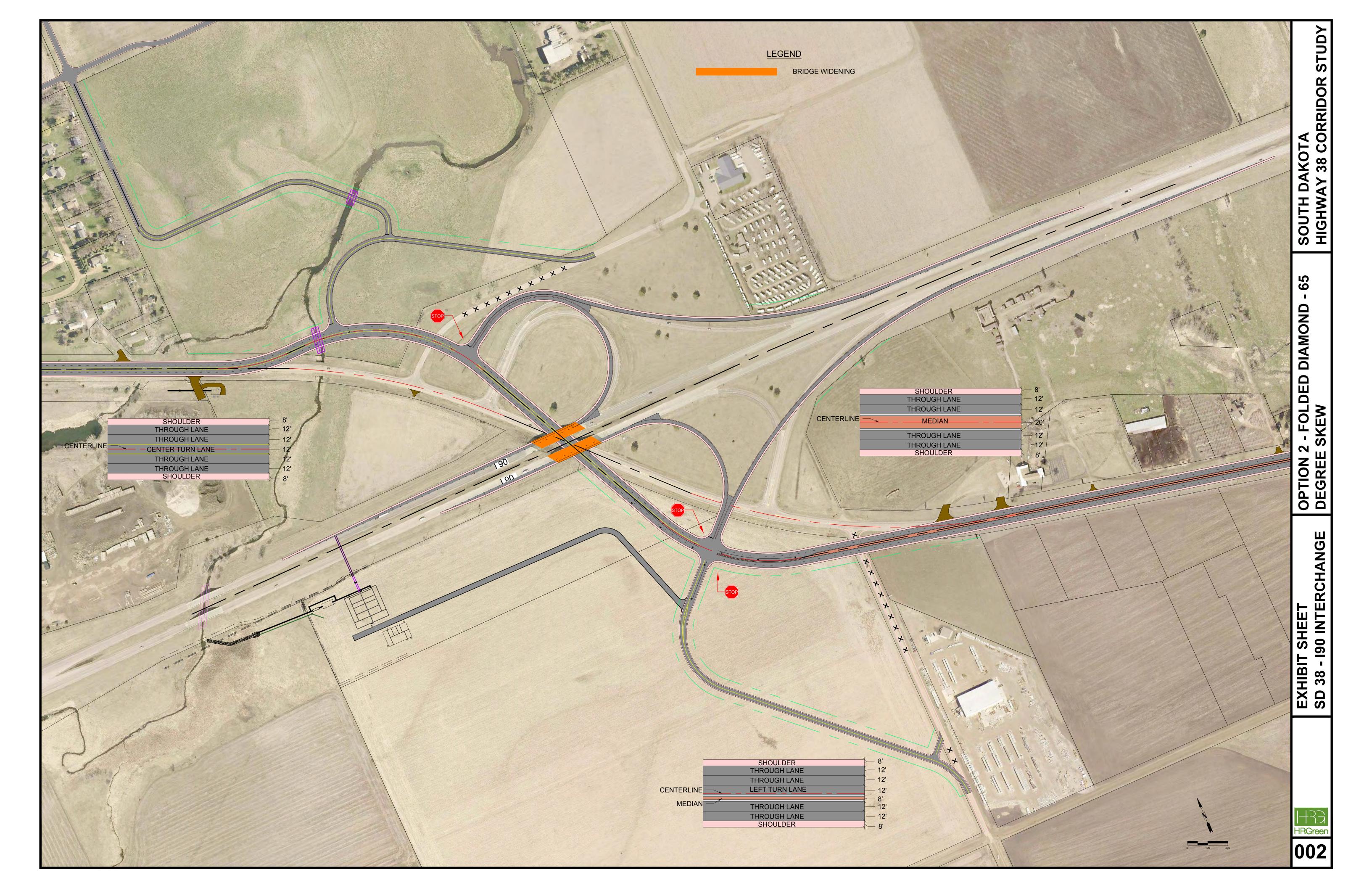


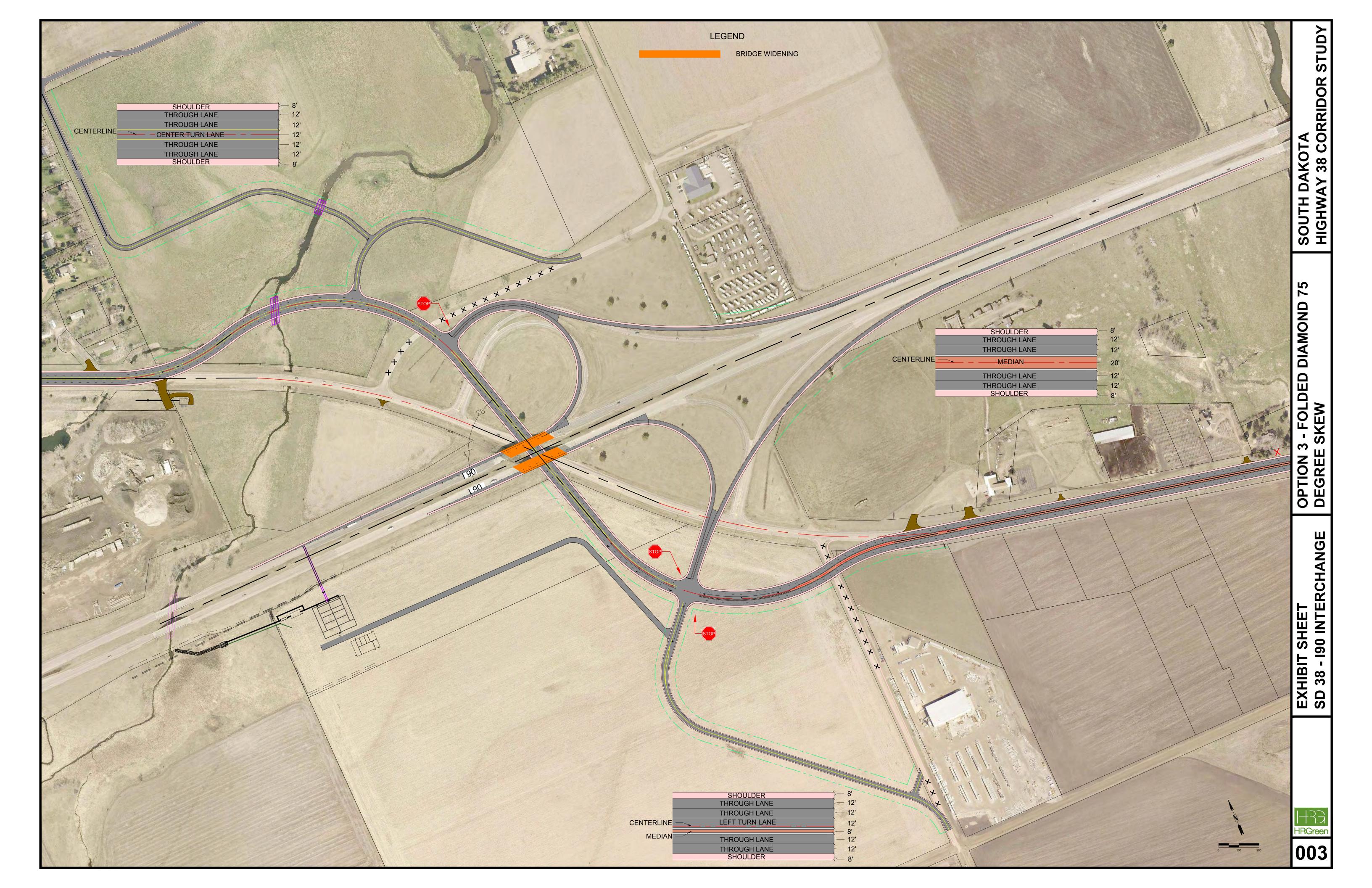
# APPENDIX E: I-90 EXIT 390 INTERCHANGE BUILD ALTERNATIVES

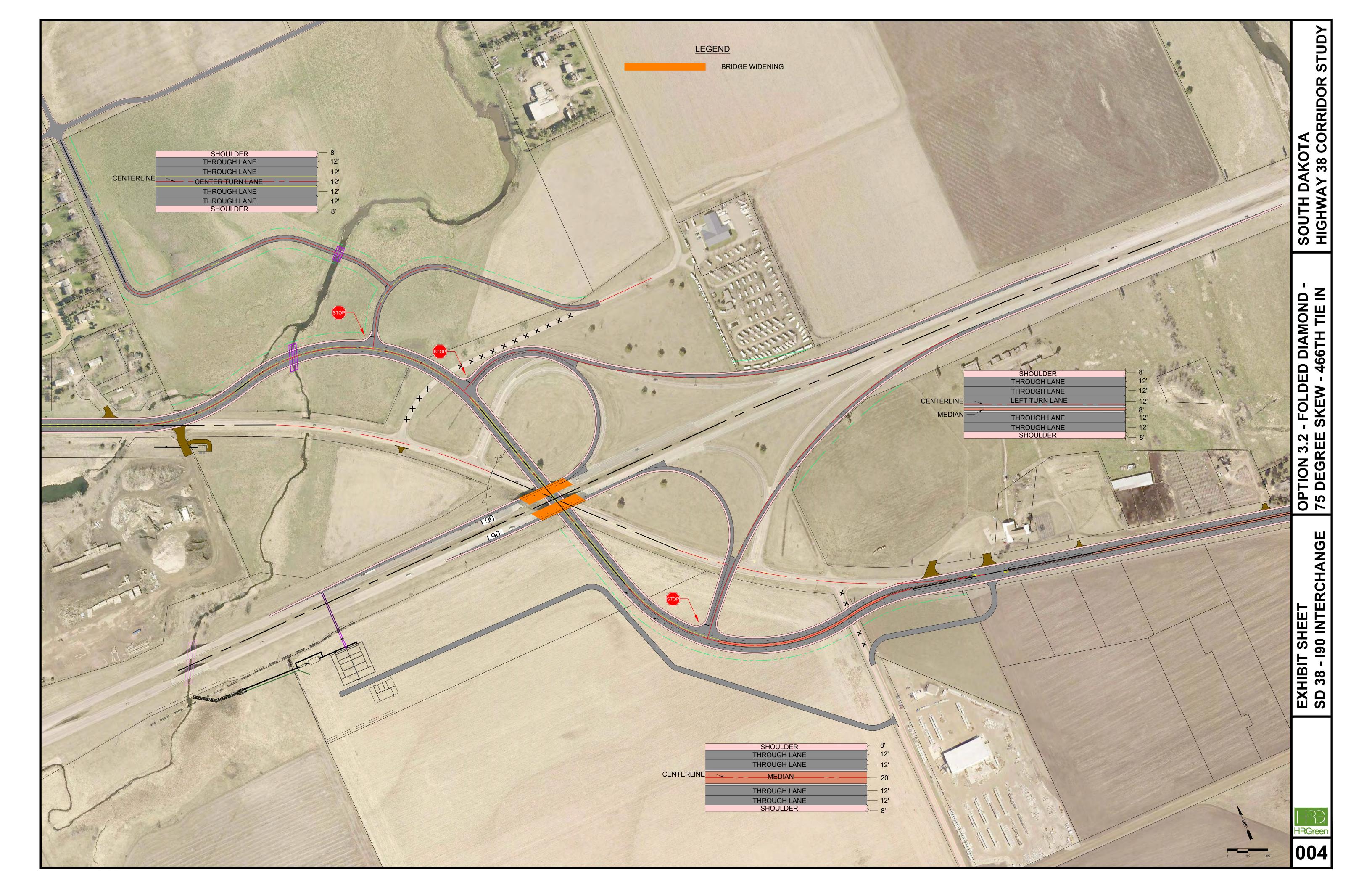
**SD38 Corridor Study** 

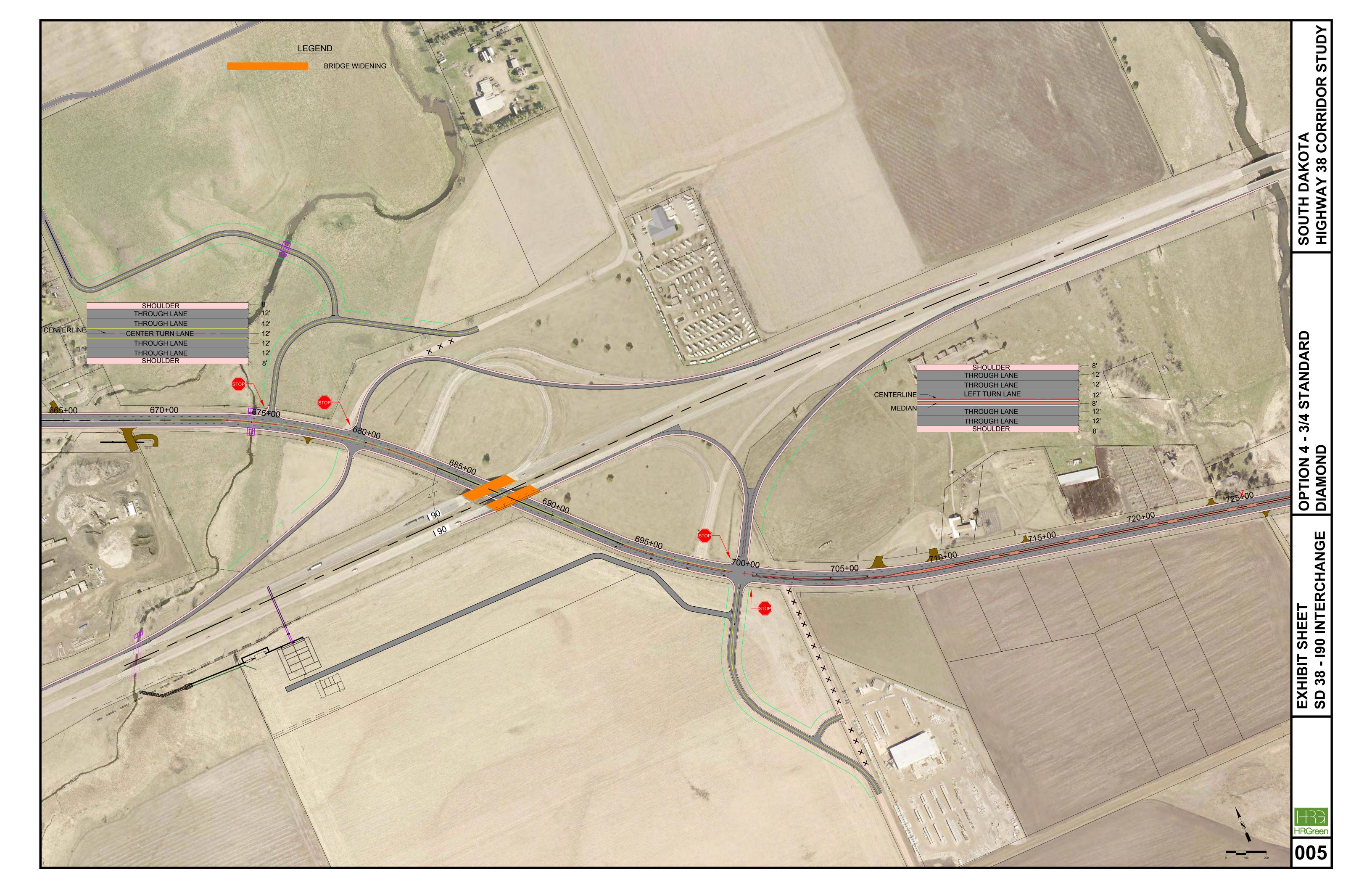


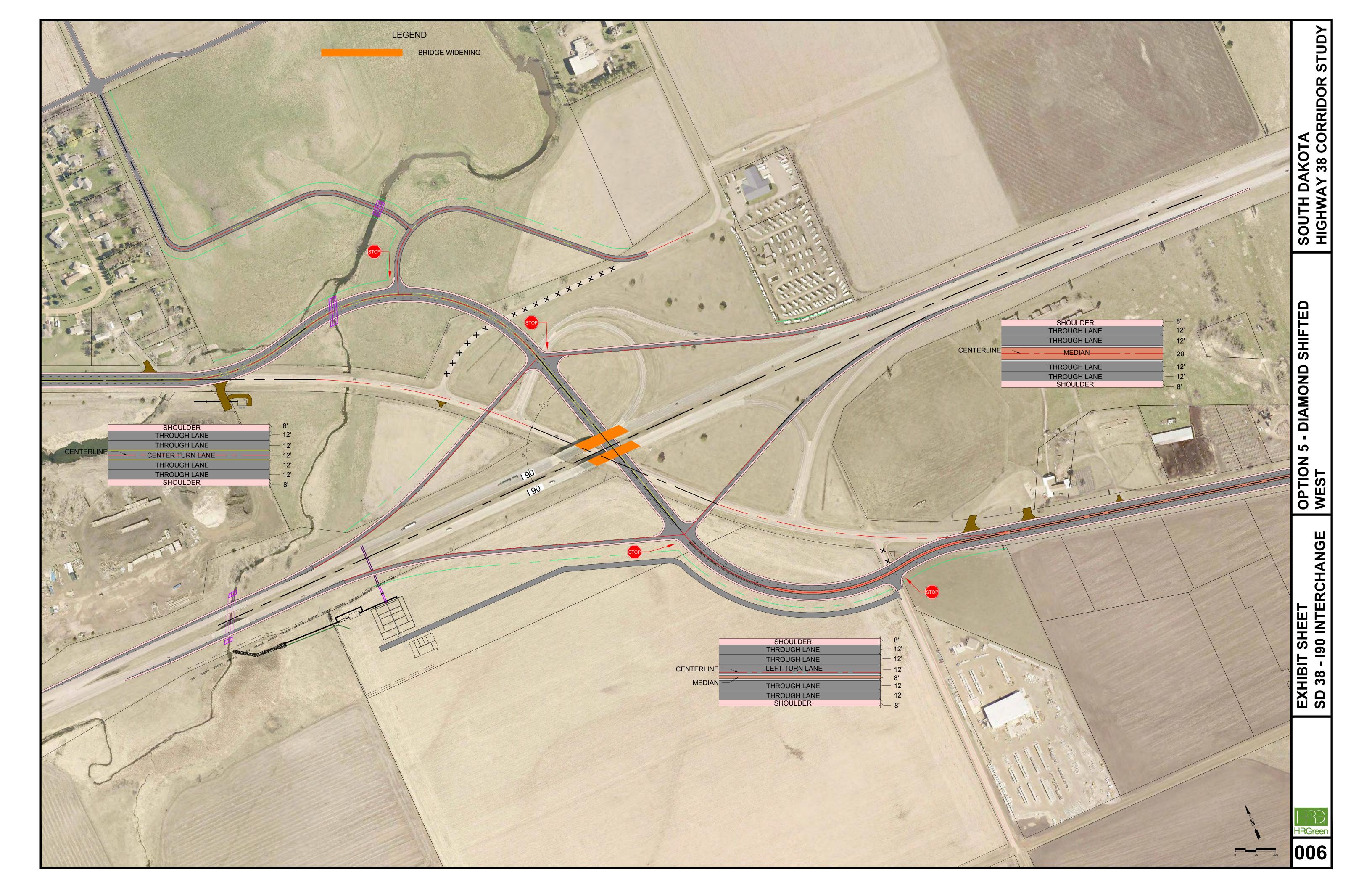


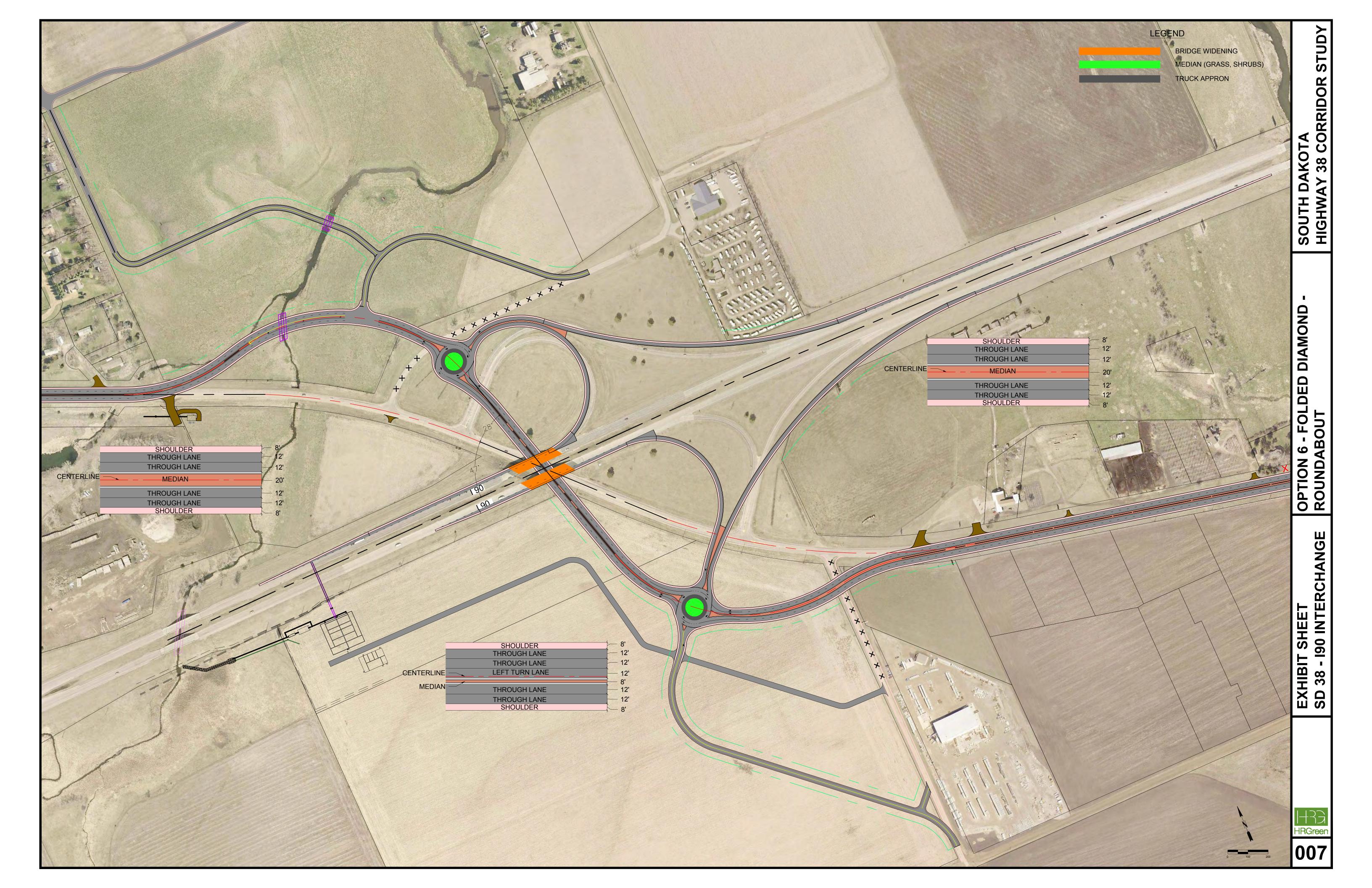


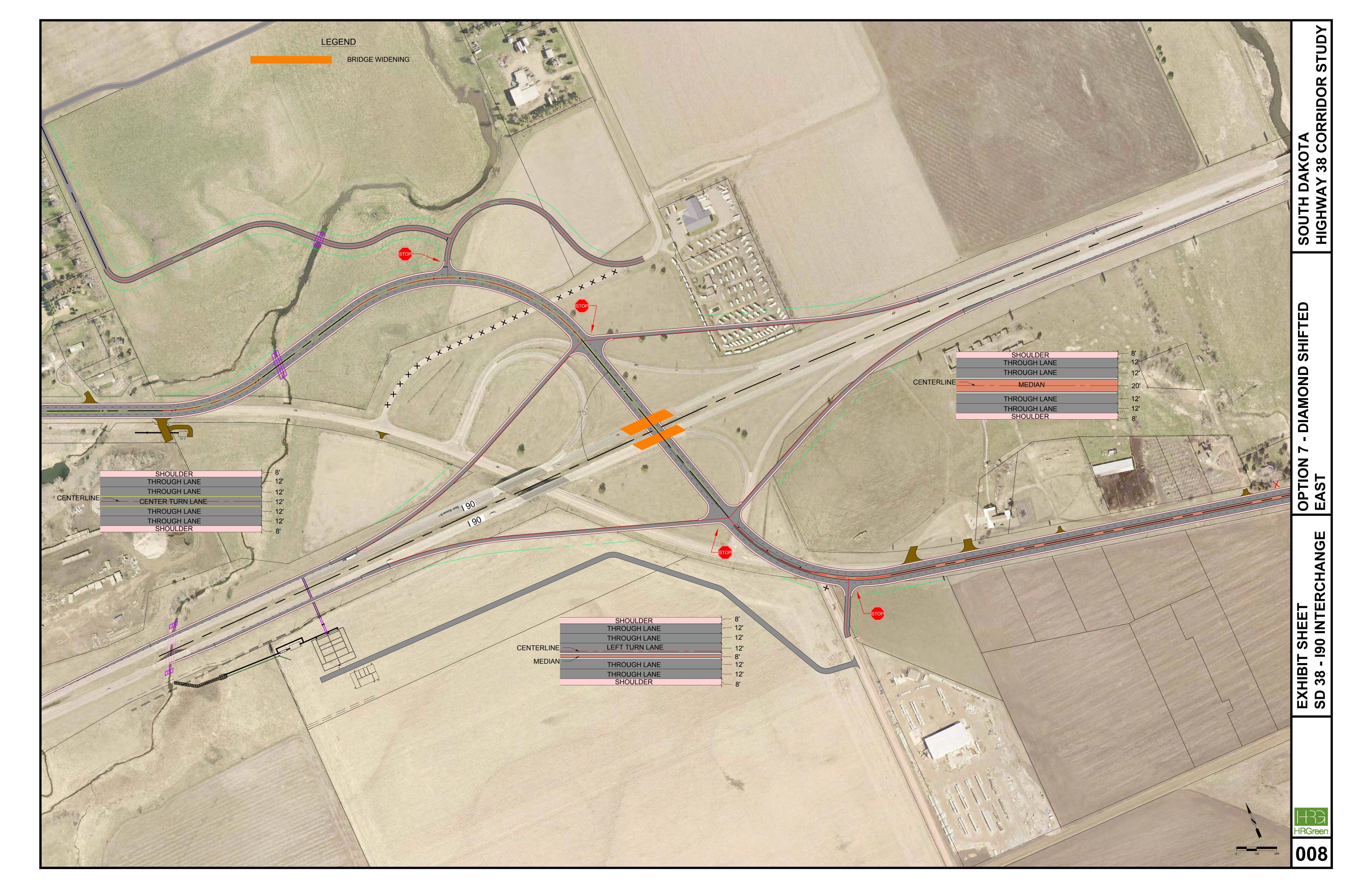


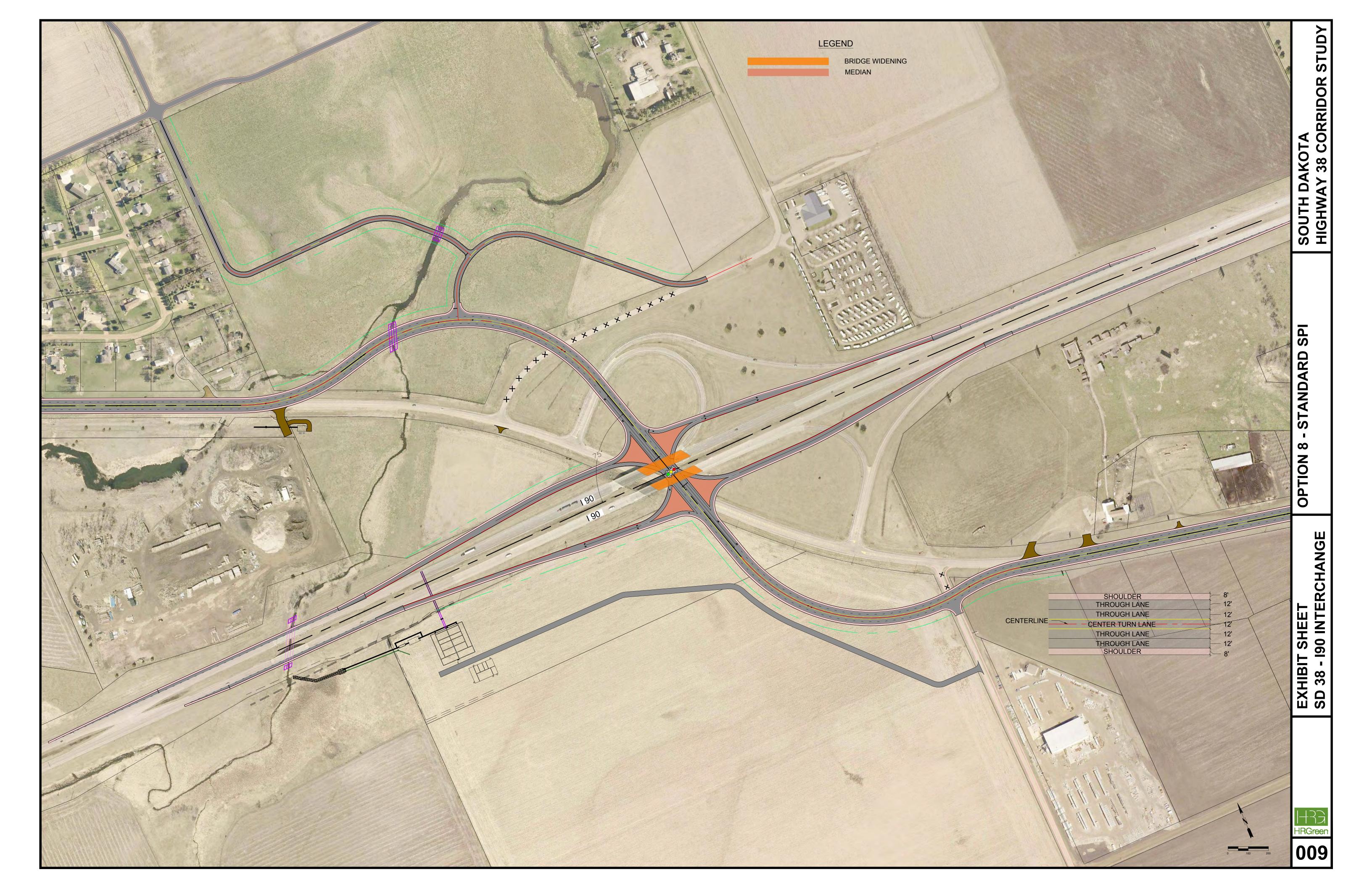


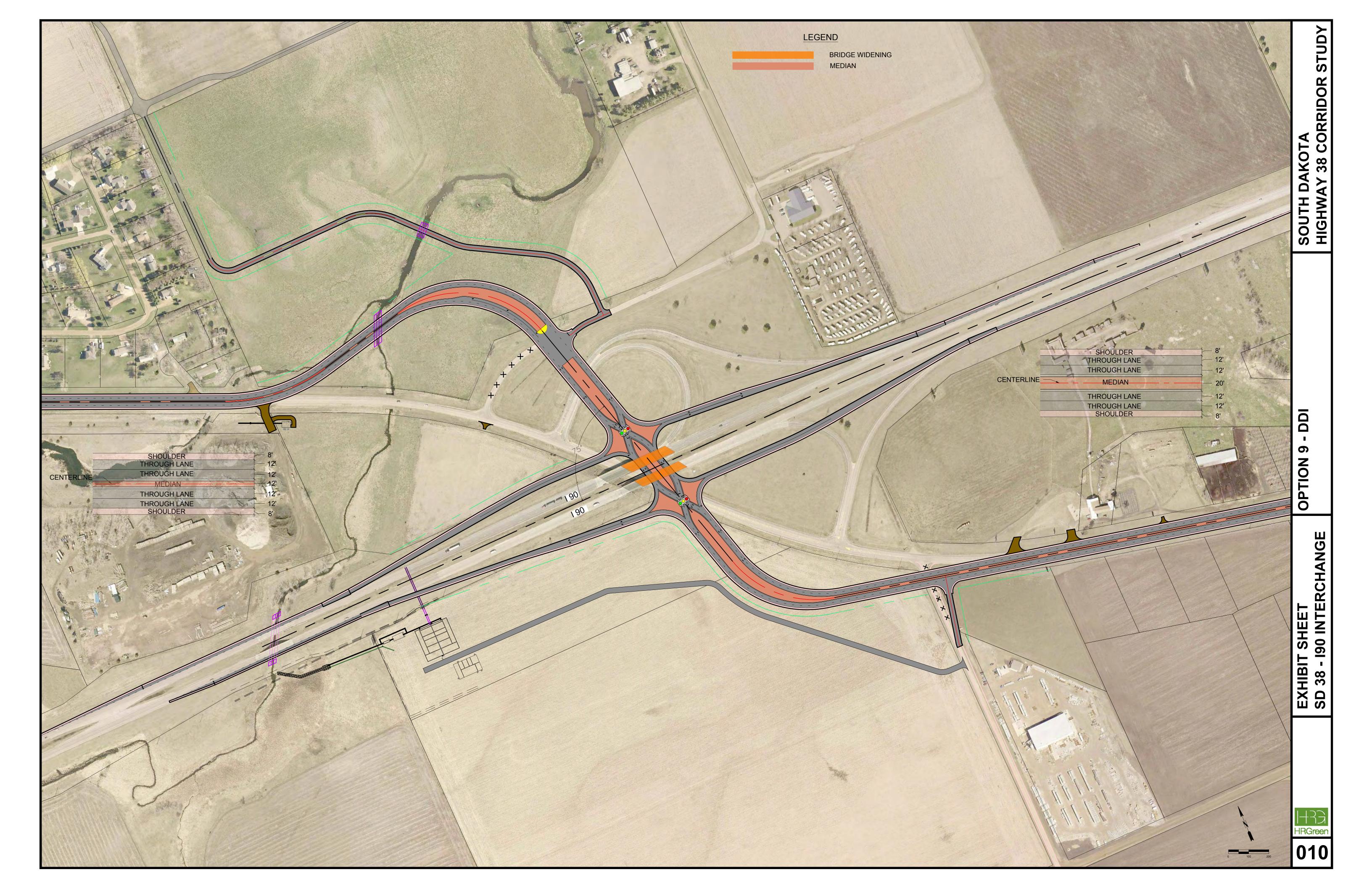








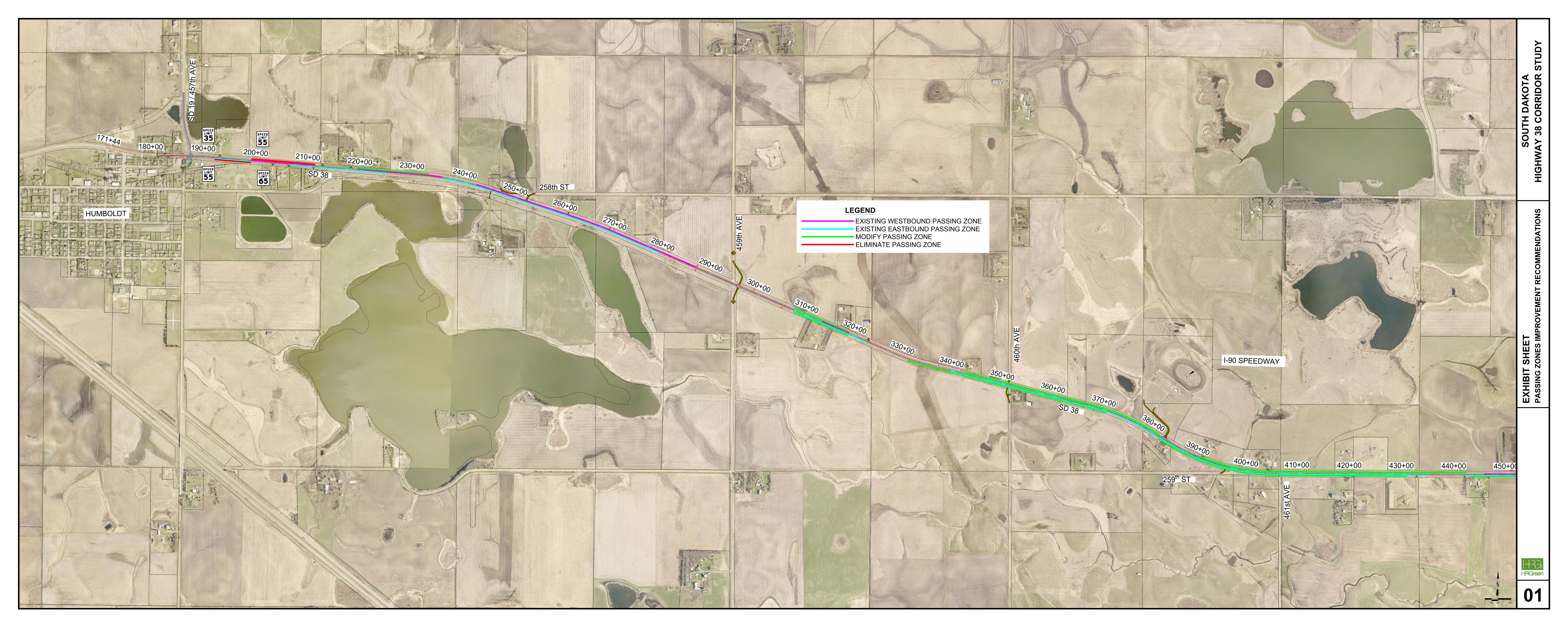


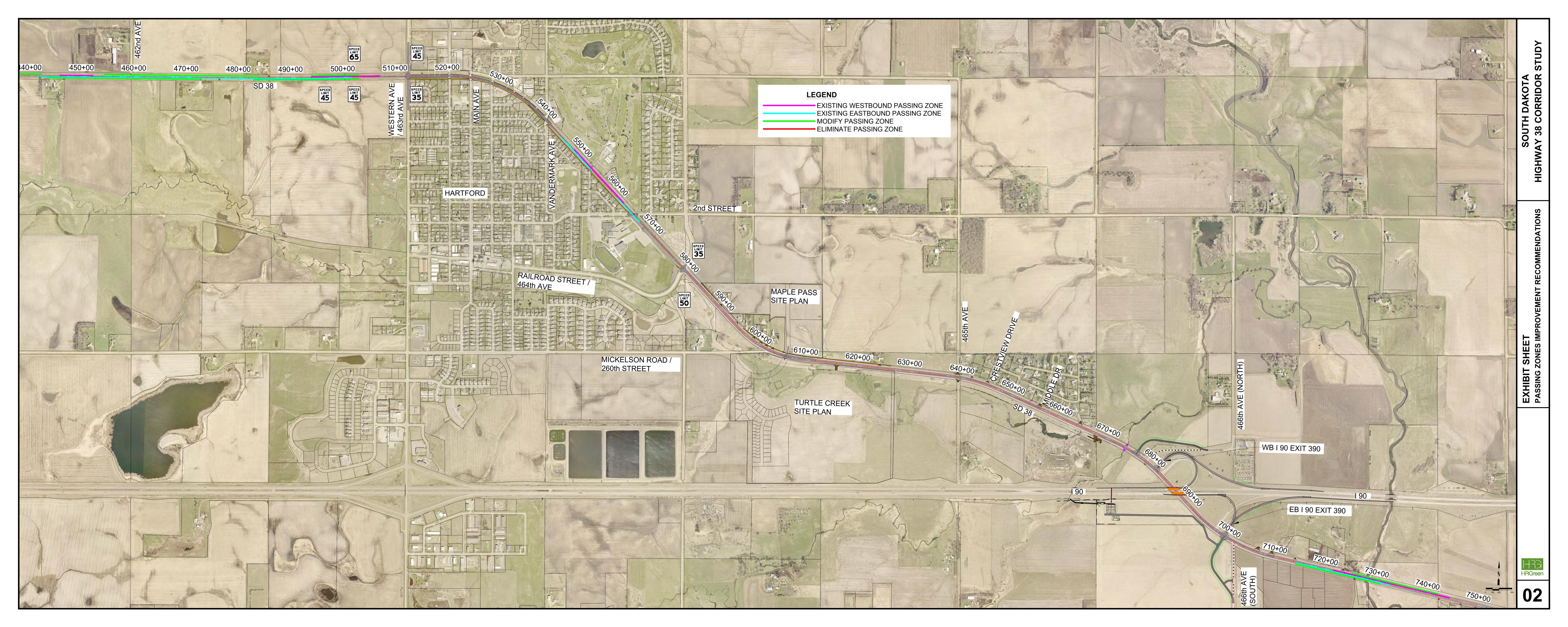


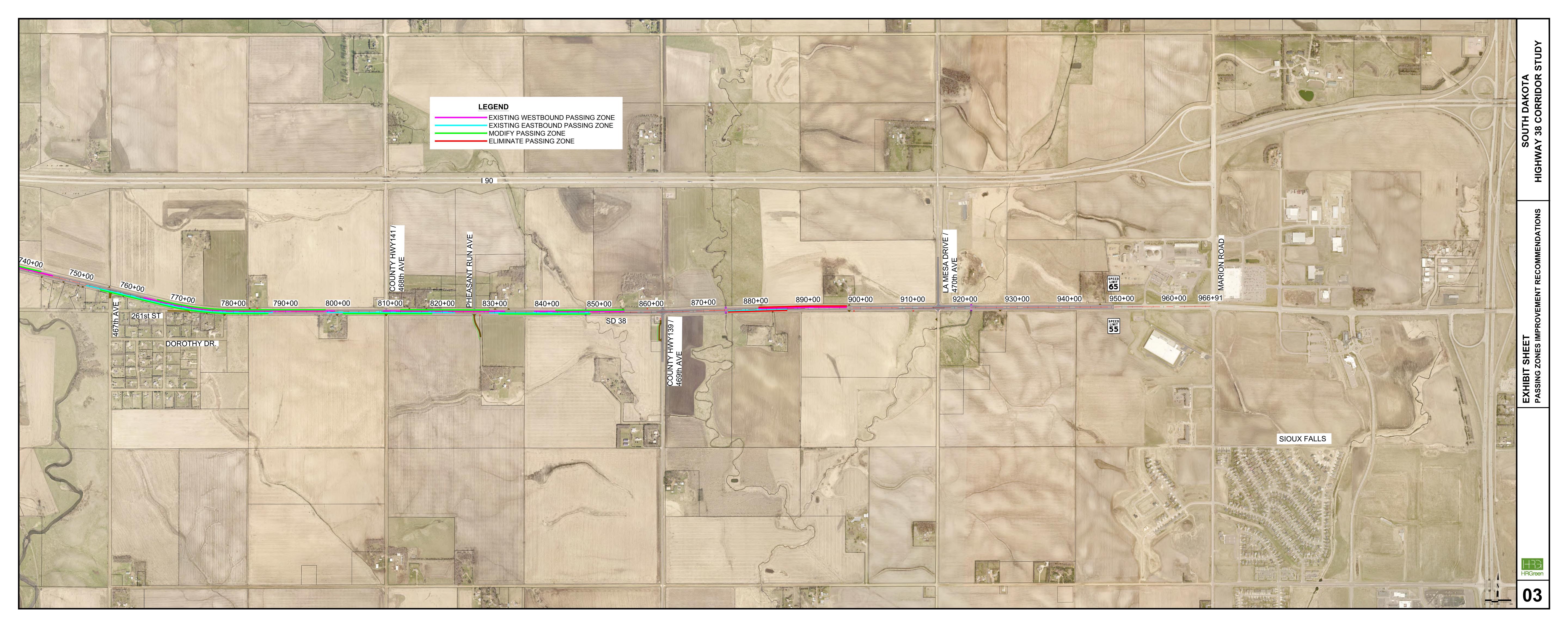
## **APPENDIX F: SHORT-TERM IMPROVEMENTS**

**SD38 Corridor Study** 

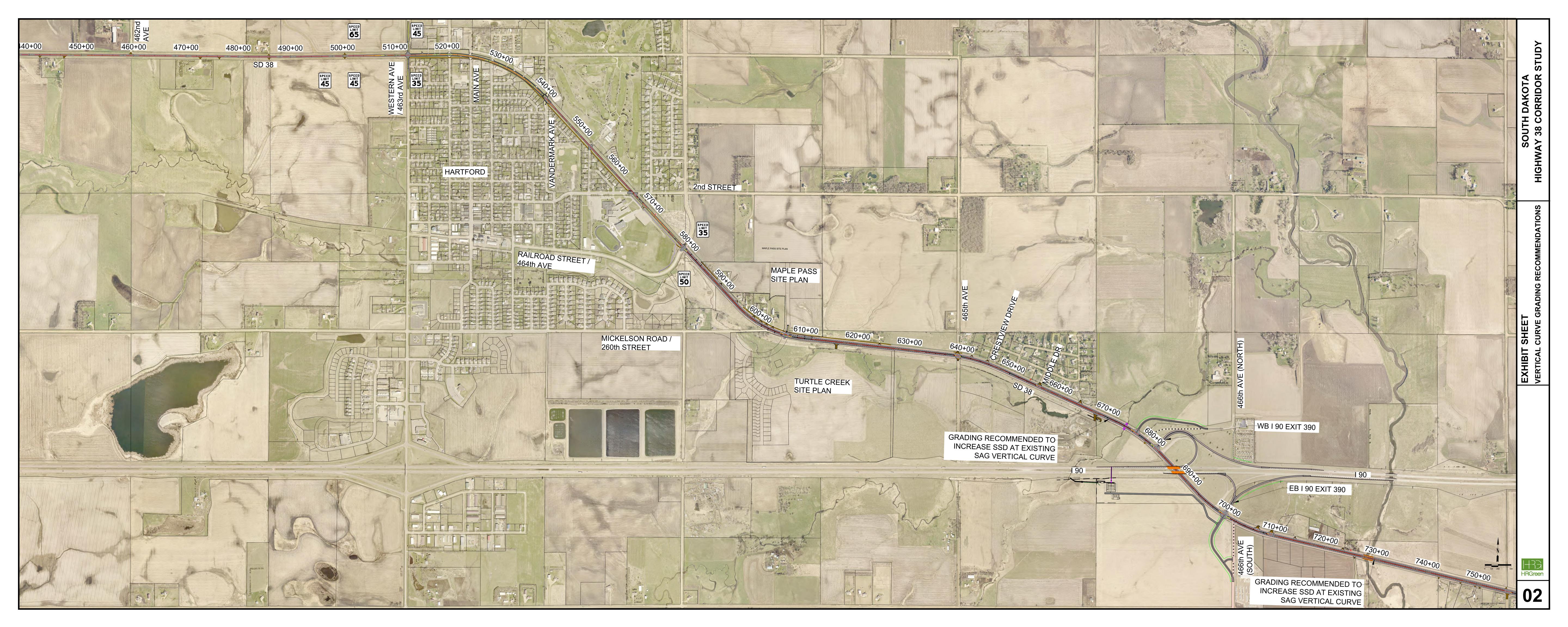


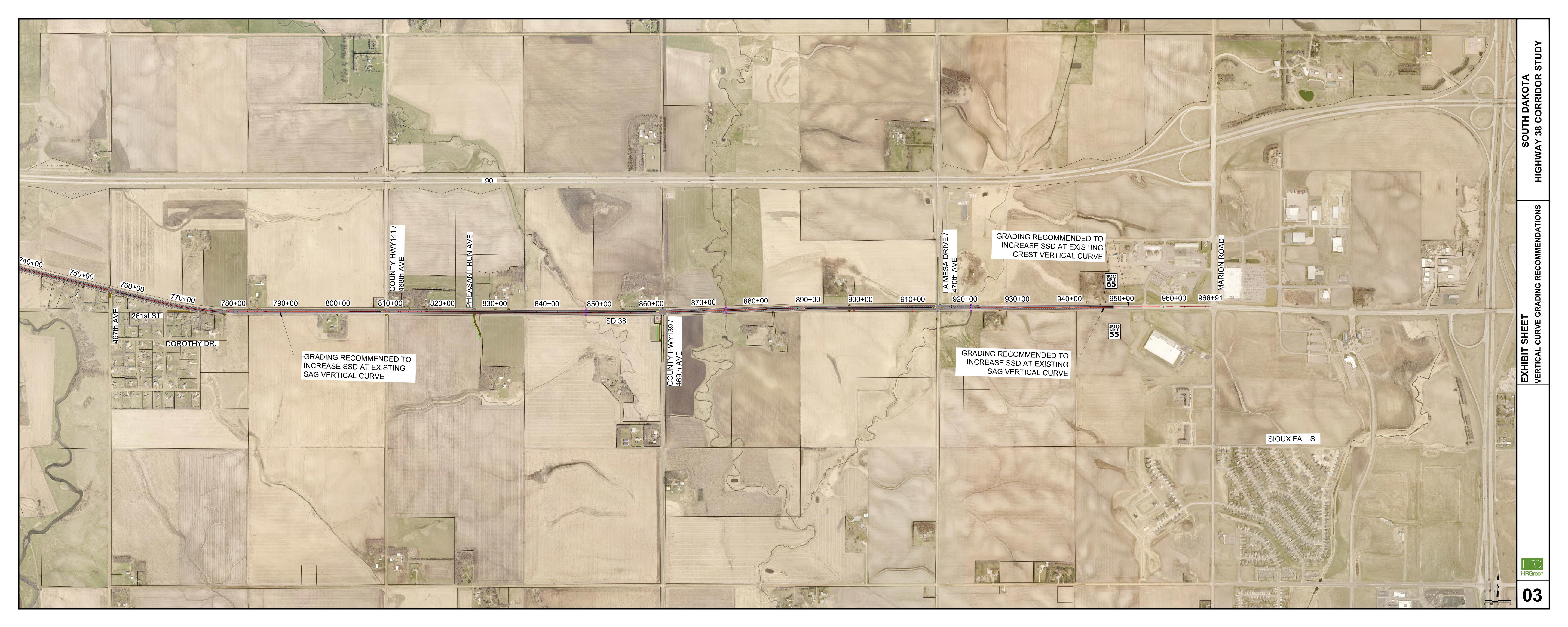


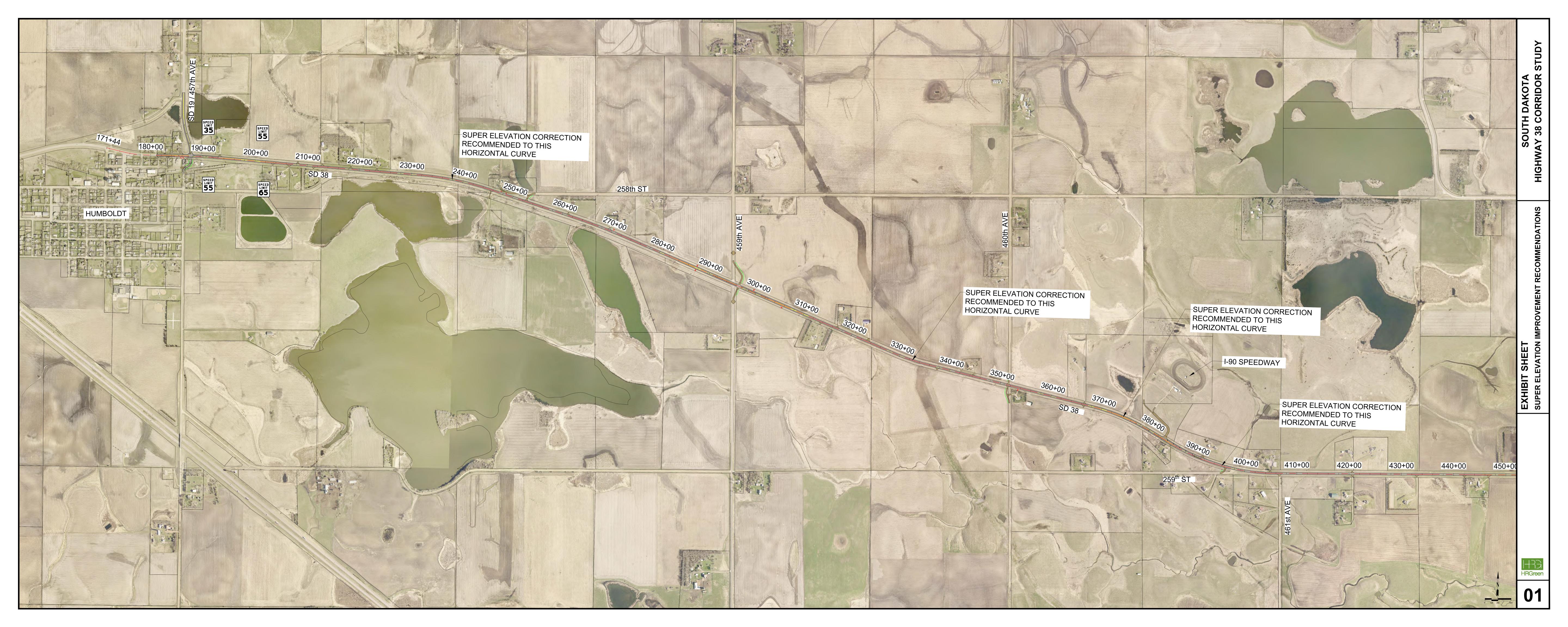


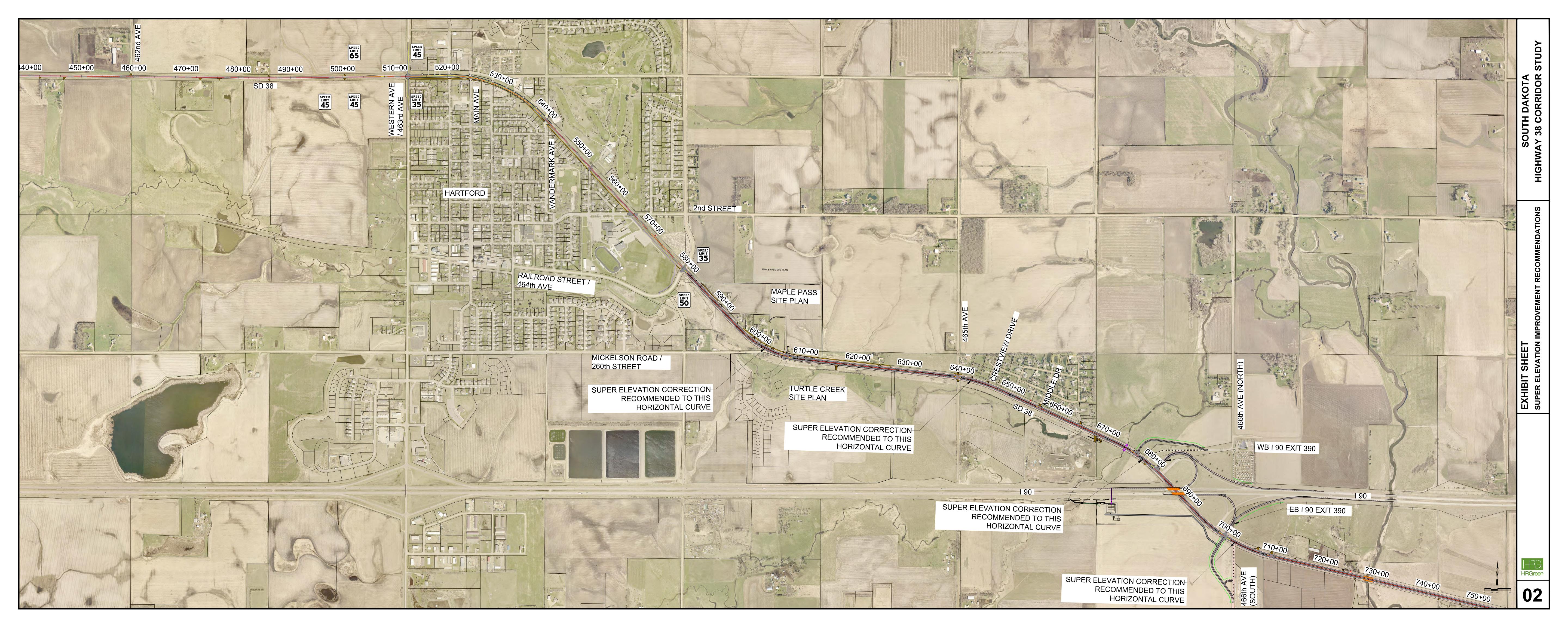














## APPENDIX G: COST ESTIMATES FOR PROJECT SEGMENTS

**SD38 Corridor Study** 



